

TRANSMITTAL LETTER

PUBLICATION:

Publication 219M

DATE:

10/7/2024

SUBJECT:

Standards for Bridge Construction September 2016 Edition Change 7

INFORMATION AND SPECIAL INSTRUCTIONS:

Incorporate the attached revisions into the September 2016 Edition of Publication 219M.

These revisions provide details for the connection of W-Beam Guide Rail (Type 31-S) to bridge barriers on the trailing end of bridges on divided highways and one-way roadways.

These standards may be used immediately and can be adopted as soon as practical on all new and existing projects without affecting letting schedules and in conjunction with the current Publication 408 Specifications. Projects let after April 11, 2025 must incorporate these new standards.

A description of the changes made to the 2016 Edition since Change 6 on March 27, 2024 are listed in the attached multi-sheet table. On the standards, light red highlighting indicates Change 7 revisions to details and notes.

Comments or questions concerning this Publication may be directed to the Bureau of Bridge.

CANCEL AND DESTROY THE FOLLOWING:

Existing BC-700M Series standards need to be retained for projects under construction and for future rehabilitation work. ADDITIONAL COPIES ARE AVAILABLE FROM:

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APPROVED FOR ISSUANCE BY:

MICHAEL B. CARROLL Secretary of Transportation

BY:

Gavin E. Gray, P.E.

Chief Engineer

Highway Administration

The major revisions for each Standard Drawing are presented below. Since minor changes are not indicated, it is strongly advised that all recipients thoroughly examine the changes and revisions incorporated in this release.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-706M	1 of 2	TITLE BLOCK: • Added "– 1" to sheet title.
		PA 3-RAIL BRIDGE BARRIER: • Added "WITH THRIE-BEAM CONNECTION" to detail name.
		 GENERAL NOTES: Note 2: Revised "RAILING TUBES" to "RAIL TUBES". Note 3: Revised "RAILING POSTS" to "POSTS". Note 4: Revised "ACCORDING TO" to "AS SPECIFIED IN". Note 5: Revised "RAILING TUBES" to "RAIL TUBES".
		ELEVATION VIEW: • Added callout for "CL THRIE-BEAM RAIL"
		REFERENCE DRAWINGS: • Added RC-51M – TYPE 31 STRONG POST GUIDE RAIL.
	2 OF 2	TITLE BLOCK: • Added "– 2" to sheet title.
		Added PA 3-RAIL BRIDGE BARRIER WITH W-BEAM CONNECTION.
		Added W-BEAM CONNECTION NOTES.
BC-709M	General	Inserted new sheet 5 and renumbered subsequent sheets; updated total number of sheets.
	1 of 13 (formerly 1 of 12)	TITLE BLOCK: • Added " – 1 to sheet title"
	1 01 12)	PA TYPE 10M BRIDGE BARRIER: • Added "WITH THRIE-BEAM CONNECTION" to sheet title.
		PLAN VIEW: • Removed "TOP" from the callout for "HSS 5x5x3/8" RAIL TUBE". • Added bottom rail to detail. • Added "(TOP RAIL)" to the callout for the "BEND POINT".
		 ELEVATION VIEW: Added callout for "CL THRIE-BEAM RAIL". Added 1'-9" and 1 3/4" dimensions. Added 3/4" hole, callout and 10 3/4" dimension for the hole in the bottom rail.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	1 of 13 (formerly 1 of 12) (cont.)	 GENERAL NOTES: Note 2: Revised "RAILING TUBES" to "RAIL TUBES". Note 3: Revised "RAILING POSTS" to "POSTS". Removed "OR 50W" (2 places). Note 4: Revised "ACCORDING TO" to "AS SPECIFIED IN". Note 5: Revised "RAILING TUBES" to "RAIL TUBES". Note 11: Added "A." to the anchor bolt tightening note. Added "B. SNUG TIGHTEN ALL THREADED ANCHOR STUDS." Note 13: Revised "FOR BARRIER RAIL TO POST CONNECTION AND SIDEWALK RAIL CONNECTION" to "FOR RAIL TUBE TO POST CONNECTION AND SIDEWALK RAIL TUBE CONNECTION". REFERENCE DRAWINGS:
	2 of 13 (formerly 2 of 12)	 Added RC-51M – TYPE 31 STRONG POST GUIDE RAIL. TITLE BLOCK: Added " – 2" to sheet title. RAIL TUBE CAP DETAILS: Added "TUBE" in titles of all four views.
	3 of 13 (formerly 3 of 12)	PA TYPE 10M BRIDGE BARRIER TYPICAL SIDEWALK: • Added "WITH THRIE-BEAM CONNECTION" to detail name. PLAN VIEW: • Removed "TOP" from the callout for "HSS 5x5x3/8" RAIL TUBE". • Added bottom rail to detail. • Added "(TOP RAIL)" to the callout for the "BEND POINT". ELEVATION VIEW: • Added callout for "CL THRIE-BEAM RAIL". • Added 1'-9" and 1 3/4" dimensions. • Added 3/4" hole, callout and 10 3/4" dimension for the hole in the bottom rail.
	4 of 13 (formerly 4 of 12)	PA TYPE 10M BRIDGE BARRIER ALTERNATE SIDEWALK: • Added "WITH THRIE-BEAM CONNECTION" to detail name. PLAN VIEW: • Removed "TOP" from the callout for "HSS 5x5x3/8" RAIL TUBE". • Added bottom rail to detail. • Added callout "(TOP RAIL)" to the callout for the "BEND POINT". ELEVATION VIEW: • Added callout for "CL THRIE-BEAM RAIL". • Added 1'-9" and 1 3/4" dimensions. • Added 3/4" hole, callout and 10 3/4" dimension for the hole in the bottom rail.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M	5 OF 13	New Sheet – W-BEAM CONNECTION ELEVATIONS.
(cont.)	(NEW)	Added ELEVATIONS and W-BEAM CONNECTION NOTES to show how the W-Beam Terminal Connection is attached to the PA Type 10M Bridge Barrier.
BC-713M	General	Inserted new sheets 7 and 8 and renumbered subsequent sheets; updated total number of sheets.
	1 of 16 (formerly 1 of 14)	TITLE BLOCK: • Added " – 1" to sheet title.
		PA BRIDGE BARRIER: • Added "WITH THRIE-BEAM CONNECTION" to detail name.
		PLAN: • Revised callout for "DETAIL C" to "DETAIL B".
		ELEVATION VIEW: • Added callout for "CL THRIE-BEAM RAIL". • Added 1'-9" dimension.
		GENERAL NOTES: • Note 2: Revised "RAILING TUBES" to "RAIL TUBES". • Note 3: • Revised "RAILING POSTS" to "POSTS". • Removed "OR 50W" (2 places). • Note 4: • Revised "ACCORDING TO" to "AS SPECIFIED IN". • Added "CONNECTOR PLATES". • Note 5: Revised "RAILING TUBES" to "RAIL TUBES".
		Note 9: Revised "FOR BARRIER RAIL TO POST CONNECTION AND SIDEWALK RAIL CONNECTION" to "FOR RAIL TUBE TO POST CONNECTION AND SIDEWALK RAIL TUBE CONNECTION".
		REFERENCE DRAWINGS: • Added RC-51M – TYPE 31 STRONG POST GUIDE RAIL.
	2 of 16 (formerly 2 of 14)	TITLE BLOCK: • Added " – 2" to sheet title.
	2 01 1 1)	DETAIL B – BASE PLATE DETAIL: • Removed "DETAIL B" from detail name.
		SIDEWALK RAIL ROD ANCHOR PLATE DETAIL: • Revised "ANCHOR" to "CONNECTOR" in detail name.
		NOTES: • Note 1: Revised "ANCHOR" to "CONNECTOR" (2 places).
	3 of 16 (formerly 3 of 14)	TITLE BLOCK: • Added " – 3" to sheet title.
	5 01 11)	Renamed DETAIL C to DETAIL B.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	4 of 16 (formerly 4 of 14)	PA BRIDGE BARRIER TYPICAL SIDEWALK: • Added "WITH THRIE-BEAM CONNECTION" to detail name.
	+ 01 1+ <i>)</i>	PLAN: • Revised callout for "ANCHOR PLATE" to "CONNECTOR PLATE". • Revised callout for "DETAIL C" to "DETAIL B".
		ELEVATION VIEW: • Added callout for "CL THRIE-BEAM RAIL". • Added 1'-9" dimension.
		SECTION C-C BARRIER SECTION: • Revised "ANCHOR PLATE" to "CONNECTOR PLATE" in web hole callout.
	5 of 16 (formerly 5 of 14)	PA BRIDGE BARRIER ALTERNATE SIDEWALK: • Added "WITH THRIE-BEAM CONNECTION" to sheet title.
	3 0111,	PLAN: • Revised callout for "ANCHOR PLATE" to "CONNECTOR PLATE". • Revised callout for "DETAIL C" to "DETAIL B".
		 ELEVATION VIEW: Added callout for "CL THRIE-BEAM RAIL". Added 1'-9" dimension.
		 Removed the 8" dimension for the sidewalk height. SECTION E-E BARRIER SECTION: Revised "ANCHOR PLATE" to "CONNECTOR PLATE" in web hole callout.
	6 of 16 (formerly 6 of 14)	PA BRIDGE BARRIER RAISED SIDEWALK: • Added "WITH THRIE-BEAM CONNECTION" to sheet title.
		PLAN: • Revised callout for "ANCHOR PLATE" to "CONNECTOR PLATE". • Revised callout for "DETAIL C" to "DETAIL B".
		ELEVATION VIEW: • Added callout for "CL THRIE-BEAM RAIL". • Added 1'-9" dimension.
		SECTION F-F BARRIER SECTION: • Revised "ANCHOR PLATE" to "CONNECTOR PLATE" in web hole callout.
	7 OF 16 (NEW)	New Sheet – W-BEAM CONNECTION ELEVATIONS – 1
	(11111)	Added ELEVATIONS and NOTES to show how the W-Beam Terminal Connection is attached to the PA Type 10M Bridge Barrier.
	8 OF 16 (NEW)	New Sheet – W-BEAM CONNECTION ELEVATIONS – 2
	(2.211)	Added ELEVATIONS and NOTES to show how the W-Beam Terminal Connection is attached to the PA Type 10M Bridge Barrier.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	12 of 16 (formerly	Renamed DETAIL D to DETAIL C.
(centi)	10 of 14)	Renamed DETAIL E to DETAIL D.
		SECTION S-S:
		Revised callout for "DETAIL E" to "DETAIL D".
		SECTION U-U: PLAN:
		Revised callout for "DETAIL D" to "DETAIL C".
	13 of 16	SECTION V-V:
	(formerly 11 of 14)	• Revised callout for "DETAIL F" to "DETAIL E".
		Renamed DETAIL F to DETAIL E.
		POST AND BASE PLATE:
		• Revised callout for "DETAIL B," to "THE BASE PLATE DETAIL ON".



TRANSMITTAL LETTER

PUBLICATION:

Publication 219M

DATE:

3/27/2024

SUBJECT:

Standards for Bridge Construction September 2016 Edition Change 6

INFORMATION AND SPECIAL INSTRUCTIONS:

Incorporate the attached revised standard BC-719M into the September 2016 Edition of Publication 219M.

BC-719M has been revised to bring the Department's temporary bridge barriers into compliance with the AASHTO Manual for Assessing Safety Hardware (MASH 2016).

This standard may be used immediately and can be adopted as soon as practical on all new and existing projects without affecting letting schedules and in conjunction with the current Publication 408 Specifications. Projects let after October 11, 2024 must incorporate these new standards.

A description of the changes made to the 2016 Edition since Change 5 on February 14, 2023 are listed in the attached multi-sheet table. On the standard, light purple highlighting indicates Change 6 revisions to details and notes.

Comments or questions concerning this Publication may be directed to the Bureau of Bridge.

CANCEL AND DESTROY THE FOLLOWING:

Existing BC-700M Series standards need to be retained for projects under construction and for future rehabilitation work.

ADDITIONAL COPIES ARE AVAILABLE FROM:

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APPROVED FOR ISSUANCE BY:

MICHAEL B. CARROLL Secretary of Transportation

BY

Richard W. Runyen, P.E. Director, Bureau of Bridge, Highway Administration

The major revisions for the Standard Drawing are presented below. Since minor changes are not indicated, it is strongly advised that all recipients thoroughly examine the changes and revisions incorporated in this release.

STANDARD S	SHEET	DESCRIPTION OF CHANGES
· · · · · · · · · · · · · · · · · · ·	1 of 7 formerly 1 of 8)	GENERAL NOTES: Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". Note 7: First Sentence: Added "(ASTM A709, GRADE 36)" after "WASHER". Note 8: Revised Note to the following: THE FOLLOWING TWO TYPE OF INSTALLATIONS ARE PERMITTED: TYPE 1: TRAFFIC PRESENT ON ONE SIDE OF THE TEMPORARY BARRIER. PROVIDE ANCHORS ON THE TRAFFIC SIDE OF THE TEMPORARY BARRIER. TYPE 2: TRAFFIC PRESENT ON BOTH SIDES OF THE TEMPORARY BARRIER. TYPE 2: TRAFFIC PRESENT ON BOTH SIDES OF THE TEMPORARY BARRIER. THE 42" TEMPORARY SINGLE FACE CONCRETE BARRIER IS NOT PERMITTED FOR TYPE 2 INSTALLATIONS. Note 9: Revised Note to the following: MASH DESIGNATIONS: THE 32" TEMPORARY CONCRETE MEDIAN BARRIER WITH 4'-0" (MAXIMUM) ANCHOR SPACINGS IS DESIGNATED AS MASH TL-3. THE 50" TEMPORARY CONCRETE MEDIAN BARRIER WITH 2'-0" (MAXIMUM) ANCHOR SPACINGS IS DESIGNATED AS MASH TL-4. THE 42" TEMPORARY SINGLE FACE CONCRETE BARRIER WITH 2'-0" (MAXIMUM) ANCHOR SPACINGS IS DESIGNATED AS MASH TL-4. THE 42" TEMPORARY SINGLE FACE CONCRETE BARRIER WITH 2'-0" (MAXIMUM) ANCHOR SPACINGS IS DESIGNATED AS MASH TL-4. Note 11: First Sentence: Removed "BARRIER OR MEDIAN". Note 12: Removed last Sentence: "SPACING OF ADHESIVE ANCHORS VARIES FROM 4'-0" TO 1'-0" AS SHOWN IN THE TABLE." Added notes 18, 19 and 20. TABLE 1: Revised table based on Barrier Type. Revised Note under table. REFERENCE DRAWINGS: Removed RC-57M. Added TC-8604 – DELINEATION.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-719M (cont.)	2 of 7 (formerly 2 of 8)	ONSTRUCTION NOTES: Note 3: Revised last three sentences from "IF BARRIERS CANNOT BE REPOSITIONED AND REBAR IS ENCOUNTERED, MOVE TO ALTERNATE BOLT POCKETS IN TYPE A AND B INSTALLATIONS. FOR EXISTING DECKS, TYPE C INSTALLATIONS WILL REQUIRE DRILLING THROUGH DECK REINFORCEMENT STEEL, ALTERNATIVELY, ONE BOLT PER BARRIER SECTION MAY BE ELIMINATED WITH APPROVAL OF THE ENGINEER. FOR NEW DECKS WITH TYPE C INSTALLATIONS, PROPERLY PLAN AND PLACE DECK REINFORCEMENT STEEL TO AVOID DAMAGE DURING DRILLING" to "IF BARRIERS CANNOT BE REPOSITIONED AND REBAR IS ENCOUNTERED, MOVE TO ALTERNATE BOLT POCKETS, FOR EXISTING DECKS DRILLING THROUGH DECK REINFORCEMENT STEEL IS PERMITTED ONLY IF THE DECK IS DEMOLISHED IN A LATER STAGE OF CONSTRUCTION. FOR NEW DECKS, PROPERLY PLAN AND PLACE DECK REINFORCEMENT STEEL TO AVOID DAMAGE DURING DRILLING" Note 5: Removed "BUT NOT TO EXCEED 2"-0" IN THE SEGMENT LENGTH ON THE BRIDGE" from the last sentence. Note 6: Note removed since it was not MASH compliant. Note 7 was renumbered to be Note 6: 1" Bullet: Revised "SECTION 1080.2 (c)". 2" Bullet: Revised "SECTION 1080.2 (c)". 4" Bullet: Revised "MANUFACTURES" to "MANUFACTURERS" and added "AND FILL THE HOLE WITH GROUT IN ACCORDANCE WITH PUBLICATION 408, SECTION 1080.2 (c)". 4" Bullet: Revised "FILL HOLES WITH POLYMER MORTAR AND CONCRETE PER BULLETIN 15, SECTION 679.2 (e), PART C, AFTER THE REMOVAL OF THE TEMPORARY BARRIER" to "FILL HOLES WITH RAPID SET CONCRETE PATCHING MATERIAL (TYPE C) AS LISTED IN BULLETIN 15, SECTION 679.2 (e), PART C, AFTER THE REMOVAL OF THE TEMPORARY BARRIER MOUNTED" to "BEHIND A TEMPORARY BARRIER NOUNTED" to "BHIND A TEMPORARY BARRIER MOUNTED" to "BHIND A TEMPORARY BARRIER MOUNTED" to "BHIND A TEMPORARY BARRIER". Note 10 was renumbered to be Note 9. TABLE 2: Revised table based on Barrier Type. Removed "TABLE 2 NOTE". SLOTTED PLATE CONNECTION — PARTIAL PLA

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-719M (cont.)	3 of 7 (formerly 3 of 8)	Removed "NOTE: SEE TABLE 1, SHEET 1 FOR SPACING AND MINIMUM REQUIRED ADHESIVE ANCHOR ULTIMATE CAPACITY." (2 places)
	0 01 0)	ALL DETAILS:
		 Added callout "SEE NOTE 4" to the deck width behind the barrier.
		ADHESIVE ANCHOR DETAIL:
		 Revised Note below title from "CONCRETE TEMPORARY BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC" to "TEMPORARY SINGLE FACE BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC".
		ADHESIVE ANCHOR ON COMPOSITE ADJACENT BOX BEAMS DETAIL:
		 Revised Note below title from "CONCRETE TEMPORARY BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT EITHER FACE" to "TEMPORARY SINGLE FACE BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC".
		** Note: Revise 2 nd sentence from "FOR 5" DECK SLAB THICKNESS, USE ANY TYPE A INSTALLATION OR TYPE B INSTALLATION WITH EITHER 1'-0" OR 2'-0" BOLT SPACINGS" to "FOR 5" DECK SLAB THICKNESS, USE 1'-0" BOLT SPACINGS"
		TYPICAL BOLT THROUGH ANCHOR DETAIL: • Revised Note below title from "CONCRETE TEMPORARY BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC" to "TEMPORARY SINGLE FACE BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC".
		ALTERNATE CONNECTION DETAIL WITH SPACER FOR HAUNCH CLEARANCE LESS THAN 2" DETAIL:
		 Revised Note below title from "CONCRETE TEMPORARY BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC" to "TEMPORARY SINGLE FACE BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC".
		• Revised the Minimum Channel Length from 5'-10" to 4'-10".
		Added callout for "THREADED ANCHOR (MINIMUM LENGTH =
		THICKNESS OF DECK + OVERLAY + THICKNESS OF HAUNCH + 11")".
		ALTERNATE BOLT THROUGH ANCHOR DETAIL:
		 Revised Note below title from "CONCRETE TEMPORARY BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC" to "TEMPORARY SINGLE FACE BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S)
		ADJACENT TO TRAFFIC".
		• Revised the Minimum Channel Length from 5'-10" to 4'-10".
		NOTES:
		Added Note 4.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-719M (cont.)	4 of 7 (formerly 4 of 8)	TITLE BLOCK: • Revised "REINFORCEMENT DETAILS" to "REINFORCEMENT DETAILS – 1".
		 ELEVATION: Removed ELEVATION shown with 2'-0" anchor pocket spacings and added the ELEVATION from original Sheet 5.
		SECTION A-A: • Revised Title "TEMPORARY GLARE SCREEN MEDIAN BARRIER 50" TYPICAL REINFORCEMENT DETAIL to "50" TEMPORARY CONCRETE MEDIAN BARRIER". • Added 2'-4" dimension. • Revised Title "TEMPORARY MEDIAN BARRIER 32" TYPICAL
		REINFORCEMENT DETAIL to "32" TEMPORARY CONCRETE MEDIAN BARRIER". o Added 2'-0" dimension. o Revised "#4" to "#4 SEE ELEVATION FOR SPACING". • Revised Title "TEMPORARY BARRIER 42" TYPICAL REINFORCEMENT BARS" to "42" TEMPORARY SINGLE FACE CONCRETE BARRIER". o Removed the "Deck linework".
		BARRIER DRAINAGE OPENING DETAIL: • Removed text for "5" WIDTH FOR 2'-0" ANCHOR HOLE SPACING". • Added "BARRIER DRAINAGE NOTES".
		NOTES: • Added Note 3.
	5 of 7 (formerly 5 of 8)	TITLE BLOCK: • Revised "END SECTION DETAILS AND REINFORCEMENT DETAILS" to "REINFORCEMENT DETAILS – 2".
		Revised Title "TRAFFIC FACE OF ALTERNATE TEMPORARY CONCRETE BARRIER AND BOTH FACES OF TYPICAL TEMPORARY CONCRETE MEDIAN BARRIER" to "TRAFFIC FACE OF SINGLE FACE TEMPORARY CONCRETE BARRIER AND BOTH FACES OF TEMPORARY CONCRETE MEDIAN BARRIER".
		 ELEVATION: ELEVATION moved to Sheet 4. Added 11" dimensions (2 places). Added B-B (2 places). Revised locations of drainage openings.
		 Revised "SEE REINFORCEMENT DETAILS THIS SHT." to "FOR REINFORCEMENT DETAILS, SEE SECTION A-A". Revised "#4, SEE REINFORCEMENT DETAILS THIS SHT." to "FOR REINFORCEMENT DETAILS, SEE SECTION A-A". Remove note under title for bolt spacing.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-719M (cont.)	5 of 7 (formerly 5 of 8) (cont.)	 PLAN AT ENDS: Added Title "REINFORCEMENT AT SLOTTED PLATE CONNECTION – PLAN (ANCHOR POCKETS NOT SHOWN)". Revised title "TEMPORARY MEDIAN BARRIER – PLAN" to "32" AND 50" TEMPORARY CONCRETE MEDIAN BARRIER". Revised title "TEMPORARY BARRIER – PLAN" to "42" TEMPORARY SINGLE FACE CONCRETE BARRIER". Removed callout "SEE REINFORCEMENT DETAILS THIS SHT.". Added "A-A" (2 places).
		SECTION B-B: • Revised Title "ALTERNATE TEMPORARY MEDIAN BARRIER 32" to "32" TEMPORARY CONCRETE MEDIAN BARRIER". • Added 2'-0" dimension. • Added the 4 7/8" and 2 3/8" dimensions. • Revised Title "TEMPORARY GLARE SCREEN MEDIAN BARRIER 50" to "50" TEMPORARY CONCRETE MEDIAN BARRIER". • Added 2'-4" dimension. • Added the 4 7/8" and 4 1/4" dimensions. • Revised Title "TEMPORARY BARRIER 42" to "42" TEMPORARY SINGLE FACE CONCRETE BARRIER". • Added 3" and 7" dimensions.
		NOTES: Note 3: Added "BARRIER ELEVATION AND".
	6 of 7 (formerly 6 of 8)	Revised Title "TEMPORARY BARRIER TYPICAL REINFORCEMENT BARS" to "42" TEMPORARY SINGLE FACE BARRIER REINFORCEMENT BARS". Revised Title "MEDIAN BARRIER 32" to "32" TEMPORARY MEDIAN BARRIER".
		Revised Title "GLARE SCREEN MEDIAN BARRIER 50" to "50" TEMPORARY MEDIAN BARRIER".
		Revised Title "TEMPORARY MEDIAN BARRIER TYPICAL REINFORCEMENT BARS" to "TEMPORARY MEDIAN BARRIER REINFORCEMENT BARS".
	7 of 7 (formerly 7 of 8)	ALTERNATE ANCHOR POCKET DETAIL: • Revised the title "PARTIAL PLAN B-B" to "PARTIAL PLAN D-D". • PLAN: Revised "A-A" to "C-C". • Revised the title "BARRIER SECTION A-A" to "BARRIER SECTION C-C". • BARRIER ELEVATION: Revised "1" CLR. (TYP.)" to "3/4" CLR (TYP)".
		 ALTERNATE ANCHOR POCKET DETAIL: Revised the title "PARTIAL PLAN B-B" to "PARTIAL PLAN F-F". PARTIAL PLAN F-F: Added 5" dimension. PLAN: Revised "A-A" to "E-E". Revised the title "BARRIER SECTION A-A" to "BARRIER SECTION E-E". BARRIER ELEVATION: Revised "1" CLR. (TYP.)" to "3/4" CLR (TYP)".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-719M (cont.)	7 of 7 (formerly 7 of 8) (cont.)	NOTES: Note 3: Revised "SEE SHEETS 4 AND 5" to "SEE SHEET 4".
	formerly 8 of 8	Sheet removed. Temporary Barrier with Pin Connected Drop-Pin Anchor is not MASH compliant.



TRANSMITTAL LETTER

PUBLICATION:

Publication 219M

DATE:

2/14/2023

SUBJECT:

Standards for Bridge Construction September 2016 Edition Change 5

INFORMATION AND SPECIAL INSTRUCTIONS:

Incorporate the attached revisions into the September 2016 Edition of Publication 219M.

These revisions introduce the PA 3-Rail Bridge Barrier which replaces the Structure Mounted Guide Rail.

These standards may be used immediately and can be adopted as soon as practical on all new and existing projects without affecting letting schedules and in conjunction with the current Publication 408 Specifications. Projects let after October 6, 2023 must incorporate these new standards.

A description of the changes made to the 2016 Edition since Change 4 of November 23, 2022 are listed in the attached multi-sheet Table. On the standards, pink highlighting indicates Change 5 revisions to details and notes.

Comments or questions concerning this Publication may be directed to the Bridge Office.

CANCEL AND DESTROY THE FOLLOWING:

Existing BC-700M Series standards need to be retained for projects under construction and for future rehabilitation work.

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APPROVED FOR ISSUANCE BY:

MICHAEL B. CARROLL
Acting Secretary of Transportation

BY:

Jonathan R. Fleming Chief Executive Highway Administration

The major revisions for each Standard Drawing are presented below. Since minor changes are not indicated, it is strongly advised that all recipients thoroughly examine the changes and revisions incorporated in this release.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-706M	All	New Standard for the PA 3-Rail Bridge Barrier.
	Sheets	
BC-798M	1 of 3	No Changes.
BC-798M	2 of 3	SKEWED BARRIER LAYOUT GUIDELINES:
		 Note 6: Added "S8 BARS FOR THE PA 3-RAIL BRIDGE
		BARRIER ARE NOT SHOWN IN THE PLAN VIEW BUT
		NEED TO BE INCLUDED. IF REQUIRED BEND THE BARS
		NEAR THE GUTTER LINE AND ROTATE BARS TO
		PROVIDE 1 1/2" MIN. CONCRETE COVER. DO NOT
		SHORTEN THE BAR LENGTH."
BC-798M	3 of 3	No Changes.



TRANSMITTAL LETTER

PUBLICATION:

Publication 219M

DATE:

11/23/2022

SUBJECT:

Standards for Bridge Construction September 2016 Edition Change 4

INFORMATION AND SPECIAL INSTRUCTIONS:

Incorporate the attached revisions into the September 2016 Edition of Publication 219M.

These standards may be used immediately and can be adopted as soon as practical on all new and existing projects without affecting letting schedules and in conjunction with the current Publication 408 Specifications. All projects let after April 30, 2023 must incorporate these new standards.

A description of the changes made to the 2016 Edition since Change 3 of February 19, 2021 are listed in the attached multi-sheet Table. On the standards, light orange highlighting indicates Change 4 revisions to details and notes.

Comments or questions concerning this Publication may be directed to the Bridge Office.

CANCEL AND DESTROY THE FOLLOWING:

Existing BC-700M Series standards need to be retained for projects under construction and for future rehabilitation work.

ADDITIONAL COPIES ARE AVAILABLE FROM:

APPROVED FOR ISSUANCE BY:

YASSMIN GRAMIAN, P.E. Secretary of Transportation

BY:

Jona han R. Fleming Chief Executive Highway Administration

Standard	Sheet	Description of Changes
BC-701M	All Sheets	Revised base plate anchor bolt spacing in the transverse direction from 5" to 4" throughout the standard.
BC-709M	General	Updated all rail tube designations throughout the standard from TS to HSS.
	1 of 12	VERTICAL V-NOTCH DETAIL: Corrected depiction of bolt and v-notch locations.
	2 of 12	SPLICE TUBE ELEVATION: Removed redundant ASTM reference from splice tube call-out.
		RAIL TUBE CAP DETAILS: Removed "HAND" from titles of 5" rail details.
	6 of 12	NOTES: Added Note 4 to mention that an alternate strip seal dam (draining to fascia) similar to that shown on BC-767M is permitted when shown on the contract plans
	7 of 12	NOTES: Added Note 4 to mention that an alternate strip seal dam (draining to fascia) similar to that shown on BC-767M is permitted when shown on the contract plans
BC-711M	General	Revised base plate anchor bolt spacing in the transverse direction from 5" to 4" throughout the standard.
	1 of 4	DETAIL A: Revised base plate thickness from 3/4" to 7/8".
		DETAIL B: Revised base plate thickness from 3/8" to 1/2".
	2 of 4	SECTION D-D: Revised base plate thickness from 3/8" to 1/2".
		SECTION C-C: Revised base plate thickness from 3/4" to 7/8".
		ANCHOR BOLT DETAIL: Added dimension to require 9" minimum effective embedment.
		DETAIL E: Revised plate thickness from 3/4" to 7/8".
BC-713M	General	Updated all rail tube designations throughout the standard from TS to HSS.
	1 of 14	GENERAL NOTES: In Note 20 added that the PA Bridge Barrier attached to a moment slab is designated MASH TL-4.
	3 of 14	ANCHOR PLATE DETAIL: Revised cut-out from squared corners to 1" radius corners. Corrected anchor plate width from 11½" to 10¼".
	4 of 14	ELEVATION - PA BRIDGE BARRIER - TYPICAL SIDEWALK: Revised middle rail tube wall thickness from 1/2" to 1/4".
	8 of 14	NOTES: Added Note 5 to mention that an alternate strip seal dam (draining to fascia) similar to that shown on BC-767M is permitted when shown on the contract plans
	9 of 14	NOTES: Added Note 5 to mention that an alternate strip seal dam (draining to fascia) similar to that shown on BC-767M is permitted when shown on the contract plans
BC-716M	1 of 2	ELEVATION: Removed the leveling pad beneath all railing posts and revised extent of splice tubes to match details on Sheet 2.
		DETAIL A: Replaced the leveling pad with leveling nuts on the anchor bolts. Replaced the maximum leveling pad height with a minimum clearance under post base.
		ALTERNATE DETAIL A: Replaced the leveling pad with leveling nuts on the anchor bolts. Replaced the maximum leveling pad height with a minimum clearance under post base.
		SECTION A-A: Replaced the leveling pad with leveling nuts on the anchor bolts.
		TYPICAL SIDEWALK DETAIL: Replaced the leveling pad with leveling nuts on the anchor bolts.
	2 of 2	TYPICAL EXPANSION PANEL DETAIL: Removed leveling pad, added Post CL, correctly extended 7½" dimension to CL Post, and added ½" dimension from CL Post to splice tube.
BC-732M	1 of 3	DETAIL A: Revised length of vertical leg of angle from 2-1/2" to 2" MIN.
BC-736M	2 of 3	DEVELOPMENT LENGTH OF STANDARD HOOKS IN TENSION: Eliminated bar sizes above #11 for conformance with AASHTO 5.10.8.2.1 and DM-4 5.10.8.2.1.
	3 of 3	DEVELOPMENT LENGTH AND LAP SPLICE LENGTH OF DEFORMED BARS IN TENSION: Eliminated bar sizes above #11 for conformance with AASHTO 5.10.8.2.1 and DM-4 5.10.8.2.1.
		DEVELOPMENT LENGTH AND LAP SPLICE LENGTH OF DEFORMED BARS IN TENSION – TABLE B: Revised the development lengths and lap lengths for horizontal bars that had been calculated incorrectly previously.
BC-752M	1 of 3	NOTES: Revised Note 4 including adding references to Publication 408.
		NOTES: Revised Note 5 to remove the PA Bridge Barrier and PA Type 10M Bridge Barrier.
		NOTES: Deleted Note 9 concerning epoxy bonding compound.
		NOTES: Rearranged Notes 6 through 9 (formerly 10).
		NOTES: Updated Pub. 408 section reference in Note 7.
		140 LC. Opudiou i db. 700 Sociali reference ili riole i.

Standard	Sheet	Description of Changes
BC-752M (cont.)	1 of 3 (cont.)	NOTES: Added new Note 10 concerning High Molecular Weight Methacrylate (HMWM).
	2 of 3	ALTERNATE TRANSVERSE CONSTRUCTION AND CRACK CONTROL JOINT: Replaced saw cut and epoxy bonding compound with HMWM.
		CONSTRUCTION JOINT DETAILS, LONGITUDINAL DETAIL: Replaced saw cut and epoxy bonding compound with HMWM.
		CONSTRUCTION JOINT DETAILS, TRANSVERSE DETAIL: Replaced saw cut and epoxy bonding compound with HMWM.
		CONSTRUCTION JOINT DETAILS: Renamed DETAIL X to DETAIL A.
		CONSTRUCTION JOINT DETAILS, DETAIL A: Revised detail for use of HMWM without saw cut instead of Epoxy Bonding Compound in saw cut.
		CONSTRUCTION JOINT DETAILS: Added two notes to details.
	3 of 3 (NEW)	Added new LATEX MODIFIED CONCRETE (LMC) WEARING SURFACE CONSTRUCTION JOINT PREPARATION details which include sequencing.
BC-754M	1 of 2	INTERMEDIATE DIAPHRAGM DETAIL: Added (SEE NOTE 21) to the Typical Connection Detail callouts (two instances) and Alternate Connection Detail callouts (two instances).
		ALTERNATE INTERMEDIATE DIAPHRAGM DETAIL: Added (SEE NOTE 21) to the Typical Connection Detail callouts (two instances) and Alternate Connection Detail callouts (two instances).
		NOTES: Added new Note 21 to provide direction for bolt spacings, edge distances and clearances at the typical and alternate connections.
	2 of 2	DIAPHRAGM CONNECTION PLATE DETAILS, DETAIL F: Added "(MIN.)" to K + 1" dimension (2 instances)
BC-756M	6 of 6	TAPPED SCREW CONNECTION FOR PRESTRESSED CONCRETE BEAM – END VIEW: Added stainless steel sheet welded to sole plate.
		TAPPED SCREW CONNECTION FOR PRESTRESSED CONCRETE BEAM, PLAN – SOLE PLATE: Added stainless steel sheet welded to sole plate and added text to tapped screw callout concerning avoiding interference with stainless steel.
BC-766M	General	Removed outdated notes and callouts that do not apply to an approach slab joint, or revised to make applicable.
		Revised barrier and sidewalk names in detail titles and callouts to match latest BD-601M.
		Revised callouts from SAW CUT OR FORMED GROOVE to SAW CUT SEAL GROOVE.
	1 of 2	GENERAL NOTES: Deleted notes 6, 12, 14, 15, and renumbered subsequent notes.
		GENERAL NOTES: Extended Note 3 to reference Pub. 408 specification for painting galvanized steel surfaces.
		GENERAL NOTES: Revised note 6 (formerly 7) to simply specify a fixed 1/2" movement classification.
		GENERAL NOTES: In note 9 (formerly 10), revised seal recess from 1/4" min. to 1/8" min. and from 1/2" max. to 1/4" max.
		GENERAL NOTES: Added Note 13 to clarify that details on sheet 2 are applicable for Type 1 approach slabs (without overlay) supported by the superstructure.
		PLAN: Added call for preformed neoprene compression seal. Removed dimension "A".
		Replaced TYPICAL JOINT DETAIL with more extensive details similar to deleted Details H and J from BD-628M.
		Moved SECTION AT BARRIER to sheet 2.
	2 of 2	Rearranged sheet.
		Added SECTION AT SPLIT MEDIAN BARRIERS from sheet 1, removed reference to Alternate Detail, added note to see Section Thru Joint with F-Shaped Barrier for additional information.
		Removed ALTERNATE DETAIL.
		Added NOTE to clarify that details on sheet 2 are applicable for Type 1 approach slabs (without overlay) supported by the superstructure.
		SECTION THRU JOINT AT TYPICAL SIDEWALK: Removed beam and underside of deck slab from detail, added note to see Section Thru Joint with F-Shaped Barrier for additional information.

Standard	Sheet	Description of Changes
BC-766M (cont.)	2 of 2 (cont.)	SECTION THRU JOINT WITH ALTERNATE SIDEWALK: Removed beam and underside of deck slab from detail, deleted spliced seal details and associated callouts, continuous seal shown as standard, added note to see Section Thru Joint with F-Shaped Barrier for additional information,
		SECTION A-A: Removed sidewalk and barrier from detail, revised Width of Basic Joint to Sawed Seal Groove, added callout for Formed Basic Joint, revised Closed Cell Foam to Closed Cell Neoprene Sponge, and added note to see joint details on sheet 1 for more information.
		SECTION C-C: Revised JOINT WIDTH to SEAL GROOVE WIDTH, revised DAM to JOINT (2 places), added callout for Formed Basic Joint, and revised additional recess for expansion from JOINT WIDTH + 1/4" to a fixed 3/4".
BC-767M	1 of 7	GENERAL NOTES: In Note 11, updated "Construction and Material Division, Bureau of Project Delivery" to "Materials Division, Bureau of Construction and Materials".
	2 of 7	TYPICAL SECTION AT ABUTMENT FIXED & EXPANSION, FOR P/S SPREAD BEAMS: Revised slab thickness to include haunch.
		TYPICAL SECTION AT PIER FIXED & EXPANSION, FOR P/S SPREAD BEAMS: Revised Slab thickness to include haunch.
	3 of 7	TYPICAL SECTION AT ABUTMENT FIXED & EXPANSION, FOR P/S ADJACENT BEAMS: Revised blockout depth to include the haunch/variable deck thickness over adjacent box beams. TYPICAL SECTION AT PIER FIXED & EXPANSION, FOR P/S ADJACENT BEAMS: Revised
		blockout depth to include the haunch/variable deck thickness over adjacent box beams.
		OVERHANG SECTION AT ABUTMENT - FIXED & EXPANSION: Added detail of typical overhang section (between the outside of the fascia beam and the barrier/gutterline) where the deck does not corbel as it does between beams.
		OVERHANG SECTION AT PIER - FIXED & EXPANSION: Added detail of typical overhang section (between the outside of the fascia beam and the barrier/gutterline) where the deck does not corbel as it does between beams.
	6 of 7	All Sections: Added shoulder cross slope to the details.
	7 of 7 (NEW)	ALTERNATE NEOPRENE STRIP SEAL DAM: Added four details for the Alternate Neoprene Strip Seal Dam which continues under the barrier to the deck fascia and eliminates the upturn miter.
BC-772M	1 of 5	GENERAL, NOTE 2: Added "PRIMARY" before "BRACING"
		Added BRACING REQIREMENT CRITERIA to clarify the requirement of primary lateral bracing, secondary lateral bracing or no lateral bracing.
		STABILITY CRITERIA, NOTE c., W _{wv} WIND PRESSURE: Added missing text "HALF OF" in description of location about which the upward wind pressure acts.
		Added title "INSTALLATION INSTRUCTIONS" over the primary bracing and secondary bracing instructions, and moved the primary bracing instructions before the secondary bracing instructions.
		INSTALLATION INSTRUCTONS, SECONDARY BRACING, NOTE a.: Revised the design method from ASD to LRFD unless noted otherwise; revised the application of the horizontal wind pressure to include interior beams in accordance with the AASHTO Guide Specification for Wind Loads on Bridges During Construction, 1st Edition (2017) which is newly referenced.
		INSTALLATION INSTRUCTONS, SECONDARY BRACING: Added Note f. which lists the design criteria for secondary bracing.
		INSTALLATION INSTRUCTONS, SECONDARY BRACING: Added Note g. concerning design method for cables and turnbuckles.
		Added WIND PRESSURE DISTRIBUTION TO GIRDERS schematic from BD-620M to illustrate the revised wind pressure distribution to exterior and interior beams in accordance with the AASHTO Guide Specification for Wind Loads on Bridges During Construction, 1st Edition (2017).
	2 of 5	PLAN and ELEVATION: Removed the turnbuckles shown at the sides leaving just the turnbuckle along the top; expanded the turnbuckle call-outs to indicate "SIZE AS REQUIRED BY DESIGN, LOCATION DETERMINED BY CONTRACTOR"; added the following note to turnbuckle call-out: "* MINIMUM WORKING LOAD IS 22 KIP FOR 5 GIRDERS. MORE THAN 5 GIRDERS, DESIGN IS REQUIRED."
		PLAN: Removed second paragraph of triple asterisk (***) note.
		ELEVATION: Replaced specified minimum cable diameter with "AS REQUIRED BY DESIGN" and noted that "DESIGN IS REQUIRED".

Standard	Sheet	Description of Changes		
BC-775M	1 of 3	DOWEL DETAIL: For consistency with BD-656M, revised reference point for 1'-0" minimum dowel length from top of substructure unit to bottom of diaphragm and added reduced minimum lengths for 17" and 21" deep beams.		
		GENERAL NOTES: In Note 2, updated terminology from "bituminous tar paper" to "asphalt-saturated paper".		
	2 of 3	TYP. TENDON PLACEMENT: Divided into two details to clarify difference between skews above or below 75°. Increased minimum diaphragm width to be consistent with 6" MIN. offset required in Detail A.		
		DETAIL A: Revised to clarify geometric requirements for tendon pocket.		
		TYPICAL SECTION OF TENDON POCKET: Clarified limits of tendon pocket depth.		
		SHEAR KEY DETAIL: Added width and tolerance of open joint between beams above the shear key.		
	3 of 3	PARTIAL PLAN – BEAMS FOR STAGED CONSTRUCTION: Divided each partial plan into two so that differences between skew angles above and below 75° could be shown.		
		STAGED CONSTRUCTION NOTES: Amended Note 2 to clarify splice chuck pocket's impact on available prestressing strand locations.		
		STAGED CONSTRUCTION NOTES: Added Note 5 to provide instructions when omitting the secondary post-tensioning duct (corresponds with DM-4 revision).		
		SECTION A-A: Moved galvanized plate to Stage 1 side of construction joint. Clarified limits of splice chuck pocked depth. Revised to show ½" joint and 4"- 6" splice chuck pocket in proper proportion to each other.		
BC-788M	1 of 12	NOTES: In Note 8, updated terminology from "bituminous material pavements" to "asphalt pavements".		
-	4 of 12	SECTION U-U: Updated terminology from "bituminous approach" to "asphalt pavement approach".		
-	8 of 12	SECTION G-G: Updated terminology from "bituminous pavement" to "asphalt pavement" and from "bituminous approach" to "asphalt pavement approach".		
		Updated detail title from BITUMINOUS APPROACH AT STRUCTURE to ASPHALT PAVEMENT APPROACH AT STRUCTURE. Also updated terminology from "bituminous approach" to "asphalt pavement approach" in call-out.		
	9 of 12	MEMBRANE WATERPROOFING DETAIL: Revised wearing course type from HMA to WMA (2 instances). Updated terminology from "bituminous wearing course" to "asphalt pavement wearing course".		
	10 of 12	NOTES: Revised Note 1 to remove reduction in waterproofing membrane for fill depths greater than 2′-0″.		
		MEMBRANE WATERPROOFING DETAIL: Revised wearing course type from HMA to WMA. Updated terminology from "bituminous wearing course" to "asphalt pavement wearing course".		
	11 of 12	Updated detail title from BITUMINOUS OVERLAY AND WATERPROOFING MEMBRANE DETAILS AT DECK DRAINS" to "ASPHALT PAVEMENR OVERLAY "		
		Updated terminology from "bituminous wearing course" to "asphalt pavement wearing course" in three details.		
	12 of 12	WATERPROOFING DETAIL AT ABUTMENT WITHOUT BACKWALL, WITH PAVING NOTCH: Updated terminology from "two-ply bituminous paper" to "two-ply asphalt-saturated paper".		
BC-798M		Switched order of sheets 2 and 3.		
	1 of 3	Renamed DETAIL D to DETAIL A and updated reference in JOINT DETAIL.		
		DETAIL A: Added 1/2" dimension of joint.		
	2 of 3	PLAN VIEW – BARRIER VERTICAL REINFORCEMENT AT SEGMENT JOINT: Revised barrier depiction and call-outs to not be specific to one barrier type. Revised note 2 to clearly prohibit the cutting of bars.		
		SKEWED BARRIER LAYOUT GUIDELINES: Deleted note 6 due to the PA Structure Mounted Guiderail being discontinued.		
		GALVANIZED STRAP CONNECTION DETAIL: Renamed SECTION A-A to SECTION B-B.		
		GALVANIZED STRAP CONNECTION DETAIL – ELEVATION: Revised detail to show a single strap. Revised distance from joint to fastener from 10″ to 9″ MIN. for consistency with BD-632M.		

Standard	Sheet	Description of Changes
BC-798M (cont.)		GALVANIZED STRAP CONNECTION DETAIL – CONNECTION STRAP: Revised strap dimensions to be minimums for consistency with BD-632M.
		GALVANIZED STRAP CONNECTION DETAIL – SECTION B-B: Revised bolt, washer and insert call-outs and added double asterisk (**) note. Removed length from strap call-out. Removed note about number of straps per connection.
		Added new END SECTION CONNECTION STRAP PLACEMENT detail.
BC-799M		SHOULDER RELIEF JOINT, PLAN and SECTION K-K: Increased shoulder relief joint and open joint in barrier from 1′-0″ to 2′-0″ for consistency with revisions to BD-628M.

OS-299 (7-08)



TRANSMITTAL LETTER

PUBLICATION:

Publication 219M September 2016 Edition Change No. 3

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February 19, 2021

SUBJECT:

Revisions to
Standards for Bridge Construction
September 2016 Edition

INFORMATION AND SPECIAL INSTRUCTIONS:

Incorporate the attached revisions into the September 2016 Edition of Publication 219M.

These standards are being issued to address bridge barriers and transitions to bridge barriers that are compliant with the AASHTO Manual for Assessing Safety Hardware (MASH 2016).

These standards may be used immediately and can be adopted as soon as practical on new and existing designs without affecting letting schedules. However, projects with T.S.&L. submissions after July 1, 2021 and projects let after April 1, 2022 shall incorporate these standards.

A description of the changes made to the 2016 Edition since Change 2 dated January 31, 2019 are listed in the attached multi-sheet Table. On the standards, light blue highlighting indicates Change 3 revisions. Highlighting of Change 1 and Change 2 revisions has been omitted for clarity.

Comments or questions concerning this Publication may be directed to the Bureau of Project Delivery, Bridge Design and Technology Division.

CANCEL AND DESTROY THE FOLLOWING:

Existing BC-700M Series standards need to be retained for projects under construction and for future rehabilitation work.

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APPROVED FOR ISSUANCE BY:

YASSMIN GRAMIAN, P.E. Secretary of Transportation

DV

Brian G. Thompson, P.E.

Director, Bureau of Project Delivery,

Highway Administration

The major revisions for each Standard Drawing are presented below. Since minor changes are not indicated, it is strongly advised that all recipients thoroughly examine the changes and revisions incorporated in this release.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-701M	1 of 3	NOTES: • Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". • Note 8: Revised "MEETING THE REQUIREMENTS OF" to "CONFORMING TO". Revised "SECTION 705.8(b)" to "SECTION 705.7(b)".
		 SECTION A-A: Revised "TYPICAL OR VERTICAL BARRIER" to ①. Revised "ALTERNATE BARRIER" to ②. Revised note "ALTERNATE SIDEWALK DETAIL SHOWN (TYP. AND ALT. CONCRETE BARRIER SIMILAR)" to "ALTERNATE SIDEWALK WITH 42" VERTICAL WALL CONCRETE BARRIER SHOWN (45", 42" AND 32" F-SHAPE CONCRETE BARRIER SIMILAR)". BASE PLATE DETAIL: Revised 2" to 1 ½" (2 locations). Revised 4" to 5".
		Added "LEGEND: ① 45" F-SHAPE, 42" F-SHAPE OR 42" VERTICAL WALL CONCRETE BARRIER. ② 32" F-SHAPE CONCRETE BARRIER.".
	2 of 3	LEVELING PAD DETAIL: Revised "AT SIDEWALK" to "AT TYPICAL SIDEWALK SHOWN (RAISED SIDEWALK SIMILAR)". Revised 5" to 4½" in section view. Revised 4" to 5" in section and plan view. Revised 2" to 1½" in plan view. BARRIER PROTECTIVE FENCE: Revised "TYPICAL OR VERTICAL BARRIER" to ①. Revised "ALTERNATE BARRIER" to ②. Revised 6" to "6" FOR ①" and "6½" FOR ②" (2 locations). Revised 4" to 5". Revised "ALTERNATE SIDEWALK DETAIL SHOWN (TYP. AND ALT.
		CONCRETE BARRIER SIMILAR) to "ALTERNATE SIDEWALK WITH 42" VERTICAL WALL CONCRETE BARRIER SHOWN (45", 42" AND 32" F-SHAPE CONCRETE BARRIER SIMILAR)". SIDEWALK DETAIL: • Added "TYPICAL" to "SIDEWALK DETAIL".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-701M (cont.)		POST BRACKET DETAIL, PLAN: Revised "3'-6" BARRIER" to ①. Revised "2'-8" BARRIER" to ②. Revised 4" to 5". Revised 2" to 1½". Added "LEGEND: ① 45" F-SHAPE, 42" F-SHAPE OR 42" VERTICAL WALL CONCRETE BARRIER. ② 32" F-SHAPE CONCRETE BARRIER.".
	3 of 3	BARRIER MOUNTED DETAILS, SECTION D-D (left): • Added callout "RAILROAD PROTECTIVE FENCE, CURVED TOP". • Revised to 10'-0" and 8'-0" heights to be measured from top of sidewalk. BARRIER MOUNTED DETAILS, SECTION D-D (right): • Added callout "RAILROAD PROTECTIVE FENCE". • Revised "TYPICAL" to "F-SHAPE". CURB MOUNTED DETAILS, SECTION D-D: • Added callout "RAILROAD PROTECTIVE FENCE, CURVED TOP, CURB MOUNTED". • Added "(RAISED SIDEWALK DETAIL SIMILAR)".
BC-703M	ALL	Standard discontinued.
BC-707M	ALL	Standard discontinued.
BC-708M	ALL	Standard discontinued.
BC-709M	1 of 12	 GENERAL NOTES: Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". Note 2: Revised "IN ACCORDANCE WITH" to "CONFORMING TO". Note 3: Revised to "PROVIDE RAILING POSTS CONFORMING TO AASHTO M270 (ASTM A709) GRADE 50 OR 50W OR ASTM A992. PROVIDE BASE PLATES CONFORMING TO AASHTO M270 (ASTM A709) GRADE 50 OR 50W. PROVIDE ANCHOR PLATES CONFORMING TO AASHTO M270 (ASTM A709) GRADE 36.". Note 4: Revised to "ALL RAILING COMPONENTS SHALL BE GALVANIZED (AFTER FABRICATION) ACCORDING TO PUBLICATION 408, SECTION 1105.02(s) UNLESS OTHERWISE SHOWN ON PLANS. GALVANIZE POSTS, BASE PLATES, ANCHOR PLATES, AND SPLICE SLEEVES ACCORDING TO ASTM A123. GALVANIZE RAIL TUBES ACCORDING TO WITH ASTM A123, EXCEPT COATING ON THREADED STUDS AND NUTS USED WITH THE STUDS SHALL MEET THE REQUIREMENTS OF ASTM A153 FOR CLASS C MATERIAL. GALVANIZE ALL ANCHOR HARDWARE ACCORDING TO ASTM A153 OR ASTM B695.". Note 6: Revised "IN ACCORDANCE WITH" to "CONFORMING TO".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	SHEET 1 of 12 (cont.)	 Note 7: Removed. Note 8: Revised note "8" to "7". Note 9: Revised note "10" to "9". Note 10: Revised note "11" to "10". Revised ⁷/₈" to ⁵/₈". Note 11: Revised note "11" to "10". Revised ⁷/₈" to ⁵/₈". Note 12: Removed. Note 13: Revised note "13" to "11". Note 13: Revised note "13" to "11". Note 14: Removed. Added note - "12. MILL TO BEAR IS DEFINED AS FOLLOWS: A MINIMUM OF 25% OF THE POST WEB AND COMPRESSION FLANGE END AREA MUST FIT WITHIN 1/32" OF THE BASE PLATE WITH NO GAP MORE THAN 0.040" FOR THE REMAINING 75% OF THE END AREA.". Added note - "13. FOR BARRIER RAIL TO POST CONNECTION AND SIDEWALK RAIL CONNECTION, USE AUTOMATIC WELDED THREADED ANCHOR STUDS MEETING THE REQUIREMENTS OF ASTM A108, USE HEX NUTS MEETING THE REQUIREMENTS OF ASTM A563. USE A ³/_{1/6}" THICK PLATE LOCK WASHER ON EACH STUD AND A ³/₈" THICK PLATE ASTM A709, GRADE 36 KSI WASHER. U-WASHERS SHALL MEET THE REQUIREMENTS OF ASTM A709, GRADE 36 KSI STEEL". Added note - "14. FOR ANCHOR BOLTS, USE 1" DIA. BOLTS CONFORMING TO THE REQUIREMENTS OF ASTM F1554, GRADE 105 KSI, INCLUDING THE SUPPLEMENTARY REQUIREMENT, SS, FOR CHARPY IMPACT STRENGTH. USE ASTM A563, GRADE DH HEAVY HEX NUTS. USE ONE ASTM F436 WASHER AT THE TOP.". Added note - "15. NO POST REQUIRED ADJACENT TO FLUSH JOINTS AT WINGWALL, IF POSTS LOCATED AT EXPANSION JOINT/ABUTMENT CORNER.". Added note - "16. THE CENTERLINE OF THE RAIL TUBE SPLICE TO A POST IS TO BE 1".5 MINIMUM MNO 2"-6" MAXIMUM FROM THE CENTERLINE OF THE RAIL TUBE SPLICE TO A POST IS TO BE 1"-8" MINIMUM MNO RODER TO MAINTAIN APPROPRIATE SPACING DIMENSIONS FROM THE END OF THE RAIL, EXPANSION JOINTS AND DRAINAGE SCUPPERS.". Added note - "11. ONE OR MORE 10"-0" MAXIMUM POST SPACINGS MAY BE REDUCED TO 5"-0" MINIMUM IN ORDER TO MAINTAIN APPROPRIATE SPACING DIMENSIONS FROM THE END OF THE RAIL, EXPANSION JOINTS AND DRAINAGE SCUPPERS.". Added note - "19. PROVIDE RAIL TUB

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	1 of 12 (cont.)	 Added note – "22. THE PA TYPE 10M BRIDGE BARRIER IS DESIGNATED AS MASH TL-4.". Added note – "23. FOR GUIDE RAIL TRANSITION TO PA TYPE 10M BRIDGE BARRIER, SEE RC-50M.". Added note – "24. PROVIDE VERTICAL V-NOTCHES ON BARRIER WALL FRONT AND REAR FACES AT ALL POST ANCHOR BOLT LOCATIONS. SEE DETAIL THIS SHEET.".
		 Added "BARRIER WALL" to 1'-6". Revised 2'-11" to 3'-3". Added a second row of anchor bolts and replaced anchor bar with an anchor plate. Added 4" horizontal dimension. Added "1'-5" BARRIER WALL". Added "(SEE DETAIL, SHEET 2)" to "DELINEATOR AT EACH POST". Added 3/4" to BASE PL. Added 1/2" vertical dimension. Added 1/2" horizontal dimension. Revised "ANCHOR BAR 2" X 3/8" X 6" (GALVANIZED)" to "ANCHOR PLATE 6" X 6" X 1/4" (GALVANIZED)(SEE DETAIL, SHEET 2)". Added "CONSTR. JT. & V-NOTCH (RAKED FINISH)".
		 ELEVATION-POST: Added 7³/8" vertical dimension. Revised "RAILING POST W8 x 18" to "W8 x 18 POST". Revised "½" θ DRAIN HOLE (SEAL WELD PLATED INSIDE DRAIN HOLE)" to "½" θ DRAIN HOLE (1/8" ABOVE WELD) (SEAL WELD PLATE INSIDE DRAIN HOLE)".
		PLAN-POST: • Moved to sheet 2. • Revised "PLAN – POST" to "BASE PLATE DETAIL". • Revised "* SEE NOTE" to "MILL TO BEAR (SEE NOTE 12, SHEET 1)". • Removed "RAILING" from W8 x 18 POST. • Added 2 ⁷ / ₈ " and 4" horizontal dimensions. • Added 2 additional 1 ¹ / ₈ " diameter holes.
		Removed "ANCHOR BAR DETAIL" detail. ALTERNATE SIDEWALK RAIL – BARRIER SECTION: • Moved to sheet 4. • Added "BARRIER WALL" to 1'-6". • Added a second row of anchor bolts and replaced anchor bar with an anchor plate. • Added 4" horizontal dimension. • Added 3'-10" vertical dimension. • Added "1'-5" BARRIER WALL". • Added 34" to BASE PL. • Added ½" vertical dimension. • Added ½" horizontal dimension.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	1 of 12 (cont.)	 Revised section marks "A" to "C". Added "CONSTR. JT. & V-NOTCH (RAKED FINISH)". Revised "ANCHOR BAR 2" X ³/₈" X 6" (GALVANIZED)" to "ANCHOR PLATE 6" X 6" X ¹/₄" (GALVANIZED)(SEE DETAIL SHEET 2)". Added "(SEE DETAIL, SHEET 2)" to DELINEATOR AT EACH POST. ALTERNATE SIDEWALK RAIL – ELEVATION-POST:
		 Moved to sheet 4. Added 7³/₈" vertical dimension. Removed "RAILING" from W8 x 18 POST. Revised ½" φ DRAIN HOLE ¹/₈" ABOVE WELD" to "½ φ DRAIN HOLE (¹/₈" ABOVE WELD) (SEAL WELD PLATE INSIDE DRAIN HOLE)".
		ANCHOR STUD DETAIL: • Moved to sheet 2.
		 SECTION A-A: Moved to sheet 4: Revised "SECTION A-A" to "SECTION C-C". Added "HAND" to RAIL SUPPORT ANGLE. Shown opposite hand.
		DELINEATOR DETAIL: • Moved to sheet 2.
		REFERENCE DRAWINGS: • Removed "BC-708M THRIE-BEAM TO PA TYPE 10M BRIDGE BARRIER TRANSITION CONNECTION".
		Added "PA TYPE 10M BRIDGE BARRIER" with "PLAN" and "ELEVATION" views to show new MASH barrier end transition.
		Added "BARRIER WALL GEOMETRY DETAIL".
		Added "VERTICAL V-NOTCH DETAIL".
	2 of 12	Added "RECESS SECTION".
		Added "BASE PLATE DETAIL" detail from sheet 1.
		 RAILING JOINTS ELEVATION: Added "1'-0" MIN." to OFFSET RAILING. Removed expansion joint detailing.
		DELINEATOR DETAIL: • Moved from sheet 1.
		Added "ANCHOR PLATE DETAIL".
		TYPICAL WELD AT MITERS: • Moved from sheet 3.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	2 of 12 (cont.)	RAIL TUBE CAP DETAILS: • Moved from sheet 3.
		RAIL SPLICE: • Moved from sheet 4.
		RAIL SPLICE TABLE: • Moved from sheet 4
		SPLICE TUBE: • Moved from sheet 4.
		SIDEWALK RAIL – BARRIER SECTION: • Moved to sheet 3. • Shown opposite hand. • Added "BARRIER WALL" to 1'-6". • Revised "VEHICULAR WIDTH" to "ROADWAY". • Added 7³/ ₈ " horizontal dimension. • Added 4" horizontal dimension. • Added a second row of anchor bolts and replaced anchor bar with an anchor plate. • Revised "3'-6" to 3'-10". • Added 3'-3" vertical dimension. • Added "1'-5" BARRIER WALL". • Added "3/4" to BASE PL. • Added ½" vertical dimension. • Added ½" horizontal dimension. • Added "ANCHOR PLATE 6"X6"X¼" (GALVANIZED) (SEE DETAIL, SHEET 2)". • Added "CONSTR. JT. (RAKED FINISH)". • Added 2" dimension.
		 SIDEWALK RAIL – DETAIL A: Move to sheet 3. Shown opposite hand. SECTION B-B: A Manual to sheet 3.
		 Moved to sheet 3. ELEVATION-POST: Moved to sheet 3. Shown opposite hand. Revised "VEHICULAR WIDTH" to "ROADWAY". Added 7³/₈" vertical dimension. Revised ½" \$\phi\$ DRAIN HOLE" to "½ \$\phi\$ DRAIN HOLE (1/8" ABOVE WELD) (SEAL WELD PLATE INSIDE DRAIN HOLE)".
		Removed "RAISED SIDEWALK RAIL" details.
	3 of 12	TITLE BLOCK: • Revised "END OF RAIL" to "TYPICAL SIDEWALK".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	3 of 12 (cont.)	Added "PA TYPE 10M BRIDGE BARRIER TYPICAL SIDEWALK". • "PLAN" and "ELEVATION" views.
		BARRIER SECTION: • Moved from sheet 2.
		DETAIL A: • Moved from sheet 2.
		SECTION B-B: • Moved from sheet 2.
		ELEVATION-POST: • Moved from sheet 2.
		TYPICAL WELD AT MITERS: • Moved to sheet 2. • Shown opposite hand.
		 RAIL TUBE CAP DETAIL: Moved to sheet 2. Shown opposite hand. Removed end connection angle and anchor studs along with associated notes and dimensions.
		Removed "TYPICAL RAIL" detail.
		Removed "SIDEWALK RAIL" and "TOP VIEW" details. Removed "ALTERNATE SIDEWALK RAIL" and "TOP VIEW" details.
	4 of 12	TITLE BLOCK: • Revised "END OF RAIL" to "ALTERNATE SIDEWALK".
		Added "PA TYPE 10M BRIDGE BARRIER ALTERNATE SIDEWALK". • "PLAN" and "ELEVATION" views to show new MASH barrier end transition.
		Added "RAIL TUBE END DETAIL". • "PLAN" and "ELEVATION" views to show additional handrail dimensions.
		BARRIER SECTION: • Moved from sheet 1.
		ELEVATION-POST: • Moved from sheet 1.
		SECTION C-C: • Moved from sheet 1.
		Removed "RAISED SIDEWALK RAIL" and "TOP VIEW" details.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	4 of 12 (cont.)	Removed "RAISED SIDEWALK RAIL" and "TOP VIEW" details.
(cont.)	(cont.)	Removed "END CAP SIZES FOR VARIOUS RAIL TUBES TABLE".
		 RAIL SPLICE: Moved to sheet 2. Removed "TS 5 x 3 x ³/₈ AND". Revised "SECTION C-C" to SECTION A-A". Revised section marks "C-C" to "A-A".
		RAIL SPLICE TABLE: • Moved to sheet 2. • Removed "TS 5 x 3 x ³ / ₈ ". • Removed "TS 4 x 2 x ⁵ / ₁₆ ASTM A500, GR. B OR C".
		SPLICE TUBE: • Moved to sheet 2. • Removed "AS DESIGNED".
	5 of 12	TITLE BLOCK: • Revised "MISCELLANEOUS DETAILS" to "DETAILS AT TOOTH EXPANSION DAM".
		PLAN – SKEW ANGLE ≥ 75°: • Revised "CURB" to "BARRIER WALL". • Revised section marks "E-E" to "F-F". • Revised "(AT GUTTER LINE SHOWN; AT END OF ALTERNATE SIDEWALK SIMILAR)" to "(AT GUTTER LINE SHOWN; AT ALTERNATE SIDEWALK SIMILAR)". • Corrected concrete recess.
		PLAN AT SIDEWALK – SKEW ANGLE ≥ 75°: • Revised "PLAN AT SIDEWALK – SKEW ANGLE ≥ 75°" to "PLAN AT TYPICAL SIDEWALK – SKEW ANGLE ≥ 75°". • Revised "CURB" to "BARRIER WALL". • Revised section marks "F-F" to "E-E". • Revised section marks "E-E" to "F-F". • Removed "(AT SIDEWALK SHOWN; AT RAISED SIDEWALK SIMILAR)".
		 PLAN AT SIDEWALK – SKEW ANGLE < 75°: Revised "PLAN AT SIDEWALK – SKEW ANGLE < 75°" to "PLAN AT TYPICAL SIDEWALK – SKEW ANGLE < 75°". Revised "CURB" to "BARRIER WALL". Added "SIDEWALK". Revised section marks "F-F" to "E-E". Revised section marks "E-E" to "F-F". Removed "(AT SIDEWALK SHOWN; AT RAISED SIDEWALK SIMILAR)".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	5 of 12 (cont.)	PLAN – SKEW ANGLE < 75°: Revised "CURB" to "BARRIER WALL". Revised section marks "E-E" to "F-F". Revised "(AT GUTTER LINE SHOWN; AT END OF ALTERNATE SIDEWALK SIMILAR)" to "(AT GUTTER LINE SHOWN; AT ALTERNATE SIDEWALK SIMILAR)" Corrected concrete recess.
		SECTION D-D: • Added "(NO SIDEWALK CONDITION SHOWN; ALTERNATE SIDEWALK SIMILAR)". • Revised 6" to 8".
		 SECTION F-F: Revised "SECTION F-F" to "SECTION E-E". Removed "(AT SIDEWALK). Added "SIDEWALK". Added "BARRIER WALL". Added "GUTTERLINE". Revised 6" to 8". Fixed weld on rear face.
		SECTION E-E: • Revised "SECTION E-E" to "SECTION F-F". • Added "(SECTION G-G IS OPPOSITE HAND)".
		PA TYPE 10M BRIDGE BARRIER AT EXPANSION TOOTH DAM: • Revised "PA TYPE 10M BRIDGE BARRIER AT EXPANSION TOOTH DAM" to "PA TYPE 10M BRIDGE BARRIER AT TOOTH EXPANSION DAM".
		NOTES: • Note 2: Revised "CURB" to "BARRIER WALL". • Note 3: Added "BRIDGES" (2 locations).
		Removed "SECTION F-F" at raised sidewalk. Removed "SECTION G-G" at raised sidewalk.
	6 of 12	TITLE BLOCK: • Revised "MISCELLANEOUS DETAILS" to "DETAILS AT NEOPRENE STRIP SEAL DAM".
		PLAN – SKEW ANGLE ≥ 75°: • Revised "CURB" to "BARRIER WALL". • Corrected concrete recess. • Revised "(SEE NOTE 3)" to "(SEE NOTE 2)". • Revised "(SEE NOTE 2)" to "(SEE NOTE 1)".
		PLAN AT SIDEWALK – SKEW ANGLE ≥ 75°: • Revised "PLAN AT SIDEWALK – SKEW ANGLE ≥ 75°" to "PLAN AT TYPICAL SIDEWALK – SKEW ANGLE ≥ 75°".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	SHEET 6 of 12 (cont.)	Revised "CURB" to "BARRIER WALL". Revised section marks "K-K" to "I-J". Revised section marks "J-J" to "K-K". Revised section marks "J-J" to "K-K". Removed "(AT SIDEWALK SHOWN; AT RAISED SIDEWALK SIMILAR)". Revised "(SEE NOTE 3)" to "(SEE NOTE 2)". Revised "(SEE NOTE 2)" to "(SEE NOTE 1)". PLAN AT SIDEWALK - SKEW ANGLE < 75°. Revised "PLAN AT SIDEWALK - SKEW ANGLE < 75°. Revised "PLAN AT SIDEWALK - SKEW ANGLE < 75°. Revised "CURB" to "BARRIER WALL". Added "SIDEWALK". Revised "CURB" to "BARRIER WALL". Revised section marks "I-J" to "K-K". Revised section marks "I-J" to "K-K". Revised section marks "K-K" to "I-J". Removed "(AT SIDEWALK SHOWN; AT RAISED SIDEWALK SIMILAR)". Revised "(SEE NOTE 3)" to "(SEE NOTE 2)". Revised "(SEE NOTE 2)" to "(SEE NOTE 1)". PLAN - SKEW ANGLE < 75°. Revised "GEE NOTE 3)" to "(SEE NOTE 2)". SECTION K-K: Revised "SECTION K-K" to "SECTION J-J". Revised "GEE NOTE 3)" to "(SEE NOTE 2)". SECTION I-J: Revised "SECTION SHOWALK). Added "SIDEWALK/BARRIER WALL/GUTTERLINE". SECTION J-J: Revised "SECTION J-J" to "SECTION K-K". Revised "NOTE 3" to "NOTE 2". SECTION H-H: Revised "OTE 3" to "NOTE 2". SECTION H-H: Revised "SECTION J-J" to "SECTION SHOWALK). Note 3: Revised note "3" to "2". Added "BRIDGES" (2 locations). Note 4" Revised note "3" to "2". Added "BRIDGES" (2 locations). Note 4" Revised note "3" to "2". Added "BRIDGES" (2 locations). Note 4" Revised note "3" to "3". Revised "MAXIMUM DISTANCE FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3." to "MAXIMUM DISTANCE FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3." to "MAXIMUM DISTANCE ALONG THE EXTRUSION FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3.". Removed "SECTION L-L".
		Moved from sheet 7.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	7 of 12	TITLE BLOCK: • Revised "MISCELLANEOUS DETAILS" to "DETAILS AT NEOPRENE STRIP SEAL DAM". PLAN AT ALTERNATE SIDEWALK – SKEW ANGLE ≥ 75°:
		 Revised "CURB" to "BARRIER WALL". Revised "NOTE 3" to "NOTE 2". Revised "NOTE 2" to "NOTE 1". Corrected concrete recess.
		PLAN AT ALTERNATE SIDEWALK – SKEW ANGLE < 75°: • Revised "CURB" to "BARRIER WALL". • Added "SIDEWALK". • Revised "NOTE 3" to "NOTE 2". • Revised "NOTE 2" to "NOTE 1".
		Corrected concrete recess.
		SECTION M-M: • Revised 6" to 8".
		SECTION N-N: • Revised "NOTE 3" to "NOTE 2".
		 NOTES: Note 1: Removed. Note 2: Revised note "2" to "1". Revised "CURB" to "BARRIER WALL". Note 3: Revised note "3" to "2". Added "BRIDGES" (2 locations). Note 4" Revised note "4" to "3". Revised "MAXIMUM DISTANCE FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3." to "MAXIMUM DISTANCE ALONG THE EXTRUSION FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3.".
		SECTION C-C: • Moved to sheet 6. • Revised "SECTION C-C" to "SECTION L-L". • Revised "NOTE 3" to "NOTE 2".
		Removed "SECTION L-L".
	8 of 12	TITLE BLOCK: • Revised "MISCELLANEOUS DETAILS" to "ALUMINUM PROTECTIVE BARRIER DETAILS".
		 DETAIL B: Revised "DETAIL B" to "DETAIL C". Added "FILL PLATE". Added "½" CLR." Added "THREADS OF BOLTS TO BE BURRED OFF AT FACE OF NUT AFTER CONNECTION IS MADE.".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	8 of 12 (cont.)	 DETAIL C: Revised "DETAIL C" to "DETAIL B". Removed "RAILING" from W8 x 18 POST. Revised "CURB" to "BARRIER WALL". Added "CL 11/16"θ HOLES FOR". SECTION Q-Q: PLAN:
		 Revised "CONCRETE CURB" to "BARRIER WALL" (2 locations). Revised "DETAIL C" to "DETAIL B". Removed "RAILING" from W8 x 18 POST.
		 SECTION P-P (TYPICAL): Revised 8'-6" to 8'-3". Revised 2'-10" to 2'-9" (3 locations). Removed "RAILING" from W8 x 18 POST. Revised "ANCHOR BAR 2"x3/8"x6" (GALV.)(TYP.)(SEE DETAIL, SHEET 1)" to "Anchor plate 6"x6"x ¹/₄" GALV.)(TYP.)(SEE DETAIL, SHEET 2)". Revised "CURB" to "BARRIER WALL". Revised 2'-11" to 3'-3". Revised 6'-8 ¹/₂" to 6'-5 ¹/₂". Revised "4" anchor bolts to "2".
		SECTION P-P (WITH ALTERNATE SIDEWALK): Revised 9'-0" to 8'-9". Revised 3'-0" to 2'-11" (3 locations). Added "SEE DETAIL C, THIS SHEET". Revised "ANCHOR BAR 2"x3/8"x6" (GALV.)(TYP.)(SEE DETAIL, SHEET 1)" to "ANCHOR PLATE 6"x6"x 1/4" GALV.)(TYP.)(SEE DETAIL, SHEET 2)". Revised "CURB" to "BARRIER WALL". Revised 3'-6" to 3'-10". Revised 6'-7 1/2" to 6'-4 1/2". Revised "4" anchor bolts to "2". Added "Q-Q" section callout.
		 Revised 1'-1" to "1'-5" BARRIER WALL". Revised 2'-11" to 3'-3".
	9 of 12	TITLE BLOCK: • Revised "MISCELLANEOUS DETAILS" to "ALUMINUM PROTECTIVE BARRIER DETAILS".
		POST AND BASE PLATE: • Revised 2 ½" to 2 7/8". • Revised 4 3/8" to 4". • Revised "SHEET 1" to "SHEET 2".
		POST MOUNTING ON GRADE: • Revised "CONCRETE CURB" to "BARRIER WALL".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	9 of 12 (cont.)	SECTION S-S: • Revised "TOP WIDTH" to "BARRIER WALL".
		NOTES: • Note 1: Revised "1107.02(p)" to "1113.03(h)" (2 locations).
	10 of 12	TITLE BLOCK: • Revised "MISCELLANEOUS DETAILS" to "M.S.E. WALL DETAILS".
		 TYPICAL CAST-IN-PLACE BARRIER DIMENSIONS ON M.S.E. WALLS: Revised "TYPICAL CAST-IN-PLACE BARRIER DIMENSIONS ON M.S.E. WALLS" to "CAST-IN-PLACE BARRIER DIMENSIONS ON M.S.E. WALLS". Added 4" horizontal dimension. Added a second row of anchor bolts and replaced anchor bar with an anchor plate. Added "BARRIER WALL" on 1'-6". Revised 1'-1" to "1'-5" BARRIER WALL". Revised 3'-3" to 3'-7". Added 3'-3". TYPICAL C.I.P. BARRIER WITH CEMENT CONCRETE SHOULDER ON M.S.E. WALLS: Revised "TYPICAL C.I.P. BARRIER WITH CEMENT CONCRETE SHOULDER ON M.S.E. WALLS". Added a second row of anchor bolts and replaced anchor bar with an anchor plate. TYPICAL C.I.P. BARRIER WITH BITUMINOUS SHOULDER ON M.S.E. WALLS: Revised "TYPICAL C.I.P. BARRIER WITH BITUMINOUS CONCRETE SHOULDER ON M.S.E. WALLS". Added a second row of anchor bolts and replaced anchor bar with an anchor plate.
		 CONCRETE CURB ELEVATION: Revised "CONCRETE CURB ELEVATION" to "BARRIER WALL ELEVATION". Revised "CONCRETE CURB" to "BARRIER WALL". Revised "SHOULDER" to "MOMENT SLAB".
		NOTES: • Note 2: Revised "CONCRETE CURB" to "BARRIER WALL" (2 locations). Add "8 AND".
	11 of 12	TITLE BLOCK: • Revised "MISCELLANEOUS DETAILS" to "M.S.E. WALL DETAILS".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	11 of 12 (cont.)	PRECAST BARRIER WITH BITUMINOUS SHOULDER: Revised "PRECAST BARRIER WITH ASPHALT-PAVED SHOULDER ON M.S.E. WALLS". Added a second row of anchor bolts and replaced anchor bar with an anchor plate. Revised "CONCRETE CURB" to "BARRIER WALL". PRECAST BARRIER WITH CEMENT CONCRETE SHOULDER: Revised "PRECAST BARRIER WITH CEMENT CONCRETE SHOULDER: Revised "PRECAST BARRIER WITH CEMENT CONCRETE SHOULDER: Revised "PRECAST BARRIER WITH CEMENT CONCRETE SHOULDER: Added a second row of anchor bolts and replaced anchor bar with an anchor plate. Revised "CONCRETE CURB" to "BARRIER WALL". DIMENSIONS: Added a second row of anchor bolts and replaced anchor bar with an anchor plate. Revised 3'-3" to 3'-7". Revised 11 ½" to 1'-3 ½". Revised 11 ½" to 1'-3 ½". Revised 11 ½" to 1'-3 ½". Revised 1'-1" to "T'-5" BARRIER WALL". REINFORCEMENT FOR BARRIER WITH BITUMINOUS SHOULDER: Revised "REINFORCEMENT FOR BARRIER WITH BITUMINOUS SHOULDER: Revised "REINFORCEMENT FOR BARRIER WITH BITUMINOUS SHOULDER: Revised "CONCRETE CURB" to "BARRIER WALL". REINFORCEMENT FOR BARRIER WITH CEMENT CONCRETE SHOULDER: Revised "CONCRETE CURB" to "BARRIER WALL". REINFORCEMENT FOR BARRIER WITH CEMENT CONCRETE SHOULDER: Revised "REINFORCEMENT FOR BARRIER WALL". REINFORCEMENT FOR BARRIER WITH CEMENT CONCRETE SHOULDER: Revised "CONCRETE CURB" to "BARRIER WITH CONCRETE CURB" to "BARRIER WITH CONCRETE CURB" to "BARRIER WALL". REINFORCEMENT FOR BARRIER WITH CEMENT CONCRETE SHOULDER: Revised "CONCRETE CURB" to "BARRIER WALL". REINFORCEMENT FOR BARRIER WITH CEMENT CONCRETE SHOULDER: Revised "CONCRETE CURB" to "BARRIER WALL". REVISED "CONCRETE CURB" to "B
	12 of 12	SECTION T-T: • Revised 1'-1" to "1'-5" BARRIER WALL". JUNCTION BOX NOTES: • Note 3: Revise "SIDEWALK RAIL" to "TYPICAL SIDEWALK". • Note 4: Removed.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-709M (cont.)	12 of 12 (cont.)	 OPEN JOINT NOTES: Note 2: Revised "PROVIDE CAULKING COMPOUND IN ACCORDANCE WITH SECTION 705.8(b) OF PUB 408." to "PROVIDE CAULKING COMPOUND CONFORMING TO PUBLICATION 408, SECTION 705.7(b).". Note 3: Revised "PROVIDE JOINT BACKING MATERIAL IN ACCORDANCE WITH SECTION 705.9 OF PUB. 408." to "PROVIDE JOINT BACKING MATERIAL CONFORMING TO PUBLICATION 408, SECTION 705.8.". Note 4: Revised "PROVIDE PREMOLDED EXPANSION JOINT FILLER IN ACCORDANCE WITH PUB. 408, SECTION 705.1." to "PROVIDE PREMOLDED EXPANSION JOINT FILLER CONFORMING TO PUBLICATION 408, SECTION 705.1.".
BC-711M	1 of 4	SECTION G-G: • Fixed fillet weld symbol. NOTES: • Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". • Note 6: Revised "MEETING THE REQUIREMENTS OF SECTION 705, PUBLICATION 408" to "CONFORMING TO PUBLICATION 408, SECTION 705.7(b)." REFERENCE DRAWINGS: • Deleted "BC-739M BRIDGE BARRIER TO GUIDE RAIL TRANSITION".
	2 of 4	 SECTION C-C: Added * to PROTECTIVE BARRIER. Revised 1'-6" to 1'-5". Revised 4'-6" to 4'-3". Added "*DIMENSIONS BASED ON 45" F-SHAPE CONCRETE BARRIER. FOR 42" F-SHAPE CONCRETE BARRIER AND 42" VERTICAL WALL CONCRETE BARRIER, USE 4'-6" ALUMINUM PROTECTIVE BARRIER HEIGHT AND 1'-6" PANEL HEIGHTS.". TYPICAL SLOPED BARRIER SECTION: Revised "TYPICAL SLOPED BARRIER SECTION" to "F-SHAPE CONCRETE BARRIER SECTION". Added "3'-9" OR" to 3'-6". ALTERNATE SIDEWALK SECTION: Revised "ALTERNATE SIDEWALK SECTION" to "ALTERNATE SIDEWALK WITH 42" VERTICAL WALL CONCRETE BARRIER SECTION". NOTES: Note 1: Revised "AND CONFORM TO SECTION 1103 OF PUBLICATION 408" to "CONFORMING TO PUBLICATION 408, SECTION 1103". Note 2: Revised "WHICH CONFORM TO SECTION 1103 OF PUBLICATION 408" to "CONFORMING TO PUBLICATION 408, SECTION 1103".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-711M	2 of 4 (cont.)	Note 6: Revised to "PROTECTIVE BARRIER CONNECTION DETAIL SHOWN FOR 42" AND 45" F-SHAPE CONCRETE BARRIER SECTION, TYPICAL SIDEWALK SECTION AND ALTERNATE SIDEWALK WITH 42" VERTICAL WALL CONCRETE BARRIER SECTION. FOR WIDER BARRIERS WIDTHS, HOLD INSIDE FACE FLUSH.".
BC-712M	ALL	Standard discontinued.
BC-713M	OLD 1 of 13 NEW 1 of 14	TITLE BLOCK: Revised "SHEET 1 OF 13" to "SHEET 1 OF 14". NOTES: Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". Note 2: Revised "IN ACCORDANCE WITH" to "CONFORMING TO". Note 3: Revised to "PROVIDE RAILING POSTS CONFORMING TO AASHTO M270 (ASTM A709) GRADE 50 OR 50W OR ASTM A992. PROVIDE BASE PLATES CONFORMING TO AASHTO M270 (ASTM A709) GRADE 50 OR 50W. PROVIDE ANCHOR PLATES CONFORMING TO AASHTO M270 (ASTM A709) GRADE 36.". Note 4: Revised "ALL RAILING COMPONENTS SHALL BE GALVANIZED (AFTER FABRICATION) ACCORDING TO PUBLICATION 408, SECTION 1105.02(s) UNLESS OTHERWISE SHOWN ON THE PLANS. GALVANIZE POSTS, BASE PLATES, ANCHOR PLATES AND SPLICE SLEEVES ACCORDING TO ASTM A123, GALVANIZE RAIL TUBES ACCORDING TO ASTM A123, EXCEPT COATING ON THREADED STUDS AND NUTS USED WITH THE STUDS SHALL MEET THE REQUIREMENTS OF ASTM A135 FOR CLASS C MATERIAL. GALVANIZE ALL ANCHOR HARDWARE ACCORDING TO ASTM A153 FOR CLASS C MATERIAL. GALVANIZE ALL ANCHOR HARDWARE ACCORDING TO ASTM A153 FOR CLASS C MATERIAL. GALVANIZE STHAN 1,500 FEET". Note 5: Added "WHEN RADIUS IS LESS THAN 1,500 FEET". Note 8: Removed "-2002" (3 locations). Note 9: Revised "MEETING THE REQUIREMENTS OF" to "CONFORMING TO" (3 locations). Note 10: Removed. Note 10: Removed. Note 10: Removed. Note 16: Revised note "16" to "14". Note 15: Removed. Note 15: Removed. Note 16: Revised note "10" to "16". Note 20: Revised note "20" to "17". Note 21: Revised note "21" to "16". Note 22: Revised note "22" to "19". Revised "MEETING THE REQUIREMENTS OF SCETION 705.8, PUBLICATION 408" to "ACCORDING TO PUBLICATION 408, SECTION 705.7(b)". Added note 20 — "THE PA BRIDGE BARRIER IS DESIGNATED AS MASH TL-5.". Added note 21 — "FOR GUIDE RAIL TRANSITION TO PA BRIDGE BARRIER, SEE RC-50M.".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	OLD 1 of 13 NEW 1 of 14 (cont.)	• Added note – "22. PROVIDE VERTICAL V-NOTCHES ON BARRIER WALL FRONT AND REAR FACES AT ALL POST ANCHOR BOLT LOCATIONS. SEE DETAIL THIS SHEET.". POST DETAIL: • Revised "POST DETAIL" to "ELEVATION-POST". • Revised "RAILING POST W8x31" to "W8x31 POST". • Added "FOR DELINEATOR" after CL ¾" ∮ HOLE. BARRIER WALL GEOMETRY DETAIL: • Added "REAR FACE". TYPICAL PA BRIDGE BARRIER ELEVATION: • Removed detail. TYPICAL SECTION: • Revised "TYPICAL SECTION" to "SECTION A-A BARRIER SECTION". • Added "BARRIER WALL" to 1'-6". • Revised "RAILING POST W8x31 SEE POST DETAIL" to "W8x31 POST". • Added ½" horizontal dimension. • Added ½" horizontal dimension. • Added 4'-2" vertical dimension. • Added 4'-2" vertical dimension. • Added 2'-0" BARRIER WALL. • Added "CONSTR. JOINT AND V-NOTCH (RAKED FINISH)". • Revised "SHEET 5" to "SHEET 3" (2 locations). REFERENCE DRAWINGS: • Removed "BC-712M THRIE-BEAM TO PA BRIDGE BARRIER TRANSITION CONNECTION". Added "PLAN" to show new MASH barrier end transition.
	OLD 2 of 13 NEW 2 of 14	Added "VERTICAL V-NOTCH DETAIL". TITLE BLOCK: Revised "SHEET 2 OF 13" to "SHEET 2 OF 14". RAIL SPLICE TABLE: Removed "NOTE: FOR SIDEWALK RAIL DETAILS, SEE SHEET 3.". DETAIL B RAIL SPLICE: Revised "DETAIL B RAIL SPLICE" to "RAIL TUBE SPLICE". DETAIL C POST TO BASE PLATE WELD: Revised "DETAIL C POST TO BASE PLATE WELD" to "DETAIL B BASE PLATE DETAIL". Revised "TRAFFIC" to "ROADWAY". Added 1'-2 ½" vertical dimension. Added 1'-0" horizontal dimension.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	OLD 2 of 13 NEW	SIDEWALK RAIL ROD ANCHOR PLATE DETAIL: • Added "(SEE NOTE 1)".
	2 of 14	NOTES:
	(cont.)	• Note 1: Removed.
		• Note 2: Revised note "2" to "1".
		• Added new note – "2. FOR ADDITIONAL NOTES, SEE SHEET 1.".
		ALTERNATE BARRIER SECTION:
		Moved to sheet 5.
		Shown opposite hand.
		 Revised "ALTERNATE BARRIER SECTION" to "SECTION E-E BARRIER SECTION".
		• Added "BARRIER WALL" to 1'-6".
		• Added ½" horizontal dimension.
		Added 1" horizontal dimension.
		Added 4'-2" vertical dimension.
		Added 2'-0" BARRIER WALL. By the All Programs 21" of the State o
		• Revised "RAILING POST W8x31" to "W8x31 POST".
		 Revised "SHEET 5" to "SHEET 3" (2 locations). Added "CONSTR. JOINT AND V-NOTCH (RAKED FINISH)".
		Added "ON SHEET 2".
		ALTERNATE POST DETAIL:
		Moved to sheet 5.
		Shown opposite hand. Shown opposite hand.
		Revised "ALTERNATE POST DETAIL" to "ELEVATION-POST". Revised "ALTERNATE POST NO. 21" ("YV) 21 POST". Revised "ALTERNATE POST NO. 21" ("YV) 21 POST".
		 Revised "RAILING POST W8x31" to "W8x31 POST". Added "FOR DELINEATOR" after CL ¾"
		Added "RAILING JOINTS ELEVATION".
	OLD	TITLE BLOCK:
	3 of 13 NEW	• Revised "SHEET 3 OF 13" to "SHEET 3 OF 14".
	3 of 14	TYPICAL SIDEWALK RAIL – BARRIER SECTION:
		Moved to sheet 4.
		Shown opposite hand.
		 Revised "BARRIER SECTION" to "SECTION C-C BARRIER SECTION".
		• Revised section cut "A" to "D".
		• Added "BARRIER WALL" to 1'-6".
		• Added ½" horizontal dimension.
		 Added 1" horizontal dimension. Added 2'-0" BARRIER WALL.
		 Added 2'-0" BARRIER WALL. Added "W8x31 POST".
		Added 1" to BASE PL.
		• Revised "SHEET 5" to "SHEET 3" (2 locations).
		 Added "1 ½" \$\phi\$ HOLE IN WEB FOR 1" \$\phi\$ SIDEWALK RAIL ROD (SEE ANCHOR PLATE DETAIL ON SHEET 2)".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	OLD 3 of 13 NEW 3 of 14 (cont.)	TYPICAL SIDEWALK RAIL – ELEVATION-POST: • Moved to sheet 4. • Shown opposite hand. • Revised "RAILING POST W8x31" to "W8x31 POST". • Added "1/s" \(\phi\) HOLE IN WEB FOR 1" \(\phi\) SIDEWALK RAIL ROD". • Added "FOR DELINEATOR" after CL 34" \(\phi\) HOLE. SECTION A-A: • Moved to sheet 4. • Shown opposite hand. • Revised "SECTION A-A" to SECTION D-D". • Revised "SHEET 5" to "SHEET 3". PLATE WASHER DETAIL: • Moved to sheet 4. **RAISED SIDEWALK RAIL – BARRIER SECTION: • Moved to sheet 6. • Shown opposite hand. • Revised "BARRIER SECTION" to "SECTION F-F BARRIER SECTION". • Revised section cut "A-A" to "D-D". • Added "BARRIER WALL" to 1'-6". • Added "BARRIER WALL" to 1'-6". • Added 4'-2" vertical dimension. • Added 1" borizontal dimension. • Added 2"-0" BARRIER WALL. • Added "W8x31 POST". • Added "ON SHEET 2". • Added "ON SHEET 2". • Added "ON SHEET 2". **Added "ON SHEET 2". **Added "ON SHEET 2". **Added "ON SHEET 2". **Added "FOR DELINEATOR" after CL 34" \(\phi\) HOLE. **ANCHOR PLATE DETAIL. • Moved to sheet 5. **BARRIER RAIL ANCHOR STUD DETAIL: • Moved from sheet 5. **DELINEATOR DETAIL: • Moved from sheet 5. **DELINEATOR DETAIL: • Moved from sheet 5. **DELINEATOR DETAIL: • Moved from sheet 5.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	OLD 3 of 13 NEW 3 of 14 (cont.)	TYPICAL RAIL TOP POST DETAIL: • Moved from sheet 5. SIDEWALK RAIL ANCHOR STUD DETAIL: • Moved from sheet 5.
		RAIL TUBE CAP DETAIL: • Moved from sheet 4. NOTES: • Note 1: Removed. • Note 2: Removed. • Note 3: Removed. • Note 4: Removed. • Note 5: Removed. • Note 5: Removed. • Note 6: Removed. • Note 6: Removed. • Revised note "7" to "2". • Note 8: Removed. • Note 9: Removed.
		 Note 9: Removed. Added new note – "1. COMPLETE JOINT PENETRATION GROOVE WELD. GRIND FLUSH ON OUTSIDE FACE. SHOW SPECIFIC WELD SYMBOL ON SHOP DRAWINGS.".
	OLD 4 of 13 NEW 4 of 14	TITLE BLOCK: Revised "END OF RAIL" to "TYPICAL SIDEWALK". Revised "SHEET 4 OF 13" to "SHEET 4 OF 14". Added "PA BRIDGE BARRIER TYPICAL SIDEWALK": "PLAN" "ELEVATION" SECTION C-C BARRIER SECTION: Moved from sheet 3. ELEVATION-POST: Moved from sheet 3. SECTION D-D: Moved from sheet 3. PLATE WASHER DETAIL: Moved from sheet 3. NOTES: Note 1: Removed. Note 2: Removed. Note 3: Revised note "3" to "1". Note 4: Removed. Removed "TYPICAL RAIL ELEVATION".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	OLD 4 of 13	Removed "TYPICAL SIDEWALK RAIL ELEVATION".
	NEW 4 of 14	Removed "ALTERNATE SIDEWALK RAIL ELEVATION".
	(cont.)	RAIL TUBE CAP DETAIL – END VIEW: • Moved to sheet 3.
		RAIL TUBE CAP DETAIL – ELEVATION VIEW: • Moved to sheet 3. • Shown opposite hand. • Removed "PL ½" x3½" x1'-10"". • Removed 2³/16" dimension. • Removed "CL ⁷ /8" θ BOLT (ASTM A307)". • Removed "BOTTOM RAIL TUBE MEMBER SHOWN, TOP RAIL TUBE SIMILAR BUT WITHOUT BOLT AND PL ½" x3½" x1'-10"". RAIL TUBE CAP DETAIL – DETAIL D: • Moved to sheet 3. • Shown opposite hand. • Revised "DETAIL D" to "DETAIL C". • Revised "NOTE 4" to "NOTE 1". • Revised 9 ¼" to 8 ¼". • Revised 2'-3" to 2'-0".
	NEW 5 of 14	New sheet added. PA BRIDGE BARRIER ALTERNATE SIDEWALK DETAILS. Added "PA BRIDGE BARRIER ALTERNATE SIDEWALK": • "PLAN" • "ELEVATION"
		Added "NOTES".
		SECTION E-E BARRIER SECTION: • Moved from sheet 2.
		ELEVATION-POST: • Moved from sheet 2.
	OLD 5 of 13 NEW 6 of 14	TITLE BLOCK: • Revised "MISCELLANEOUS RAILING" to "RAISED SIDEWALK". • Revised "SHEET 5 OF 13" to "SHEET 6 OF 14".
		Added "PA BRIDGE BARRIER RAISED SIDEWALK": • "PLAN" • "ELEVATION"
		SECTION F-F BARRIER SECTION: • Moved from sheet 3.
		ELEVATION-POST: • Moved from sheet 3.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	OLD 5 of 13 NEW 6 of 14 (cont.)	NOTES: Note 1: Removed. Note 2: Removed. Note 3: Revised note "3" to "1". Removed "TOP VIEW". Removed "RAISED SIDEWALK RAIL ELEVATION". ANCHOR PLATE DETAIL: Moved to sheet 3. Revised 11½" to 10¼". Revised 3½" to 2½". Revised 1¾" to 1½". Revised 1¾" to 1½". Revised 1¾" dimension. BARRIER RAIL ANCHOR STUD DETAIL: Moved to sheet 3. DELINEATOR DETAIL: Moved to sheet 3. TYPICAL RAIL TO POST DETAIL: Moved to sheet 3. Shown opposite hand. Revised "SHEET 3" to "SHEET 4". U-WASHER: Moved to sheet 3.
	OLD 6 of 13 NEW 7 of 14	 Moved to sheet 3. TITLE BLOCK: Revised "SHEET 6 OF 13" to "SHEET 7 OF 14". PLAN – SKEW ANGLE ≥ 75°: Revised "CURB" to "BARRIER WALL". Revised section marks "A-A" to "G-G". Revised section marks "B-B" to "H-H". Corrected concrete recess. PLAN AT SIDEWALK – SKEW ANGLE ≥ 75°: Added "TYPICAL" (2 locations). Revised "CURB" to "BARRIER WALL". Revised section marks "C-C" to "J-J". Revised section marks "D-D" to "K-K". Revised section marks "B-B" to "H-H".

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	OLD 6 of 13 NEW 7 of 14 (cont.)	PLAN AT SIDEWALK – SKEW ANGLE < 75°: Added "TYPICAL" (2 locations). Revised "CURB" to "BARRIER WALL". Added "SIDEWALK". Revised section marks "C-C" to "J-J". Revised section marks "B-D" to "K-K". Revised section marks "B-B" to "H-H". PLAN – SKEW ANGLE < 75°: Revised "CURB" to "BARRIER WALL". Revised section marks "A-A" to "G-G". Revised section marks "B-B" to "H-H". Corrected concrete recess. SECTION C-C (2 locations): Added "TYPICAL". Revised "SECTION C-C" to "SECTION J-J". Revised weld symbol. SECTION D-D: Revised "SECTION D-D" to "SECTION K-K". Added "TYPICAL". SECTION A-A: Revised "SECTION A-A" to "SECTION G-G".
	OLD 7 of 13 NEW 8 of 14	NOTES: Note 3: Revised "-9°F" to -10°F". Added "BRIDGES" (2 locations). TITLE BLOCK: Revised "SHEET 7 OF 13" to "SHEET 8 OF 14". PLAN − SKEW ANGLE ≥ 75°: Revised "CURB" to "BARRIER WALL". Revised section marks "A-A" to "L-L". Revised section marks "B-B" to "M-M". Corrected concrete recess. PLAN AT SIDEWALK − SKEW ANGLE ≥ 75°: Added "TYPICAL" (2 locations). Revised "CURB" to "BARRIER WALL". Revised section marks "C-C" to "N-N". Revised section marks "D-D" to "P-P". Revised section marks "B-B" to "M-M". Corrected concrete recess.

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-713M (cont.)	OLD 7 of 13 NEW 8 of 14 (cont.)	PLAN AT SIDEWALK – SKEW ANGLE < 75°: • Added "TYPICAL" (2 locations). • Revised "CURB" to "BARRIER WALL". • Added "SIDEWALK". • Revised section marks "C-C" to "N-N". • Revised section marks "D-D" to "P-P". • Revised section marks "B-B" to "M-M". • Revised detail to show skew correctly. PLAN – SKEW ANGLE < 75°: • Revised "CURB" to "BARRIER WALL". • Revised section marks "A-A" to "L-L". • Revised section marks "B-B" to "M-M". • Corrected concrete recess. SECTION C-C (2 locations): • Revised "SECTION C-C" to "SECTION N-N". • Added "TYPICAL". • Added "TYPICAL". • Added "SIDEWALK/BARRIER WALL/GUTTERLINE". SECTION A-A: • Revised "SECTION A-A" to "SECTION L-L". SECTION B-B: • Revised "SECTION B-B" to "SECTION M-M". NOTES: • Note 1: Revised "D-D" to "P-P". Revised "SHEET 8" to "SHEET 9". • Note 3: Revised "-9°F" to -10°F". Added "BRIDGES" (2 locations). • Note 4: Revised to "MAXIMUM DISTANCE ALONG THE EXTRUSION FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3".
	OLD 8 of 13 NEW 9 of 14	TITLE BLOCK: • Revised "SHEET 8 OF 13" to "SHEET 9 OF 14". SECTION D-D (2 locations): • Revised "SECTION D-D" to "SECTION P-P". • Added "TYPICAL". • Corrected concrete recess, at raised sidewalk. PLAN AT ALTERNATE SIDEWALK − SKEW ANGLE ≥ 75°: • Revised "CURB" to "BARRIER WALL". • Revised section marks "F-F" to "Q-Q". • Revised section marks "E-E" to "R-R". • Correct concrete recess. PLAN AT ALTERNATE SIDEWALK − SKEW ANGLE < 75°: • Revised how joint is detailed at bend. • Revised "CURB" to "BARRIER WALL". • Replaced "GUTTERLINE" with "SIDEWALK". • Revised section marks "F-F" to "Q-Q". • Revised section marks "F-F" to "Q-Q".

STANDARD	SHEET	DESCRIPTION OF CHANGES			
BC-713M (cont.)	OLD 8 of 13 NEW 9 of 14 (cont.)	SECTION F-F: • Revised "SECTION F-F" to "SECTION Q-Q". SECTION E-E: • Revised "SECTION E-E" to "SECTION R-R".			
		NOTES: Note 1: Revised to "FOR LOCATION OF SECTION P-P, SEE SHEET 8.". Note 3: Revised "-9°F" to -10°F". Added "BRIDGES" (2 locations). Note 4: Revised to "MAXIMUM DISTANCE ALONG THE EXTRUSION FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3".			
	OLD 9 of 13 NEW 10 of 14	TITLE BLOCK: Revised "SHEET 9 OF 13" to "SHEET 10 OF 14". DETAIL A: Revised "DETAIL A" to "DETAIL E". Added "TACK WELD NUT TO CONNECTION PLATE" DETAIL B: Revised "DETAIL B" to "DETAIL D". Removed "RAILING" (2 locations). SECTION B-B: PLAN: Revised "SECTION B-B: PLAN" to "SECTION U-U: PLAN". Revised "SECTION B-B: PLAN" to "SECTION U-U: PLAN". Revised "DETAIL B" to "DETAIL D". Removed "RAILING" (2 locations). SECTION A-A (2 locations): Revised section "A-A" to "S-S". Revised section marks "B-B" to "U-U". Revised "DETAIL A" to "DETAIL E". SECTION C-C: Revised section "C-C" to "T-T". RAILING END SECTION: Revised "SECTION A-A" to "SECTION S-S". TYPICAL POST SECTION: Revised section marks "A-A" to "S-S".			
		NOTES: • Revised to "1. SEE SHEET 11 FOR NOTES.".			
	OLD 10 of 13 NEW 11 of 14	TITLE BLOCK: • Revised "SHEET 10 OF 13" to "SHEET 11 OF 14". ELEVATION: • Revised section marks "J-J" to "V-V".			

STANDARD	SHEET	DESCRIPTION OF CHANGES			
BC-713M (cont.)	OLD 10 of 13 NEW 11 of 14	 Revised "DETAIL C" to DETAIL F". Removed "RAILING". POST AND BASE PLATE:			
	(cont.)	Revised "DETAIL C" to "DETAIL B".			
		 SECTION J-J: Revised "SECTION J-J" to "SECTION V-V". Revised "DETAIL C" to "DETAIL F". Added "BARRIER WALL" to 1'-6". 			
		DETAIL C: • Revised "DETAIL C" to "DETAIL F".			
		NOTES: • Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". • Note 2: Added "PUBLICATION" (2 locations).			
	OLD 11 of 13 NEW	TITLE BLOCK: • Revised "SHEET 11 OF 13" to "SHEET 12 OF 14".			
	12 of 14	CAST-IN-PLACE BARRIER DIMENSIONS ON M.S.E. WALLS: • Revised "BARRIER WALL GEOMETRY DETAIL" to "DETAIL A". • Added 4'-2" vertical dimension.			
		C.I.P. BARRIER WITH BITUMINOUS CONCRETE SHOULDER ON M.S.E. WALLS: • Revised "C.I.P. BARRIER WITH BITUMINOUS CONCRETE SHOULDER			
		ON M.S.E. WALLS" to "C.I.P. BARRIER WITH ASPHALT-PAVED SHOULDER ON M.S.E. WALLS". • Revised "ASPHALT JOINT SEALANT (AC-20)" to "ASPHALT RUBBER SEALING COMPOUND [PUB.408, SECTION 705.4(g)]".			
		Added "(P.C.P.)" to PREFORMED CELLULAR POLYSTYRENE. CONCRETE DARRIED WALL ELEVATION ON M.S.E. WALL.			
		CONCRETE BARRIER WALL ELEVATION ON M.S.E. WALL: • Labeled "BARRIER WALL".			
		NOTES: • Note 2: Revised "SHEET 7" to "SHEET 8 AND 9".			
		Added "LEGEND: C.C.N.S. CLOSED CELL NEOPRENE SPONGE".			
	OLD 12 of 13 NEW	TITLE BLOCK: • Revised "SHEET 12 OF 13" to "SHEET 13 OF 14".			
	13 of 14	PRECAST BARRIER WITH BITUMINOUS CONCRETE SHOULDER ON M.S.E. WALLS: • Revised "PRECAST BARRIER WITH BITUMINOUS CONCRETE SHOULDER ON M.S.E. WALLS" to "PRECAST BARRIER WITH ASPHALT-PAVED SHOULDER ON M.S.E. WALLS".			

STANDARD	SHEET	DESCRIPTION OF CHANGES				
BC-713M (cont.)	OLD 12 of 13 NEW 13 of 14	 Revised "ASPHALT JOINT SEALANT (AC-20)" to "ASPHALT RUBBER SEALING COMPOUND [PUB.408, SECTION 705.4(g)]". Added "(P.C.P.)" to PREFORMED CELLULAR POLYSTYRENE. 				
	(cont.)	PRECAST BARRIER WITH CEMENT CONCRETE SHOULDER ON M.S.E. WALLS:				
		Added "(P.C.P.)" to PREFORMED CELLULAR POLYSTYRENE.				
		PRECAST BARRIER DIMENSIONS ON M.S.E. WALLS: • Revised "FINISHED CURB AS PER DETAIL D, SHEET 2. (TYP.)" to "FINISH BARRIER WALL AS PER DETAIL A, SHEET 1 (TYP.)". • Added 2'-0" to BARRIER WALL.				
		REINFORCEMENT FOR PRECAST BARRIER WITH BITUMINOUS CONCRETE SHOULDER:				
		Revised: "REINFORCEMENT FOR PRECAST BARRIER WITH BITUMINOUS CONCRETE SHOULDER" to "REINFORCEMENT FOR PRECAST BARRIER WITH ASPHALT-PAVED SHOULDER".				
		 NOTES: Note 4: Revised to "USE SILICONE JOINT SEALING MATERIAL AS SPECIFIED IN PUBLICATION SECTION 705.4(a).". Note 5: Revised "PROVIDE REINFORCEMENT AS PER DETAIL A, SHEET 3, BC-799M." to "PROVIDE LEVELING CONCRETE IN ACCORDANCE WITH BC-799M, SHEET 3, DETAIL A.". 				
		Note 6: Revised "SHEET 11" to "SHEET 12". Added "LEGEND: C.C.N.S. CLOSED CELL NEOPRENE SPONGE".				
	OLD 13 of 13 NEW	TITLE BLOCK: • Revised "SHEET 13 OF 13" to "SHEET 14 OF 14".				
	14 of 14	ELEVATION: • Revised section marks "A-A" to "W-W".				
		SECTION A-A: • Revised "SECTION A-A" to "SECTION W-W". • Revised section marks "B-B" to "X-X".				
		SECTION B-B: • Revised "SECTION B-B" to "SECTION X-X"				
		SECTION A-A: • Revised "SECTION A-A" to "SECTION Y-Y".				
		PLAN: • Added "BARRIER WALL" to 1'-6". • Revised section marks "A-A" to "Y-Y".				
		NOTES: • Revised title "NOTES" to OPEN JOINT NOTES".				

STANDARD	SHEET	DESCRIPTION OF CHANGES				
BC-713M (cont.)	OLD 13 of 13 NEW 14 of 14 (cont.)	 Note 2: Revised "SECTION 705.8(b) OF PUB. 408" to "PUBLICATION 408 SECTION 705.8(b)". Note 3: Revised "SECTION 705.9 OF PUB. 408" to "PUBLICATION 408 SECTION 705.9". Added new note – "4. PROVIDE PREMOLDED EXPANSION JOINT FILLER IN ACCORDANCE WITH PUBLICATION 408 SECTION 705.1.". Note 4: Revised note "4" to "5". Note 5: Revised note "5" to "6". Added "JUNCTION BOX NOTES".				
BC-718M	ALL	Standard discontinued.				
BC-720M	1 of 1	 ELEVATION: Revised "PAVEMENT" to "PAYMENT". Added "SIDEWALK" callout. Revised note "BRIDGE HAND RAILING ON 2'-8" VERTICAL WALL SHOWN, BRIDGE HAND RAILING ON ALTERNATE CONCRETE BARRIER, TYPICAL CONCRETE BARRIER AND 3'-6" VERTICAL WALL SIMILAR, SEE NOTE 8" to "BRIDGE HAND RAILING ON ALTERNATE SIDEWALK WITH 42" VERTICAL WALL CONCRETE BARRIER SHOWN, BRIDGE HAND RAILING ON 32", 42" OR 45" F-SHAPE CONCRETE BARRIER AND 32" OR 42" VERTICAL WALL CONCRETE BARRIER SIMILAR, SEE NOTE 8". SECTION A-A: Revised 2'-8" to 3'-6". NOTES: Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". Note 7: Revised "MEETING THE REQUIREMENTS OF" to "CONFORMING TO". Revised "SECTION 705.8(b)" to "SECTION 705.7(b)". Note 8: Revised "FOR TYPICAL CONCRETE BARRIER OR 3'-6" VERTICAL WALL BRIDGE BARRIER" to "FOR BRIDGE HAND RAILING ON 42" OR 45" F-SHAPE CONCRETE BARRIER OR 42" VERTICAL WALL CONCRETE BARRIER OR 42" VERTICAL WALL CONCRETE BARRIER". REFERENCE DRAWINGS: Deleted "BC-739M BRIDGE BARRIER TO GUIDE RAIL TRANSITION". Added "RC-50M GUIDE RAIL TO BRIDGE BARRIER TRANSITIONS". 				
BC-721M	1 of 2	 CONDUIT DETAILS AT ENDS OF BRIDGE: ELEVATION and PLAN VIEW – revised to remove 10° flare and show new end barrier transition. JUNCTION BOX JB25: Revised "(SECTION 1101 OF PUB. 408)" to "(PUBLICATION 408, SECTION 1101)". 				

STANDARD	SHEET	DESCRIPTION OF CHANGES				
BC-721M (cont.)	1 of 2 (cont.)	NOTES: Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". Revised "PUB." to "PUBLICATION". Note 2: Revised "FOR PROPER GROUNDING OF GALV. STEEL CONDUIT OR NON-METALLIC CONDUIT, AS SPECIFIED IN PUB. 408 SECTION 910." to "GROUND LIGHTING POLES, UNDERPASS LUMINARIES, AND METAL JUNCTION BOXES ON STRUCTURES IN ACCORDANCE WITH PUBLICATION 408, SECTION 910.3(q).". "Note 5: Revised "PARAPETS" to "BARRIERS". REFERENCE DRAWINGS: Deleted "BC-739M BRIDGE BARRIER TO GUIDE RAIL TRANSITION" Added "RC-50M GUIDE RAIL TO BRIDGE BARRIER TRANSITIONS".				
	2 of 2	CONDUIT EXPANSION NOTES: • Note 2: Revised "PER" to "ACCORDING TO". • Note 4: Revised "PER PUB." to "IN ACCORDANCE WITH PUBLICATION". NOTES: • Note 1: Removed. • Note 2: Removed.				
BC-722M	1 of 2	BLISTER DIMENSIONS: • Revised "F-BARRIER TYPE" to "F-SHAPE BARRIER". • Revised "TYP." to "42" & 45"". • Revised "ALT." to "32"" ALTERNATE – PLAN: • Revised "TYPICAL" to "42" & 45" F-SHAPE CONCRETE". • Revised "ALTERNATE" to "32" F-SHAPE CONCRETE". NOTES: • Note 1: Revised "WORKMANSHIP" to "PERFORM WORK".				
BC-734M	OLD 1 of 3 NEW 1 of 2	NOTES: • Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". • Note 8: Deleted "(2) ATTACHING BASE PLATES FOR FENCE, PEDESTRIAN RAILING, PROTECTIVE BARRIERS AND BRIDGE RAILING POSTS TO CONCRETE DECKS OR PARAPETS.". TITLE BLOCK: • Revised "SHEET 1 OF 3" to "SHEET 1 OF 2".				
	OLD 2 of 3	DELETED SHEET				
	OLD 3 of 3 NEW 2 of 2	CONSTRUCTION NOTES: • Note 1: Revised "SECURITY" to "SECURELY".				

STANDARD	SHEET	DESCRIPTION OF CHANGES					
BC-734M (cont.)	OLD 3 of 3 NEW 2 of 2 (cont.)	TITLE BLOCK: • Revised "SHEET 3 OF 3" to "SHEET 2 OF 2".					
BC-739M	ALL	Standard discontinued					
BC-752M	1 of 2	 NOTES: Note 1: Revised "WORKMANSHIP" to "PERFORM WORK". Note 5: Revised to "OPEN JOINT DETAILS AND MODIFIED DEFLECTION JOINTS APPLY TO THE FOLLOWING TYPES OF BARRIERS: 32", 42" AND 45" F-SHAPE CONCRETE BARRIERS, 32 AND 50" SPLIT CONCRETE MEDIAN BARRIERS, 32" AND 50" CONCRETE MEDIAN BARRIERS, ALTERNATE SIDEWALK DETA RAISED SIDEWALK DETAIL, 32" AND 42" VERTICAL CONCRETE BARRIERS, AND BARRIER WALLS FOR THE PA BRIDGE BARRIE AND PA TYPE 10M BRIDGE BARRIER.". Note 6: Revised to "PROVIDE CAULKING COMPOUND CONFORM TO PUBLICATION 408, SECTION 705.7(b).". Note 7: Revised to "PROVIDE JOINT BACKING MATERIAL CONFORMING TO PUBLICATION 408, SECTION 705.9.". Note 9: Revised to "PROVIDE AN EPOXY BONDING COMPOUND, TYPE 1, GRADE 3, CONFORMING TO PUBLICATION 408, SECTION 705.1.". Note 10: Revised to "PROVIDE PREMOLDED EXPANSION JOINT FILLER CONFORMING TO PUBLICATION 408, SECTION 705.1.". 					
BC-767M	1 of 6	 Note 2: Revised "WORKMANSHIP" to "PERFORM WORK". Note 11: Revise "MOVEMENT CLASSIFICATION OF SEAL TO BE NOT LESS THAN THE CLASSIFICATION SPECIFIED ON THE DESIGN DRAWINGS. ALL SEALS TO CONFORM TO THE REQUIREMENTS OF SECTION 705 OF PUBLICATION 408. OBTAIN APPROVAL FOR USE OF THE SEAL FROM THE CHIEF MATERIALS ENGINEER, LABORATORY TESTING SECTION, INNOVATION AND SUPPORT SERVICES DIVISION, BUREAU OF PROJECT DELIEVER." to "PROVIDE SEALS WITH MOVEMENT CLASSIFICATION NOT LESS THAN THE CLASSIFICATION SPECIFIED ON THE DESIGN DRAWINGS. ALL SEALS SHALL CONFORM TO THE REQUIREMENTS OF PUBLICATION 408, SECTION 705. OBTAIN APPROVAL FOR USE OF THE SEAL FROM THE CHIEF MATERIALS ENGINEER, LABORATORY TESTING SECTION, CONSTRUCTION AND MATERIALS DIVISION, BUREAU OF PROJECT DELIEVERY.". SECTION B-B: Revised detail for a 4" curb height and angle for steel extrusion. 					
	4 of 6	PLAN AT SIDEWALK: • Revised "PLAN AT SIDEWALK" to "PLAN AT TYPICAL SIDEWALK".					

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-767M (cont.)	4 of 6 (cont.)	SECTION AT SIDEWALK: • Revised "SECTION AT SIDEWALK" to "SECTION AT TYPICAL SIDEWALK".
	5 of 6	SECTION AT BARRIER: • Revised "SPACING OF THE SCREWS IS THE SAME FOR THE ALTERNATE BARRIER." to "SPACING OF THE SCREWS IS THE SAME FOR ALL F-SHAPE BARRIERS.".
		SECTION AT SPLIT CONCRETE DIVISOR: • Revised "SECTION AT SPLIT CONCRETE DIVISOR" to "SECTION AT SPLIT CONCRETE MOUNTABLE DIVISOR".
BC-779M	1 of 9	 GENERAL NOTES: Note 2: Revised "WORKMANSHIP" to "WORK QUALITY". Added note – "22. PROVIDE VERTICAL V-NOTCHES ON BARRIER WALL FRONT AND REAR FACES AT ALL POST ANCHOR BOLT LOCATIONS FOR SOUND BARRIERS MOUNTED ON TOP OF BARRIERS ON BRIDGES, RETAINING WALLS AND MOMENT SLABS. SEE DETAIL SHEET 8.".
	2 of 9	 MATERIAL NOTES: Note 3: Revised "THAT MEET THE REQUIREMENTS OF" to "CONFORMING TO". Note 4: Revised "THAT MEET THE REQUIREMENTS OF" to "CONFORMING TO". Note 12: Revised "SECTION 705.8(b)" to "SECTION 705.7(b)". Note 14: Revised "SECTION 705.9" to "SECTION 705.8".
	3 of 9	BARRIER MOUNTED/RETAINING WALL MOUNTED SOUND BARRIER ELEVATION: • Revised "(SEE NOTE 7) to "(SEE NOTE 8)". BARRIER MOUNTED SOUND BARRIER ON MOMENT SLAB TYPICAL SECTION: • Added ▲ to 3'-6". • Revised "SEE NOTE 4" to "SEE NOTE 5". • Revised "SEE NOTES 5 & 6" to "See NOTES 6 & 7". BARRIER MOUNTED SOUND BARRIER ON BRIDGE TYPICAL SECTION: • Revised 3'-4" to 3'-7". • Revised 3'-6" to 3'-9". BARRIER MOUNTED SOUND BARRIER ON RETAINING WALL TYPICAL SECTION: • Revised 3'-4" to 3'-7". • Revised 3'-6" to 3'-9".

STANDARD	SHEET	DESCRIPTION OF CHANGES			
BC-779M (cont.)	3 of 9 (cont.)	 NOTES: Note 4: Revised to read "45" F-SHAPE CONCRETE BARRIER SHOWN, 42" F-SHAPE CONCRETE BARRIER AND 42" VERTICAL WALL CONCRETE BARRIER SIMILAR.". New Note 5: 42" F-SHAPE CONCRETE BARRIER SHOWN, 42" VERTICAL WALL CONCRETE BARRIER SIMILAR. Renumbered notes 5, 6, 7 to 6, 7, 8. LEGEND: Added ▲ 45" F-SHAPE CONCRETE BARRIER NOT PERMITTED ON MOMENT SLAB. 			
	7 of 9	Detail E: • Removed 3'-4" dimension. • Revised Note to read "45" F-SHAPE CONCRETE BARRIER SHOWN, 42" F-SHAPE CONCRETE BARRIER AND 42" VERTICAL WALL CONCRETE BARRIER SIMILAR.".			
	8 of 9	Added "VERTICAL V-NOTCH DETAIL".			
BC-799M	1 of 13	 GENERAL NOTES: Note 5: Revised "WORKMANSHIP" to "PERFORM WORK". Note 27: Revised "IN ACCORDANCE WITH PUB." to "AS SPECIFIED IN PUBLICATION". New Note: "29. THE 45" F-SHAPE CONCRETE BARRER IS NOT PERMITTED ON MOMENT SLABS.". New Note: "30. THE 42" F-SHAPE CONCRETE BARRIER ATTACHED TO A MOMENT SLAB IS DESIGNATED AS MASH TL-4. THE 32" F-SHAPE CONCRETE BARRIER ATTACHED TO MOMENT SLAB IS DESIGNATED AS MASH TL-3. THE ALTERNATE SIDEWALK WITH 42" VERTICAL WALL CONCRETE BARRIER ATTACHED TO A MOMENT SLAB IS DESIGNATED AS MASH TL-2.". REFERENCE DRAWINGS: Deleted "BC-739M BRIDGE BARRIER TO GUIDE RAIL TRANSITION". Added "RC-50M GUIDE RAIL TO BRIDGE BARRIER TRANSITIONS". 			
	3 of 13	DRAINAGE DITCH DETAIL: • Revised "SOIL IN ACCORDANCE WITH PUB. 408 SECTION 206(a)1.a" to "SOIL CONFORMING TO PUBLICATION 408, SECTION 206.2(a)1.a".			
	4 of 13	 TYPICAL CAST-IN-PLACE BARRIER DIMENSIONS: Revised Title to "CAST-IN-PLACE 42" F-SHAPE CONCRETE BARRIER DIMENSIONS". Added note "32" F-SHAPE CONCRETE BARRIER SIMILAR" and "(SEE NOTE 29, SHEET 1)". 			
		TYPICAL C.I.P. BARRIER WITH CEMENT CONCRETE SHOULDER: • Revised title to "C.I.P. F-SHAPE CONCRETE BARRIER WITH CEMENT CONCRETE SHOULDER".			

STANDARD	SHEET	DESCRIPTION OF CHANGES				
BC-799M (cont.)	4 of 13 (cont.)	TYPICAL C.I.P. BARRIER WITH BITUMINOUS SHOULDER: • Revised title to "C.I.P. F-SHAPE CONCRETE BARRIER WITH ASPHALT-PAVED SHOULDER".				
	5 of 13	PRECAST BARRIER WITH BITUMINOUS SHOULDER: • Revised title to "PRECAST F-SHAPE CONCRETE BARRIER WITH ASPHALT-PAVED SHOULDER".				
		PRECAST BARRIER WITH CONCRETE SHOULDER: • Revised title to "PRECAST F-SHAPE CONCRETE BARRIER WITH CEMENT CONCRETE SHOULDER".				
		TYPICAL PRECAST BARRIER DETAILS: • Revised title to "PRECAST 42" F-SHAPE CONCRETE BARRIER DETAILS".				
		REINFORCEMENT FOR BARRIER WITH BITUMINOUS SHOULDER: • Revised title to "REINFORCEMENT FOR BARRIER WITH ASPHALT-PAVED SHOULDER".				
		DIMENSIONS: • Revised "½" CHAMFER" to "½" x ½" CHAMFER". • Added "½" x ½" CHAMFER".				
		NOTES: • Note 7: Revised "AS PER PUB." to "AS SPECIFIED IN PUBLICATION".				
	6 of 13	TYPE 1 AND TYPE 2 OPEN JOINT IN PRECAST BARRIER: • Revised title to "TYPE 1 AND TYPE 2 OPEN JOINT IN PRECAST F-SHAPE CONCRETE BARRIER".				
		BARRIER MOMENT SLAB NOTES: • Note 1: Revised "AS PER PUBLICATION 408, SEC.705.4(a)" to "AS SPECIFIED IN PUBLICATION 408, SECTION 705.4(a)".				
	7 of 13	SIDEWALK BARRIER SECTION: • Revised title to "ALTERNATE SIDEWALK WITH 42" VERTICAL WALL CONCRETE BARRIER SECTION".				
		BARRIER TO GUIDE RAIL TRANSITION: • Revised PLAN and ELEVATION for new barrier transition.				
		 ALTERNATE TRAFFIC BARRIER: Revised title to "32" F-SHAPE CONCRETE BARRIER". Revised "TYPICAL" to "F-SHAPE CONCRETE". Added R=1". 				
		• Added "½" x ½" CHAMFER" (2 locations).				
		BARRIER WITH BITUMINOUS SHOULDER: • Revised title to "BARRIER WITH ASPHALT-PAVED SHOULDER".				

STANDARD	SHEET	DESCRIPTION OF CHANGES			
BC-799M (cont.)	8 of 13	DRAINAGE PIPE NOTES: • Note 1: Revised "MEETING REQUIREMENTS OF PUB." to "CONFORMING TO PUBLICATION". Deleted "(SOL 431-10-04, DATED MARCH 17, 2010)".			
	9 of 13	PLAN – SHOULDER DETAILS AT INLET: • Added "2" CLR". • Added "(ROADWAY ITEM)" to "INLET". SHOULDER RELIEF JOINT – DETAIL D: • Revised "PG 46-40" to "PG 64S-22". • Revised "SECTION 470.2(a)" to "SECTION 471.2(a)". SHOULDER RELIEF JOINT – SECTION K-K: • Revised note "ADJUST FOR SIDEWALK AND ALTERNATE BARRIER" to "DIMENSIONS SHOWN ARE FOR 42" F-SHAPE CONCRETE BARRIER AND ALTERNATE SIDEWALK WITH 42" VERTIAL WALL CONCRETE BARRIER. ADJUST FOR 32" F-SHAPE CONCRETE BARRIER.". SECTION L-L – BARRIER WITH BITUMINOUS SHOULDER: • Revised title to "BARRIER WITH ASPHALT-PAVED SHOULDER".			

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OS-299 (7-08)



TRANSMITTAL LETTER

PUBLICATION:

Publication 219M September 2016 Edition Change No. 2

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January 31, 2019

SUBJECT:

Revisions to
Standards for Bridge Construction
September 2016 Edition

INFORMATION AND SPECIAL INSTRUCTIONS:

Incorporate the attached revisions into the September 2016 Edition of Publication 219M.

These standards may be used immediately and can be adopted as soon as practical on all new and existing designs without affecting letting schedules. All projects let after June 28, 2019 must incorporate these new standards.

A description of the changes made to the 2016 Edition since Change 1 of August 4, 2017 are listed in the attached multi-sheet Table. On the standards, light green highlighting indicates Change 2 revisions to details and notes; light yellow highlighting indicates Change 1 revisions.

CANCEL AND DESTROY THE FOLLOWING:

Existing BC-700M Series standards need to be retained for projects under construction and for future rehabilitation work.

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APPROVED FOR ISSUANCE BY:

LESLIE S. RICHARDS
Secretary of Transportation

BY:

Melissa J. Batula, P.E.

Acting Director, Bureau of Project Delivery,

Highway Administration

Standard	Sheet	Description of Changes
BC-700M	1 Sht.	Added new BC-790M and revised approval date for standards included in Change 2.
BC-701M	3 of 3	LEVELING PAD DETAIL - SECTION: added note to leveling pad thickness dimension: * - LEVELING PAD CAN BE CONTINUOUSLY POURED MONOLITHICALLY WITH CURB. LEVELING PAD CAN BE ELIMINATED IF SLOPE/GRADE ON TOP OF CURB IS LESS THAN 1%.
BC-706M	1 of 2	RAIL AT END OF BRIDGE - PLAN and SECTION B-B: updated bolt designation from ASTM A325 to ASTM F3125 GRADE A325.
BC-707M	4 of 5	Updated bolt designations from ASTM A325 and A325 to ASTM F3125 GRADE A325. (five instances)
	5 of 5	POST MOUNTING ON GRADE: updated bolt designation from ASTM A325 to ASTM F3125 GRADE A325.
BC-709M	1 of 12	GENERAL NOTES: removed Note 15 because it is an exact repeat of Note 11.
	3 of 12	Removed the $**$ note stating the end connection angle is a roadway item because it is a bridge item, and removed all $**$ note indicators.
<u> </u>		RAIL TUBE CAP - ELEVATION VIEW: increased corner cope size from 3/4" to 1 1/8".
	4 of 12	Removed the $**$ note stating that the end connection angle is a roadway item because it is a bridge item, and removed all $**$ note indicators.
		SPLICE TUBE – ELEVATION: revised the steel designation for the 1/8" thick fill plate from ASTM A709 to ASTM 1101 OR EQUIVALENT.
	8 of 12	DETAIL B and DETAIL C: added ASTM F3125 GRADE A325 designation and SEE NOTE 1 ON SHEET 9 to bolt callout.
		DETAIL C: revised bolt designation from H.S. to ASTM F3125 GRADE A325.
		SECTION Q-Q: PLAN: removed bolt callout because it is in Detail C.
		SECTION P-P (TYPICAL) and ELEVATION: updated bolt designation from ASTM A325 to ASTM F3125 GRADE A325.
	9 of 12	ELEVATION and DETAIL D: added (SEE NOTE 1) to bolt callout.
		SECTION J-J corrected to be SECTION S-S
		DETAIL D: revised bolt designation from H.S. to ASTM F3125 GRADE A325.
	10 of 12	TYPICAL C.I.P. BARRIER WITH SHOULDER ON M.S.E. WALLS (two places): increased #4 rebar's horizontal leg from 12" to 1'-7".
		PLAN - BARRIER MOMENT SLAB details: revised moment slab length from (VARIES) 30' MIN., 40'-0" MAX. to 30'-0"; and in third line, revised ONE PAVEMENT JOINT to TWO PAVEMENT JOINTS; and revised RC-20M to RC-27M in NOTE B.
	11 of 12	PRECAST BARRIER WITH BITUMINOUS SHOULDER and PRECAST BARRIER WITH CEMENT CONCRETE SHOULDER details: removed 1'-6" MIN. dimension which is already shown in corresponding reinforcement detail at bottom of sheet.
		NOTE A: revised RC-20M to RC-27M.
BC-712M	1 of 3	GENERAL NOTES: added Note 9 to refer to Sheet 2 for Transition Without Inlet Placement.
		PLAN VIEW and ELEVATION VIEW: removed WITHOUT INLET PLACEMENT SIMILAR from note below titles
	2 of 3	Added new sheet for Thrie-Beam to PA Bridge Barrier Connection without Inlet Placement. Previous sheet 2 renumbered sheet 3.
BC-713M	1 of 13	NOTES: In notes 4, 10 and 18 revised ANCHOR BARS to ANCHOR PLATES.
		TYPICAL SECTION: revised post anchorage from two bars to an anchor plate; revised callout from ANCHOR BAR to ANCHOR PLATE.
	2 of 13	ALTERNATE BARRIER SECTION: revised post anchorage from two bars to an anchor plate; revised callout from ANCHOR BAR to ANCHOR PLATE.
	3 of 13	BARRIER SECTIONS (two places): revised post anchorage from two bars to an anchor plate; revised callout from ANCHOR BAR to ANCHOR PLATE.
	4 of 13	RAIL TUBE CAP DETAIL - END VIEW: increased corner cope size from 3/4" to 1 1/8".
	5 of 13	Revised ANCHOR BAR DETAIL to ANCHOR PLATE DETAIL in which two bars replaced with 1/4"x11 1/2"x1'-3" plate with 4 1/2"x8" cut-out.
	9 of 13	Added SEE NOTE 2 ON SHEET 10 to bolt callouts (five instances).
		DETAIL A and DETAIL B: added ASTM F3125 GRADE A325 designation to bolt callout.

Standard	Sheet	Description of Changes
9 of 13	9 of 13	DETAIL B: revised bolt designation from H.S. to ASTM F3125 GRADE A325.
(cont.)	(cont.)	SECTION A-A (two places): revised post anchorage from two bars to an anchor plate.
		SECTION A-A (TYPICAL): updated bolt designation from ASTM A325 to ASTM F3125 GRADE A325.
		ELEVATION: updated bolt designation from A325 to ASTM F3125 GRADE A325.
		Added NOTE: SEE SHEET 10 FOR NOTES.
BC-713M	10 of 13	Added (SEE NOTE 2) to all bolt callouts (three instances).
	44 540	DETAIL C: added ASTM F3125 GRADE A325 designation to bolt callout (two places).
	11 of 13	Three C.I.P. Barrier Sections: revised post anchorage from two bars to an anchor plate.
		C.I.P. BARRIER WITH CEMENT CONCRETE SHOULDER ON M.S.E. WALLS: increased #4 rebar's horizontal leg from 1'-0" to 1'-7".
		C.I.P. BARRIER WITH BITUMINOUS CONCRETE SHOULDER ON M.S.E. WALLS: increased #5 rebar's inclined leg from 1'-6" MIN. to 2'-0" MIN.
		PLAN - BARRIER MOMENT SLAB details: revised moment slab length from (VARIES) 30' MIN., 40'-0" MAX. to 30'-0"; and in third line, revised ONE PAVEMENT JOINT to TWO PAVEMENT JOINTS; and revised RC-20M to RC-27M in NOTE B.
	12 of 13	In five barrier sections: revised post anchorage from two bars to an anchor plate.
		PRECAST BARRIER WITH BITUMINOUS SHOULDER and PRECAST BARRIER WITH CEMENT CONCRETE SHOULDER details: removed 1'-6" MIN. dimension which is already shown in corresponding reinforcement detail at bottom of sheet.
		NOTE A: revised RC-20M to RC-27M.
BC-719M	6 of 8	TEMPORARY BARRIER TYPICAL REINFORCEMENT BARS: revised a dimension from 1'-6" to 1'-7"
BC-726M	5 shts.	Made numerous revisions throughout the standard based on recommendations from the Bridge Grid Flooring Manufacturers Association.
	1 of 5	GENERAL NOTES, Note 9: revised to PROVIDE A 3/4" DIAMETER LEVELING BOLT THAT IS EITHER: ASTM A307 GRADE A HEADED BOLT OR EQUIVALENT. OR A WELDED ASSEMBLY CONSISTING OF THREADED ROD AND HEX NUT. • THREADED ROD: ASTM A307, ASTM F1554 GRADE 36, OR EQUIVALENT • HEX NUT: ASTM A194 OR ASTM A563 FURNISH LEVELING BOLTS UNCOATED UNLESS REQUIRED TO BE GALVANIZED. SEE SHEET 4 FOR DETAILS GENERAL NOTES: added Note 10 – USE THE 5 3/16" MAIN BEARING BAR WITH OR WITHOUT THE MIDDLE RIB FOR FULL DEPTH CONCRETE DECKS. GENERAL NOTES: added Note 11 - HOT DIP GALVANIZE PANELS PER PUB. 408, SECTION 1105.02(S). SECTION A-A: added callout of main bearing bar in full-depth side of detail with reference to Note 10. SECTION C-C: added FORM PAN SHIPPED LOOSE AND FIELD INSTALLED callout; added SHOP INSTALLED to outer pan callout; and added reference to Note 10 in main bearing bar callout. SECTION E-E: added 20 GA SHEET METAL FORM PAN SHIPPED LOOSE AND FIELD INSTALLED callout.
		MAIN BEARING BAR – 41/4" BAR WITHOUT RIB: corrected dimension line for bottom flange thickness of 4 1/4" bar.
	2 of 5	TYPICAL EXPANSION JOINT DETAILS: added trim plate weld callout CONTRACTOR HAS OPTION TO SHOP WELD BEARING BAR TO TRIM PLATE; added ALLOW 2" MIN BETWEEN GRID COMPONENTS AND TRIM PLATE callout for dimension between cross bar and trim plate; and added REMOVE BOTTOM ROUND BAR FOR FULL DEPTH DECK to bottom round bar callout.

Standard	Sheet	Description of Changes
BC-726M (cont.)	2 of 5 (cont.)	TYPICAL HAUNCH FORM DETAIL: SECTION VIEW: added +1/2" TO -1" to end of bulkhead pan dimension; added callout OMITBEAMS to cross bar; added FIELD INSTALLED to the beginning and added MIN. after 16 GAGE for haunch angle callout. DETAIL A: revised TYPICAL to SEE EXTRUSION NOTE for top weld note; added 3/16"
		dimension callout for trim plate fillet weld; and added extrusion note ONE PIECE EXTRUSION IN LIEU OF TWO PIECE MEMBER (EXTRUSION AND PLATE COMBINATION) IS PERMITTED. WELD IN ACCORDANCE WITH AASHTO/AWS D1.5M SPECIFICATIONS;
	3 of 5	END SECTION DETAIL: moved flat pans to mid-depth of main bearing bars.
		SCUPPER INSTALLATION DETAILS - PLAN VIEW: replaced weld note with WELD ALL BARS TO DRAIN FRAME; replaced dimensions with AS SHOWN ON PLANS; and revised callout for bars to be cut.
		FORMED ANGLE – WELDED STRAP: In strap callout deleted WELDED BETWEEN HAUNCH ANGLES and added (OR ALTERNATE THREADED ROD).
		MAIN BAR SPLICE AT PANEL ENDS: added AND SUPLPEMENTAL BARS to weld callouts (three instances)
	4 of 5	Renamed FIELD WELD DETAIL to OPTIONAL FIELD WELD DETAIL WITHOUT HAUNCH
		LEVELING BOLT DETAIL: revised bolt to consist of a threaded rod with a hex nut that is connected via a plug weld and added THREADED ROD (A307, F1554 GR 36, OR EQUAL) callout.
		Added SHEAR CONNECTION AT PARTIALLY AND FULLY FILLED GRID DECKS detail.
		PARTIAL TRANSVERSE SECTION THRU GRID DECK – SECTION VIEW: changed drip ledge to form pan dimension from 1/2" to 1/2" TO 1"; replaced FIELD FORMS AT OVERHANG BY CONTRACTOR. FORM PANS IN GRID DECK OMITTED with FORM PANS SHOP INSTALLED. FIELD INSTALLED FORMS OPTIONAL. (FULL DEPTH OVERHAND) for form pan callout; added form pan line work; and added note MAIN BAR CAMBERING AS PERMITTED BY AWS D1.5 PUB.408.
		END TRIM PLATE WELD DETAIL: revised 1 3/4" to 1 1/2" for dimension in between top of bearing bar and end trim plate.
		LEVELING PLATE WELD DETAIL: added second note • THE LEVELING NUT MAY BE PLACED UNDER THE MAIN BAR WHEN CONDITIONS PERMIT. ALTERNATE LEVELING DETAILS PERMITTED AS APPROVED BY THE DISTRICT BRIDGE ENGINEER.
		LEVELING PLATE WELD DETAIL – PLAN VIEW: replaced dimensions with AS REQUIRED.
		LEVELING PLATE WELD DETAIL - SECTION I-I: removed 5/8" and 11/16" from hex nut and plate hole centerline callout; changed 2" to AS REQUIRED for dimensions between hex nut centerline and inner cross bars; removed 3" x 1/2" x 5" LG. from end of leveling plate callout.
	5 of 5	CONCRETE GRID TRANSVERSE SPLICE BETWEEN PANELS (two places): added note OPTIONAL BOLTED SPLICE PERMITTED AS APPROVED BY THE DISTRICT BRIDGE ENGINEER.
		SECTION J-J and SECTION K-K: added solid lines for precast concrete surfaces; revised line work of pans; added FLAT PAN FIELD INSTALLED (TYP.) callout.; and added 8" C.C. spacing to stud callout
		Added note: NOTE: SPLICE DETAILS CAN ALSO BE USED FOR CAST-IN-PLACE WITHOUT BLOCKOUT CLOSURE POURS.
BC-732M	1 of 3	SUPPORT AT P/S CONCRETE DEAM: added SEE DETAIL X callout and circle.
		Added new DETAIL X detail.
		NOTES: In Note 16 deleted text in last 2 lines after 'TABLES'.
	3 of 3	Removed notes below each portion of Table.
BC-734M	1 of 3	NOTE 8, item (1): inserted TRANSITION after GUIDE RAIL.
BC-736M	1 of 3	GENERAL NOTES, Note 1: reworded to refer to Pub. 408 Specifications.
[WELDED WIRE FABRIC table: corrected designation W2.0xW2.0 to be W2.1xW2.1.
	2 of 3	Moved all content related to development length and lap splice length of deformed bars in tension (Table A, Notes, Guidelines) to sheet 3.

Standard	Sheet	Description of Changes
BC-736M (cont.)	2 of 3 (cont.)	DEVELOPMENT LENGTH AND LAP SPLICE LENGTH OF DEFORMED BARS IN COMPRESSION: revised AASHTO article numbers from 5.11.2.2.1 to 5.10.8.2.2a and from 5.11.5.5.1 to 5.10.8.4.5a to comply with AASHTO 8th Edition (2017). DEVELOPMENT LENGTH OF STANDARD HOOKS IN TENSION: removed reference to Grade 40; revised AASHTO article number from 5.11.2.4.1 to 5.10.8.2.4a to comply with AASHTO 8th
	3 of 3	Edition (2017), and revised factor in Note 1 from 0.7 to 0.8. Moved TABLE A, NOTES, and GUIDELINES here from Sheet 2.
		Class C splices have been removed based on AASHTO 8th Edition (2017) reinforcement design changes.
		Lap lengths and development lengths in Tables A, B and C have been revised and updated based on AASHTO 8th Edition (2017) reinforcement design changes.
		TABLES A, B and C: removed references to Grade 40 and revised TOP BARS to HORIZONTAL BARS with a note to SEE NOTE 3.
		NOTES FOR DEFORMED BARS IN TENSION: Revised and updated the notes for Development Length and Lap Splice Length and made the definition of horizontal bars (formerly top bars) a separate item 3.
BC-739M	2 of 2	SECTION G-G and SECTION H-H: corrected the safety wingwall rear face to be vertical instead of battered.
BC-751M	1 of 7	NOTES: deleted Note 11 and re-numbered remaining notes.
	3 of 7	DRAIN BOX DETAIL – PLAN and DETAIL F - PLAN: replaced 'ADHESIVE' with 'x 3 3/4" EXPANSION' in anchor bolts callout; revised size from 3/4" to 3/8"; and deleted *NOTE above details.
BC-753M	1 of 3	NOTES: added Note 19. OPTIONAL METHOD FOR STUD LOCATION IS TO STAGGER STUD ROWS TRANSVERSELY ACROSS DECK. BY NOT PLACING STUDS AT SAME LOCATION ON ALL THE BEAMS. SEE STAGGERED STUD ROW ON SHEET 3. [This note is based on PSU research report, "Bridge Deck Cracking: Effects on In-Service Performance, Prevention, and Remediation".]
	2 of 3	LONGITUDINAL-TRANSVERSE STIFFENER INTERSECTION DETAILS – PLAN VIEW: added CONTINUOUS THROUGH INTERMEDIATE STIFFENERS ONLY to longitudinal stiffener callout.
	3 of 3	New sheet with GIRDER HAUCH STIFFENER DETAIL and STAGGERED STUD ROW PLAN details.
BC-754M	I of 2	DETAIL A and DETAIL B: corrected the bolts spacings because they violated the minimum spacing.
		NOTES, Notes 11, 12 and 13: updated bolt designation from ASTM A325 to ASTM F3125 GRADE 325.
BC-755M	1 of 4	ELEVATION – EXPANSION BEARING and ELEVATION – FIXED BEARING: added 1/2" MIN. BOLT THREAD PROTRUSION callout to anchor bolt.
	2 of 4	SECTION A-A: added 1/2" MIN. BOLT THREAD PROTRUSION call-out to anchor bolt.
	3 of 4	Added 1/2" MIN. BOLT THREAD PROTRUSION callout to anchor bolt (three places).
	4 of 4	EXPANSION BEARINGS end elevations (three places): revised distance from edge of beam flange to edge of sole plate from 0 MIN. to 1/2" MIN. and added 1/2" MIN. BOLT THREAD PROTRUSION callout to anchor bolt.
BC-762M	1 of 7	GENERAL NOTES, Note 1: removed first sentence about reinforcement requirements and added REINFORCEMENT before BARS in remaining sentence.
		GENERAL NOTES, Note 3: revise note to require galvanized steel in all cases and painting only if specified [for consistency with changes to Pub. 408, Sect. 1026]
<u>.</u>	7 of 7	GENERAL NOTES, Note 4: added GALVANIZED after steel grade designation.
	7 of 7	TYP. DRAIN BOX INSTALLATION @ PIERS - FOR P/S BEAMS: removed steel plates from the top of the end diaphragms.
DC 766M	2 of 2	SECTION H-H: added missing object line for side of drain box.
BC-766M BC-767M	2 of 2 1 of 6	SECTION A-A: revised to indicate joint between end of DECK SLAB and APPROACH SLAB. GENERAL NOTES, Note 1: removed first sentence about reinforcement requirements and added
DO-7 07 101	1 01 0	REINFORCEMENT BARS after STEEL in remaining sentence.

Standard	Sheet	Description of Changes
BC-767M (cont.)	1 of 6 (cont.)	GENERAL NOTES, Note 3: Revise note to require galvanized of steel in all cases and painting only if specified (for consistency with changes to Pub. 408, Sect. 1026).
		GENERAL NOTES, Note 4: added GALVANIZED after steel grade designation.
	2 of 6	DIMENSION "A" TABLE: added USE 2 1/2" MIN. FOR DIMENSION "A" to note below table.
BC-770M	1 of 4	GENERAL NOTES, notes 3, 11, and 13: updated bolt designation from A325 to ASTM F3125 GRADE A325.
		GENERAL NOTES, note 3: added DIAMETER after 7/8".
BC-776M	2 of 7	MATERIAL NOTES, Note 3: deleted second bullet regarding soft converted metric sizes.
	4 of 7	PRECAST CONCRETE PANEL - ELEVATION: revised lap splice length of #4 perimeter rebar from 11" to 1'-3" for uncoated or galvanized and from 1'-4" to 1'-6" for epoxy coated.
		LEGEND FOR WELDED WIRE FABRIC: for C and D definitions, replaced SIZE with CROSS SECTIONAL AREA IN SQ. INCHES MULTIPLIED BY 100.
	5 of 7	ELEVATION – SLOPED TOP and ELEVATION – LEVEL TOP: revised lap splice length of #4 perimeter rebar from 11" to 1'-3" for uncoated or galvanized and from 1'-4" to 1'-6" for epoxy coated.
	7 of 7	ACCESS DOOR DETAIL: revised lap splice length of #4 perimeter rebar from 11" to 1'-3" for uncoated or galvanized and from 1'-4" to 1'-6" for epoxy coated.
BC-777M	7 of 12	DETAIL 3 - ELEVATION: added 3" dimensions from optional construction joint in caisson to ties.
		VERTICAL SPLICE NOTES: updated WWF from 4x4-D4.0 x D4.0 to 4x4-W4.0xW4.0.
	9 of 12	DETAIL 5 - ELEVATION: added 3" dimensions from optional construction joint in caisson to ties.
	10 of 12	DETAIL 6 - ELEVATION: added 3" dimensions from optional construction joint in caisson to ties.
BC-779M	5 of 9	PRECAST CONCRETE PANEL – ELEVATION details: revised lap splice length of #4 perimeter rebar from 11" to 1'-3" for uncoated or galvanized and from 1'-4" to 1'-6" for epoxy coated.
BC-780M	4 of 8	STANDARD PANEL – ELEVATION and STANDARD SLOPED PANEL – ELEVATION: revised lap splice length of #5 perimeter rebar from 1'-1" to 1'-7" for uncoated or galvanized and from 1'-8" to 1'-11" for epoxy coated. LEGEND FOR WELDED WIRE FABRIC: for C and D definitions, replaced SIZE with CROSS
		SECTIONAL AREA IN SQ. INCHES MULTIPLIED BY 100.
	5 of 8	Updated bolt designations from ASTM A325 to ASTM F3125 GRADE A325 (three instances).
BC-783M	2 of 4	ELEVATION VIEW: revised welded wire mesh size from 3x3-W10xW10 to 2x2-W4.0xW4.0. REINFORCED CONCRETE REPAIR TYPE 2 NOTES, Note 10: revised 3x3-W10xW10 to 2x2-
[W4.0xW4.0.
	3 of 4	Updated welded wire mesh designations from 2x2-W4XW4 to 2x2-W4.0xW4.0 (four instances).
	4 of 4	Updated welded wire mesh designations from 4"x4"-W8XW8 to 4x4-W2.1xW2.1 (five instances).
BC-788M	4 of 12	ABUTMENT PLANS (two places): added SPONGE between NEOPRENE and WASHER in callouts.
	5 of 12	SECTION-ABUTMENT WITH BACKWALL - PRESTRESSED AND STEEL I-BEAM: added waterproofing membrane; adjusted height of protective panel; added CONSTR. JT. at base of backwall.
	9 of 12	MEMBRANE WATERPFROOFING DETAIL: in bituminous wearing course callout revised FJ-1, LEVELING COURSE to HMA WEARING COURSE (LEVELING), 4.75MM MIX (two places).
	10 of 12	MEMBRANE WATERPFROOFING DETAIL: in bituminous wearing course callout revised FJ-1, LEVELING COURSE to HMA WEARING COURSE (LEVELING), 4.75MM MIX.
BC-790M	1 of 1	Initial Release of Standard: POST-TENSIONING OF CONCRETE GIRDERS GROUTING SPECIFICATIONS
BC-794M	1 of 1	GENERAL NOTES: added Note 5 - INSERTS TO BE GALVANIZED OR COMPLETELY ZINC- ELECTROPLATED.
		UTILITIES SUPPORTED BY I-BEAMS: added, SEE NOTE 5 to insert callouts (two instances).
BC-798M	1 of 3	DETAIL D: added joint dimension of 1 1/2" MAX.
		TYP. BOX SECTION SHOWING STRAND LOCATIONS: removed H and S dimensions.
		POST-TENSION CONNECTION DETAILS – TYPICAL STRAND & DETAILS: removed AND ACCEPTANCE BY ENGINEER from callout for "CUT STRANDS".

Standard	Sheet	Description of Changes
BC-798M (cont.)	1 of 3 (cont.)	INSTRUCTIONS FOR POST-TENSIONING NOTES, item 11: removed AND APPROVED after WITNESSED.
		INSTRUCTIONS FOR POST-TENSIONING NOTES, item 13: removed second sentence.
		INSTRUCTIONS FOR POST-TENSIONING NOTES: added item 18 - ALTERNATE POST- TENSIONING SEQUENCE VARYING FROM DETAILS SHOWN ON THIS STANDARD MUST BE DETAILED ON SHOP DRAWINGS AND ACCEPTED BY DISTRICT BRIDGE ENGINEER.
		INSTRUCTIONS FOR POST-TENSIONING NOTES: added item 19 - POST-TENSIONING DUCTS MUST BE ADEQUATELY SECURED TO PREVENT DEFLECTION DURING CONCRETE PLACEMENT. DUCTS THAT ARE NOT STRAIGHT MUST BE ACCEPTED BY CHIEF STRUCTURAL MATERIALS ENGINEER.
BC-799M	1 of 13	GENERAL NOTES, Note 21: replaced statement in quotes with "I CERTIFY THAT ALL ASUMPTIONS MADE IN DESIGNING THIS WALL HAVE BEEN VALIDATED THROUGH CONSTRUCTION DETAILS, SPECIAL NOTES AND/OR INSTRUCTIONS TO THE FABRICATOR, ERECTOR AND CONTRACTOR".
	2 of 13	BRIDGE ABUTMENT: removed "R" roadway item symbol and SEE BD628M from BRIDGE APPROACH SLAB callout.
	3 of 13	CRASH WALL REQUIREMENTS: in title, replaced BRIDGES OVER with M.S.E. WALLS NEAR.
		C.I.P. CONCRETE COPING DETAIL: added EMBEDMENT to 1'-6" MIN. dimension; removed quantity from #4 DOWELS callout.
		DETAIL A – ELEVATION: replaced EMBEDDED 12" INTO PANEL with DOWELS.
	4 of 13	TYPICAL C.I.P. BARRIER WITH CEMENT CONCRETE SHOULDER: revised vertical reinforcement across slab/barrier construction joint, replaced 90 degree hook with 180 degree hook at end of slab top rebar, and eliminated 90 degree hook from end of slab bottom rebar.
		TYPICAL C.I.P. BARRIER WITH BITUMINOUS SHOULDER: increased #6 rebar's horizontal leg from 2'-3" MIN. to 2'-4" MIN.
	5 of 13	PRECAST BARRIER WITH BITUMINOUS SHOULDER: In Note A, revised RC-20M to RC-27M.
		REINFORCEMENT FOR BARRIER WITH CEMENT CONCRETE SHOULDER: added MIN. to 1'-6" vertical rebar lap length.
	6 of 13	PLAN - BARRIER MOMENT SLAB details (two places): revised moment slab length from (VARIES) 30' MIN., 40'-0" MAX. to 30'-0"; and in third line, revised ONE PAVEMENT JOINT to TWO PAVEMENT JOINTS; and revised RC-20M to RC-27M in NOTE B.
	10 of 13	TIE STRIP LOCATION: updated bolt designation from A325 to ASTM F3125 GRADE A325.
	11 of 13	TIE STRIP LOCATION: updated bolt designation from A325 to ASTM F3125 GRADE A325.

OS-299 (7-08)



TRANSMITTAL LETTER

PUBLICATION:

Publication 219M September 2016 Edition Change No. 1

DATE:

August 4, 2017

SUBJECT:

Revisions to Standards for Bridge Construction September 2016 Edition

INFORMATION AND SPECIAL INSTRUCTIONS:

Incorporate the attached revisions into the September 2016 Edition of Publication 219M.

The revisions pertain primarily to:

- * Manual for Assessing Safety Hardware (MASH) 2016 Edition.
- *Adding a new Standard Drawing for Type 31 Strong Post Guide Rail (RC-51M) (31" height to top of W-beam rail element).
- *Deleting an existing Standard Drawing for Type 2 Strong Post Guide Rail (RC-52M) (27 3/4" height to top of W-Beam rail element).

These revised Standard Drawings should be adopted on all new and existing designs as soon as possible without affecting any letting schedules and in conjunction with the current Publication 408 Specifications. Regardless, revised standards must be used on projects let after December 31, 2017.

STANDARD	SHEET	DESCRIPTION OF CHANGES			
BC-703M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).			
		Revised W-Beam to Thrie-Beam Transition Sections in the two elevation views between Post 3 and Post 5 from a symmetrical shape to an asymmetrical shape.			
BC-706M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).			
		Revised Elevation A-A to indicate RC-51M.			
BC-708M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).			
		Revised Note 4 to indicate RC-51M.			
BC-712M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).			
		Revised W-Beam to Thrie-Beam Transition Section in Elevation View between Post 5 and Post 7 from a symmetrical shape to an asymmetrical shape.			
		Revised Note 4 to indicate RC-51M.			

STANDARD	SHEET	DESCRIPTION OF CHANGES
BC-734M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).
BC-739M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).
		Revised Note 5 and Section C-C to indicate RC-51M.
		Revised Note 6 to indicate Test Level 4 (TL-4) equivalence and Test Level 3 (TL-3) equivalence based on NCHRP Report 350 criteria.
	Sheets 1-2	Revised Elevation Views for Typical Concrete Bridge Barriers and Alternate Concrete Bridge Barriers to indicate Type 31-SC Guide Rail and RC-51M.
BC-741M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).
BC-743M	Sheet 1 Sheet 2	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail). Revised details for Typical Shoulder Installations to indicate RC-51M. Revised Table "A" to indicate column as "MINIMUM UNOBSTRUCTED DISTANCE" rather than "REQUIRED CLEARANCES". Modified note under Table "A" that refers to the column for "MINIMUM UNOBSTRUCTED DISTANCE". Revised values in Table "A" of minimum unobstructed distances for Type 31 Strong Post Guide Rail, Type 2 Weak Post Guide Rail, and Median Barrier.
BC-744M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).
	Sheet 2	Revised details for Typical Shoulder Installations to indicate RC-51M. Revised Table "A" to indicate column as "MINIMUM UNOBSTRUCTED DISTANCE" rather than "REQUIRED CLEARANCES". Modified note under Table "A" that refers to the column for "MINIMUM UNOBSTRUCTED DISTANCE". Revised values in Table "A" of minimum unobstructed distances for Type 31 Strong Post Guide Rail, Type 2 Weak Post Guide Rail, and Median Barrier.
BC-745M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).
	Sheet 2	Revised details for Typical Shoulder Installations to indicate RC-51M.
		Revised Table "A" to indicate column as "MINIMUM UNOBSTRUCTED DISTANCE" rather than "REQUIRED CLEARANCES".
		Modified note under Table "A" that refers to the column for "MINIMUM UNOBSTRUCTED DISTANCE".
		Revised values in Table "A" of minimum unobstructed distances for Type 31 Strong Post Guide Rail, Type 2 Weak Post Guide Rail, and Median Barrier.
BC-747M	Sheet 1	Revised reference drawing to RC-51M (Type 31 Strong Post Guide Rail).
	Sheet 5	Revised Table "A" to indicate column as "MINIMUM UNOBSTRUCTED DISTANCE" rather than "REQUIRED CLEARANCES".
		Modified note under Table "A" that refers to the column for "MINIMUM UNOBSTRUCTED DISTANCE".

STANDARD BC-747M (cont.)	ANGES ble "A" of minimum unobstructed distances for Type Rail, Type 2 Weak Post Guide Rail, and Median				
CANCEL AND DESTROY THE FOLLOWING: The following revised BC-700M Series standards need to be retained for projects under construction and for future rehabilitation work: Index Sheet - Sept. 30, 2016 BC-703M - Sept. 30, 2016 BC-706M - Sept. 30, 2016 BC-712M - Sept. 30, 2016 BC-712M - Sept. 30, 2016 BC-734M - Sept. 30, 2016 BC-739M - Sept. 30, 2016 BC-741M - Sept. 30, 2016 BC-743M - Sept. 30, 2016			ADDITIONAL COPIES ARE AVAILABLE FROM: PennDOT SALES STORE (717) 787-6746 phone (717) 787-8779 fax ra-penndotsalesstore.state.pa.us PennDOT website - www.dot.state.pa.us Click on Forms, Publications & Maps DGS warehouse (PennDOT employees ONLY)		
BC-744M - Sept. 30, 2016 BC-745M - Sept. 30, 2016 BC-747M - Sept. 30, 2016			LESLIE S. RICHARDS Secretary of Transportation BY: Brian G. Thompson, P.E. Director, Bureau of Project Delivery, Highway Administration		

OS-299 (11-13)



TRANSMITTAL LETTER

PUBLICATION:

Publication 219M September 2016 Edition

DATE: October 5, 2016

SUBJECT:

Standards for Bridge Construction, BC-700M Series September 2016 Edition

INFORMATION AND SPECIAL INSTRUCTIONS:

These standards may be used immediately and can be adopted as soon as practical on all new and existing designs without affecting letting schedules. All projects with T.S. & L. submissions after December 2, 2016 should incorporate these new standards.

The 2016 Edition incorporates Changes 1 through 3 issued for the 2010 Edition.

A description of the changes made to the 2010 Edition since Change 3 of Nov. 21, 2014 and additional revisions of each standard are listed in the attached multi-sheet Table. Note highlighted details and/or notes on each standard are revisions made since Change 3.

CANCEL AND DESTROY THE FOLLOWING:

Existing BC-700M Series standards need to be retained for projects under construction and for

future rehabilitation work.

ADDITIONAL COPIES ARE AVAILABLE FROM:

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APPROVED FOR ISSUANCE BY:

Leslie S. Richards – Secretary of Transportation

Bv:

Brian G. Thompson, P.E.,

Director of Bureau of Project Delivery,

Highway Administration

STANDARD	SHEET	DESCRIPTION OF CHANGES			
BC-700M	1 sht.	Added BC-726M and revised Approval Dates for new Edition.			
BC-701M	1 of 3	Added Note 13 which was previously displayed on Typical Fence Elevation. NOTE 6: added PERMIT after JOINTS. TYPICAL FENCE ELEVATION: added FABRIC after PROTECTIVE FENCE callouts.			
	2 of 3	POST BRACKET DETAIL: added 3 3/4" vertical distance to horizontal shield pipe.			
	3 of 3	ELEVATION: added FABRIC after to call-out for Mesh Diamond Chain Link Fence. Added 2" MAX. dimension for space between mesh and top of wall. SECTIONS D-D: removed 4" dimension for height of bottom rail above top of wall.			
BC-703M	2 shts.	Re-issued with no changes.			
BC-706M	1 of 2	Added (BEHIND W-BEAM) to callout for 5/8" Dia. Hex Head Bolt. TYPICAL SECTION - added DELINEATOR to steel post. Added TUBULAR BLOCKOUT to TSx7x3x1/4". Added ROUND HEAD for two bolt callouts.			
		NOTES: added Notes 12 & 13. ELEVATION A-A: added 5/8" DIA. HEX HEAD BOLT (BEHIND RUBRAIL). PLAN - added ROADWAY ITEM to W-BEAM RAIL and OFFSET BRACKET. Added W6X9 STEEL POST (ROADWAY ITEM) callout.			
		RAIL TUBE END CAP - added PJP GRIND TO CONTOUR (TYP.) to weld symbol.			
	2 of 2	Added new sheet with nut and bolt details.			
BC-707M	1 of 5	GENERAL NOTES: NOTE 3 - added ASTM C834 OR C920 to end of note.			
BC-708M	2 shts.	Section letters revised.			
BC-709M	3 of 12	TYPICAL WELD AT MITERS - added BENDING OF 1/2" THICK PLATE IS PERMITTED INSTEAD OF WELDING to end of callout. RAIL TUBE CAP DETAIL - decreased size of cap plate from 4 3/4" to 4 5/8".			
	4 of 12	RAIL SPLICE - added OR 1/4"x 3/4" x 3/16" PLATE ATTACHED WITH DUAL 3/16"x5/8" LONG FILLET WELDS to callout for pin/stud.			
	10 of 12	PLAN - BARRIER MOMENT SLAB - added (ROADWAY) to tie bars/bolts callout.			
	11 of 12	PRECAST BARRIER WITH BIT. SHOULDER - increase spacing of top transverse #5 reinforcement from 11" to 12".			
BC-711M	2 of 4	ACCIDENT PREVENTION SIGN - added R = 1/2" (TYP.) callout to lower right corner of sign.			
	4 of 4	PARTIAL INSIDE ELEVATION - removed GROUND CONNECTIONS TO BE MADE BY RAILROAD statement from tapped holes' callout.			
BC-712M	1 of 2	PLAN VIEW FOR THRIE-BEAM TO PA BRIDGE BARRIER: Type C Inlet callout- replaced RC-34M with RC-45M and RC-46M.			
BC-713M	1 of 13	NOTES: Note 9 - added THICK PLATE prior to LOCK WASHER and washer. REFERENCE DRAWINGS: Added BC-711M, BC-720M, BC-721M, BC-734M, BC-736M, BC-752M, BC-767M, BC-799M, RC-20M, and RC-50M.			
	3 of 13	Added new PLATE WASHER DETAIL. SECTION A-A: added "AND PLATE WASHER DETAIL ON THIS SHEET" to end of slotted hole callout. Added plate washer beneath two lock washer/nut connections.			
	5 of 13	TYPICAL RAIL TO POST DETAIL: added "AND PLATE WASHER DETAIL ON SHEET 3" to slotted hole callout. Added plate washer beneath two lock washer/nut connections.			
1	11 of 13	CAST-IN-PLACE PA BRIDGE BARRIER ON M.S.E. WALLS: changed STYROFOAM to PREFORMED CELLULAR POLYSTYRENE (P.C.P.) at two locations.			
	12 of 13	PRECAST BARRIER WITH BITUMINOUS CONCRETE SHOULDER ON M.S.E. WALLS: changed STYROFOAM to PREFORMED CELLULAR POLYSTYRENE. PRECAST BARRIER WITH CEMENT CONCRETE SHOULDER ON M.S.E. WALLS: changed STYROFOAM to PREFORMED CELLULAR POLYSTYRENE.			
BC-716M 1 of 2 Added ALTERNATE DETAIL A for attachment of pedestrian railing post with anchor bolts of slab. Added ALTERNATE PLATE DETAIL. DETAIL A: added galvanized anchor bolts callout.		Added ALTERNATE PLATE DETAIL.			

STANDARD	SHEET	DESCRIPTION OF CHANGES	
BC-716M	2 of 2	TYPICAL DETAIL AT POST: added RAILING HEIGHT to two heights.	
BC-718M	1 sht.	Re-issued with no changes.	
BC-719M	1 of 8	NOTES: added NOTE 17 -BOLT THROUGH ANCHORS ARE NOT PERMITTED IN RECENTLY POURED DECKS WITHOUT APPROVAL OF DISTRICT BRIDGE ENGINEER. NOTE 8, TYPE B: added STANDARD WIDTH prior to LANES OF TRAFFIC in first line. Added Reference Drawings.	
	3 of 8	NOTES: added NOTE 3 - FOR SPACING AND MINIMUM REQUIRED ADHESIVE ANCHOR ULTIMATE CAPACITY SEE TABLE 1, SHEET 1.	
7 of 8 Added three Notes		Added three Notes.	
BC-720M	1 sht.	ELEVATION: added 1'-5" spacings for railing posts to center of light pole. SECTION A-A & SECTION C-C: added MIN. to wall thickness dimension.	
BC-721M	1 of 2	EXPANSION AND DEFLECTION JOINT FITTINGS - added (SEE NOTE 2) to deflection fitting's ground connection callout.	
	2 of 2	CONDUIT EXPANSION NOTES, Note 2: added ARTICLE prior to NEC314. EXPOSED CONDUIT CONNECTIONS AT EXPANSION JOINTS: added CONDUIT EXPANSION prior to NOTE 1 in conduit callout.	
BC-722M	2 of 2	Barrier Pedestrian Fence Post to Light Pole spacing increased from 1'-0" to 1'-1". Added 3" space between end of Pedestrian Railing to light pole and removed 1'-6" dimension.	
BC-723M	10 shts.	Minor notes changes made throughput.	
BC-726M	5 shts.	Initial release.	
BC-731M	1 sht.	Re-issued with no changes.	
BC-732M	1 of 3	TYPICAL LONGITUDINAL SECTION: added note regarding deck top reinforcement mat orientation. NOTES: added Notes 14, 15 and 16.	
BC-734M	3 shts.	Re-issued with no changes.	
BC-735M	1 sht.	Re-issued with no changes.	
BC-736M	3 shts.	Re-issued with no changes.	
BC-739M		Re-issued with no changes.	
BC-741M	1 of 6	NOTES TO FABRICATOR, 1st bullet point: reworded first sentence to recommend use of Centermount structure types to carry DMS/VMS. Added mention of overhead sign structures not represented by BD-649M must be designed by PE.	
		GENERAL NOTES: revised Note on bolt hole diameter for bolts. Added Note 13 to require checking the clear distance between bolt holes and to end of member.	
	2 of 6	ALTERNATE FOUNDATION, Note: added , #13-602-BDTD AND #14-603-BDTD FOR SUPPORT OF CENTER-MOUNT DMS SIGN STRUCTURES.	
	3 of 6	ALTERNATE FOUNDATION, Note: added , #13-602-BDTD AND #14-603-BDTD FOR SUPPORT OF CENTER-MOUNT DMS SIGN STRUCTURES. ALTERNATE CAISSON FOUNDATIONS table: caissons for unavailable larger wall thickness 24" and 26" pipe sizes were removed.	
	4 of 6	PIPE CAPS table: pipe caps for unavailable larger wall thickness 24" and 26" pipe sizes were removed. COLUMN BASES: bases for unavailable larger wall thickness 24" and 26" pipe sizes were removed.	
	5 of 6	NOTES, 3rd bullet point: removed TC-8716.	
BC-743M	1 of 10	GENERAL NOTES: revised Note on bolt hole diameter for bolts. Added Note 13 to require checking the clear distance between bolt holes and to end of member.	
	3 of 10	ALTERNATE CAISSON FOUNDATIONS table: caissons for unavailable larger wall thickness 24" and 26" pipe sizes were removed.	
	4 of 10	COLUMN BASES table: column bases for unavailable larger wall thickness 24" and 26" pipe sizes were removed.	
		PIPE CAPS table: pipe caps for unavailable larger wall thickness 24" and 26" pipe sizes were removed.	
	5 of 10	CHORD SPLICE table; chord splices for unavailable larger wall thickness 24" and 26" pipe sizes were removed.	

STANDARD	SHEET	DESCRIPTION OF CHANGES					
BC-743M (continued)	6 of 10	COPE HOLE DETAIL (TYP.): revised cope hole radius to be dependent on size of gusset plate. ALTERNATE PANEL POINT CONNECTION GUSSET PLATE DIMENSIONS table: chords sizes for unavailable larger wall thickness 24" and 26" pipe sizes were removed.					
	7 of 10	SADDLE BLOCK DIMENSIONS table: saddle blocks for unavailable larger wall thickness 24" and 2 pipe sizes were removed. TRUSS SEAT table, truss seats for unavailable larger wall thickness 24" and 26" pipe sizes were removed.					
BC-744M	1 of 12	GENERAL NOTES: revised Note on bolt hole diameter for bolts. Added Note 13 to require checking the clear distance between bolt holes and to end of member.					
	4 of 12	ALTERNATE CAISSON FOUNDATIONS table: caissons for unavailable larger wall thickness 24" and 26" pipe sizes were removed.					
	5 of 12	COLUMN BASES table; column bases for unavailable larger wall thickness 24" and 26" pipe sizes were removed PLAN OF COLUMN BASE TYPE Y: added 1/2" MIN. CLR. (TYP.) for space from 2" holes to inside of column.					
	6 of 12	COLUMN BASES - 4 POST STRUCTURES table: column bases for unavailable larger wall thickness 24" and 26" pipe sizes were removed. PLAN OF COLUMN BASE TYPE W: added 1/2" MIN. CLR. (TYP.) for space from 2" holes to inside of column.					
	8 of 12	SIGN SUPPORT BRACKET DETAIL: U-bolt dimension equation revised to 7/8" instead of 3/4". CHORD SPLICE table, removed splices					
	9 of 12	TRUSS SEAT table: truss seats for unavailable larger wall thickness 24" and 26" pipe sizes were removed.					
•	11 of 12	SECTION C-C: added CHORD O.D. + 5/8" (TYP.)					
	12 of 12	TYPICAL LIGHT FIXTURE SUPPORT DETAILS: U-bolt dimension equation revised to be CHORD O.D. plus 7/8" instead of 3/4". PIPE CAPS table: pipe caps for unavailable larger wall thickness 24" and 26" pipe sizes were removed.					
BC-745M	1 of 10	NOTES TO FABRICATOR: in 1st bullet point: replaced NOT with RECOMMENDED. Deleted remainder of note.					
		GENERAL NOTES: revised Note on bolt hole diameter for bolts. Added Note 13 to require checking the clear distance between bolt holes and to end of member. DESIGN CRITERIA: Catwalk reference changed to "3.6" under AASHTO Sign Spec.					
	4 of 10	COLUMN BASES table: column bases for unavailable larger wall thickness 24" and 26" pipe sizes were removed. PLAN OF COLUMN BASE TYPE W: added 1/2" MIN. CLR. (TYP.) for space from 2" holes to inside of column.					
	5 of 10	PIPE CAPS table: pipe caps for unavailable larger wall thickness 24" and 26" pipe sizes were removed.					
BC-747M	1 of 5	Drawing title: 200' changed to 160' and 38' changed to 27'. GENERAL NOTES: revised Note on bolt hole diameter for bolts. Added Note 11 to require checking the clear distance between bolt holes and to end of member. CAISSON BELL DIA. FOR SOFT COHESIVE SOIL FRAME STR table: bell diameters for 180' & 200' removed					
	2 of 5	END CONNECTIONS - SECTION: changed Hole Diameter to be Bolt Diameter + 1/8".					
		Component Selection Tables: 38' span designs were removed since larger pipe sizes are unavailable. Base Plate size for 27' 350 SF design case changed from 2 1/8" to 2 1/4".					
	3 of 5	Base plate thicknesses increased to either 2 1/4" or 2 1/2" for seven entries in table.					
		MAST ARM & SPLICE CONNECTION COMPONENT SELECTION, MAST & BASE CONNECTION COMPONENT SELECTION TABLES: design selections removed since larger wall thickness 24" and 26" pipe sizes are unavailable.					
	5 of 5	CAISSON COMPONENT SELECTION TABLE FRAME STRUCTURES: removed entries for 180' and 200' spans due to unavailability of larger pipe sizes.					

STANDARD SHEET DESCRIPTION OF CHANGES		DESCRIPTION OF CHANGES	
		CAISSON COMPONENT SELECTION - CANTILEVER STRUCTURES TABLE: remove 38' span selections. Also remove 27' span with a 460 SF panel area due to unavailability of larger pipe sizes.	
BC-751M	1 of 7	NOTES, No. 3: added CONFORMING TO AASHTO prior to M270.	
	2 of 7	SECTION C-C: pipe wall thickness changed from 3/4" to 3/8" at two places.	
	3 of 7	DRAIN BOX PLAN & DETAIL F: added * EMBEDMENT LENGTH ACCORDING TO MANUFACTURER'S SPECIFICATIONS to adhesive anchor bolt callout.	
	4 of 7	SPLASH BLOCK PLAN: added SPLASH BLOCK INCIDENTAL TO DOWNSPOUT to cement concrete slab callout. VIEW G-G: added INCIDENTAL TO DOWNSPOUT to SPLASH BLOCK callout.	
BC-752M	2 of 2	ALTERNATE TRANSVERSE CONSTRUCTION AND CRACK CONTROL JOINT detail was added. HAUNCH REINFORCEMENT DETAILS: added Note 3 regarding orientation of top reinforcement mat. Construction Joint details moved to Sht. 2 from Sht. 1.	
BC-753M	1 of 2	BEARING STIFFENER: replaced MILL with FINISHED- in callout for end of plate at flange. CORNER CHAMFER DETAIL: added 0" TO after X = and Y =.	
	2 of 2	DETAIL A: revised to indicate web's longitudinal stiffener running continuously and vertical stiffener being disrupted. Added fillet weld symbol. Added reference to CORNER CHAMFER DETAIL on Sheet 1.	
		ALTERNATE BOLTED SPLICE DETAIL AT MAIN MEMBER FIELD SPLICE: replaced 1 1/8" with 1 1/4" for O.D. of tubing. Corrected I.D. of tubing to be 0.688" instead of 0.668". ELEVATION: added 5/8" DIA. BAR callout	
BC-754M	1 of 2	DETAIL A and DETAIL B: modified to match the changes made to the end diaphragm configuration.	
		Note 20 was added. END DIAPHRAGM DETAIL: configuration of diagonal angles changed by attaching them at bottom flanges and mid-span of upper strut. Angle size increased from 3 1/2 x 3/8" to 5 x 1/2"	
	2 of 2	Replaced STRINGER with BEAM at eight locations.	
BC-755M	1 of 4	TABLE A ANCHOR BOLT CLEARANCE table: Dimension A values were decreased. PLAN VIEW: slot thickness and hole diameter in sole plate changed from D + 5/8" to D + 13/16". ELEVATION - EXPANSION BEARING: increased gap between hex nut and washer from 1/8" to 1/2".	
	2 of 4	OPTION I - PLAN VIEW: slot thickness and hole diameter in sole plate changed from D + 5/8" to D + 13/16".	
	3 of 4	LEGEND was added. EXPANSION BEARINGS IE and IIIE: increased gap between hex nut and washer from 1/8" to 1/2".	
BC-756M	1 of 6	ANCHOR BOLT DETAIL 1: replaced 6" DIA. with 2" LARGER THAN ANCHOR BOLT for blockout. Changed to NONSHRINK grout. GENERAL NOTE 9: replaced MIL-S-8660 with SAE-AS8660.	
BC-757M	3 shts.	Re-issued with no changes.	
BC-762M	3 - 6 of 7	SECTIONs: added FOR DECK TOP REINFORCEMENT MAT: TRAVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.	
BC-766M	1 of 2	SECTION AT BARRIER: replaced STANDARD CURB with TYPICAL BARRIER. GENERAL NOTES: In Note 7, replaced THE MATERIALS AND RESTING DIVISION, BOCM with CHIEF MATERIALS ENGINEER, LABORATORY TESTING SECTION, INNOVATION AND SUPPORT SERVICES DIVISION, BOPD.	
	2 of 2	SECTION A-A: replaced preformed expansion joint MATERIAL with FILLER in callout. SECTION C-C: added WIDTH to Joint dimension.	
BC-767M	1 of 6	GENERAL NOTES: In Note 11, replaced THE MATERIALS AND RESTING DIVISION, BOCM with CHIEF MATERIALS ENGINEER, LABORATORY TESTING SECTION, INNOVATION AND SUPPORT SERVICES DIVISION, BOPD.	
	2 & 3 of 6	SECTIONs: added FOR DECK TOP REINFORCEMENT MAT: TRAVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.	
	6 of 6	PLAN: added > 15 degrees to SKEW ANGLE callout.	
BC-770M	4 shts.		
BC-772M	2 of 5	ELEVATION: added STEEL ANGLE (TYP.) to TOP FLANGE EDGE PROTECTION callout.	
BC-775M	1 of 3	GENERAL NOTES: added Note 5 regarding recessing of strands at end of beam.	

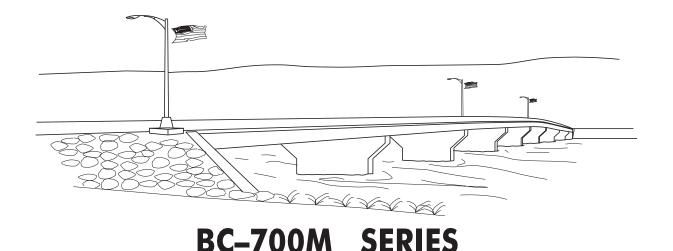
STANDARD	SHEET	DESCRIPTION OF CHANGES				
		GROUTED RECESS FOR STRANDS AT BEAM ENDS: added Note 3 - PAINT BEAM ENDS PRIOR TO SHIPMENT OR STORAGE. DOWEL DETAIL: added STAINLESS STEEL to dowel callout. ACCEPTABLE DRIP NOTCH DETAILS: added Option D.				
BC-775M (continued)	2 of 3	SHEAR KEY DETAIL: added OR CCNS WITH DOUBLE SIDED ADHESIVE STRIP after backer r in callout.				
	3 of 3	Added VERTICAL ADJUSTMENT DEVICE details to be used in conjunction with BD-605M.				
		GENERAL NOTES: minor changes within the notes.				
	4 of 7	PRECAST CONCRETE PANEL: added symbol to Panel Height which refers to the first note under LEGEND. LIFTING INSERT LOCATION & TWO POINT PICK-UP details: added symbol to Panel Length which refers to the first note under LEGEND.				
	5 of 7	PRECAST CONCRETE PANEL: added symbol to Panel Height which refers to the first note under LEGEND.				
	7 of 7	ACCESS DOOR DETAIL: added symbol to Panel Length which refers to the first note under LEGEND.				
BC-777M	7 of 12	WWF VERTICAL SPLICE DETAIL FOR PRECAST CONCRETE POST detail relocated from Sht. 3 and was revised.				
BC-778M	1 of 10	GENERAL NOTES: Note 16 - ENGINEER was replaced with REPRESENTATIVE. MATERIAL NOTES: Note 5 – SECTION 1105.02(c)3a replaced with SECTION 1105.02(c)2b. Note 8, 2 nd bullet point – added AND AFTER THE PANELS ARE INSTALLED to end of statement.				
BC-779M	2 of 9	MATERIAL NOTES: Note 7, 1st bullet point – replaced ASTM A325 with ASTM A307, GRADE A for bolt specification. In 4th bullet point – removed AND PAINT after GALVANIZE.				
	3 & 4 of 9	BARRIER MOUNTED/RETAINING WALL MOUNTED SOUND BARRIER ELEVATION: replaced SPECIFIED with REQUIRED in steel cable connection callout.				
	5 of 9	PRECAST CONCRETE PANEL Elevations - replaced SPECIFIED with REQUIRED in steel cable connection callouts.				
	9 of 9	ELEVATION, SECTION F-F and BOLT DETAIL: replaced A325 with ASTM A307, GRADE A in bolt callouts.				
BC-780M	1 of 8	GENERAL NOTES: Note 20 - ENGINEER was replaced with REPRESENTATIVE.				
	2 of 8	MATERIAL NOTES: Note 7, 1st bullet point - replaced A325 with A 307 for bolt specification.				
	5 of 8	ELEVATION & SECTION E-E: added circle symbol to various footing dimensions which refers to the first note under LEGEND.				
	7 of 8	LEGEND: added circle symbol which denotes AS REQUIRED BY DESIGN REFER TO CONTRACT DRAWINGS. TWO POINT & FOUR POINT PICK-UP details: added circle symbol to Panel Height dimensions.				
BC-781M	1 sht.	Re-issued with no changes.				
BC-782M	1 sht.	Note 4, which restricted use of slope walls in urban or suburban environments, was removed.				
BC-783M 1 of 4 DECK REPAIRS AND LATEX MODIFIED CONCRETE OVERLAY: added FOR DEC REINFORCEMENT MAT: TRAVERSE BARS SHOWN ON TOP, SIMILAR WHEN LO		DECK REPAIRS AND LATEX MODIFIED CONCRETE OVERLAY: added FOR DECK TOP REINFORCEMENT MAT: TRAVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.				
BC-788M	1 of 12	On three details, increased Closed Cell Neoprene Sponge thickness from 1/4" to 1/2" THICKER THAN BEARING PAD.				
	2 of 12	In three details, increased Closed Cell Neoprene Sponge thickness from 1/4" to 1/2" THICKER THAN BEARING PAD at four callouts. DETAIL "B": increased Neoprene Sponge Washer thickness from 1/4" to 1/2" THICKER THAN BEARING PAD.				
	3 of 12	In four details, increased Continuous Strip of Closed Cell Neoprene Sponge's thickness from 1" to 1 1/4".				
	4 of 12	In three details, increased Closed Cell Neoprene Sponge thickness from 1/4" to 1/2" THICKER THAN BEARING PAD at four callouts. ABUTMENT PLANS & SECTION U-U: increased Neoprene Washer thickness from 1/4" to 1/2" THICKER THAN BEARING PAD.				

STANDARD	SHEET	DESCRIPTION OF CHANGES			
BC-788M (continued)	5 of 12	BOX BEAMS WITHOUT BACKWALL & P/S AND STEEL I-BEAM WITHOUT BACKWALL details: increased Closed Cell Neoprene Sponge thickness from 1/4" to 1/2" THICKER THAN BEARING PAD.			
	8 of 12	two details, increased Closed Cell Neoprene Sponge thickness from 1/4" to 1/2" THICKER THAN EARING PAD at three callouts. /ATERPROOFING DETAIL - * Note, 4th line - removed PIER prior to FOOTING.			
	11 of 12	SECTION AT ABUTMENT & PANEL ANCHOR details: increased Closed Cell Neoprene Sponge thickness from 1/4" to 1/2" THICKER THAN BEARING PAD.			
BC-794M	1 sht.	AT ABUTMENTS: replaced UTILITY with PIPE OR MAIN.			
BC-798M	1 of 3 Removed solid triangle note regarding tendon placement in walls and slabs of culvert. TYPICAL STRAND & DETAILS: in dimension callout for strand extension length, replaced PRESTRESSING with TENSIONING.				
	2 of 3	TIE BOLT DETAIL - PRECAST CHANNEL BEAM: washer specification revised from ASTM 436-86 to ASTM F436.			
BC-799M	1 of 13	TYPICAL FILL SECTION: revised vertical dimension from ground line to weep hole from 1'-0" to 6". GENERAL NOTES: Note 6: removed 2nd bullet point regarding Traffic Barrier and Sidewalk Barrier design specifications.			
	7 of 13	SIDEWALK BARRIER SECTION: added Railing on top of wall with callout regarding authorization. Increase rebar cover from 1 1/2" to 2". Wall plus Moment Slab height changed to 5'-7 1/2" from 5'-9 1/2". Added 3'-6" barrier wall height.			
	10 of 13	SECTIONS M-M, N-N, P-P & Q-Q: revised shape of panel's horizontal joint.			
	12 of 13	TYPICAL PANEL LAYOUT: removed Panel Dowels & Tapered Holes centerlines from square panels on right side of detail. NOTE 8: increased PVC Rod diameter from 5/8" to 3/4". Also changed length of 5/8" diameter galvanized steel to 12".			

COMMONWEALTH OF PENNSYLVANIA



BUREAU OF PROJECT DELIVERY STANDARDS FOR BRIDGE CONSTRUCTION



SEPTEMBER 2016 EDITION

INDEX OF STANDARDS FOR BRIDGE CONSTRUCTION with Strike-Off Letters' red markups

THESE STANDARDS MAY BE REFERRED TO ON THE DESIGN DRAWINGS IN LIEU OF SHOWING SPECIFIC DETAILS PROVIDED COORDINATING INFORMATION IS SHOWN ON THE DESIGN DRAWINGS.

Click on the desired Standard to view.

Highlighting throughout the standards indicates revisions to the September 2016 Edition. The highlighting color indicates whether the most recent revision was part of Change #1, #2, #3, #4, #5, #6 or #7. Refer to the legend on this index sheet or on the first sheet of each standard.

To obtain a clean printout without highlighting, select "Print" and in the dialogue box under "Comments and Forms" select "Document" from the pull-down menu.

	0, 201, 10	DETAILS	THOUIDED COOKDINA
STD. DWG.	TITLE	NO.OF SHTS.	DATE
BC-700M	INDEX OF STANDARDS	1	OCT. 7, 2024
BC-701M	PROTECTIVE FENCE	3	NOV. 23, 2022
BC 703M	THRIE BEAM TO VERTICAL WALL BRIDGE BARRIER TRANSITION CONNECTION		DISCONTINUED
BC-706M	PA 3-RAIL BRIDGE BARRIER	2	OCT. 7, 2024)
- BC-707M	PA HT BRIDGE BARRIER		DISCONTINUED
	THRIE-BEAM TO PA TYPE 10M BRIDGE BARRIER TRANSITION CONNECTION		DISCONTINUED
BC-709M	PA TYPE 10M BRIDGE BARRIER	13)	OCT. 7, 2024
BC-711M	ALUMINUM PROTECTIVE BARRIER	4	NOV. 23, 2022
BC-712M	THRIE-BEAM TO PA BRIDGE BARRIER TRANSITION CONNECTION		DISCONTINUED
BC-713M	PA BRIDGE BARRIER	16)	OCT. 7, 2024
BC-716M	ALUMINUM PEDESTRIAN RAILING	2	NOV. 23, 2022
BC-718M	ALTERNATE RAILING DETAILS	_	DISCONTINUED
BC-719M	TEMPORARY CONCRETE BARRIER, STRUCTURE MOUNTED	7	MAR. 27, 2024
BC-720M	ALUMINUM OR STEEL BRIDGE HAND RAILING	1	FEB. 19, 2021
BC-721M	ELECTRICAL DETAILS	2	FEB. 19, 2021
BC-722M	LIGHTING POLE ANCHORAGE	2	FEB. 19, 2021
BC-723M	BRIDGE ANTI-ICING SYSTEM	10	SEPT. 30, 2016
BC-726M	STEEL GRID REINFORCED CONCRETE BRIDGE DECK FOR BEAM BRIDGES	5	JAN. 31, 2019
BC-731M	CEMENT CONCRETE SLOPE WALL	1	SEPT. 30, 2016
BC-732M	PERMANENT METAL DECK FORMS	3	NOV. 23, 2022
BC-734M BC-735M	ANCHOR SYSTEMS WALL CONSTRUCTION AND EXPANSION JOINT DETAILS	2 1	FEB. 19, 2021 SEPT. 30, 2016
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BC-736M	REINFORCEMENT BAR FABRICATION DETAILS	3	NOV. 23, 2022
BC 739M	BRIDGE BARRIER TO GUIDE RAIL		DISCONTINUED
	-TRANSITION		
BC-741M	OVERHEAD SIGN STRUCTURES-CANTILEVER AND CENTER-MOUNT STRUCTURES STRUT LENGTHS UP TO 40'	6	AUG. 4, 2017
BC-743M	OVERHEAD SIGN STRUCTURES-2 POST PLANAR TRUSS SPANS FROM 30'TO 100'	10	AUG. 4, 2017
BC-744M	OVERHEAD SIGN STRUCTURES-2 POST AND 4 POST TRI-CHORD TRUSS SPANS FROM 60' TO 240'	12	AUG. 4, 2017
BC-745M	OVERHEAD SIGN STRUCTURES-4 POST 4 CHORD TRUSS SPANS FROM 100' TO 200'	10	AUG. 4, 2017
BC-747M	MONOPIPE SIGN STRUCTURES	5	AUG. 4, 2017
BC-751M	BRIDGE DRAINAGE	7	JAN. 31, 2019
		<u> </u>	<u> </u>

BC-752M CONCRETE DECK SLAB DETAILS 3 NOV. 23, 2022	STD. DWG.	TITLE	NO.OF SHTS.	DATE
BC-754M STEEL DIAPHRAGMS FOR STEEL BEAM/ GIRDERS ONLY) GIRDERS ONLY) BC-755M BEARINGS 4 JAN. 31, 2019 BC-756M HIGH LOAD MUTI ROTATIONAL 6 NOV. 23, 2022 BC-757M STEEL PILE TIP REINFORCEMENTS & 3 SEPT. 30, 2016 BC-757M STEEL PILE TIP REINFORCEMENTS & 3 SEPT. 30, 2016 BC-762M TOOTH EXPANSION DAM FOR PRESTRESSED 7 JAN. 31, 2019 BC-762M CONCRETE & STEEL BEAM BRIDGES 7 JAN. 31, 2019 BC-766M PREFORMED NEOPRENE COMPRESSION 2 NOV. 23, 2022 BC-766M PREFORMED NEOPRENE COMPRESSION 2 NOV. 23, 2022 BC-767M NEOPRENE STRIP SEAL DAM FOR PRESTRESSED 7 NOV. 23, 2022 BC-767M NEOPRENE STRIP SEAL DAM FOR PRESTRESSED 7 NOV. 23, 2022 BC-770M STEEL MID-SPAN DIAPHRAGMS FOR P/S 4 JAN. 31, 2019 CONCRETE AASHTO I-BEAM AND PA BULB-TEE BEAM BRIDGES 5 NOV. 23, 2022 BC-775M PRESTRESSED CONCRETE BEAM BRACING 5 NOV. 23, 2022 BC-775M MISCELLANEOUS PRESTRESS DETAILS 3 NOV. 23, 2022 BC-776M GROUND MOUNTED SOUND BARRIERS 7 JAN. 31, 2019 BC-777M GROUND MOUNTED SOUND BARRIERS 10 SEPT. 30, 2016 BC-779M GROUND MOUNTED SOUND BARRIERS 10 SEPT. 30, 2016 BC-780M GROUND BARRIER WALLS 8 JAN. 31, 2019 BC-781M RANDOM STONE SLOPE WALL 1 SEPT. 30, 2016 BC-782M GABION SLOPE WALL 1 SEPT. 30, 2016 BC-783M REINFORCED CONCRETE REPAIR 4 JAN. 31, 2019 BC-788M TYPICAL WATERPROOFING AND 12 NOV. 23, 2022 BC-790M POST-TENSIONING OF CONCRETE GIRDERS 1 JAN. 31, 2019 BC-790M POST-TENSIONING OF CONCRETE GIRDERS 1 JAN. 31, 2019 BC-799M MECHANICALLY STABILIZED EARTH 13 NOV. 23, 2022 BC-799M MECHANICALY STABILIZE		CONCRETE DECK SLAB DETAILS		NOV. 23, 2022
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PRECAST CONCRETE PANELS BC-777M GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS BC-778M GROUND MOUNTED SOUND BARRIERS STEEL POSTS BC-779M STRUCTURE MOUNTED SOUND BARRIER WALLS BC-780M OFFSET SOUND BARRIER WALLS BC-780M OFFSET SOUND BARRIER WALLS BC-781M RANDOM STONE SLOPE WALL BC-782M GABION SLOPE WALL DETAILS BC-783M REINFORCED CONCRETE REPAIR BC-783M REINFORCED CONCRETE REPAIR BC-788M TYPICAL WATERPROOFING AND EXPANSION DETAILS BC-790M POST-TENSIONING OF CONCRETE GIRDERS BC-794M UTILITY ATTACHMENT & SUPPORT DETAILS, PRESTRESSED BRIDGES BC-799M MECHANICAL CONNECTION DETAILS BC-799M MECHANICAL CONNECTION DETAILS BC-799M MECHANICALLY STABILIZED EARTH 12 JAN. 31, 2019 BC-799M MECHANICALLY STABILIZED EARTH 13 NOV. 23, 2022	BC-775M	MISCELLANEOUS PRESTRESS DETAILS	3	NOV. 23, 2022
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BC-782M GABION SLOPE WALL DETAILS BC-783M REINFORCED CONCRETE REPAIR BC-788M TYPICAL WATERPROOFING AND EXPANSION DETAILS BC-790M POST-TENSIONING OF CONCRETE GIRDERS GROUT SPECIFICATIONS BC-794M UTILITY ATTACHMENT & SUPPORT DETAILS, PRESTRESSED BRIDGES BC-798M MECHANICAL CONNECTION DETAILS BC-799M MECHANICALLY STABILIZED EARTH 13 NOV. 23, 2022	BC-780M		8	JAN. 31, 2019
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EXPANSION DETAILS BC-790M POST-TENSIONING OF CONCRETE GIRDERS GROUT SPECIFICATIONS BC-794M UTILITY ATTACHMENT & SUPPORT DETAILS, PRESTRESSED BRIDGES BC-798M MECHANICAL CONNECTION DETAILS BC-799M MECHANICALLY STABILIZED EARTH 13 NOV. 23, 2022	BC-783M	REINFORCED CONCRETE REPAIR	4	JAN. 31, 2019
GROUT SPECIFICATIONS BC-794M UTILITY ATTACHMENT & SUPPORT 1 JAN. 31, 2019 DETAILS, PRESTRESSED BRIDGES BC-798M MECHANICAL CONNECTION DETAILS 3 FEB. 14, 2023 BC-799M MECHANICALLY STABILIZED EARTH 13 NOV. 23, 2022	BC-788M		12	NOV. 23, 2022
DETAILS, PRESTRESSED BRIDGES BC-798M MECHANICAL CONNECTION DETAILS BC-799M MECHANICALLY STABILIZED EARTH 13 NOV. 23 2022	BC-790M		1	JAN. 31, 2019
BC-799M MECHANICALLY STABILIZED EARTH 13 NOV. 23, 2022	BC-794M		1	JAN. 31, 2019)
	BC-798M	MECHANICAL CONNECTION DETAILS	3	FEB. 14, 2023
	BC-799M	MECHANICALLY STABILIZED EARTH RETAINING WALLS	13	NOV. 23, 2022

SEPTEMBER 2016 EDITION

SEE CHANGE #1 FOR AUG. 4, 2017 STANDARD REVISIONS.

SEE CHANGE #2 FOR JAN. 31, 2019 STANDARD REVISIONS.

SEE CHANGE #3 FOR FEB. 19, 2021 STANDARD REVISIONS.

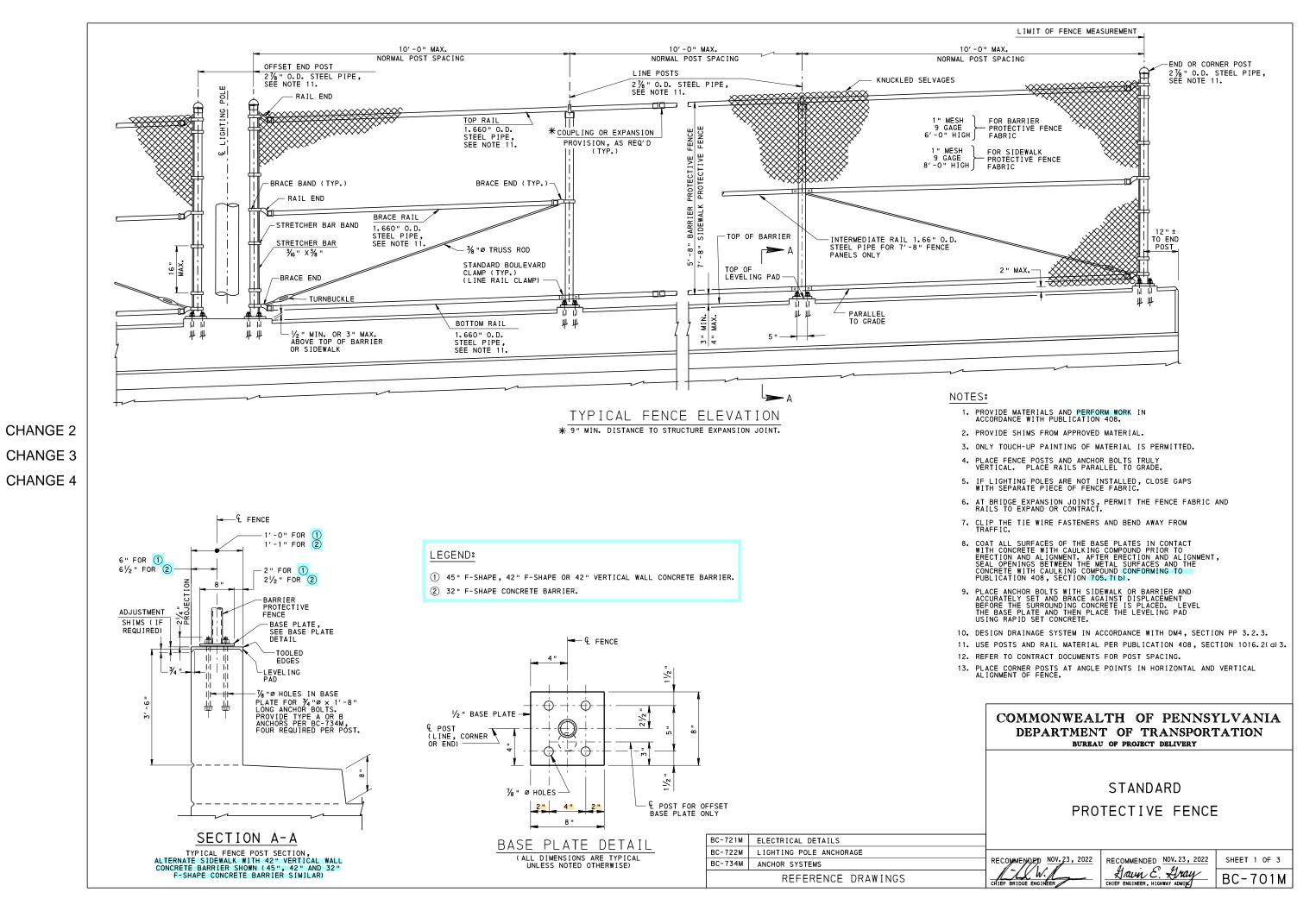
SEE CHANGE #4 FOR NOV. 23, 2022 STANDARD REVISIONS.

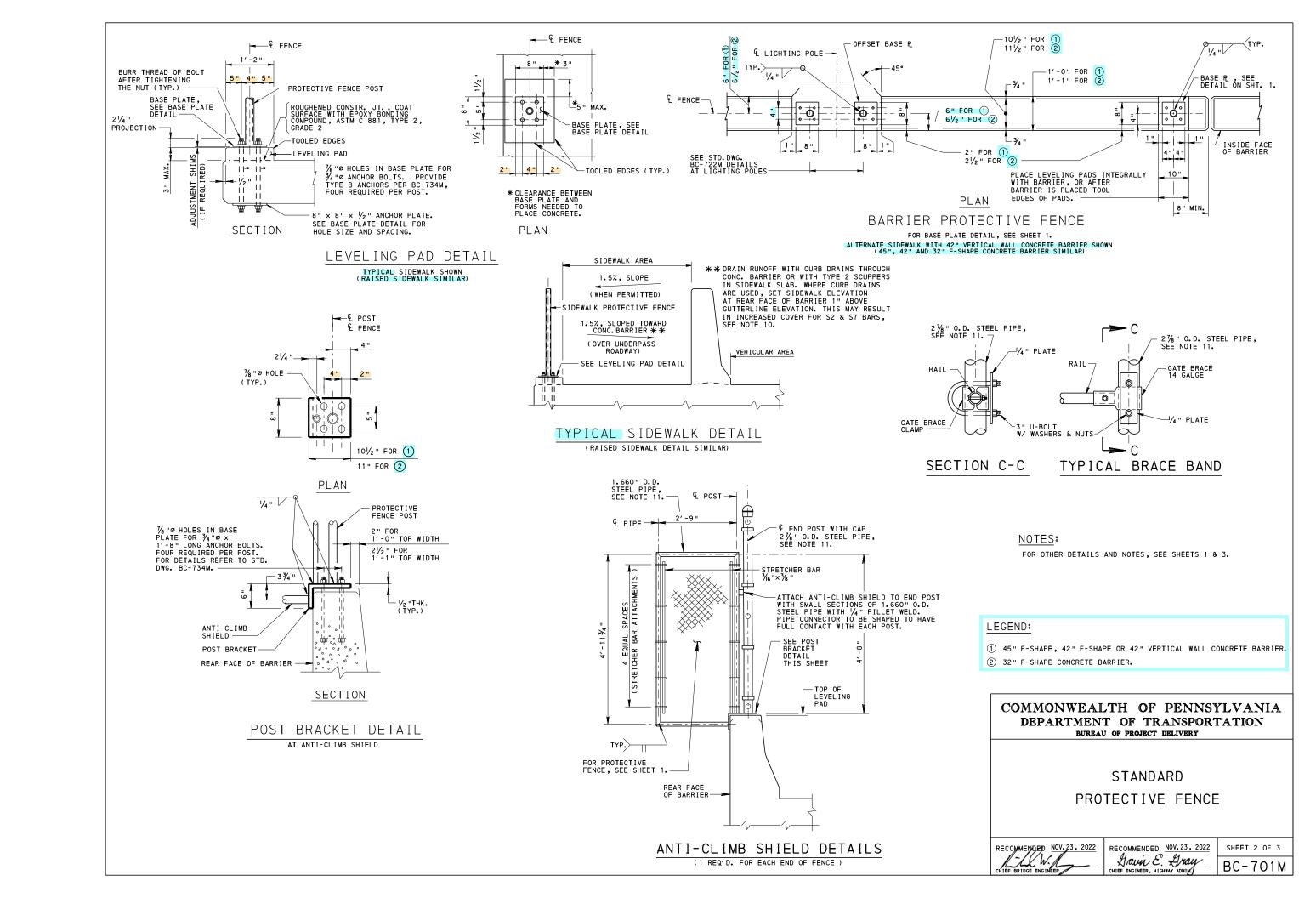
SEE CHANGE #5 FOR FEB. 14, 2023 STANDARD REVISIONS.

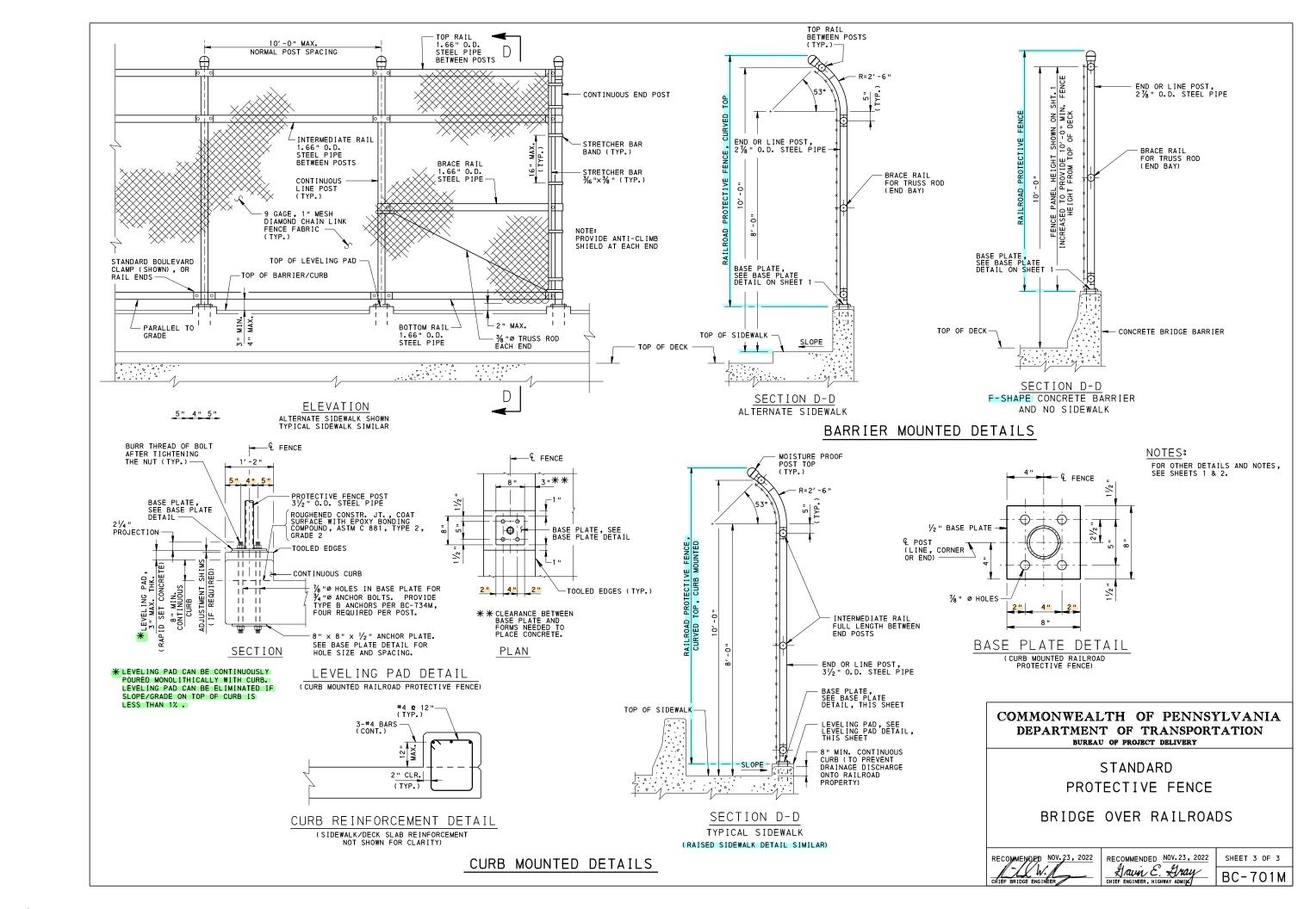
SEE CHANGE #6 FOR MAR. 27, 2024 STANDARD REVISIONS.

SEE CHANGE #7 FOR OCT. 7, 2024 STANDARD REVISIONS.

BC-700M







GENERAL NOTES:

- 1. PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH PUBLICATION
- 2. PROVIDE RAIL TUBES CONFORMING TO ASTM A500 OR A501, GRADE AS SPECIFIED BASED ON PROVIDED WALL THICKNESS.
- PROVIDE POSTS CONFORMING TO AASHTO M270 (ASTM A709), GRADE 50 OR ASTM A992. PROVIDE BASE PLATES CONFORMING TO AASHTO M270 (ASTM A709), GRADE 50.
- 4. ALL RAILING COMPONENTS SHALL BE GALVANIZED (AFTER FABRICATION)

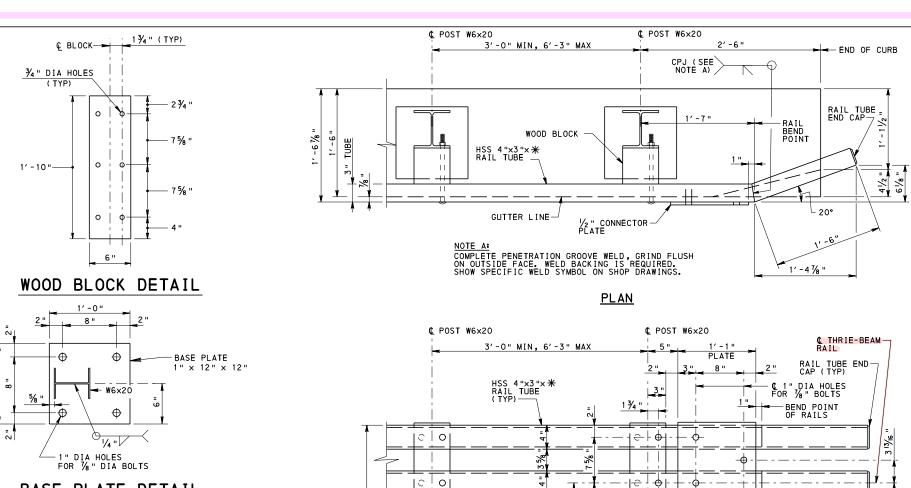
 AS SPECIFIED IN PUBLICATION 408, SECTION 1105.02(s), UNLESS OTHERWISE SHOWN ON THE PLANS. GALVANIZE RAIL TUBES, POSTS, AND BASE PLATES ACCORDING TO ASTM A123. GALVANIZE ALL HARDWARE ACCORDING TO ASTM
- 5. THE RAIL TUBES ARE SHOP BENT OR FABRICATED TO FIT HORIZONTAL CURVE WHEN RADIUS IS LESS THAN 1,500 FEET.

CHANGE 5

CHANGE 7

- 6. STEEL TUBE TOLERANCES:
 A. STRAIGHTNESS: THE PERMISSIBLE VARIATION FOR STRAIGHTNESS SHALL
 BE 1/8" TIMES THE NUMBER OF FEET OF THE TOTAL LENGTH DIVIDED BY
- B. TWIST: SPECIFIED DIMENSION OF THE LONGEST SIDE IN INCHES FROM OVER 4" TO 6" INCLUSIVE: 0.087" MAX TWIST IN THE FIRST 3 FEET AND IN EACH ADDITIONAL 3 FEET.
- NOTE: TWIST IS MEASURED BY HOLDING DOWN ONE END OF SQUARE OR RECTANGULAR TUBE ON A FLAT SURFACE PLATE WITH THE BOTTOM SIDE OF THE TUBE PARALLEL TO THE SURFACE PLATE AND NOTING THE HEIGHT DIFFERENCE BETWEEN THE TWO CORNERS AT THE OPPOSITE END OF THE BOTTOM SIDE OF THE TUBE.
- 7. ALL WELDING SHALL CONFORM TO THE PROVISIONS OF AASHTO/AWS BRIDGE WELDING CODE D1.5, EXCEPT USE AASHTO/AWS STRUCTURAL WELDING CODE D1.1 FOR WELDING NOT COVERED IN D1.5.
- 8. FOR ANCHOR BOLTS, USE 7_8 " DIAMETER BOLTS CONFORMING TO ASTM A449 OR ASTM F1554, GRADE 105 KSI. USE ASTM A563, GRADE DH HEAVY HEX NUTS. USE ONE ASTM F436M WASHER AT THE TOP.
- FOR RAIL TUBE TO POST CONNECTION, USE 5/8" DIAMETER ROUND HEAD BOLTS CONFORMING TO ASTM A307. USE ASTM A563, GRADE A HEX NUTS.
- 10. BOLT TIGHTENING PROCEDURES ARE AS FOLLOWS: A. SNUG TIGHTEN ALL ANCHOR BOLTS. TIGHTEN THE NUTS AN ADDITIONAL 1/3 TURN USING A WRENCH.
 - B. SNUG TIGHTEN THE RAIL TO POST BOLT.
- 11. PROVIDE EPOXY COATED MECHANICAL SPLICES IN ACCORDANCE WITH PUBLICATION 408, SECTION 1002.2(c).
- 12. PROVIDE WOOD BLOCKS IN ACCORDANCE WITH PUBLICATION 408, SECTION
- 13. ONE OR MORE 6'-3" MAXIMUM POST SPACINGS MAY BE REDUCED TO 3'-0" MINIMUM IN ORDER TO MAINTAIN APPROPRIATE SPACING DIMENSIONS FROM THE END OF RAIL AND EXPANSION JOINTS.
- 14. LOCATE RAIL SPLICES AT EXPANSION JOINTS AND AT OTHER LOCATIONS WHERE NECESSARY. PROVIDE RAILS AS LONG AS PRACTICAL, WITH A MINIMUM OF THREE POSTS BETWEEN SPLICES, UNLESS OTHERWISE REQUIRED
- 15. PROVIDE RAIL TUBES CONTINUOUS OVER NOT LESS THAN TWO RAILING POSTS. NO WELDED BUTT SPLICES WILL BE ALLOWED IN THE RAIL TUBE SECTIONS.
- 16. PLACE POST AND POST ANCHOR BOLTS NORMAL TO GRADE AND RAILS PARALLEL TO GRADE.
- 17. COAT ALL SURFACES OF THE BASE PLATE IN CONTACT WITH CONCRETE WITH CAULKING COMPOUND PRIOR TO ERECTION. AFTER ERECTION AND ALIGNMENT, SEAL OPENINGS BETWEEN METAL SURFACES AND THE CONCRETE WITH CAULKING COMPOUND MEETING THE REQUIREMENTS OF PUBLICATION 408 . SECTION 705, 7(b) .
- 18. DO NOT USE DEFLECTION JOINTS WITH PA 3-RAIL BRIDGE BARRIERS.
- 19. PROVIDE POST SPACINGS ON THE PLANS.
- 20. THE PA 3-RAIL BRIDGE BARRIER IS DESIGNATED AS MASH TL-3.
- 21. FOR GUIDE RAIL TRANSITION TO PA 3-RAIL BRIDGE BARRIER, SEE RC-50M.

TUBE MEMBERS				
RAIL TUBE		SPLICE TUBE		
MATERIAL	THICKNESS	MATERIAL	THICKNESS	
A500 GR. C	0.188"	A500 GR. C	0.188"	
A500 GR. B	0. 25 "	A500 GR. B	0. 25 "	
A500 GR.A OR A501	0.313"	A500 GR.A OR A501	0.25"	



BASE PLATE DETAIL

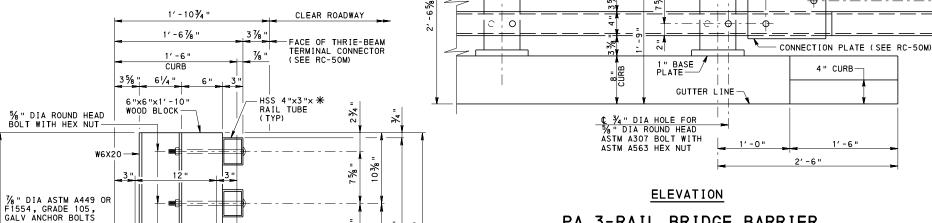
W/ LEVELING NUTS, PLAIN WASHERS AND

3/4 "X 3/4 " CHAMFER (TYP) —

LEVELING NUT (TYP)

RC CURB

CONSTRUCTION JOINT A V-NOTCH (RAKED FINISH)



-6% 73%

3%

WEARING COURSE

RC DECK OR BOX -

CULVERT TOP SLAB

(IF REQUIRED)

PA 3-RAIL BRIDGE BARRIER WITH THRIE-BEAM CONNECTION

(GUIDE RAIL AND ANCHOR BOLTS OMITTED FOR CLARITY)
(WITH CURB SHOWN, WITHOUT CURB SIMILAR)

NOTE: PRIOR TO CONSTRUCTING CURB AND DECK, ANCHOR BOLTS SHALL BE INSTALLED WITH EITHER A TEMPLATE OR ACTUAL POST W/ BASEPLATE INSTALLED TO ENSURE PROPER ANCHOR BOLT ALIGNMENT AND PLACEMENT. * FOR TUBE THICKNESS, SEE TUBE RAIL SPECIFICATIONS TABLE.

DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

COMMONWEALTH OF PENNSYLVANIA

PA 3-RAIL BRIDGE BARRIER MISCELLANEOUS DETAILS - 1

BARRIER SECTION DECK / SLAB REINFORCEMENT NOT SHOWN FOR CLARITY

-1" BASE PL

← 2" CLR (TYP)

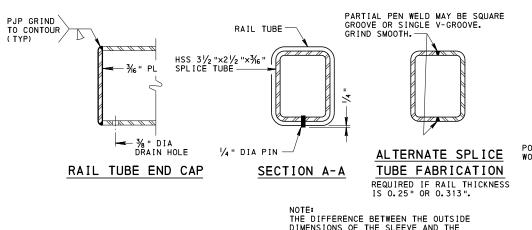
RC-50M GUIDE RAIL TO BRIDGE BARRIER TRANSITIONS RC-51M TYPE 31 STRONG POST GUIDE RAIL BC-734M ANCHOR SYSTEMS REFERENCE DRAWINGS

RECOMMENDED OCT. 7, 2024 Kutu S. Hanger

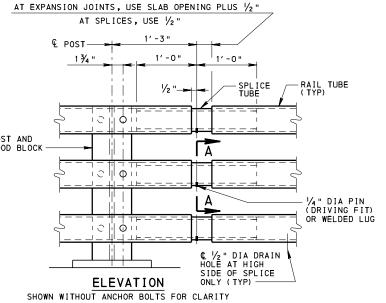
RECOMMENDED OCT. 7, 2024 Havin E. Lyray
HEF ENGINEER, HIGHWAY ADMIN.

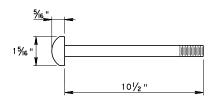
SHEET 1 OF 2 BC-706M

TUBE RAIL SPECIFICATIONS



NOTE:
THE DIFFERENCE BETWEEN THE OUTSIDE
DIMENSIONS OF THE SLEEVE AND THE
INSIDE DIMENSIONS OF THE RAIL SHALL
NOT EXCEED 1/8 " ALONG EITHER AXIS.





RAIL TO POST BOLT

% " ROUND HEAD BOLT

RECOMMENDED OCT. 7, 2024

CHIEF BRIDGE ENGINEER

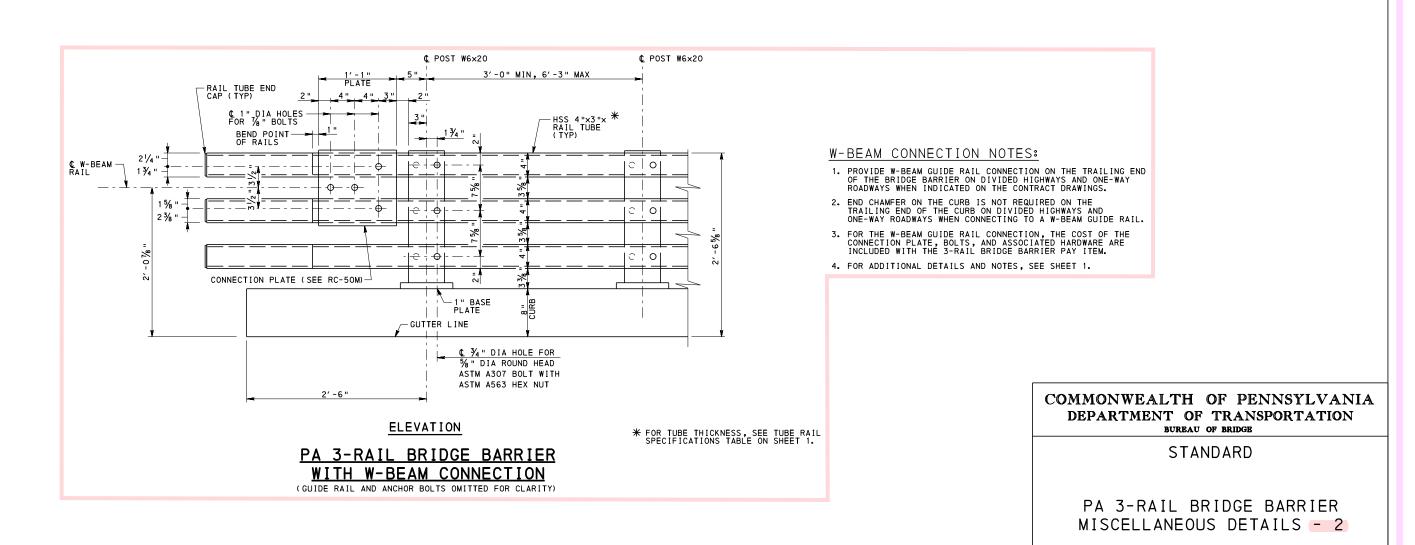
RECOMMENDED OCT. 7, 2024

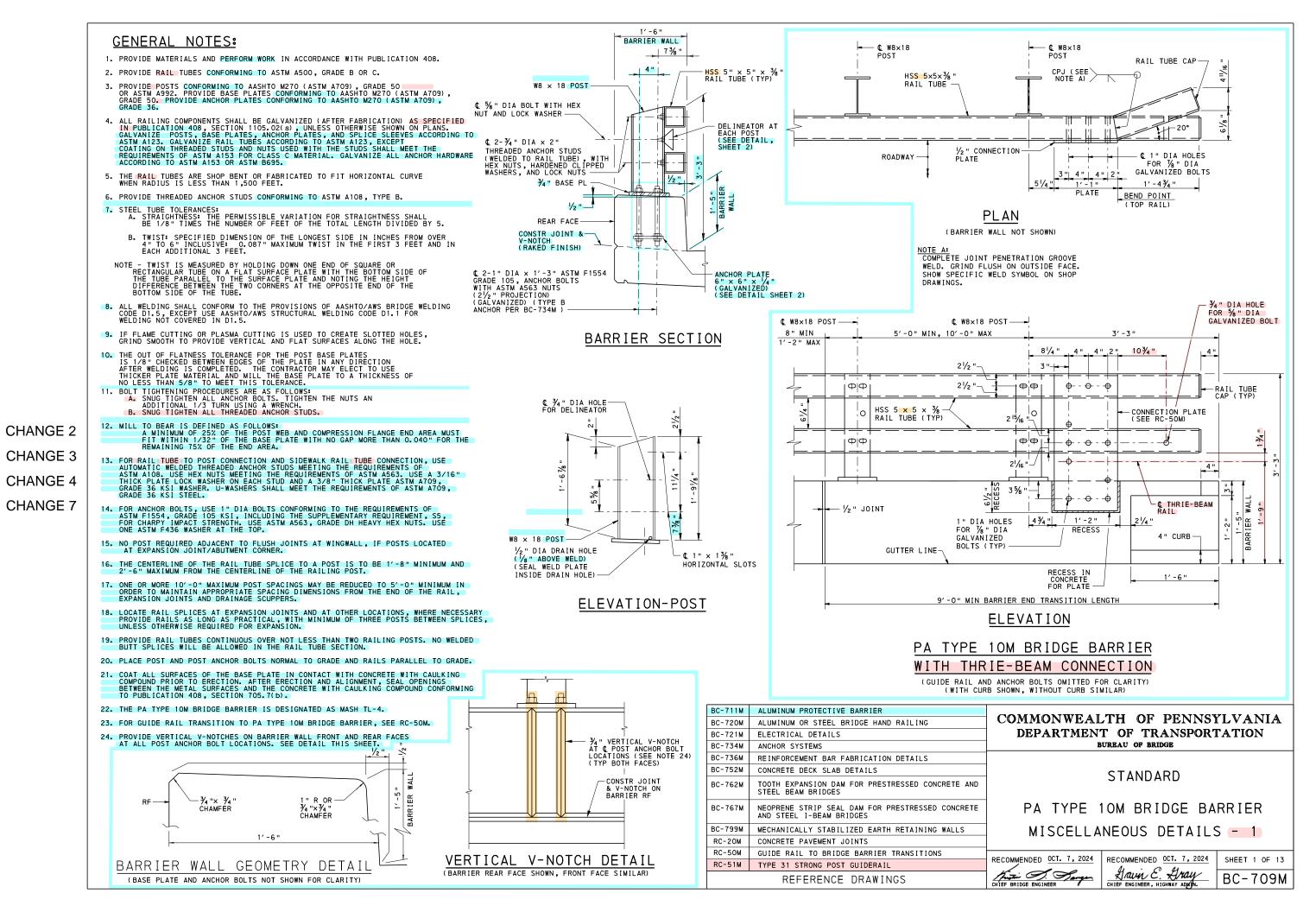
Havin E. Hray
CHIEF ENGINEER, HIGHWAY ADMIN.

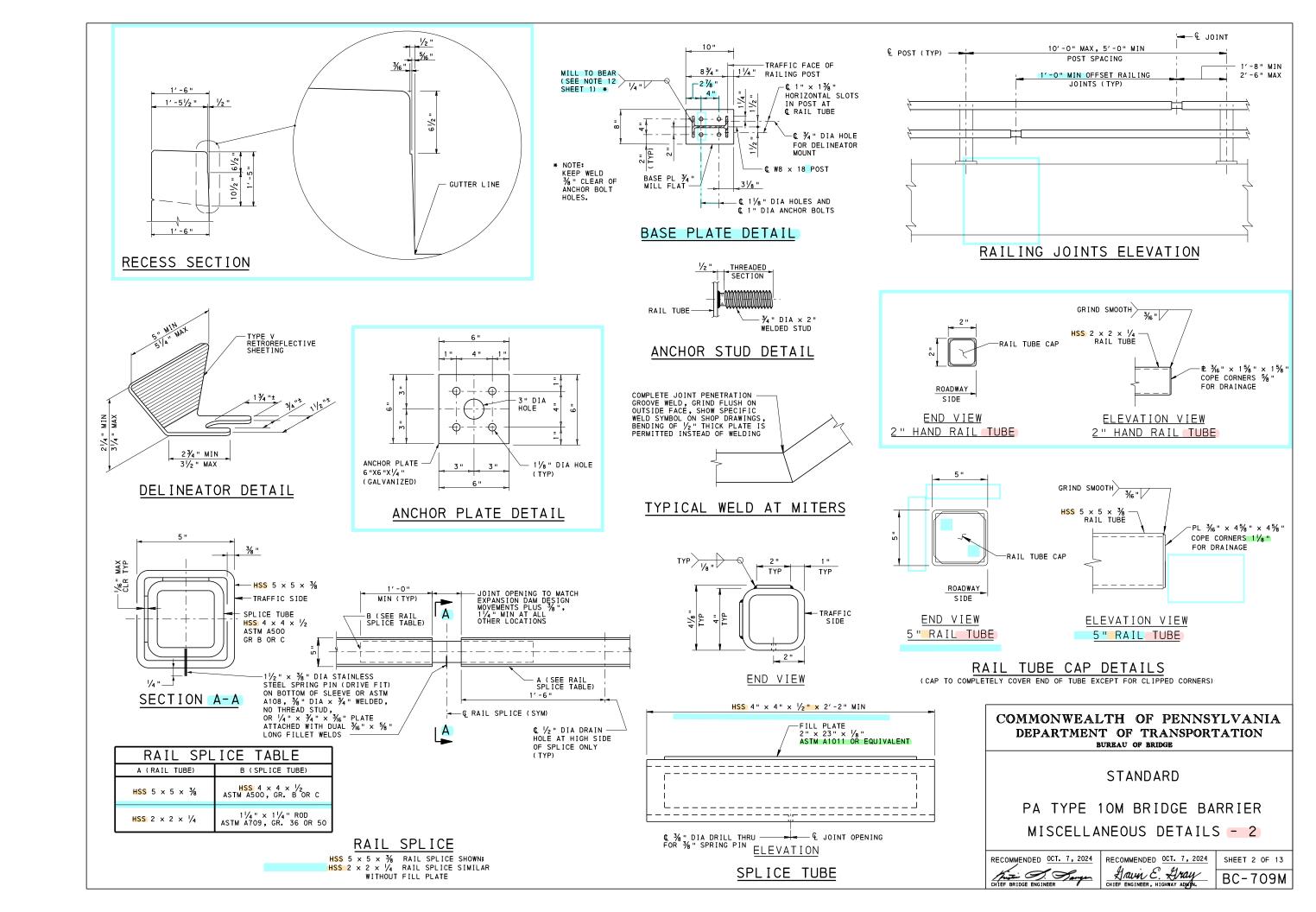
SHEET 2 OF 2

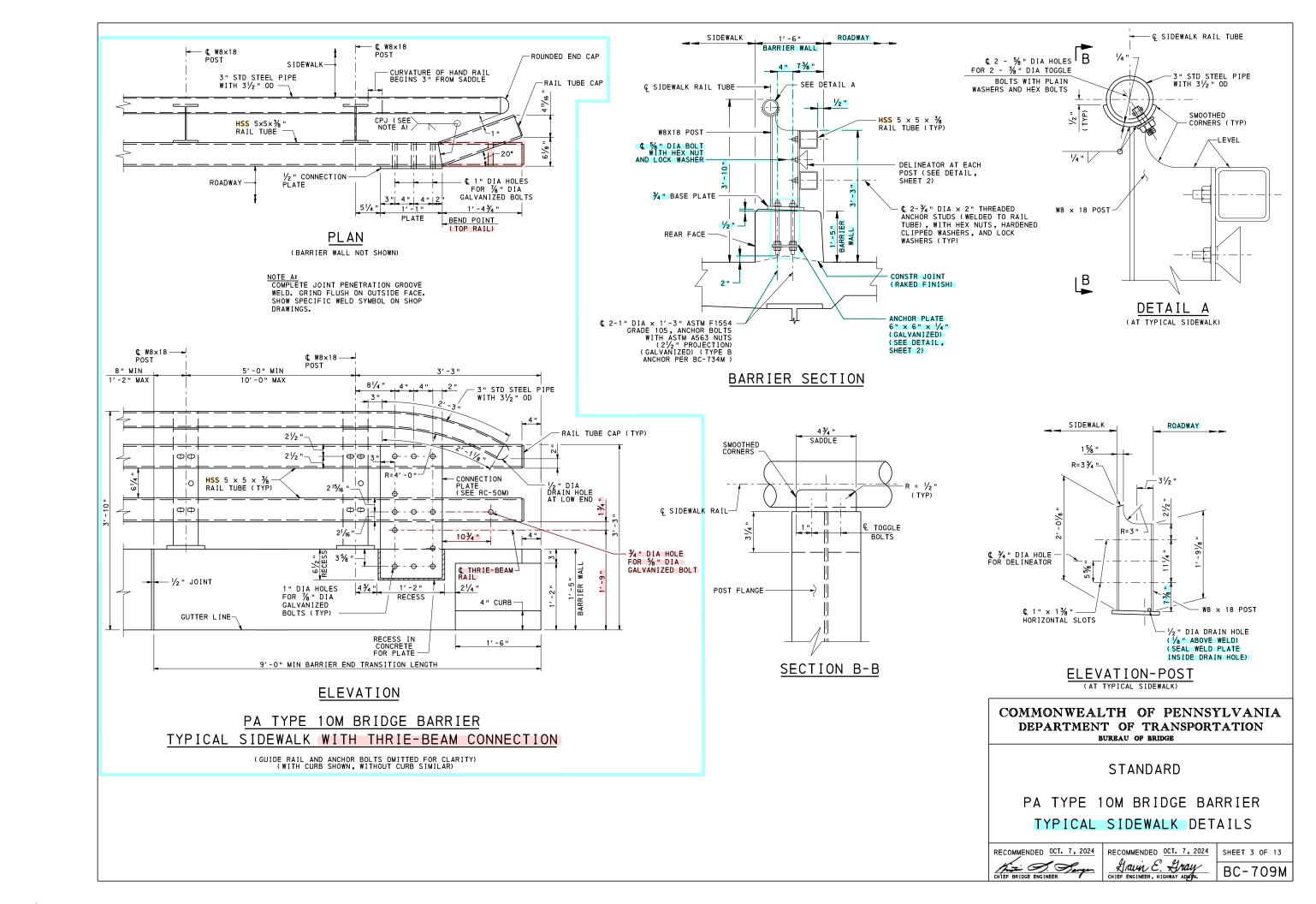
BC-706M

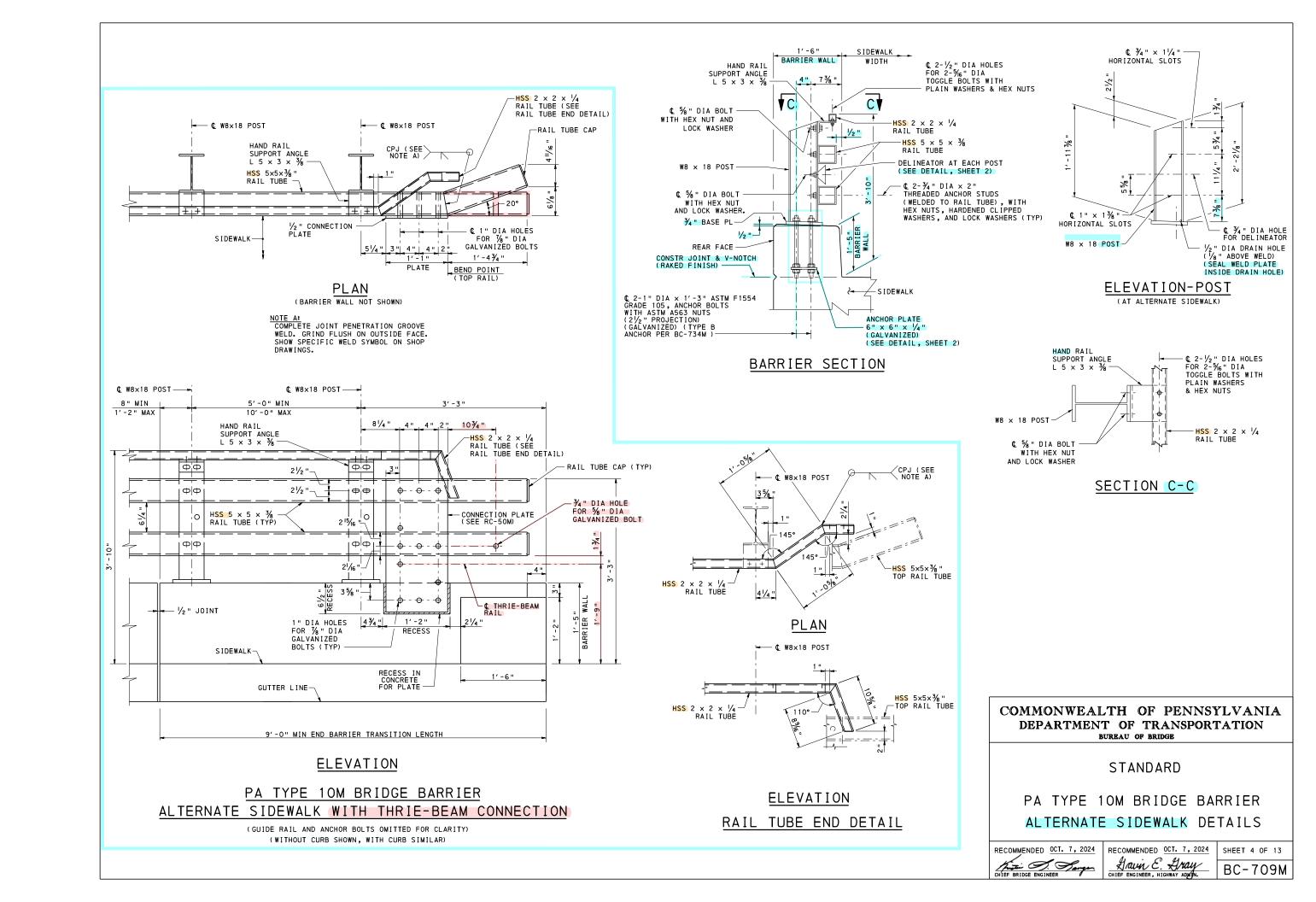
TUBE SPLICE DETAILS

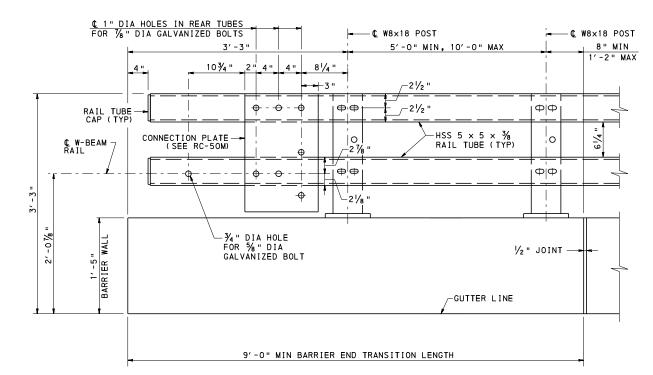








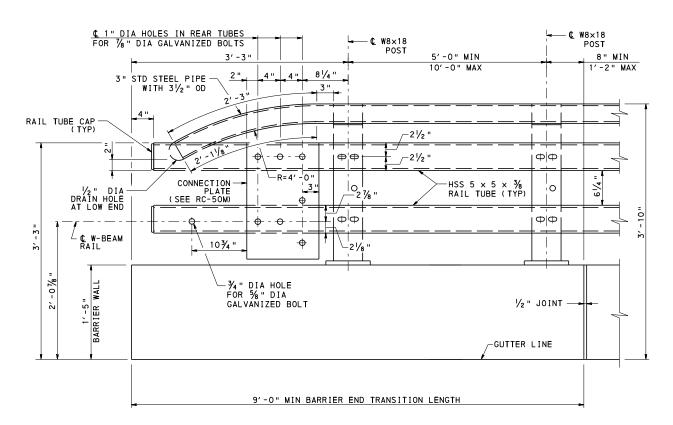




ELEVATION

PA TYPE 10M BRIDGE BARRIER WITH W-BEAM CONNECTION

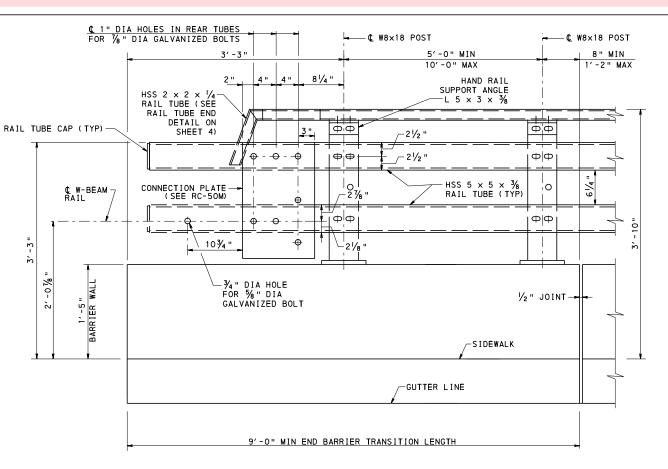
(GUIDE RAIL AND ANCHOR BOLTS OMITTED FOR CLARITY)



ELEVATION

PA TYPE 10M BRIDGE BARRIER TYPICAL SIDEWALK WITH W-BEAM CONNECTION

(GUIDE RAIL AND ANCHOR BOLTS OMITTED FOR CLARITY)



ELEVATION

PA TYPE 10M BRIDGE BARRIER ALTERNATE SIDEWALK WITH W-BEAM CONNECTION

(GUIDE RAIL AND ANCHOR BOLTS OMITTED FOR CLARITY)

W-BEAM CONNECTION NOTES:

- PROVIDE W-BEAM GUIDE RAIL CONNECTION ON THE TRAILING END OF THE BRIDGE BARRIER ON DIVIDED HIGHWAYS AND ONE-WAY ROADWAYS WHEN INDICATED ON THE CONTRACT DRAWINGS.
- 2. END CHAMFERS ON THE BARRIER WALL ARE NOT REQUIRED ON THE TRAILING END OF THE BARRIER WALL ON DIVIDED HIGHWAYS AND ONE-WAY ROADWAYS WHEN CONNECTING TO A W-BEAM GUIDE RAIL.
- 3. FOR THE W-BEAM GUIDE RAIL CONNECTION, THE COST OF THE CONNECTION PLATE, BOLTS, AND ASSOCIATED HARDWARE ARE INCLUDED WITH THE PA TYPE 10M BRIDGE BARRIER PAY ITEM.
- 4. FOR ADDITIONAL DETAILS AND NOTES, SEE SHEETS 1-4.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

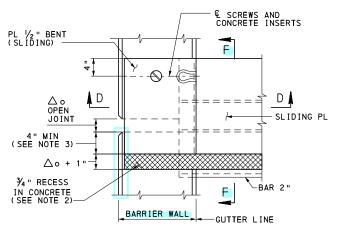
PA TYPE 10M BRIDGE BARRIER W-BEAM CONNECTION ELEVATIONS

RECOMMENDED OCT. 7, 2024 CHIEF BRIDGE ENGINEER

RECOMMENDED OCT. 7, 2024 Havin E. Hray

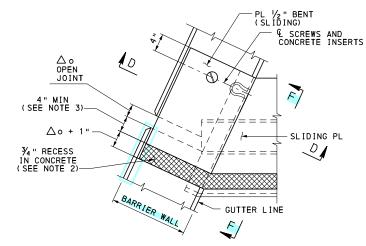
HIEF ENGINEER, HIGHWAY ADMIN.

SHEET 5 OF 13 BC-709M



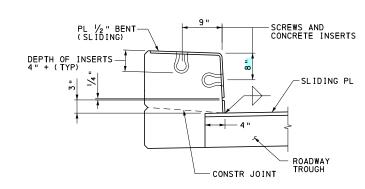
PLAN - SKEW ANGLE ≥ 75°

(AT GUTTER LINE SHOWN;
AT ALTERNATE SIDEWALK SIMILAR)



PLAN - SKEW ANGLE < 75°

(AT GUTTER LINE SHOWN;
AT ALTERNATE SIDEWALK SIMILAR)

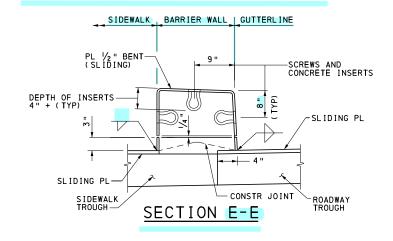


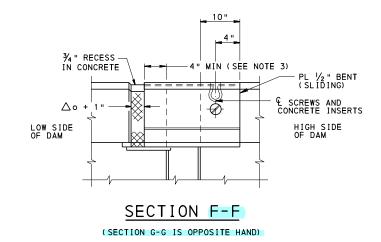
SECTION D-D

(NO SIDEWALK CONDITION SHOWN;
ALTERNATE SIDEWALK SIMILAR)

SIDEWALK SLIDING PL G SCREWS AND CONCRETE INSERTS PL //2 " BENT (SLIDING) SIDEWALK BARRIER WALL GUTTER LINE

PLAN AT TYPICAL SIDEWALK- SKEW ANGLE ≥ 75°



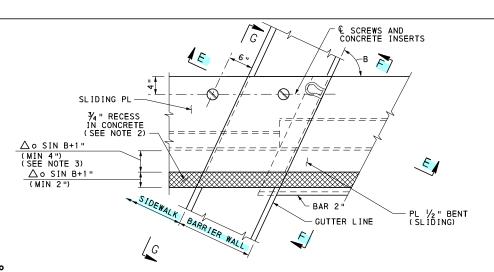


PA TYPE 10M BRIDGE BARRIER AT TOOTH EXPANSION DAM

(RAILING POST AND TUBE RAILS NOT SHOWN)

NOTES:

- 1. FOR △ o SEE BC-762M
- 2. FORM CONCRETE RECESS AREA IN BARRIER WALL AND GRIND TO PROVIDE SMOOTH SURFACE. APPLY ONE COAT OF ASPHALT CEMENT PAINT WA-1 OR PERFORMANCE GRADED ASPHALT CEMENT PG 64-22 TO ALLOW BENT SLIDING PLATE TO MOVE FREELY WITHOUT FRICTION.
- 3. MAINTAIN 4" MINIMUM BETWEEN EDGE OF STEEL TO THE EDGE OF CONCRETE AT TEMPERATURE OF -10°F FOR STEEL BRIDGES AND 10°F FOR PRESTRESSED (P/S) CONCRETE BRIDGES.
- 4. MAXIMUM DISTANCE FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3".



PLAN AT TYPICAL SIDEWALK- SKEW ANGLE < 75°



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PA TYPE 10M BRIDGE BARRIER

DETAILS AT TOOTH EXPANSION DAM

RECOMMENDED OCT. 7, 2024

CHIEF BRIDGE ENGINEER

RECOMMENDED OCT. 7, 2024

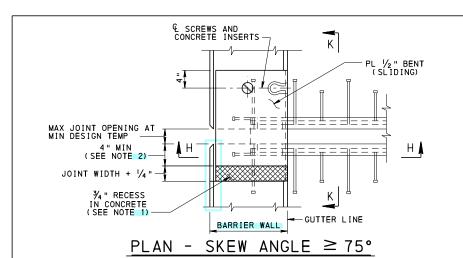
Havin E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN.

E. Gray

BC-709M

SHEET 6 OF 13

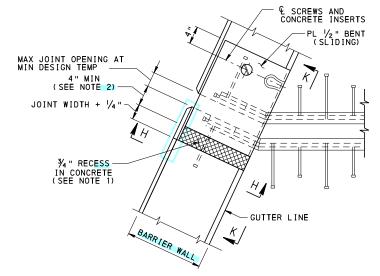


© SCREWS AND CONCRETE INSERTS SIDEWALK SLIDING PL PL ½" BENT (SLIDING) MAX JOINT OPENING AT MIN DESIGN TEMP 4" MIN (SEE NOTE 2) J JOINT WIDTH + 1/4 ¾ " RECESS IN CONCRETE -GUTTER LINE SIDEWALK BARRIER WALL

¢ SCREWS AND CONCRETE INSERTS SIDEWALK SLIDING PL PL ½" BENT (SLIDING) **⊗** 3/4" RECESS IN CONCRETE (SEE NOTE 1) MAX JOINT OPENING AT MIN DESIGN TEMP 4" MIN (SEE NOTE 2) JOINT WIDTH + 1/4" -GUTTER LINE

PLAN AT TYPICAL SIDEWALK- SKEW ANGLE ≥ 75°

PLAN AT TYPICAL SIDEWALK- SKEW ANGLE < 75°



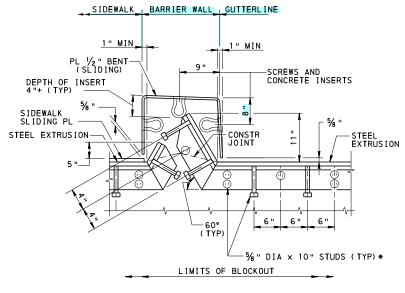
PL ½" BENT (SLIDING)

DEPTH OF INSERT 4"+ (TYP) —

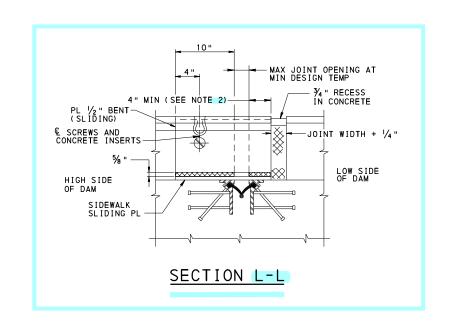
PLAN - SKEW ANGLE < 75°

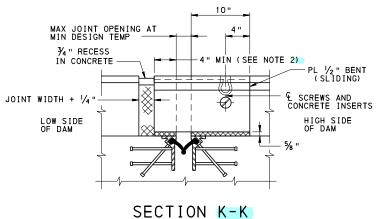
<u>1" MIN ≥ 75°</u> SEE BC-767M<75°

-SCREWS AND CONCRETE INSERTS



SECTION J-J





NOTES:

- FORM CONCRETE RECESS AREA IN BARRIER WALL AND GRIND TO PROVIDE SMOOTH SURFACE. APPLY ONE COAT OF ASPHALT CEMENT PAINT WA-1 OR PERFORMANCE GRADED ASPHALT CEMENT PG 64-22 TO ALLOW BENT SLIDING PLATE TO MOVE FREELY WITHOUT FRICTION.
- MAINTAIN 4" MINIMUM BETWEEN EDGE OF STEEL TO THE EDGE OF CONCRETE AT TEMPERATURE OF -10°F FOR STEEL BRIDGES AND 10°F FOR PRESTRESSED (P/S) CONCRETE BRIDGES.
- MAXIMUM DISTANCE ALONG THE EXTRUSION FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3 ".
- 4. ALTERNATE STRIP SEAL DAM (NOT SHOWN) SIMILAR TO THAT ON BC-767M, SHEET 7, IS PERMITTED IF SHOWN ON THE CONTRACT PLANS.

DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

COMMONWEALTH OF PENNSYLVANIA

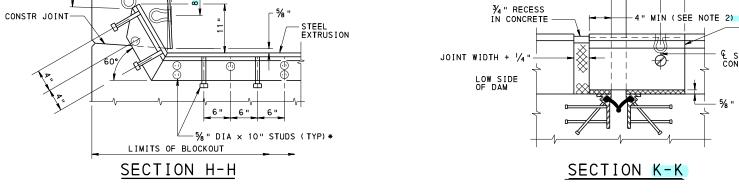
STANDARD

PA TYPE 10M BRIDGE BARRIER

DETAILS AT NEOPRENE STRIP SEAL DAM - 1

RECOMMENDED OCT. 7, 2024

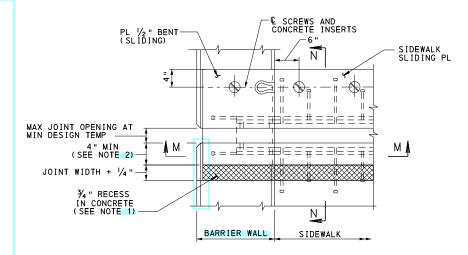
RECOMMENDED OCT. 7, 2024 SHEET 7 OF 13 Havin E. Hray
HEF ENGINEER, HIGHWAY ADMYN. BC-709M



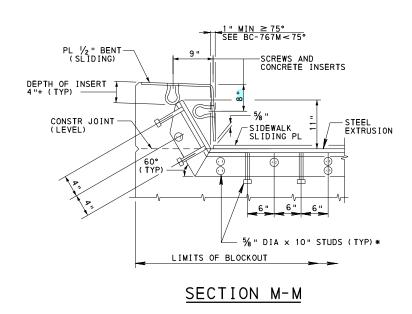
PA TYPE 10M BRIDGE BARRIER AT NEOPRENE STRIP SEAL DAM

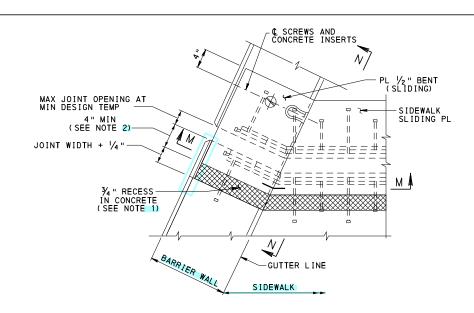
(RAILING POST AND TUBE RAILS NOT SHOWN)

* IF 10" STUDS CANNOT BE ACCOMMODATED IN THE SPACE AVAILABLE, REQUEST SPECIFIC LENGTH APPROVAL FROM THE DISTRICT BRIDGE ENGINEER AT THE SHOP DRAWINGS APPROVAL STAGE.

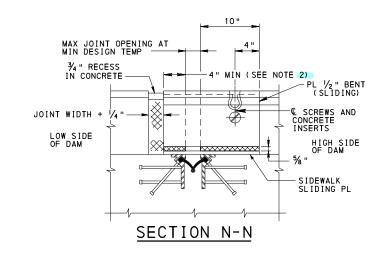


PLAN AT ALTERNATE SIDEWALK- SKEW ANGLE ≥ 75°





PLAN AT ALTERNATE SIDEWALK- SKEW ANGLE < 75°



NOTES:

- 1. FORM CONCRETE RECESS AREA IN BARRIER WALL AND GRIND TO PROVIDE SMOOTH SURFACE. APPLY ONE COAT OF ASPHALT CEMENT PAINT WA-1 OR PERFORMANCE GRADED ASPHALT CEMENT PG 64-22 TO ALLOW BENT SLIDING PLATE TO MOVE FREELY WITHOUT FRICTION.
- 2. MAINTAIN 4" MINIMUM BETWEEN EDGE OF STEEL TO THE EDGE OF CONCRETE AT TEMPERATURE OF -10°F FOR STEEL BRIDGES AND 10°F FOR PRESTRESSED (P/S) CONCRETE BRIDGES.
- 3. MAXIMUM DISTANCE ALONG THE EXTRUSION FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3".
- 4. ALTERNATE STRIP SEAL DAM (NOT SHOWN) SIMILAR TO THAT ON BC-767M, SHEET 7, IS PERMITTED IF SHOWN ON THE CONTRACT PLANS.

PA TYPE 10M BRIDGE BARRIER AT NEOPRENE STRIP SEAL DAM

(RAILING POST AND TUBE RAILS NOT SHOWN)

STANDARD

PA TYPE 10M BRIDGE BARRIER

COMMONWEALTH OF PENNSYLVANIA

DEPARTMENT OF TRANSPORTATION
BUREAU OF BRIDGE

DETAILS AT NEOPRENE STRIP SEAL DAM - 2

RECOMMENDED OCT. 7, 2024

CHIEF BRIDGE ENGINEER

RECOMMENDED OCT. 7, 2024

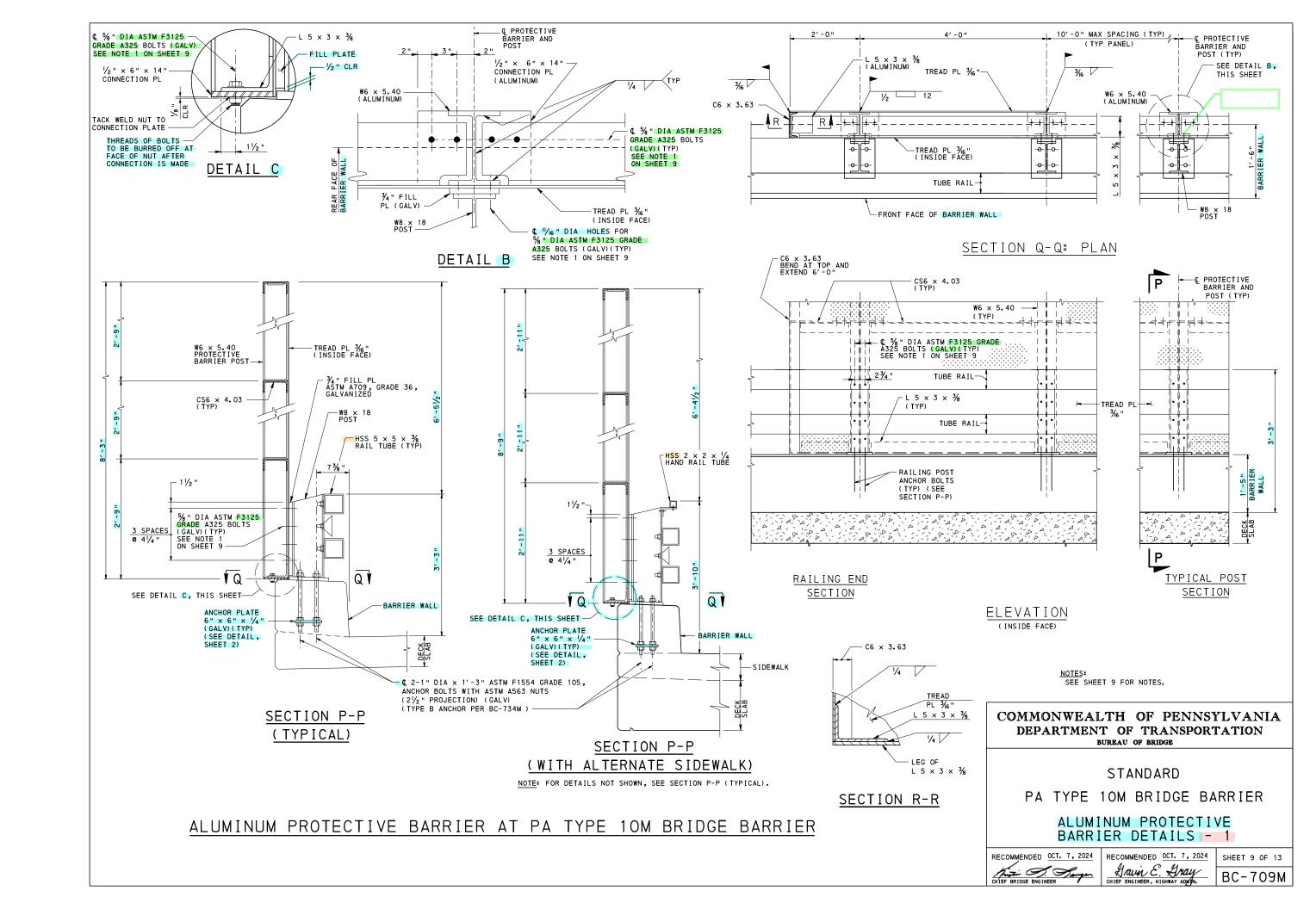
LAWN E. HAY

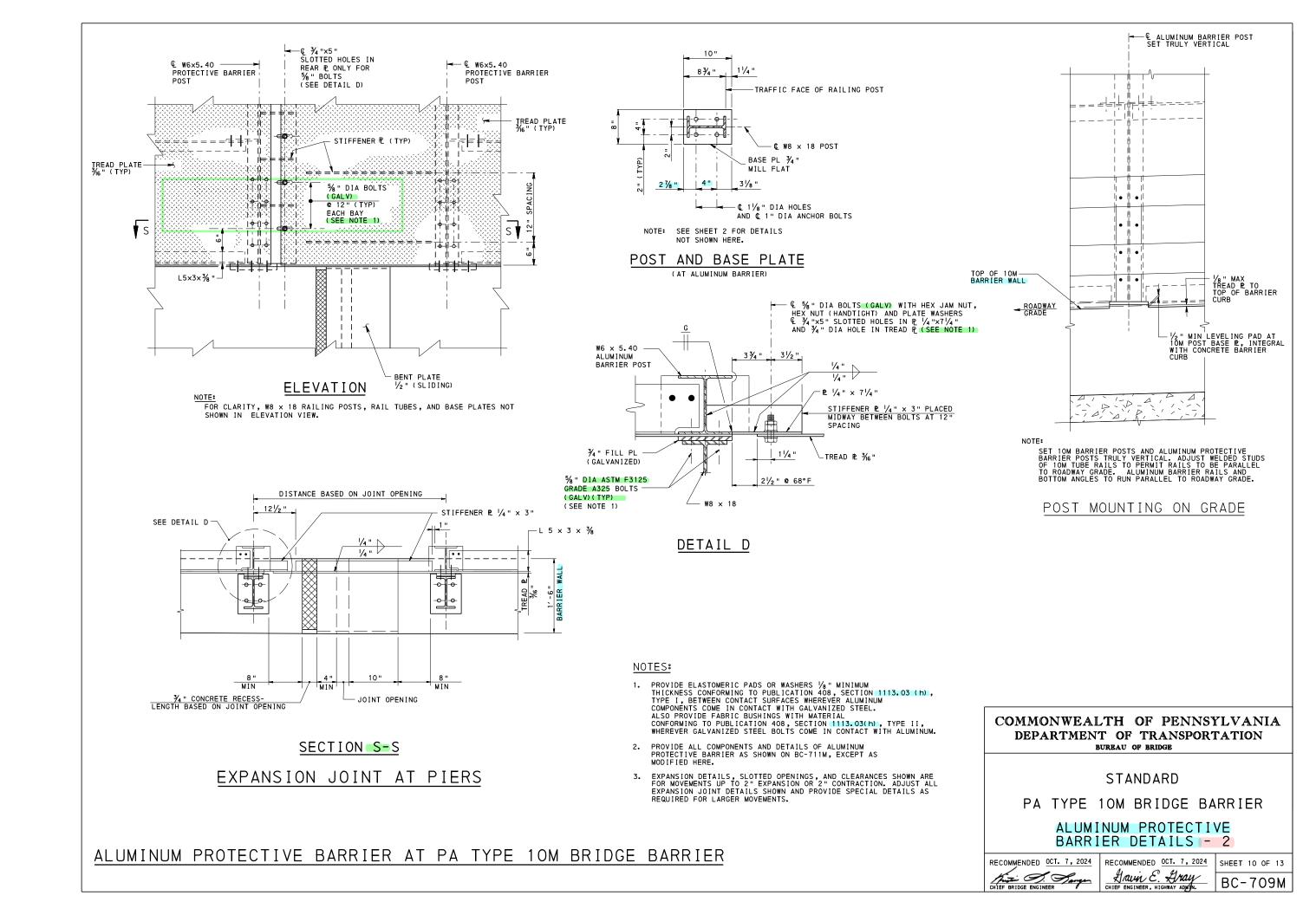
CHIEF ENGINEER, HIGHWAY ADMIN.

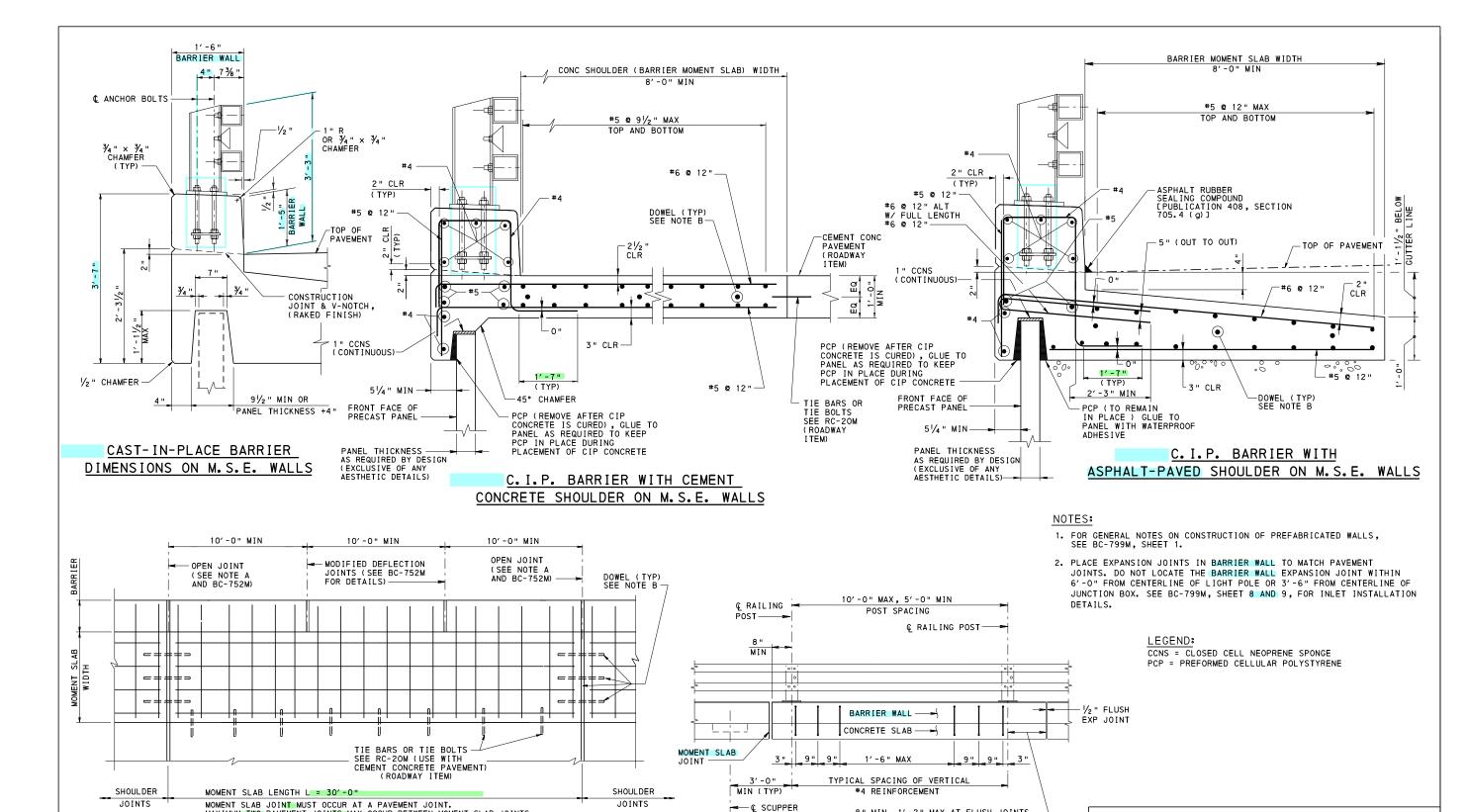
SHEET 8 OF 13

BC-709M

* IF 10" STUDS CANNOT BE ACCOMMODATED IN THE SPACE AVAILABLE, REQUEST SPECIFIC LENGTH APPROVAL FROM THE DISTRICT BRIDGE ENGINEER AT THE SHOP DRAWINGS APPROVAL STAGE.







OR METAL

CURB DRAIN

MAXIMUM TWO PAVEMENT JOINTS MAY OCCUR BETWEEN MOMENT SLAB JOINTS. NOTE A: PROVIDE OPEN JOINTS IN BARRIER AT SAME LOCATIONS AS THOSE PROVIDED FOR THE MOMENT SLAB. NOTE B: USE TYPE D OR E JOINT PER RC-27M. USE SAME JOINT AS PROVIDED IN PAVEMENT.

PLAN - BARRIER MOMENT SLAB (C.I.P. BARRIER)

BARRIER WALL ELEVATION

8" MIN, 1'-2" MAX AT FLUSH JOINTS.

(ON ONE SIDE OF JOINT) (TYP)

(RAILING POST AND VERTICAL REINFORCEMENT SPACING)

PA TYPE 10M CAST-IN-PLACE BARRIER ON M.S.E. WALLS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

PA TYPE 10M BRIDGE BARRIER

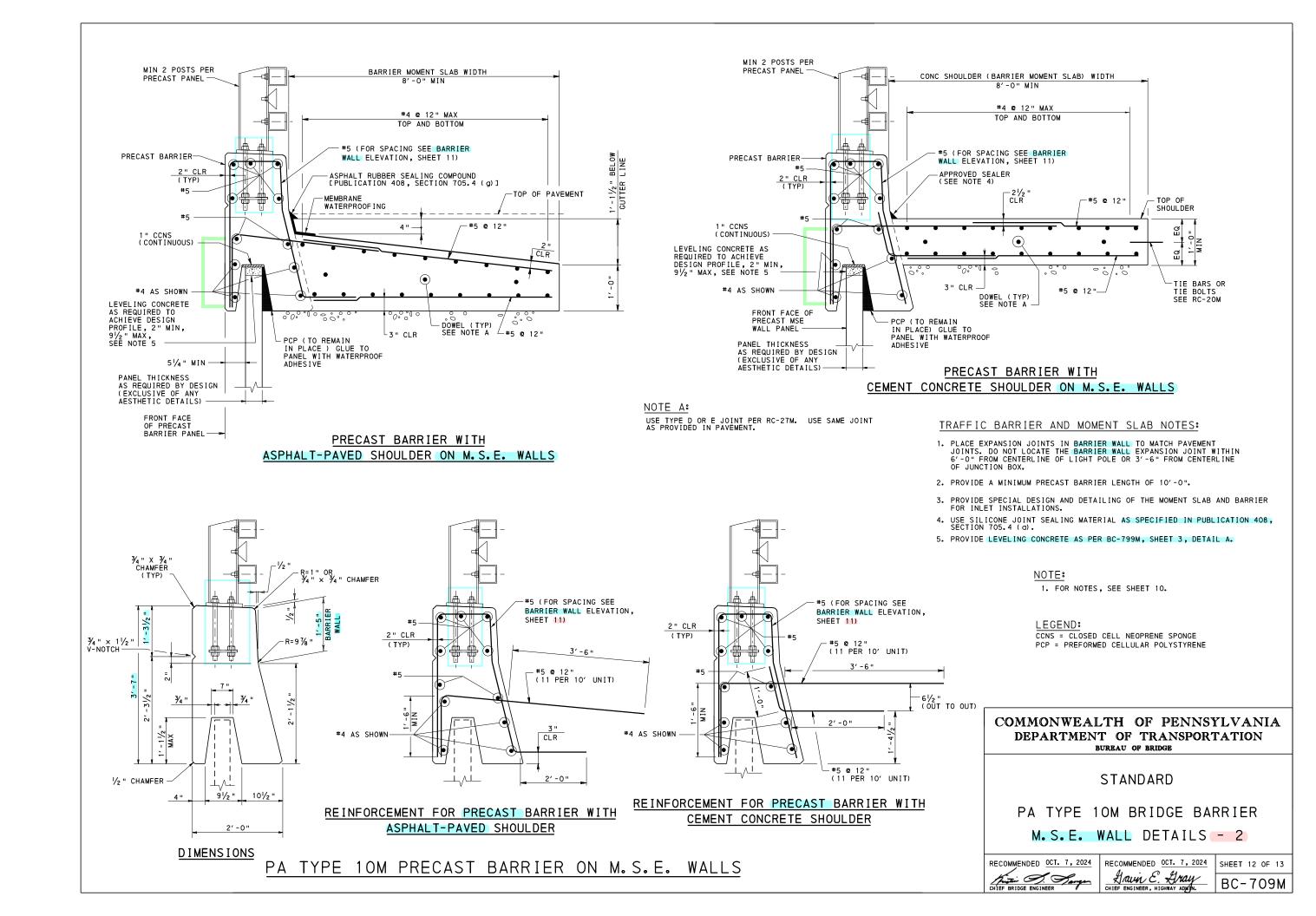
M. S. E. WALL DETAILS - 1

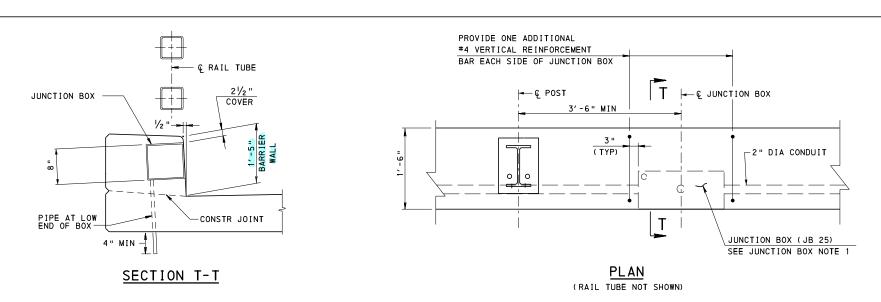
RECOMMENDED OCT. 7, 2024 CHIEF BRIDGE ENCINEED

RECOMMENDED 0CT. 7, 2024 SHEET 11 OF 13 Havin E. Hray

CHIEF ENGINEER, HIGHWAY ADMYN.

BC-709M

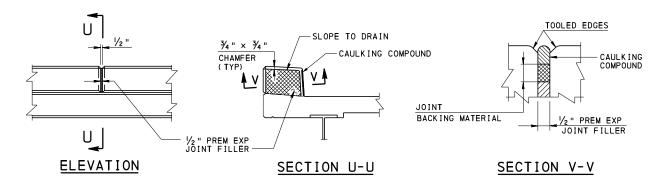




JUNCTION BOX NOTES:

- JUNCTION BOX MAY BE LOCATED EITHER TO THE LEFT OR TO THE RIGHT OF THE LIGHTING POLE.
- JUNCTION BOXES ARE ONLY REQUIRED, IF SPECIFIED ON THE CONTRACT DRAWINGS.
- 3. FOR TYPICAL SIDEWALK, PLACE JUNCTION BOX ON SIDEWALK SIDE.

PA TYPE 10M BRIDGE BARRIER ALTERNATE JUNCTION BOX DETAIL

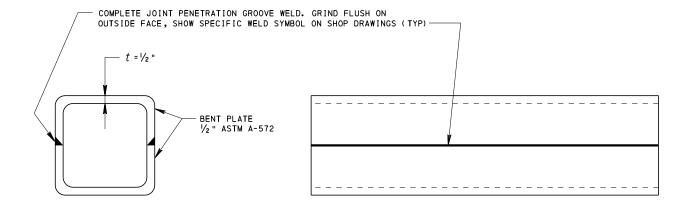


OPEN JOINT DETAIL (RAILING POST AND TUBE NOT SHOWN)

PA TYPE 10M BRIDGE BARRIER AT OPEN JOINT

OPEN JOINT NOTES:

- 1. FOR LOCATION OF CONSTRUCTION JOINTS AND OPEN JOINTS, REFER TO DESIGN DRAWINGS.
- PROVIDE CAULKING COMPOUND CONFORMING TO PUBLICATION 408, SECTION 705.7 (b).
- PROVIDE JOINT BACKING MATERIAL CONFORMING TO PUBLICATION 408, SECTION 705.8.
- 4. PROVIDE PREMOLDED EXPANSION JOINT FILLER CONFORMING TO PUBLICATION 408, SECTION 705.1.
- 5. PROVIDE 2" CLEAR ON ALL REINFORCEMENT UNLESS NOTED.
- 6. FOR ADDITIONAL NOTES, SEE SHEET 1.



END VIEW ELEVATION

> <u>ALTERNATE</u> RAIL SPLICE SLEEVE

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

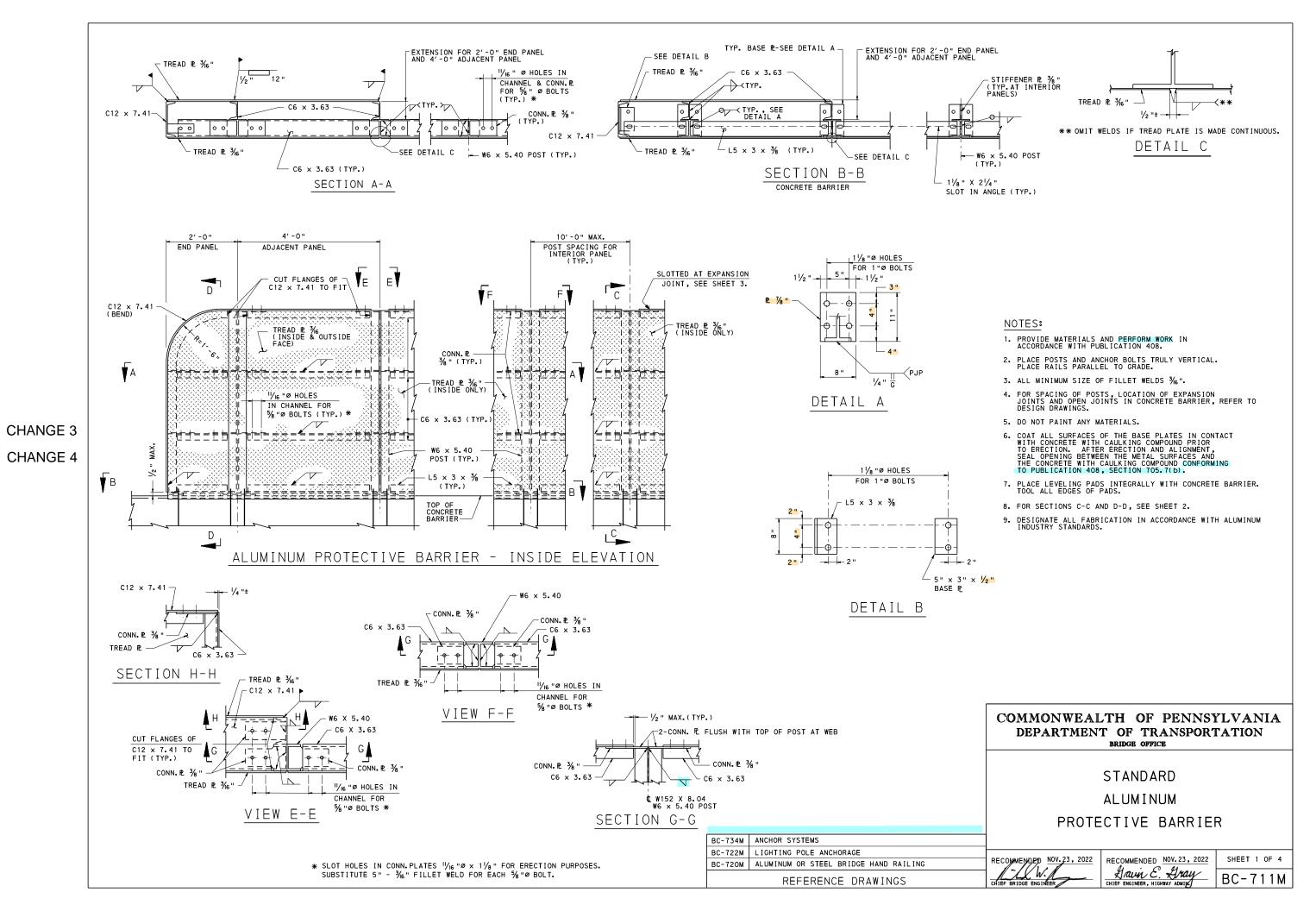
STANDARD

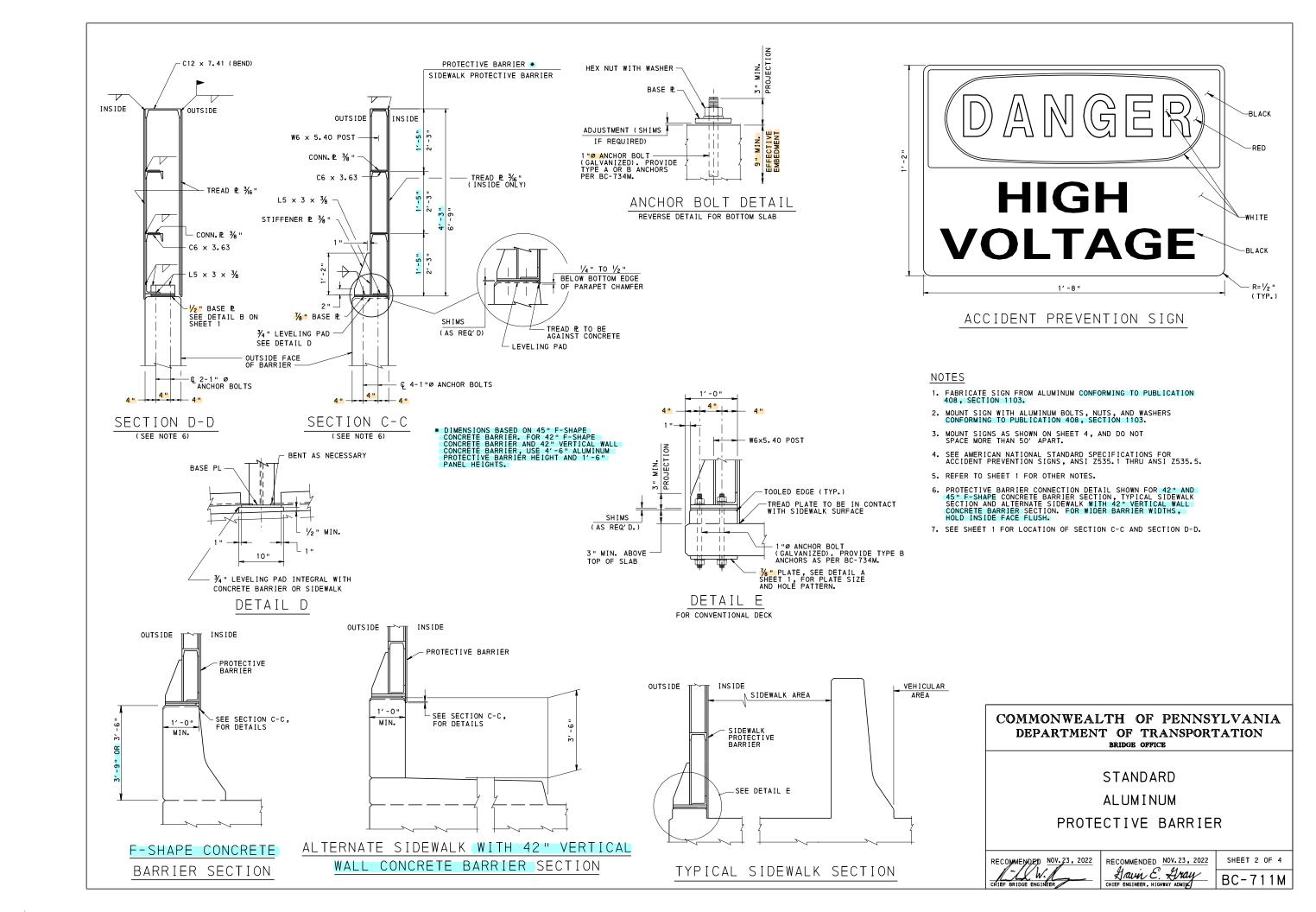
PA TYPE 10M BRIDGE BARRIER MISCELLANEOUS DETAILS

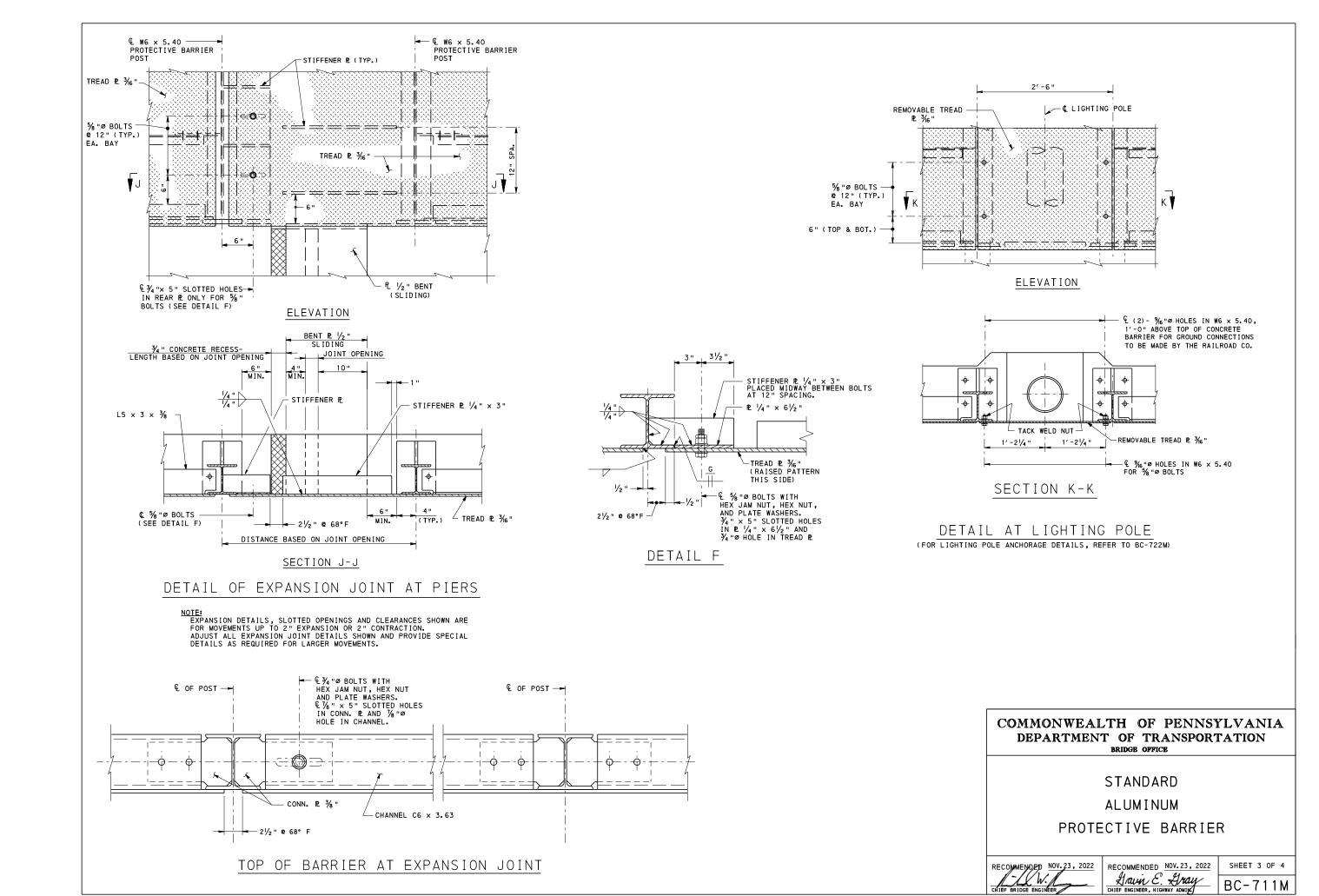
RECOMMENDED OCT. 7, 2024 CHIEF BRIDGE ENGINEER

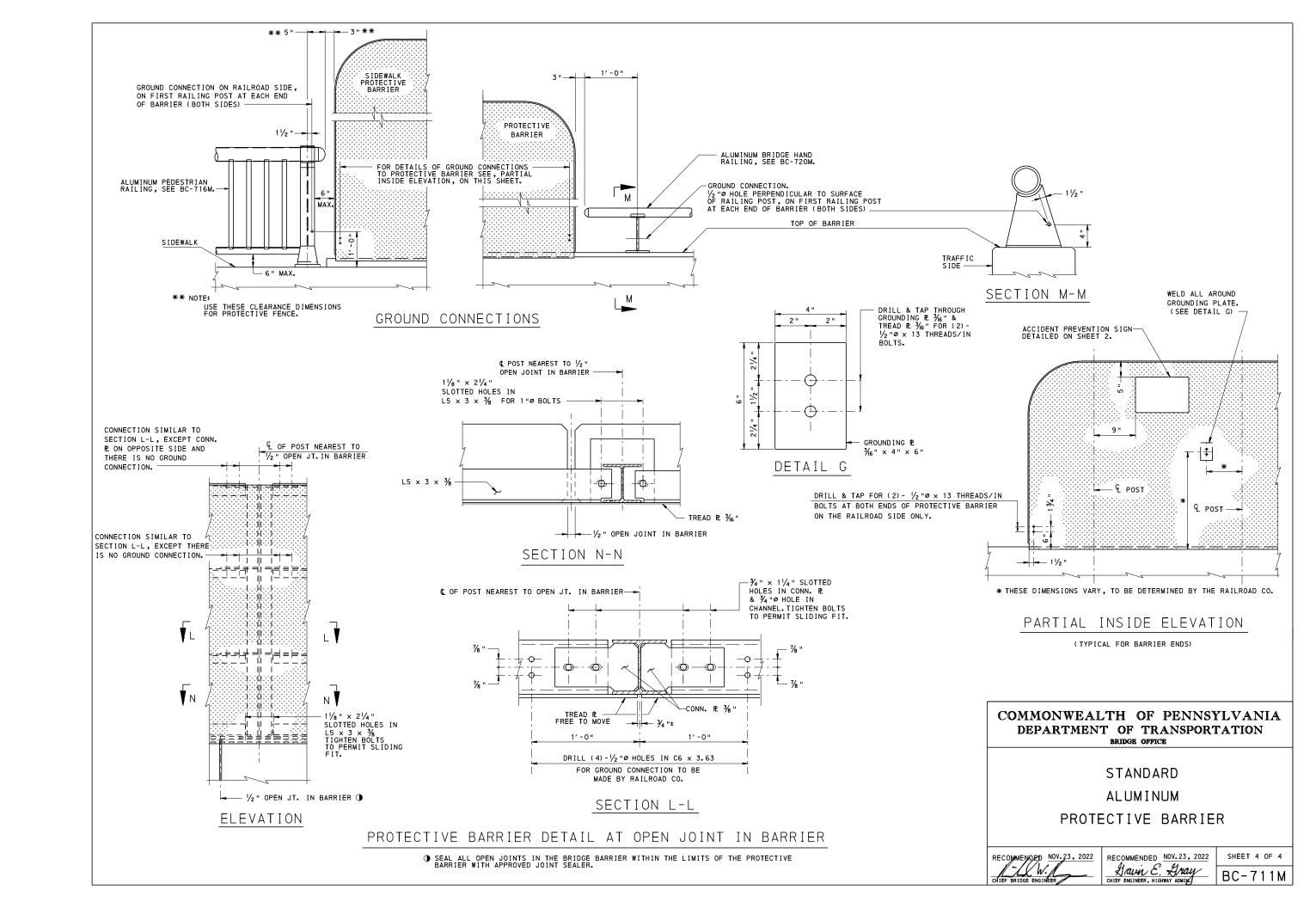
Havin E. Hray
CHIEF ENGINEER, HIGHWAY ADMIN.

RECOMMENDED <u>0CT. 7, 2024</u> | SHEET 13 OF 13 BC-709M









GENERAL NOTES:

- 1. PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH PUBLICATION 408.
- 2. PROVIDE RAIL TUBES CONFORMING TO ASTM A500, GRADE B.
- PROVIDE POSTS CONFORMING TO AASHTO M270 (ASTM A709), GRADE 50 OR ASTM A992. PROVIDE BASE PLATES CONFORMING TO AASHTO M270 (ASTM A709), GRADE 50. PROVIDE ANCHOR PLATES CONFORMING TO AASHTO M270 (ASTM A709),
- 4. ALL RAILING COMPONENTS SHALL BE GALVANIZED (AFTER FABRICATION) AS SPECIFIED IN PUBLICATION 408, SECTION 1105.02(s), UNLESS OTHERWISE SHOWN ON THE PLANS. GALVANIZE POSTS, BASE PLATES, ANCHOR PLATES, CONNECTOR PLATES AND SPLICE SLEEVES ACCORDING TO ASTM A123. GALVANIZE RAIL TUBES ACCORDING TO ASTM A123, EXCEPT COATING ON THREADED STUDS AND NUTS USED WITH THE STUDS SHALL MEET THE REQUIREMENTS OF ASTM A153 FOR CLASS C MATERIAL. GALVANIZE ALL ANCHOR HARDWARE ACCORDING TO ASTM A153 OR ASTM B695.
- 5. THE RAIL TUBES ARE SHOP BENT OR FABRICATED TO FIT HORIZONTAL CURVE WHEN RADIUS IS LESS THAN 1,500 FEET.

- 6. STEEL TUBE TOLERANCES:

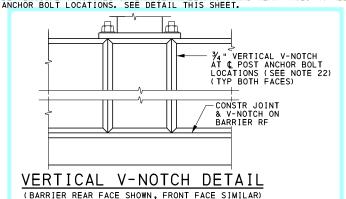
 A. STRAIGHTNESS: THE PERMISSIBLE VARIATION FOR STRAIGHTNESS SHALL
 BE 1/8 " TIMES THE NUMBER OF FEET OF THE TOTAL LENGTH DIVIDED BY 5.

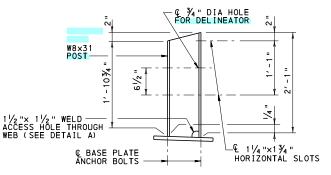
 B. TWISTS SPECIFIED DIMENSION OF THE LONGEST SIDE IN INCHES FROM OVER
 4 " TO 6" INCLUSIVE: 0.087" MAX TWIST IN THE FIRST 3 FEET AND IN EACH
 ADDITIONAL 3 FOOT.

 NOTE TWIST IS MEASURED BY HOLDING DOWN ONE END OF SQUARE OR
 RECTANGULAR TUBE ON A FLAT SURFACE PLATE WITH THE BOTTOM SIDE OF
 THE TUBE PARALLEL TO THE SURFACE PLATE AND NOTING THE HEIGHT
 DIFFERENCE BETWEEN THE TWO CORNERS AT THE OPPOSITE END OF THE
 BOTTOM SIDE OF THE TUBE.
- 7. MILL TO BEAR IS DEFINED AS FOLLOWS: A MINIMUM OF 25% OF THE POST WEB AND COMPRESSION FLANGE END AREA MUST FIT WITHIN 1/2" OF THE BASE PLATE WITH NO GAP MORE THAN 0.040" FOR THE REMAINING 75% OF THE END AREA.
- 8. ALL WELDING SHALL CONFORM TO THE PROVISIONS OF AASHTO/AWS BRIDGE WELDING CODE D1.5, EXCEPT USE AASHTO/AWS BRIDGE WELDING CODE D1.1 FOR WELDING NOT COVERED IN D1.5.
- 9. FOR RAIL TUBE TO POST CONNECTION AND SIDEWALK RAIL TUBE CONNECTION, USE AUTOMATIC WELDED THREADED ANCHOR STUDS CONFORMING TO ASTM A108. USE HAX NUTS CONFORMING TO ASTM A563. USE A 36" THICK PLATE LOCK WASHER ON EACH STUD AND A 36" THICK PLATE ASTM A709, GRADE 36 KSI WASHER. U-WASHERS CONFORMING TO ASTM A709, GRADE 36 KSI STEEL.
- R ANCHOR BOLTS, USE 11/8" DIA BOLTS CONFORMING TO THE REQUIREMENTS ASTM F1554, GRADE 105 KSI, INCLUDING THE SUPPLEMENTARY REQUIREMENT, FOR CHARPY IMPACT STRENGTH. USE ASTM A563, GRADE DH HEAVY HEX NUTS. E ONE ASTM F436,21/4" O.D. CLIPPED WASHER AT THE TOP OR ALTERNATIVELY E A RECTANGULAR % "x2"x3", ASTM A709, GRADE 36 KSI WASHER WITH 15/6"
- 11. BOLT TIGHTENING PROCEDURES ARE AS FOLLOWS:

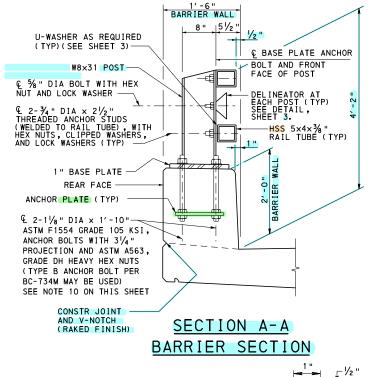
 A. SNUG TIGHTEN ALL ANCHOR BOLTS. TIGHTEN THE NUTS AN ADDITIONAL 1/3 TURN USING A WRENCH.

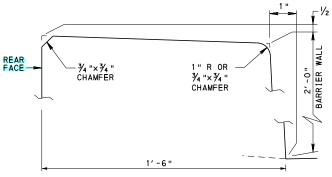
 B. INSTALL RAILING PROVIDING A SMOOTH FACE TO TRAFFIC. INSTALL U-SHAPE WASHERS PROVIDING A SNUG-FIT CONNECTION BETWEEN THE RAIL AND POST. SNUG-TIGHTEN ALL THREADED ANCHOR STUDS. REFER TO SHEET 5 FOR U-WASHER DETAIL.
- 12. IF FLAME CUTTING OR PLASMA CUTTING IS USED TO CREATE SLOTTED HOLES, GRIND SMOOTH TO PROVIDE VERTICAL AND FLAT SURFACES ALONG THE HOLE.
- 13. THE OUT OF FLATNESS TOLERANCE FOR THE POST BASE PLATES
 IS 1/8 " CHECKED BETWEEN EDGES OF THE PLATE IN ANY DIRECTION
 AFTER WELDING IS COMPLETED. THE CONTRACTOR MAY ELECT TO USE
 THICKER PLATE MATERIAL AND MILL THE BASE PLATE TO A THICKNESS OF
 NO LESS THAN 7/8 " TO MEET THIS TOLERANCE.
- 14. THE CENTERLINE OF THE RAIL TUBE SPLICE TO A POST IS TO BE 1'-6" MINIMUM AND 2'-6" MAXIMUM FROM THE CENTERLINE OF THE RAILING POST.
- 15. ONE OR MORE 7'-6" MAXIMUM POST SPACINGS MAY BE REDUCED TO 4'-0" MINIMUM IN ORDER TO MAINTAIN APPROPRIATE SPACING DIMENSIONS FROM THE END OF THE RAIL, EXPANSION JOINTS AND DRAINAGE SCUPPERS.
- 16. LOCATE RAIL SPLICES AT EXPANSION JOINTS AND AT OTHER LOCATIONS WHERE NECESSARY. PROVIDE RAILS AS LONG AS PRACTICAL, WITH A MINIMUM OF THREE POSTS BETWEEN SPLICES, UNLESS OTHERWISE REQUIRED FOR EXPANSION.
- 17. PROVIDE RAIL TUBES CONTINUOUS OVER NOT LESS THAN THREE RAILING POSTS. NO WELDED BUTT SPLICES WILL BE ALLOWED IN THE RAIL TUBE SECTIONS.
- 18. PLACE POST AND POST ANCHOR BOLTS NORMAL TO GRADE AND RAILS PARALLEL TO GRADE.
- 19. COAT ALL SURFACES OF THE BASE PLATE IN CONTACT WITH CONCRETE WITH CAULKING COMPOUND PRIOR TO ERECTION. AFTER ERECTION AND ALIGNMENT, SEAL OPENINGS BETWEEN THE METAL SURFACES AND THE CONCRETE WITH CAULKING COMPOUND CONFORMING TO PUBLICATION 408, SECTION 705.7(b).
- 20. THE PA BRIDGE BARRIER IS DESIGNATED AS MASH TL-5. THE PA BRIDGE BARRIER ATTACHED TO A MOMENT SLAB IS DESIGNATED AS MASH TL-4.
- 21. FOR GUIDE RAIL TRANSITION TO PA BRIDGE BARRIER, SEE RC-50M.
- 22. PROVIDE VERTICAL V-NOTCHES ON BARRIER WALL FRONT AND REAR FACES AT ALL POST ANCHOR BOLT LOCATIONS. SEE DETAIL THIS SHEET.



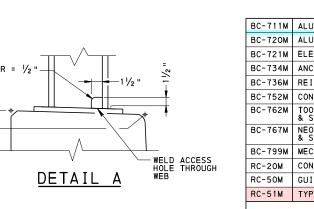


ELEVATION-POST





BARRIER WALL GEOMETRY DETAIL (BASE PLATE AND ANCHOR BOLTS NOT SHOWN FOR CLARITY)



BC-711M ALUMINUM PROTECTIVE BARRIER ALUMINUM OR STEEL BRIDGE HAND RAILING FLECTRICAL DETAILS ANCHOR SYSTEMS BC-736M REINFORCEMENT BAR FABRICATION DETAILS CONCRETE DECK SLAB DETAILS TOOTH EXPANSION DAM FOR PRESTRESSED CONCRETE & STEEL BEAM BRIDGES NEOPRENE STRIP SEAL DAM FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES MECHANICALLY STABILIZED EARTH RETAINING WALLS CONCRETE PAVEMENT JOINTS GUIDE RAIL TO BRIDGE BARRIER TRANSITIONS RC-51M TYPE 31 STRONG POST GUIDE RAIL REFERENCE DRAWINGS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

PA BRIDGE BARRIER MISCELLANEOUS DETAILS - 1



RECOMMENDED OCT. 7, 2024 Havin E. Hray
HIEF ENGINEER, HIGHWAY ADMIN.

BC-713M

SHEET 1 OF 16

CHANGE 2 CHANGE 3

CHANGE 7

CHANGE 4

PLAN (BARRIER WALL NOT SHOWN) - C W8×31 -- € W8×31 POST 4'-0" MIN, 7'-6" MAX 3'-33/4" 10" MIN 1'-2" MAX Α DEVELOPED LENGTH 0:0 | O¦O | HSS 5x4x3/k RAIL TUBÉ 10 -1509 SPA @ 010 00 -0 -RAII TUBE - RATI CAP TUBE _4 "_ CAP C THRIE-BEAM CONNECTION PLATE -6--ASSEMBLY (SEE RC-50M) 2' -0 BARRIER - ½" JOINT 1" DIA HOLES FOR 18" DIA GALVANIZED BOLTS (TYP) GUTTER LINE-1'-6" 9'-0" MIN BARRIER END TRANSITION LENGTH A ELEVATION PA BRIDGE BARRIER WITH THRIE-BEAM CONNECTION (GUIDE RAIL AND ANCHOR BOLTS OMITTED FOR CLARITY)
(WITH CURB SHOWN, WITHOUT CURB SIMILAR)

- (C. W8×31

-160°

103/8"

SEE DETAIL BOOK SHEET 3

2'-0" DEVELOPED LENGTH

- C W8×3

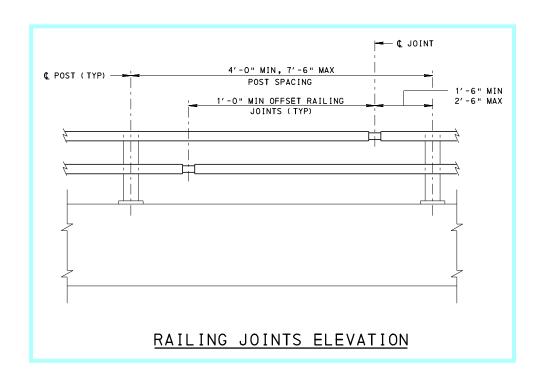
HSS 5×4×3/8

RAIL TUBE

ROADWAY

BOTTOM OF TOP RAIL

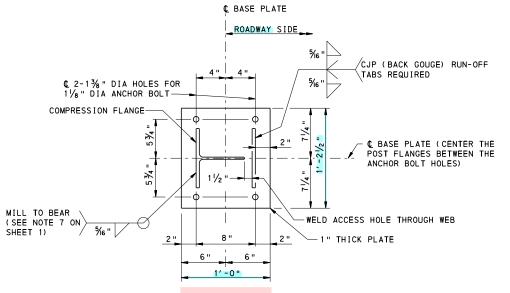
POST



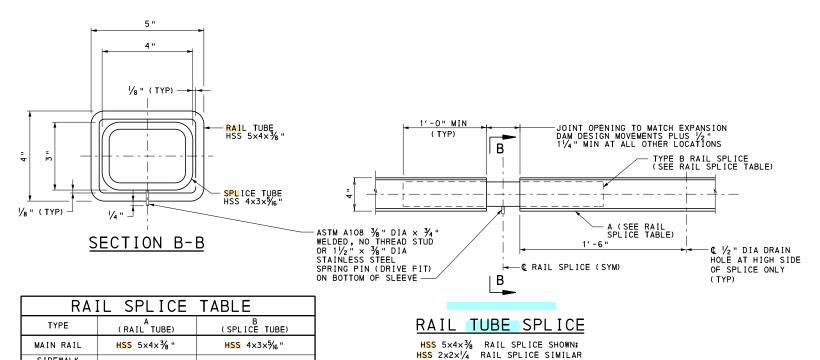
SIDEWALK RAIL

HSS 2x2x1/4"

1 1/4 "x1 1/4" ROD, 36 KSI

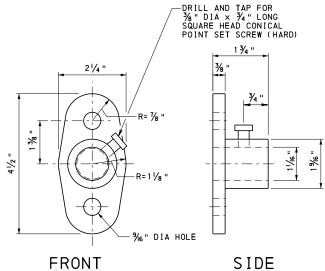


BASE PLATE DETAIL



1" DIA SIDEWALK RAIL ROD —— ASTM A709, GRADE 36 KSI	6"	15/6" OD TUBING (106" ID) CARBON STEEL - ASTM A513 OR A519
£		
SAME AS OPENING IN BARRIER FOR SPLICE EXPANSION JOINTS IN SUPERSTRUCTURE. // SPLICES AT OTHER LOCATIONS		DRILL AND TAP FOR % " DIA × ¾ " LONG SQUARE HEAD CONICAL POINT SET SCREW (HARD)

SIDEWALK RAIL ROD SPLICE



SIDEWALK RAIL ROD

CONNECTOR PLATE DETAIL

(SEE NOTE 1)

NOTES:

- 1. IN LIEU OF FABRICATED CONNECTOR PLATE, USE CAST OR OTHER TYPE OF CONNECTOR PLATE SUBJECT TO SHOP DRAWING APPROVAL.
- 2. FOR ADDITIONAL NOTES, SEE SHEET 1.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

PA BRIDGE BARRIER
MISCELLANEOUS DETAILS - 2

RECOMMENDED OCT. 7, 2024

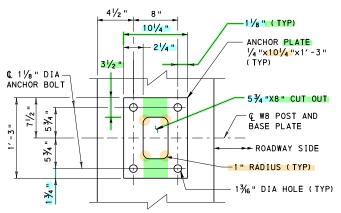
CHIEF BRIDGE ENGINEER

RECOMMENDED OCT. 7, 2024

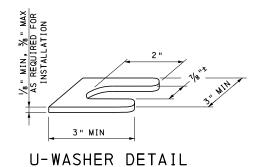
Havin E. Gray

CHIEF ENGINEER, HIGHWAY ADMIN.

SHEET 2 OF 16
BC-713M

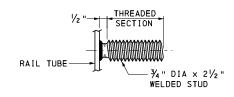


ANCHOR PLATE DETAIL



U-WASHER NOTES:

- 1. THIS U-WASHER IS PROVIDED TO ADJUST FOR "FIT" PROBLEMS IN THE FIELD.
- 2. PROVIDE ONE U-WASHER PER STUD AS REQUIRED.



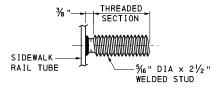
BARRIER RAIL ANCHOR STUD DETAIL

2¾" MIN

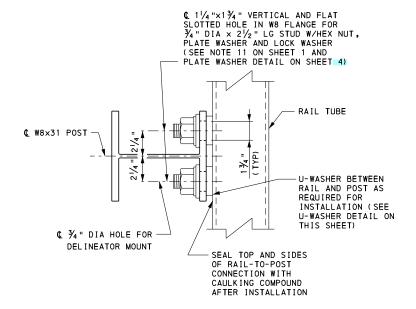
31/2" MAX

DELINEATOR DETAIL

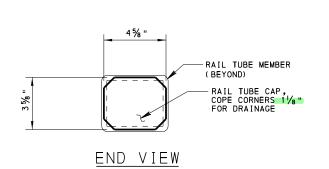
TYPE V RETROREFLECTIVE SHEETING

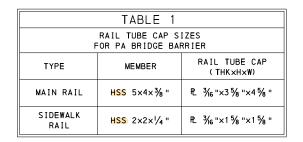


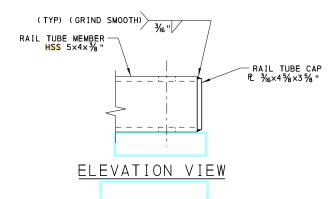
SIDEWALK RAIL ANCHOR STUD DETAIL



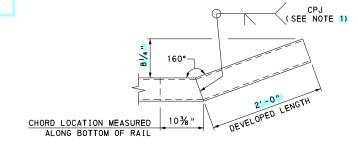
TYPICAL RAIL TO POST DETAIL







2¹/₄ " MIN 3¹/₄ " MAX



DETAIL B (TOP RAIL ONLY)

NOTES:

- 1. COMPLETE JOINT PENETRATION GROOVE WELD. GRIND FLUSH ON OUTSIDE FACE. SHOW SPECIFIC WELD SYMBOL ON SHOP DRAWINGS.
- 2. FOR ADDITIONAL NOTES, SEE SHEET 1.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

PA BRIDGE BARRIER MISCELLANEOUS DETAILS - 3

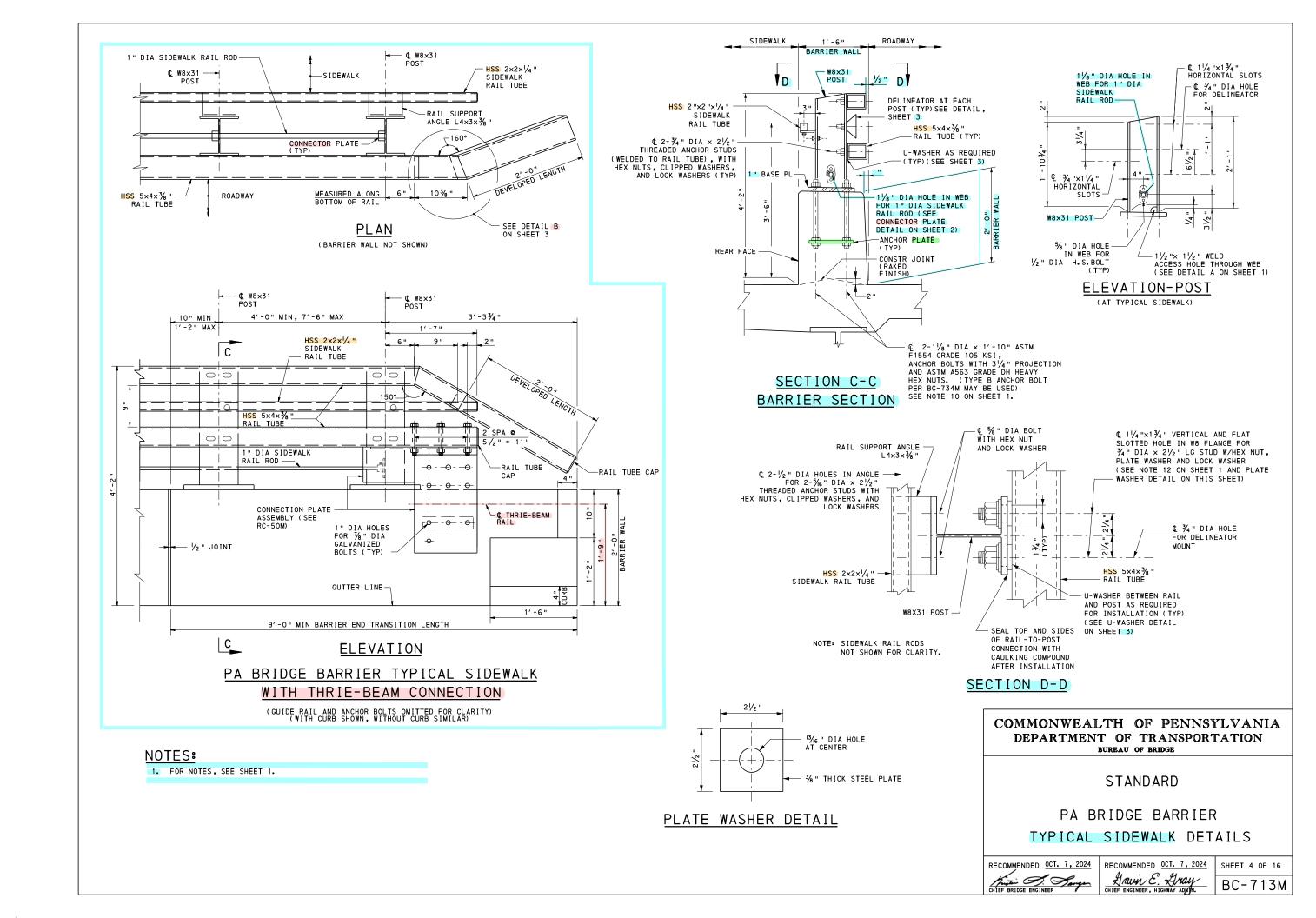
RECOMMENDED OCT. 7, 2024 CHIEF BRIDGE ENGINEER

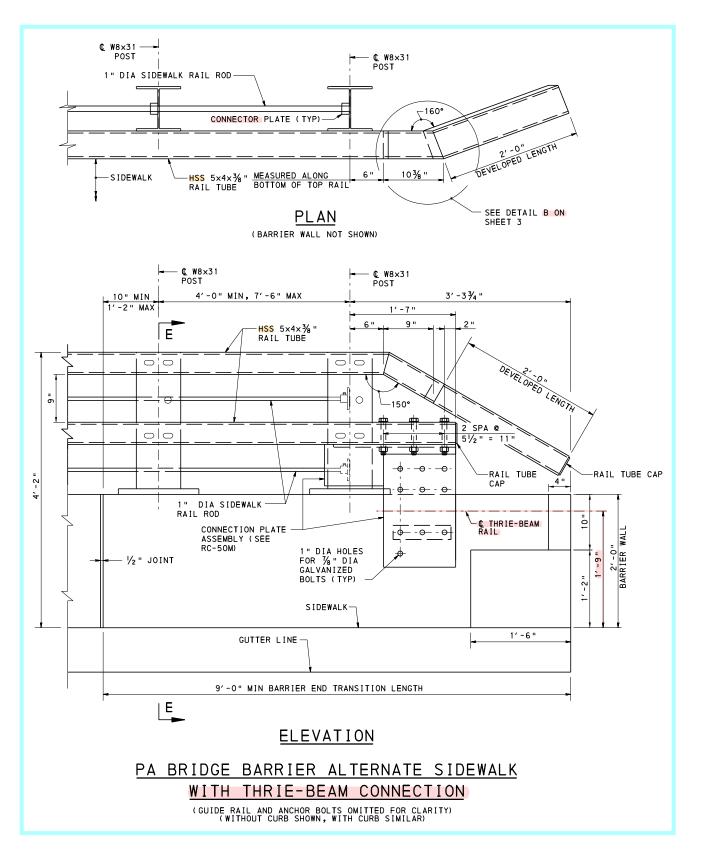
RECOMMENDED OCT. 7, 2024 Havin E. Hray

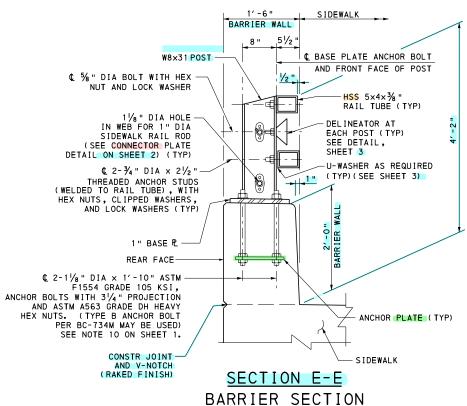
CHIEF ENGINEER, HIGHWAY ADMYN.

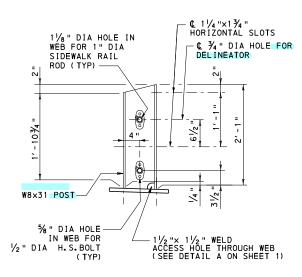
SHEET 3 OF 16 BC-713M

RAIL TUBE CAP DETAIL









ELEVATION-POST

(AT ALTERNATE SIDEWALK)

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

PA BRIDGE BARRIER ALTERNATE SIDEWALK DETAILS

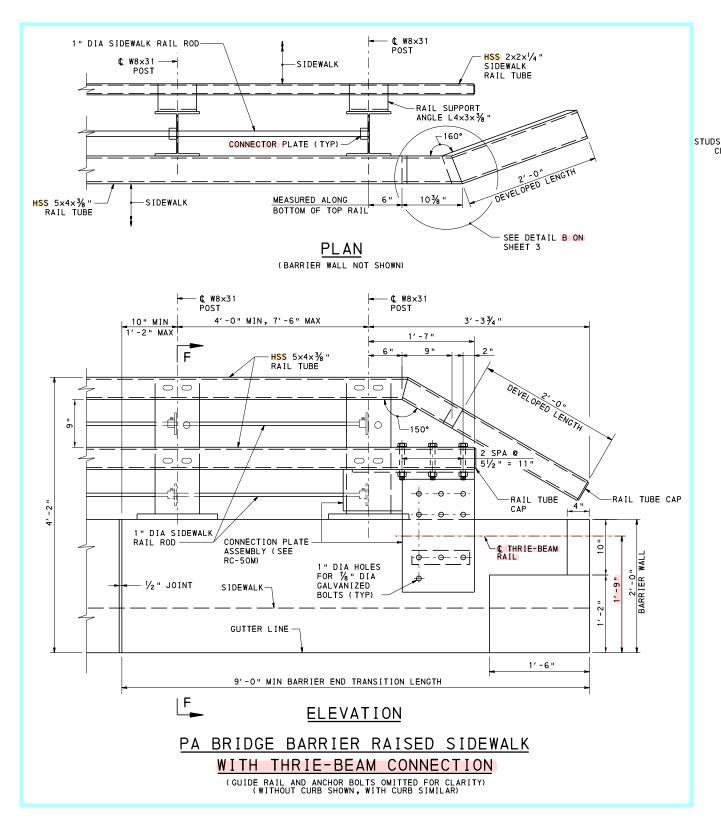
RECOMMENDED OCT. 7, 2024 CHIEF BRIDGE ENGINEER

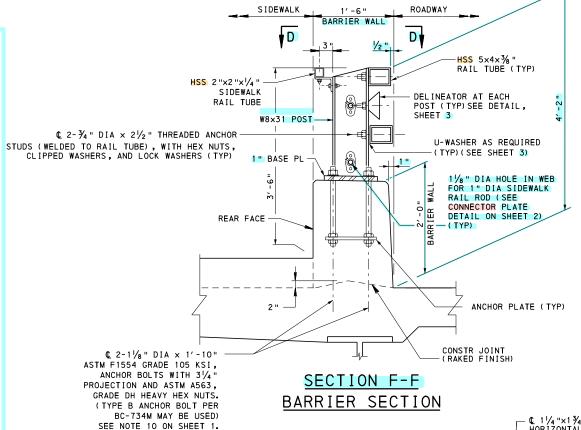
RECOMMENDED OCT. 7, 2024 Havin E. Gray
CHIEF ENGINEER, HIGHWAY ADMIN.

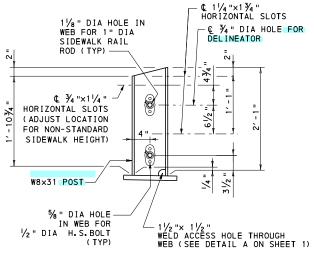
SHEET 5 OF 16 BC-713M

NOTES:

1. FOR NOTES, SEE SHEET 1.







ELEVATION-POST (AT RAISED 8" SIDEWALK)

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

PA BRIDGE BARRIER RAISED SIDEWALK DETAILS

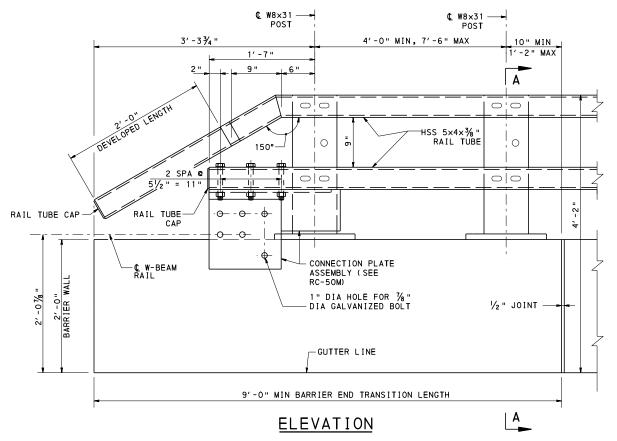
RECOMMENDED OCT. 7, 2024 CHIEF BRIDGE ENGINEER

Havin E. Gray
CHIEF ENGINEER, HIGHWAY ADMIN.

RECOMMENDED OCT. 7, 2024 SHEET 6 OF 16 BC-713M

NOTES:

1. FOR NOTES, SEE SHEET 1

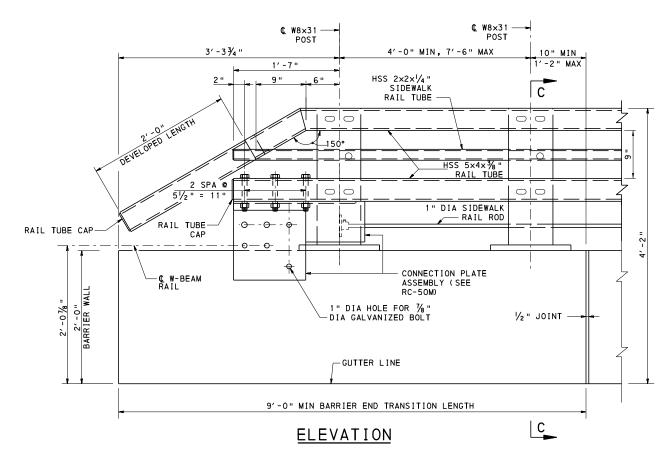


PA BRIDGE BARRIER WITH W-BEAM CONNECTION

(GUIDE RAIL AND ANCHOR BOLTS OMITTED FOR CLARITY)

NOTES:

- PROVIDE W-BEAM GUIDE RAIL CONNECTION ON THE TRAILING END OF THE BRIDGE BARRIER ON DIVIDED HIGHWAYS AND ONE-WAY ROADWAYS WHEN INDICATED ON THE CONTRACT DRAWINGS.
- 2. END CHAMFERS, ON THE BARRIER WALL, ARE NOT REQUIRED ON THE TRAILING END OF THE BARRIER WALL ON DIVIDED HIGHWAYS AND ONE-WAY ROADWAYS WHEN CONNECTING TO A W-BEAM RAIL.
- 3. FOR THE W-BEAM GUIDE RAIL CONNECTION, THE COST OF THE CONNECTION PLATE, BOLTS, AND ASSOCIATED HARDWARE ARE INCLUDED WITH THE PA BRIDGE BARRIER PAY ITEM.
- 4. FOR ADDITIONAL DETAILS AND NOTES, SEE SHEETS 1-6.
- 5. FOR SECTION A-A, SEE SHEET 1.
- 6. FOR SECTION B-B, SEE SHEET 4.



PA BRIDGE BARRIER TYPICAL SIDEWALK WITH W-BEAM CONNECTION

(GUIDE RAIL AND ANCHOR BOLTS OMITTED FOR CLARITY)

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

W-BEAM CONNECTIONS ELEVATIONS - 1

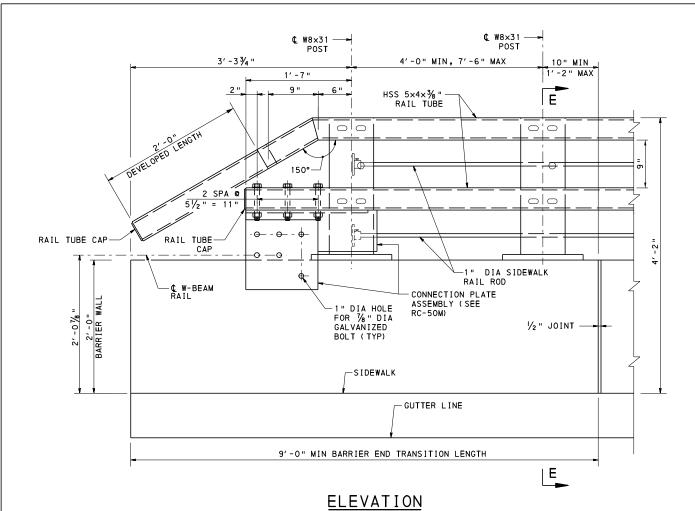
RECOMMENDED OCT. 7, 2024

RECOMMENDED OCT. 7, 2024 Havin E. Hray.

CHIEF ENGINEER, HIGHWAY ADMYN.

SHEET 7 OF 16 BC-713M

CHIEF BRIDGE ENGINEER



POST POST 3'-33/4" 4'-0" MIN. 7'-6" MAX 10" MIN _ 1'-2" MAX 1'-7" HSS 5×4×3/8" -RAIL TUBE 00 010 - - - - -RAIL TUBE CAP-RAIL TUBE--1" DIA SIDEWALK −Ç W-BEAM RAIL CONNECTION PLATE ASSEMBLY (SEE 2' -0" SARIER WALL FOR % " DIA GALVANIZED ½" J0INT — GUTTER LINE 9'-0" MIN BARRIER END TRANSITION LENGTH ELEVATION

© W8×31 — -

PA BRIDGE BARRIER ALTERNATE SIDEWALK WITH W-BEAM CONNECTION

(GUIDE RAIL AND ANCHOR BOLTS OMITTED FOR CLARITY)

PA BRIDGE BARRIER RAISED SIDEWALK WITH W-BEAM CONNECTION

(GUIDE RAIL AND ANCHOR BOLTS OMITTED FOR CLARITY)

NOTES:

- PROVIDE W-BEAM GUIDE RAIL CONNECTION ON THE TRAILING END OF THE BRIDGE BARRIER ON DIVIDED HIGHWAYS AND ONE-WAY ROADWAYS WHEN INDICATED ON THE CONTRACT DRAWINGS.
- 2. END CHAMFERS, ON THE BARRIER WALL, ARE NOT REQUIRED ON THE TRAILING END OF THE BARRIER WALL ON DIVIDED HIGHWAYS AND ONE-WAY ROADWAYS WHEN CONNECTING TO A W-BEAM RAIL.
- 3. FOR THE W-BEAM GUIDE RAIL CONNECTION, THE COST OF THE CONNECTION PLATE, BOLTS, AND ASSOCIATED HARDWARE ARE INCLUDED WITH THE PA BRIDGE BARRIER PAY ITEM.
- 4. FOR ADDITIONAL DETAILS AND NOTES, SEE SHEETS 1-6.
- 5. FOR SECTION E-E, SEE SHEET 5.
- 6. FOR SECTION F-F, SEE SHEET 6.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

¢ w8×31 —-

STANDARD

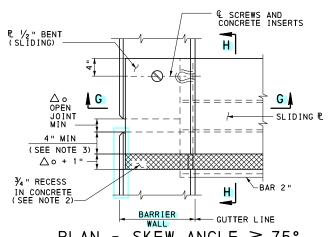
W-BEAM CONNECTIONS ELEVATIONS - 2

RECOMMENDED OCT. 7, 2024 CHIEF BRIDGE ENGINEER

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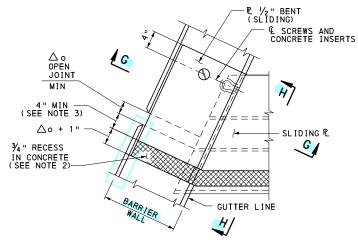
SHEET 8 OF 16 Havin E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN. BC-713M



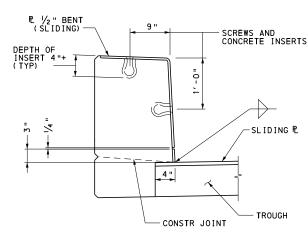
PLAN - SKEW ANGLE ≥ 75°

(AT GUTTER LINE SHOWN; AT END OF ALTERNATE SIDEWALK SIMILAR)



PLAN - SKEW ANGLE < 75°

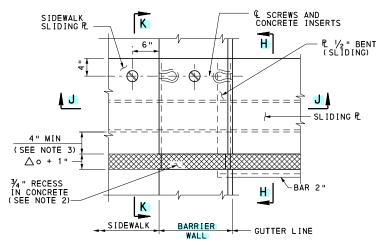
(AT GUTTER LINE SHOWN; AT END OF ALTERNATE SIDEWALK SIMILAR)



SECTION G-G

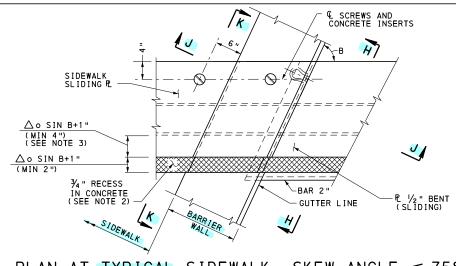
NOTES:

- 1. FOR △ o SEE BC-762M
- FORM CONCRETE RECESS AREA IN BARRIER WALL AND GRIND TO PROVIDE SMOOTH SURFACE. APPLY ONE COAT OF ASPHALT CEMENT PAINT WA-1 OR PERFORMANCE GRADED ASPHALT CEMENT PG 64-22 TO ALLOW BENT SLIDING PLATE TO MOVE FREELY WITHOUT FRICTION.
- MAINTAIN 4" MINIMUM BETWEEN EDGE OF STEEL TO THE EDGE OF CONCRETE AT TEMP. OF -10°F FOR STEEL BRIDGES AND 10°F FOR P/S CONCRETE BRIDGES.
- 4. MAXIMUM DISTANCE FROM EDGE OF EXTENSION OR BEND TO FIRST



PLAN AT TYPICAL SIDEWALK- SKEW ANGLE ≥ 75°

(AT TYPICAL SIDEWALK SHOWN; RAISED SIDEWALK SIMILAR)

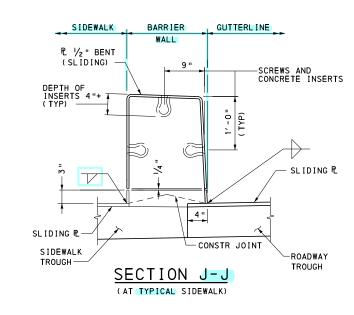


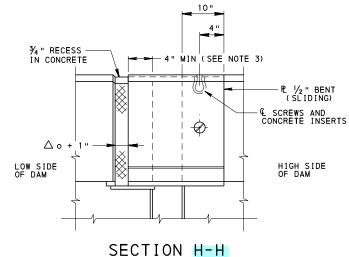
PLAN AT TYPICAL SIDEWALK- SKEW ANGLE < 75°

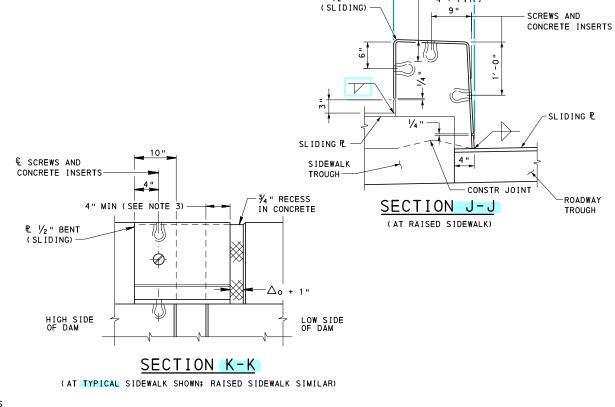
SIDEWALK

PL 1/2" BENT

(AT TYPICAL SIDEWALK SHOWN; RAISED SIDEWALK SIMILAR)







PA BRIDGE BARRIER AT TOOTH EXPANSION DAM

(RAILING POST AND TUBE RAILS NOT SHOWN) (AT TYPICAL SIDEWALK SHOWN; RAISED SIDEWALK SIMILAR)

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

BARRIER

GUTTERL INE

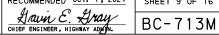
- DEPTH OF INSERTS 4"+ (TYP)

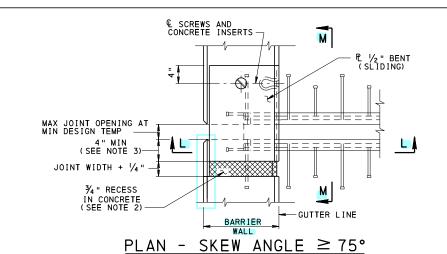
STANDARD

PA BRIDGE BARRIER DETAILS AT TOOTH EXPANSION DAM

RECOMMENDED OCT. 7, 2024 CHIEF BRIDGE ENGINEER

RECOMMENDED OCT. 7, 2024 SHEET 9 OF 16





MAX JOINT OPENING AT MIN DESIGN TEMP 4" MIN (SEE NOTE 3)-

JOINT WIDTH +1/4"

3/4" RECESS

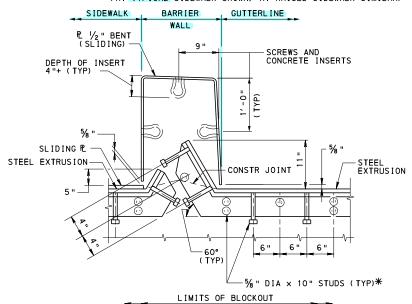
IN CONCRETE (SEE NOTE 2)

& SCREWS AND CONCRETE INSERTS SIDEWALK SLIDING PL PL 1/2" BENT (SLIDING) MAX JOINT OPENING AT MIN DESIGN TEMP 4" MIN (SEE NOTE 3) JOINT WIDTH + 1/4" 3/4" RECESS IN CONCRETE (SEE NOTE 2) -GUTTER LINE SIDEWALK BARRIER _

© SCREWS AND CONCRETE INSERTS 10 SIDEWALK SLIDING PL MAX JOINT OPENING AT MIN DESIGN TEMP 4" MIN (SEE NOTE 3) JOINT WIDTH + 1/4" 3/4" RECESS IN CONCRETI GUTTER LINE

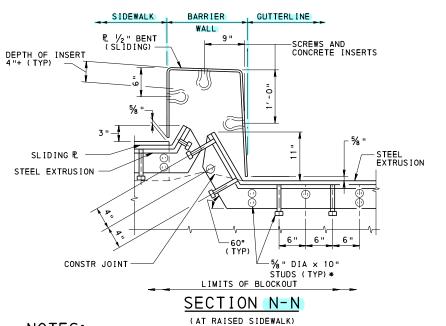
PLAN AT TYPICAL SIDEWALK- SKEW ANGLE ≥ 75°

(AT TYPICAL SIDEWALK SHOWN; AT RAISED SIDEWALK SIMILAR)



PLAN AT TYPICAL SIDEWALK- SKEW ANGLE < 75°

(AT TYPICAL SIDEWALK SHOWN; RAISED SIDEWALK SIMILAR)



PLAN - SKEW ANGLE < 75°

GUTTER LINE

PL 1/2" BENT (SLIDING)

SECTION N-N (AT TYPICAL SIDEWALK)

MAX JOINT OPENING AT MIN DESIGN TEMP 3/4" RECESS IN CONCRETE 4" MIN (SEE NOTE 3) ·R ½" BENT (SLIDING) JOINT WIDTH - C SCREWS AND CONCRETE INSERTS 1/2 " RECESS IN CONCRETE LOW SIDE HIGH SIDE OF DAM SECTION M-M

* IF 10" STUDS CANNOT BE ACCOMMODATED IN THE SPACE AVAILABLE, REQUEST SPECIFIC LENGTH APPROVAL FROM THE DISTRICT BRIDGE ENGINEER AT THE SHOP DRAWINGS APPROVAL STAGE.

€ SCREWS AND CONCRETE INSERTS -SCREWS AND CONCRETE INSERTS DEPTH OF INSERT CONSTR JOINT-STEEL EXTRUSION 6" 6" -% " DIA × 10" STUDS (TYP)* LIMITS OF BLOCKOUT SECTION L-L

PA BRIDGE BARRIER AT NEOPRENE STRIP SEAL DAM

(RAILING POST AND TUBE RAILS NOT SHOWN)

NOTES:

- 1. FOR SECTION P-P DETAILS, SEE SHEET 9.
- FORM CONCRETE RECESS AREA IN BARRIER WALL AND GRIND TO PROVIDE SMOOTH SURFACE. APPLY ONE COAT OF ASPHALT CEMENT PAINT WA-1 OR PERFORMANCE GRADED ASPHALT CEMENT PG 64-22 TO ALLOW BENT SLIDING PLATE TO MOVE FREELY WITHOUT FRICTION.
- MAINTAIN 4" MINIMUM BETWEEN EDGE OF STEEL TO THE EDG AT TEMP. OF -10°F FOR STEEL BRIDGES AND 10°F FOR P/S
- MAXIMUM DISTANCE ALONG THE EXTRUSION FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3".
- 5. ALTERNATE STRIP SEAL DAM (NOT SHOWN) SIMILAR TO THAT SHOWN ON BC-767M,

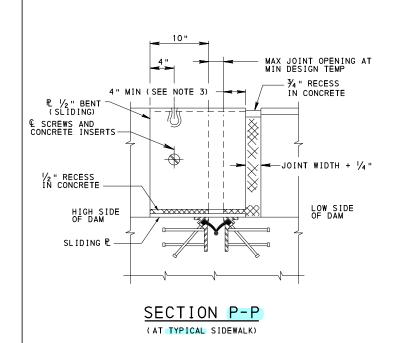
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

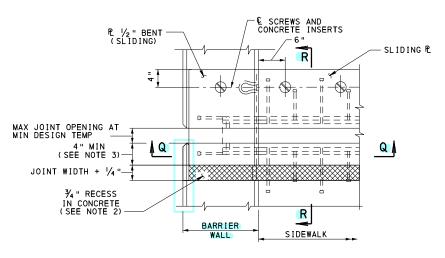
STANDARD PA BRIDGE BARRIER DETAILS AT NEOPRENE STRIP SEAL DAM - 1

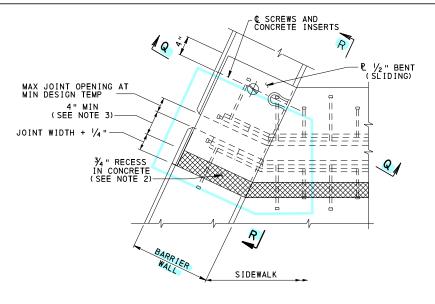
RECOMMENDED OCT. 7, 2024

RECOMMENDED 0CT. 7, 2024 | SHEET 10 OF 16 Havin E. Hray
HIEF ENGINEER, HIGHWAY ADJUN.

BC-713M

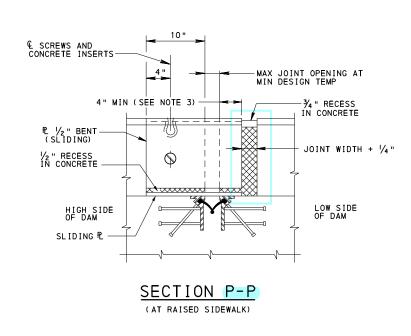


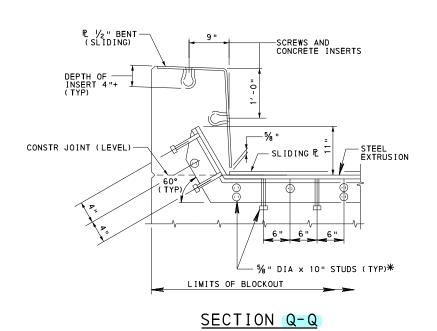


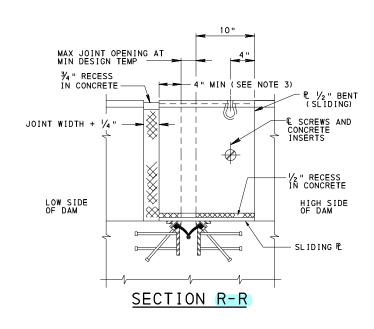


PLAN AT ALTERNATE SIDEWALK- SKEW ANGLE ≥ 75°

PLAN AT ALTERNATE SIDEWALK- SKEW ANGLE < 75°







PA BRIDGE BARRIER AT NEOPRENE STRIP SEAL DAM

(RAILING POST AND TUBE RAILS NOT SHOWN)

NOTES:

- 1. FOR LOCATION OF SECTION P-P, SEE SHEET 8.
- FORM CONCRETE RECESS AREA IN BARRIER WALL AND GRIND TO PROVIDE SMOOTH SURFACE. APPLY ONE COAT OF ASPHALT CEMENT PAINT WA-1 OR PERFORMANCE GRADED ASPHALT CEMENT PG 64-22 TO ALLOW BENT SLIDING PLATE TO MOVE FREELY WITHOUT FRICTION.
- 3. MAINTAIN 4" MINIMUM BETWEEN EDGE OF STEEL TO THE EDGE OF CONCRETE AT TEMP. OF -10°F FOR STEEL BRIDGES AND 10°F FOR P/S CONCRETE BRIDGES.
- 4. MAXIMUM DISTANCE ALONG THE EXTRUSION FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3 ".
- 5. ALTERNATE STRIP SEAL DAM (NOT SHOWN) SIMILAR TO THAT SHOWN ON BC-767M, SHEET 7, IS PERMITTED IF SHOWN ON THE CONTRACT PLANS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

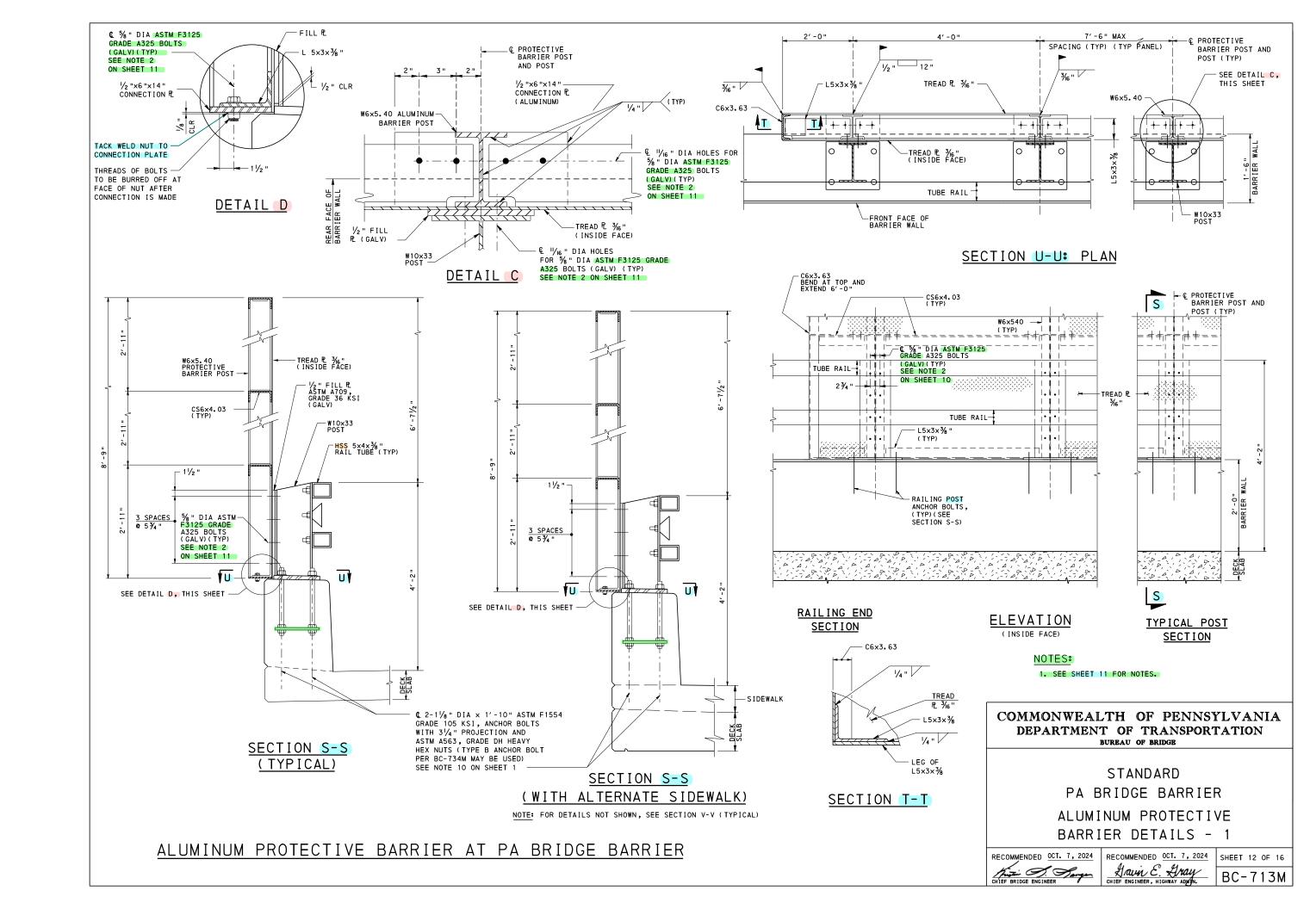
STANDARD PA BRIDGE BARRIER DETAILS AT NEOPRENE STRIP SEAL DAM - 2

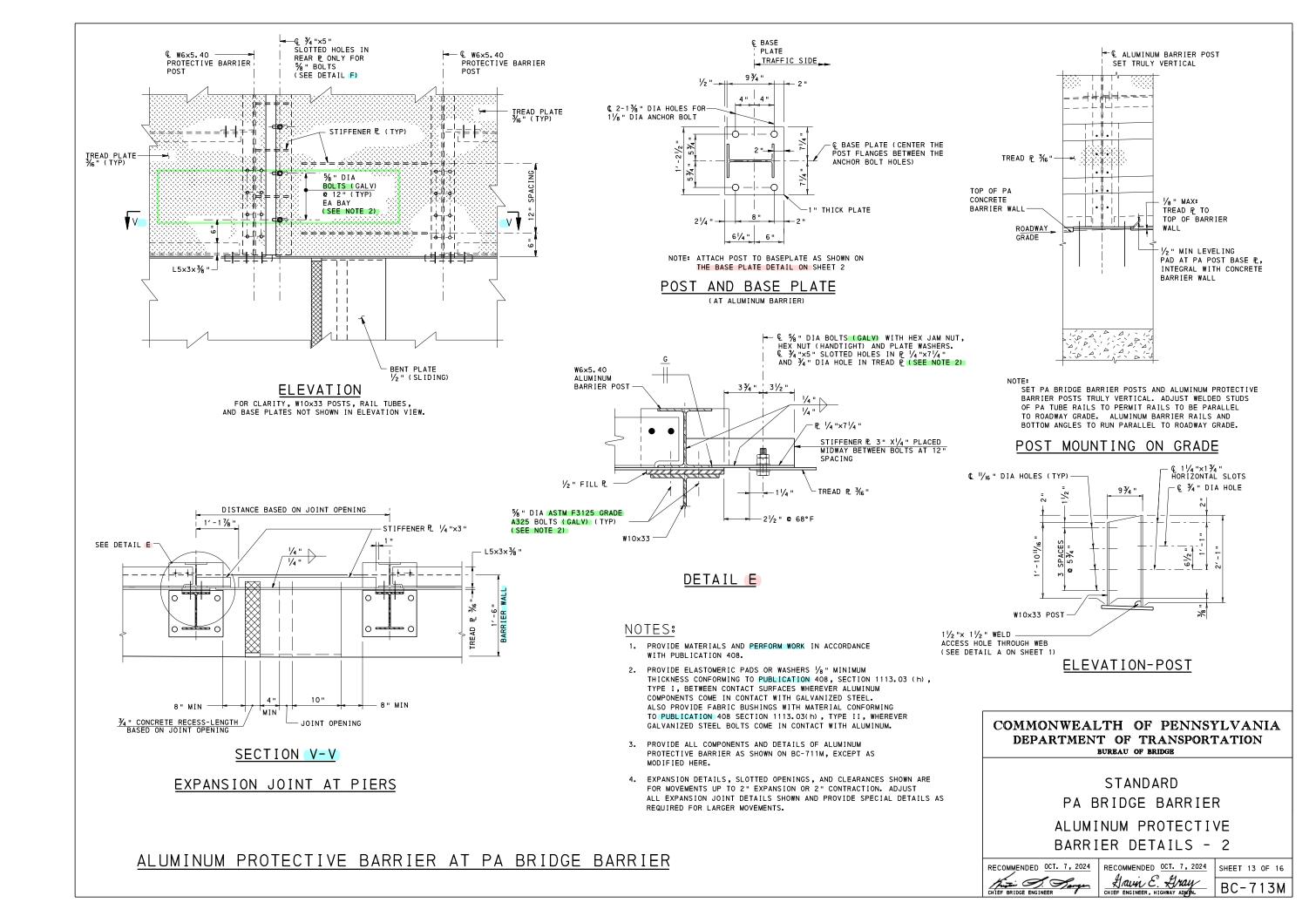
RECOMMENDED OCT. 7, 2024 Bustin S. Hange

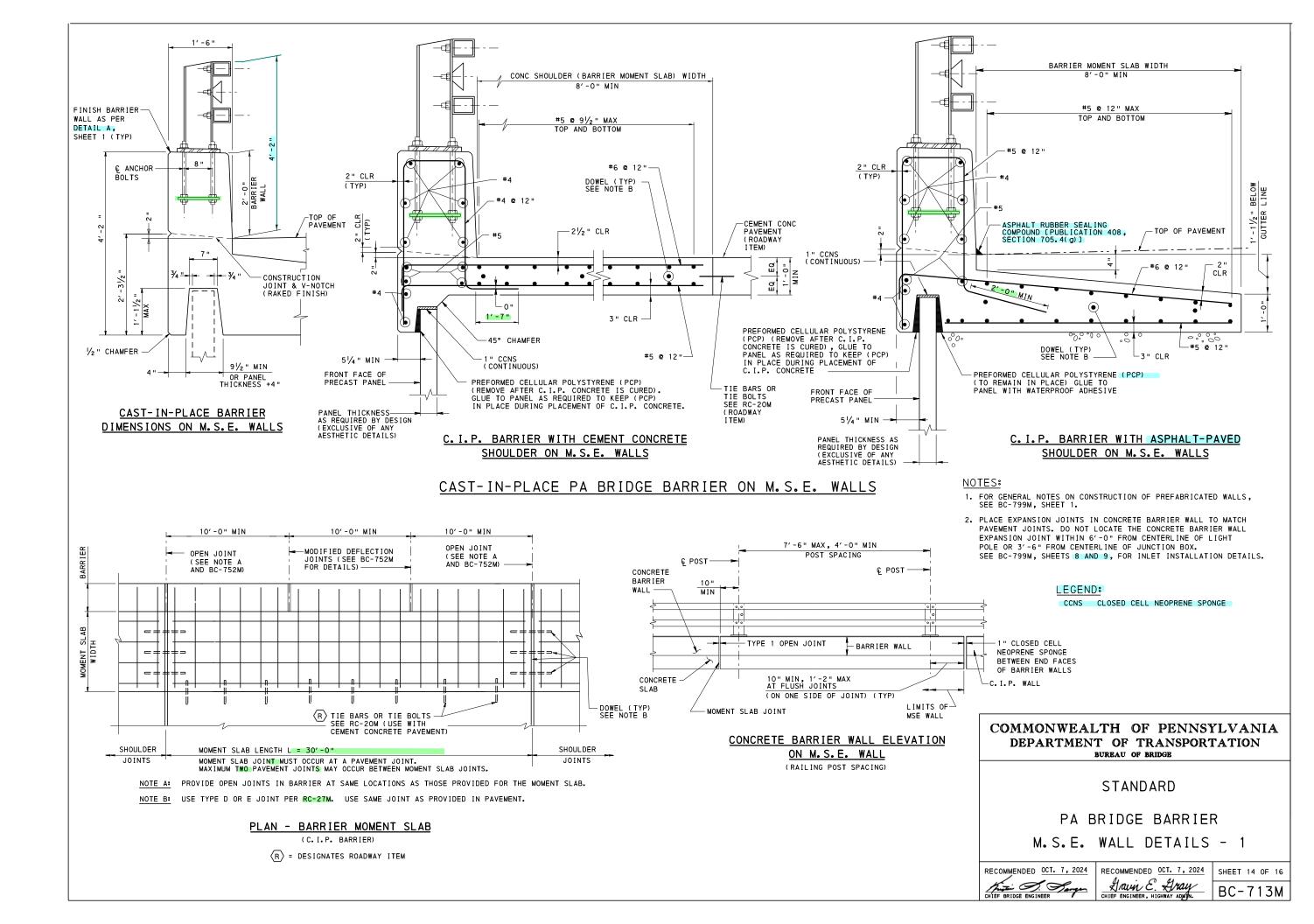
RECOMMENDED 0CT. 7, 2024 | SHEET 11 OF 16

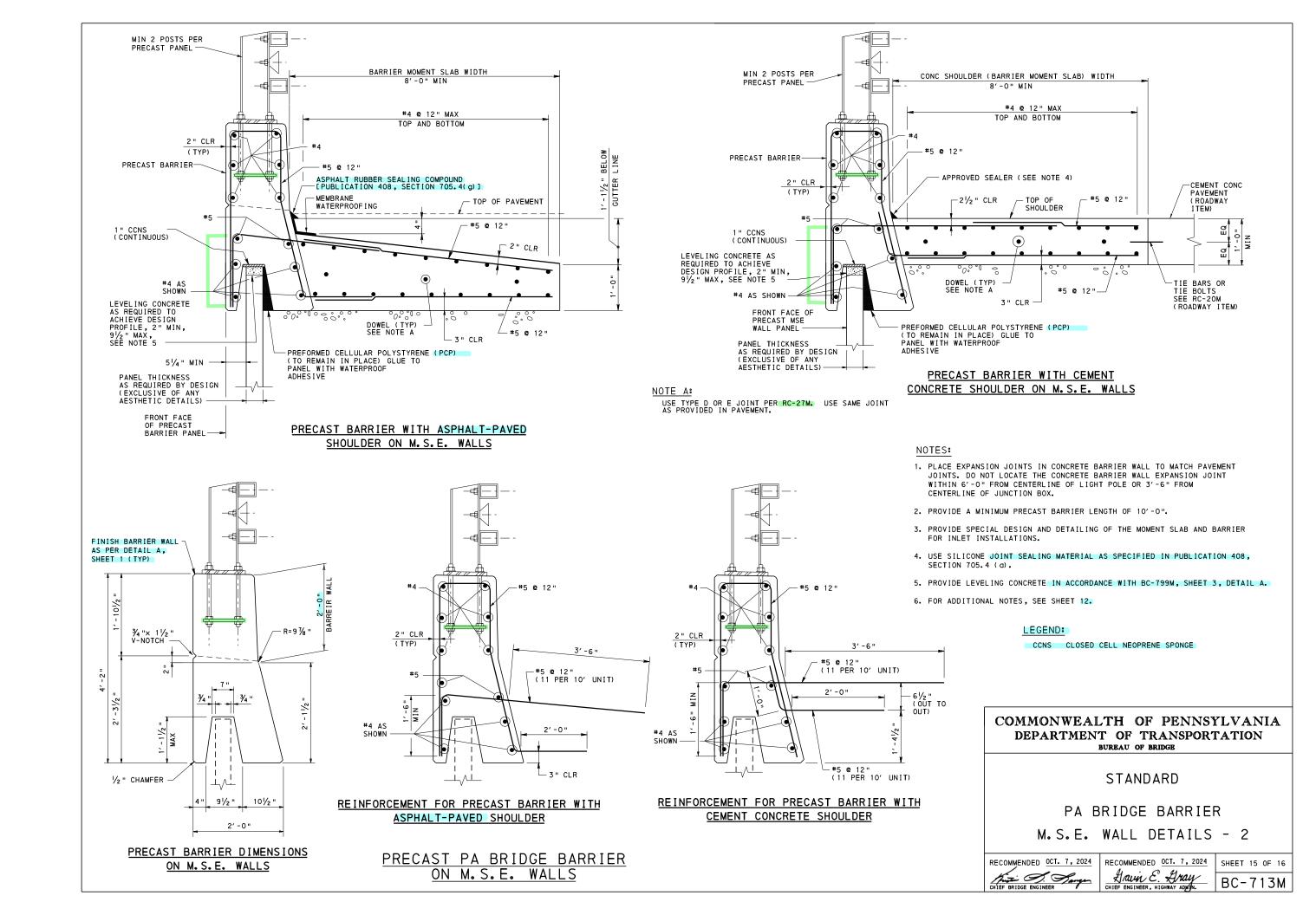
Havin E. Gray
HEF ENGINEER, HIGHWAY ADMIN. BC-713M

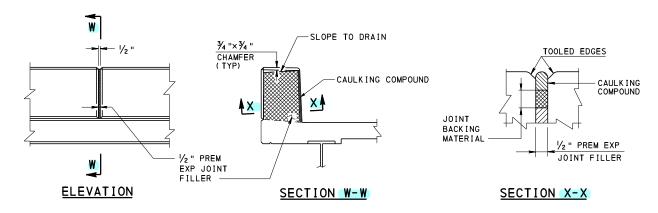
* IF 10" STUDS CANNOT BE ACCOMMODATED IN THE SPACE AVAILABLE, REQUEST SPECIFIC LENGTH APPROVAL FROM THE DISTRICT BRIDGE ENGINEER AT THE SHOP DRAWINGS APPROVAL STAGE.









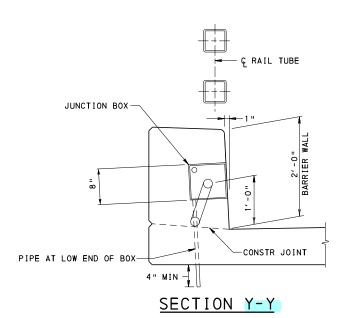


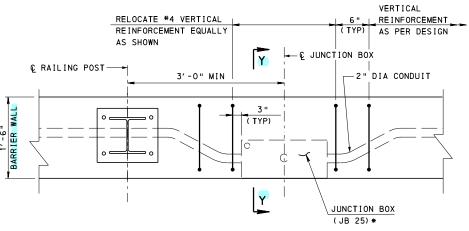
PA BRIDGE BARRIER AT OPEN JOINT

(RAILING POST AND TUBE NOT SHOWN)

JUNCTION BOX NOTES:

- 1. JUNCTION BOXES ARE ONLY REQUIRED, IF SPECIFIED ON THE CONTRACT DRAWINGS.
- 2. FOR TYPICAL SIDEWALK, PLACE JUNCTION BOX ON SIDEWALK SIDE.
- FOR RAISED SIDEWALK, PLACE JUNCTION BOX ON SIDEWALK SIDE ABOVE THE RAISED SIDEWALK.





PLAN

(RAIL TUBE NOT SHOWN) * JUNCTION BOX MAY BE LOCATED EITHER TO THE LEFT OR TO THE RIGHT OF THE LIGHTING POLE.

PA BRIDGE BARRIER ALTERNATE JUNCTION BOX DETAIL

OPEN JOINT NOTES:

- 1. FOR LOCATION OF CONSTRUCTION JOINTS AND OPEN JOINTS, REFER TO DESIGN DRAWINGS.
- PROVIDE CAULKING COMPOUND CONFORMING TO PUBLICATION 408, SECTION 705.7 (b).
- 3. PROVIDE JOINT BACKING MATERIAL CONFORMING TO PUBLICATION 408,
- 4. PROVIDE PREMOLDED EXPANSION JOINT FILLER CONFORMING TO PUBLICATION 408, SECTION 705.1.
- 5. PROVIDE 2" CLEAR ON ALL REINFORCEMENT UNLESS NOTED.
- 6. FOR ADDITIONAL NOTES, SEE SHEET 1.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD

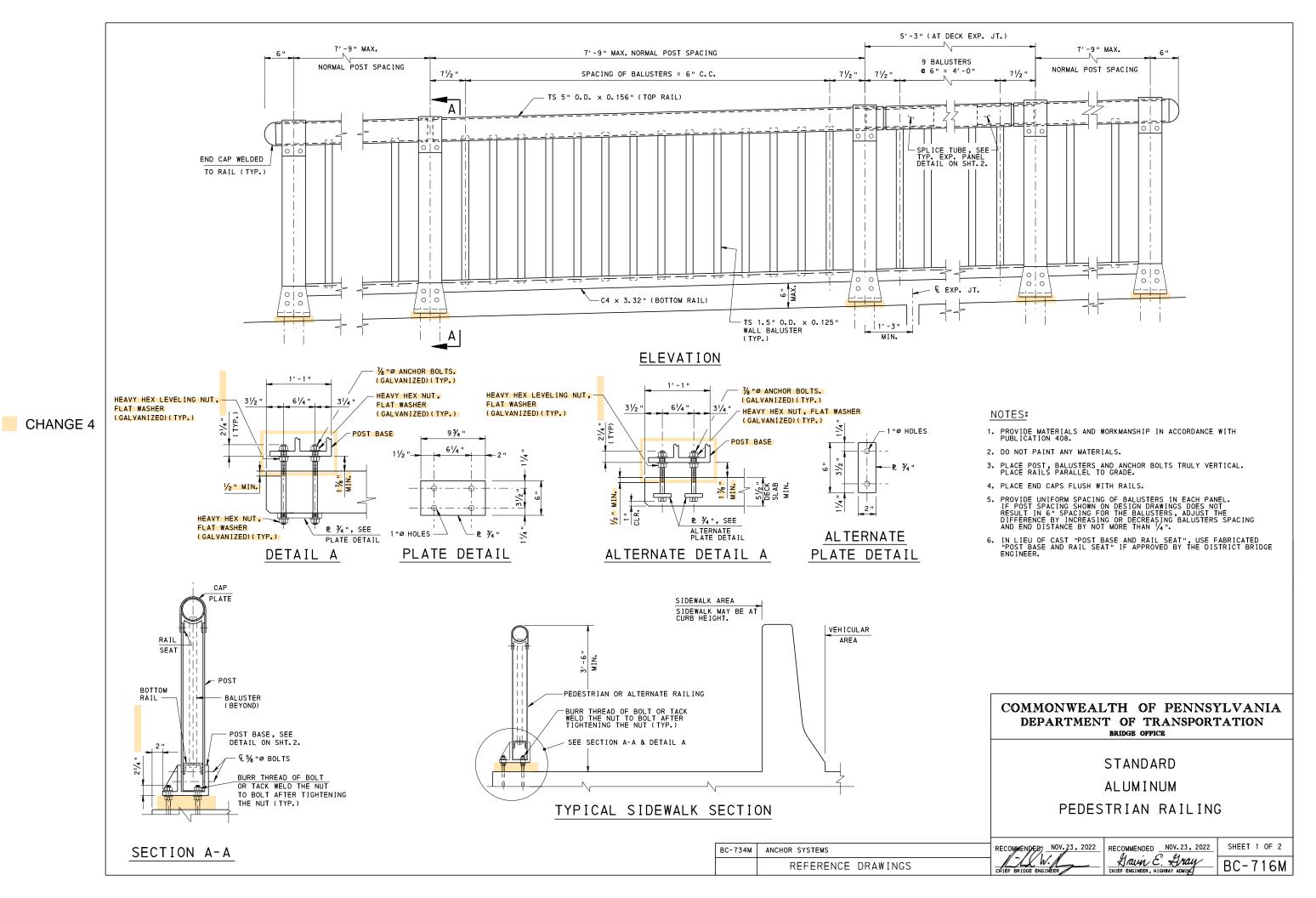
PA BRIDGE BARRIER MISCELLANEOUS DETAILS

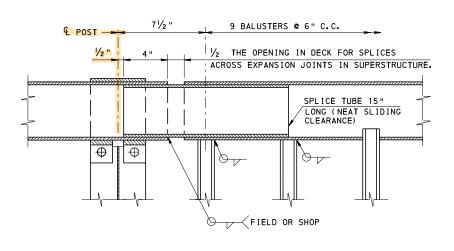
RECOMMENDED OCT. 7, 2024 CHIEF BRIDGE ENGINEER

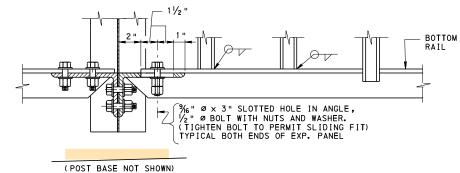
RECOMMENDED 0CT. 7, 2024 SHEET 16 OF 16 Havin E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN.

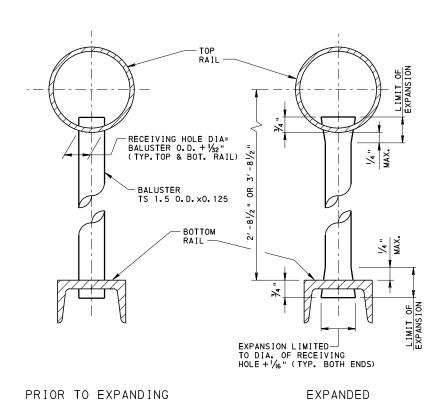
BC-713M





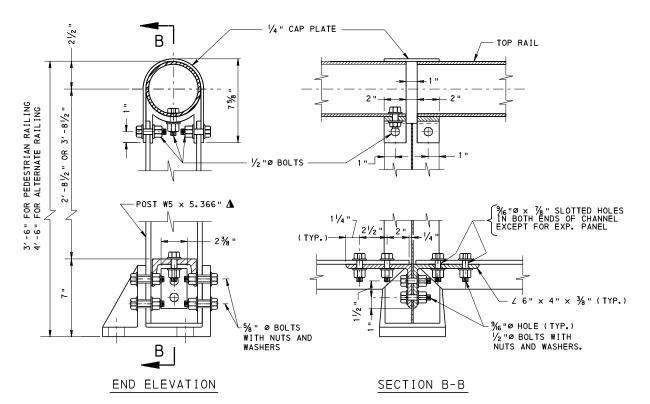


TYPICAL EXPANSION PANEL DETAIL



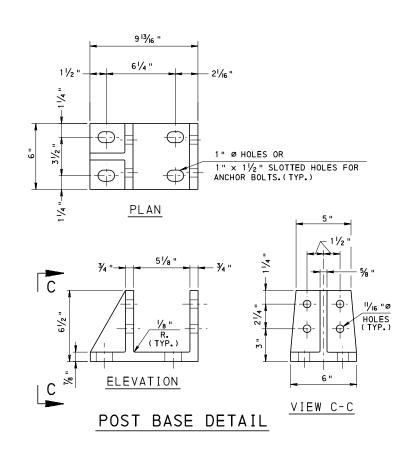
BALUSTER DETAILS

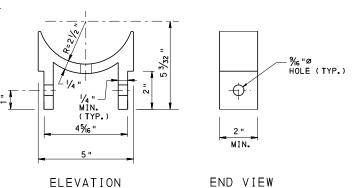
EXPAND FULL CIRCUMFERENCE OF BALUSTER WITHIN THE LIMIT OF EXPANSION.



TYPICAL DETAIL AT POST

▲ 2'-11 1/8 " LONG FOR 3'-6" RAILING HEIGHT 3'-11 1/8" LONG FOR 4'-6" RAILING HEIGHT





RAIL SEAT DETAIL

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD
ALUMINUM
PEDESTRIAN RAILING

RECOMMENDED NOV.23, 2022

RECOMMENDED NOV. 23, 2022 SHEET 2 OF 2

Haw E. Lyay

CHIEF ENGINEER, HICHNAY ADMIN

GENERAL NOTES:

- PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH PUBLICATION 408 AND APPLICABLE SPECIAL PROVISIONS.
- 2. MATERIAL STRENGTH: REINFORCEMENT STEEL fy = 60 KSI CONCRETE FOR BARRIERS f'c = 3.5 KSI (CLASS AA CONCRETE)
- PROVIDE STRUCTURAL STEEL CONFORMING TO AASHTO M270, GRADE 50, (ASTM A 709, GRADE 50) UNLESS OTHERWISE NOTED.
- 4. PROVIDE 1" DIA. ASTM F 1554 GRADE 105 OR ASTM A 193 GRADE B7 (105 KSI YIELD) ANCHOR BOLT, HOT-DIPPED GALVANIZED IN ACCORDANCE WITH THE REQUIREMENTS OF CLASS C OF ASTM A 153 FOR ALL BOLT THROUGH ANCHORS. ADDITIONAL REQUIREMENTS FOR ASTM F 1554 SUPPLEMENT SFOR ASTM F 1554 SUPPLEMENT S5 GRADE 105 CHARPY IMPACT REQUIREMENTS AT -20°F.
- 5. PROVIDE 1" DIA. ASTM A 193 GRADE B7 (105 KSI YIELD), HOT-DIPPED GALVANIZED ANCHOR BOLT IN ACCORDANCE WITH THE REQUIREMENTS OF CLASS C OF ASTM A 153 FOR ALL ADHESIVE ANCHORS. ADDITIONAL REQUIREMENT FOR ASTM A 193 INCLUDES ASTM F 1554 SUPPLEMENT S5 GRADE 105 CHARPY IMPACT REQUIREMENTS AT -20°F.
- 6. PROVIDE HEAVY HEX NUTS IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02 (c) 3. a OR ASTM A 194 GRADE 7, SUPPLEMENT 3 AT -20°F, HOT-DIPPED GALVANIZED IN ACCORDANCE WITH THE REQUIREMENTS OF CLASS C OF ASTM A 153. ZINC COATING, OVERTAPPING OF THE NUT, AND LUBRICATION REQUIREMENTS SHALL BE IN ACCORDANCE WITH ASTM A 563.
- 7. PROVIDE 3" DIAMETER × 1/4" THICK PLATE WASHER (ASTM A709, GRADE 36) WITH A 1 1/8" DIAMETER HOLE IN THE MIDDLE, ALL OTHER WASHERS ARE IN ACCORDANCE WITH ASTM F 436 TYPE 1. WASHERS ARE TO BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH THE REQUIREMENTS OF CLASS C OF ASTM A 153.
- 8. THE FOLLOWING TWO TYPES OF INSTALLATIONS ARE PERMITTED:

TYPE 1: TRAFFIC PRESENT ON ONE SIDE OF THE TEMPORARY BARRIER. PROVIDE ANCHORS ON THE TRAFFIC SIDE OF THE TEMPORARY BARRIER.

TYPE 2: TRAFFIC PRESENT ON BOTH SIDES OF THE TEMPORARY BARRIER.
PROVIDE ANCHORS ON BOTH SIDES OF THE TEMPORARY BARRIER. THE
42" TEMPORARY SINGLE FACE CONCRETE BARRIER IS NOT PERMITTED
FOR TYPE 2 INSTALLATIONS.

9. MASH DESIGNATIONS:

CHANGE 2

CHANGE 6

- THE 32" TEMPORARY CONCRETE MEDIAN BARRIER WITH 4'-0" (MAXIMUM) ANCHOR SPACINGS IS DESIGNATED AS MASH TL-3.
- THE 50" TEMPORARY CONCRETE MEDIAN BARRIER WITH 2'-0" (MAXIMUM) ANCHOR SPACING IS DESIGNATED AS MASH TL-4.
- THE 42" TEMPORARY SINGLE FACE CONCRETE BARRIER WITH 2'-0" (MAXIMUM) ANCHOR SPACINGS IS DESIGNATED AS MASH TL-4.
- 10. INDICATE INSTALLATION TYPE ON THE CONTRACT DRAWINGS.
- 11. ANCHORS MUST BE INSTALLED IN THE END HOLES OF EACH BARRIER SEGMENT. KEEP ANCHOR SPACING UNIFORM ALONG THE FULL LENGTH OF THE BRIDGE TO THE EXTENT POSSIBLE.
- 12. WHEN USING ADHESIVE ANCHORS FOR THE FACE(S) OF BARRIERS ADJACENT TO TRAFFIC, INSTALL ANCHORS TO SATISFY THE SPACING AND STRENGTH REQUIREMENTS OF TABLE 1.
- 13. ADHESIVE ANCHORS MAY BE USED FOR ALL INSTALLATIONS EXCEPT WHERE THE DECK HAS CONCRETE STRENGTHS LESS THAN 3000 PSI, IS IN POOR CONDITION AND/OR ADEQUATE PULL OUT CANNOT BE ACHIEVED AS PER TABLE 2 ON SHEET 2. BOLT THROUGH ANCHORS MUST BE USED IF SPECIFICALLY INDICATED ON CONTRACT DRAWINGS. BOLT THROUGH ANCHORS MUST ALSO BE USED IF THE DECK IS PENETRATED DURING THE DRILLING PROCESS.
- 14. ADHESIVE ANCHORS FOR TEMPORARY BARRIERS ARE PERMITTED ON BRIDGE CONSTRUCTION PROJECTS THAT EXTEND CONTINUOUSLY FOR A MAXIMUM OF THREE YEARS.
- 15. IDENTIFY THE PLAN LOCATION OF THE BARRIER ON THE BRIDGE DECK. POSITION BARRIER SEGMENTS TO LOGICALLY ACCOMMODATE THE ENDS OF THE STRUCTURE, EXPANSION DAMS AND OTHER OBSTACLES.
- 16. TRAFFIC TRANSITIONS AND LANE MERGING MUST BE OFF THE BRIDGE.
- 17. BOLT THROUGH ANCHORS ARE NOT PERMITTED IN RECENTLY POURED DECKS WITHOUT APPROVAL OF DISTRICT BRIDGE ENGINEER.
- 18. FOR DEAD LOAD CALCULATIONS, THE WEIGHT OF THE BARRIERS ARE AS FOLLOWS:
 - 32" TEMPORARY CONCRETE MEDIAN BARRIER = 490 LB/FT
 - 50" TEMPORARY CONCRETE MEDIAN BARRIER = 840 LB/FT
 - 42" TEMPORARY SINGLE FACE CONCRETE BARRIER = 690 LB/FT
- 19. THE TEMPORARY BARRIER WEIGHT DOES NOT COUNT AGAINST THE CONSTRUCTION LOADING LIMITS AS SPECIFIED IN PUBLICATION 408, SECTION 105.17. HOWEVER, THE TEMPORARY BARRIER WEIGHT IS TO BE INCLUDED WITH THE FACTORED DEAD LOAD IN DETERMINING THE CONSTRUCTION LOAD CAPACITIES.
- 20. FOR DELINEATION AND BARRIER LINE PAINTING, SEE PUBLICATION 111, STANDARD DRAWING TC-8604.

TABLE 1

MINIMUM REQUIRED ADHESIVE ANCHOR ULTIMATE CAPACITY BASED ON CONCRETE AND BOND STRENGTH

BARRIER TYPE	ANCHOR	ANCHOR LOADS (KIPS)	
	SPACING	SHEAR	TENSION
32" MEDIAN BARRIER	4′-0"	8	14
50" MEDIAN BARRIER	2'-0"	11	22
42" SINGLE-FACE BARRIER	2'-0"	10	21

SHEAR AND TENSION VALUES ARE MINIMUM CAPACITIES REQUIRED. IF BOTH VALUES ARE NOT MET OR EXCEEDED BY THE ANCHOR PROVIDED, A CLOSER SPACING MUST BE

> COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD TEMPORARY CONCRETE BARRIER, STRUCTURE MOUNTED GENERAL NOTES

TC-8604 DELINEATION

REFERENCE DRAWINGS

RECOMMENDED MAR. 27, 2024

SHEET 1 OF 7

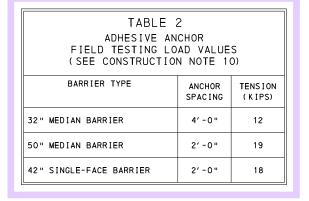
RECOMMENDED MAR. 27, 2024

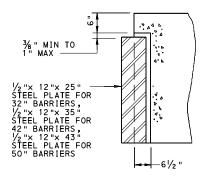
Club & Size BC-719M

ACTING CHIEF ENGINEER, HIGHEAY ADMIN.

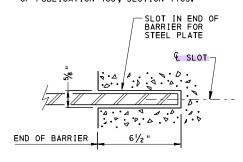
CONSTRUCTION NOTES:

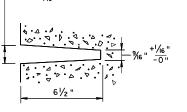
- DRILL BY MEANS WHICH WILL NOT DAMAGE THE ADJACENT CONCRETE. SUPPORT BENEATH THE DECK AS NECESSARY TO AVOID SPALLING OF CONCRETE FOR BOLT THROUGH AND ADHESIVE ANCHORS HOLES.
- 2. DRILL INTO THE DECK USING THE HOLES IN THE TEMPORARY BARRIER AS A TEMPLATE. THE DRILL MAY BE LOCATED ANYWHERE WITHIN THE 2" SLOT, BUT MUST BE MAINTAINED VERTICALLY ±1°.
- THE BARRIERS MAY BE REPOSITIONED TO AVOID DAMAGING THE DECK REINFORCEMENT DURING DRILLING. MOVE THE BARRIER PARALLEL TO THE DIRECTION OF TRAFFIC UP TO 2" AND PERPENDICULAR TO TRAFFIC UP TO 1"; HOWEVER, A SMOOTH BARRIER FACE MUST ALWAYS BE PRESENTED TO TRAFFIC. IF BARRIERS CANNOT BE REPOSITIONED AND REBAR IS ENCOUNTERED, MOVE TO ALTERNATE BOLT POCKETS. FOR EXISTING DECKS, DRILLING THROUGH DECK REINFORCEMENT STEEL IS PERMITTED ONLY IF THE DECK IS DEMOLISHED IN A LATER STAGE OF CONSTRUCTION. FOR NEW DECKS, PROPERLY PLAN AND PLACE DECK REINFORCEMENT STEEL TO AVOID DAMAGE DURING DRILLING.
- MATCH THE ENDS OF THE SEGMENTS WITH THE LOCATION OF THE EXPANSION DAMS AS CLOSELY AS POSSIBLE. BOLTING OF A SEGMENT ON EACH SIDE OF AN EXPANSION DAM IS NOT PERMITTED. FOR OTHER OBSTACLES THAT DO NOT INVOLVE MOVEMENT, SUCH AS SCUPPERS, BOLTING A SEGMENT ON EACH SIDE OF THE OBSTACLE IS
- THE END SEGMENT OF THE TEMPORARY BARRIER AT THE END OF THE BRIDGE MAY EXTEND PARTIALLY OFF THE BRIDGE. CONNECT THE END OF THE SEGMENT OFF THE BRIDGE TO THE ADJACENT SEGMENT OF THE ROADWAY BARRIER. POSITION BARRIER SEGMENTS SUCH THAT THE LARGEST POSSIBLE PORTION OF THE END SEGMENT IS PLACED ON THE BRIDGE. INSTALL ANCHORS AT THE SAME SPACING USED ON THE
- 6. TREATMENT OF ANCHOR HOLES AFTER REMOVAL OF TEMPORARY BARRIERS:
 - FOR ADHESIVE ANCHORS MOUNTED ON NEW DECKS AND EXISTING DECKS THAT WILL NOT BE DEMOLISHED IN A LATER STAGE OF CONSTRUCTION, CORE THE ANCHORS TO COMPLETELY REMOVE THE ANCHOR AFTER THE REMOVAL OF THE TEMPORARY BARRIER AND FILL THE HOLE WITH GROUT IN ACCORDANCE WITH PUBLICATION 408, SECTION 1080.2 (c).
 - FOR ADHESIVE ANCHORS INSTALLED USING A MANUFACTURERS RELEASING AGENT, THE CONTRACTOR MAY REMOVE THE ANCHOR. REDRILL THE HOLE TO REMOVE THE EPOXY USING THE SAME SIZE HOLE WHEN INSTALLING THE ADHESIVE ANCHOR AND FILL THE HOLE WITH GROUT IN ACCORDANCE WITH PUBLICATION 408, SECTION
 - FOR ADHESIVE ANCHORS MOUNTED ON EXISTING DECKS THAT WILL BE DEMOLISHED
 IN A LATER STAGE OF CONSTRUCTION, CUT THE PROJECTION OF THE ANCHOR
 ABOVE THE DECK AND GRIND SMOOTH AND FLUSH WITH THE TOP SURFACE OF THE
 DECK IMMEDIATELY AFTER TEMPORARY BARRIER REMOVAL.
 - FOR BOLT THROUGH ANCHORS MOUNTED ON NEW DECKS OR MOUNTED ON EXISTING DECKS THAT WILL BE REOPENED TO TRAFFIC AFTER TEMPORARY BARRIER REMOVAL, FILL THE HOLES WITH RAPID SET CONCRETE PATCHING MATERIAL (TYPE C) AS LISTED IN BULLETIN 15, SECTION 679.2(a)5, AFTER THE REMOVAL OF THE TEMPORARY BARRIER.
- 7. THE MINIMUM DECK WIDTH BEHIND A TEMPORARY BARRIER MOUNTED SUCH THAT TRAFFIC EXISTS ALONG ONE FACE IS 2" FOR DECKS WITHOUT OVERLAYS AND 12" FOR DECKS WITH AN OVERLAY. ADDITIONAL OFFSET MAY BE IDENTIFIED ON THE CONTRACT DRAWINGS, IF PRACTICAL, TO ALLOW CONTRACTOR ACCESS FOR PARTIAL WIDTH CONSTRUCTION.
- 8. ANCHORS ARE REQUIRED ON THE TRAFFIC SIDE OF THE TEMPORARY BARRIER.
- 9. FIELD TEST LOADING VALUES ARE 85% OF THE ADHESIVE ANCHOR TENSILE





1. PROVIDE STEEL PLATES MEETING THE REQUIREMENTS OF PUBLICATION 408, SECTION 1105.





ELEVATION - SLOT DETAIL

PARTIAL PLAN - SLOT DETAIL

PERMISSIBLE TAPER

SLOTTED PLATE CONNECTION

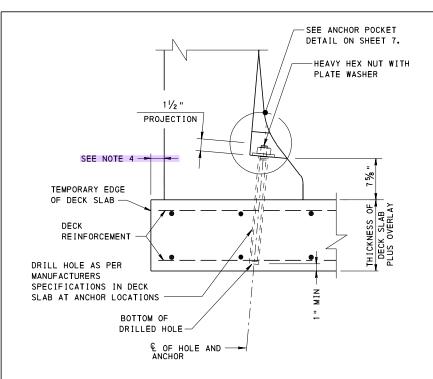
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD TEMPORARY CONCRETE BARRIER. STRUCTURE MOUNTED CONSTRUCTION NOTES AND SLOTTED PLATE CONNECTION

RECOMMENDED MAR. 27, 2024

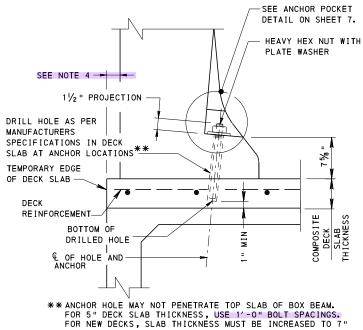
RECOMMENDED MAR. 27, 2024 ACTING CHIEF ENGINEER, HIGHRAY ADMIN. BC-719M

SHEET 2 OF 7



ADHESIVE ANCHOR

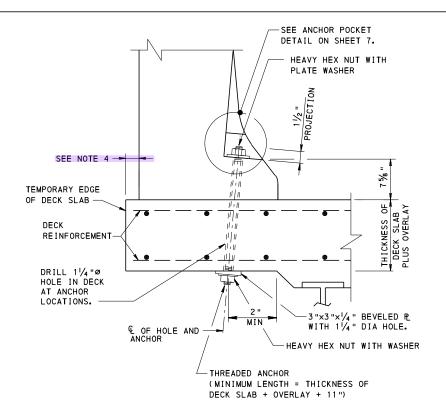
(TEMPORARY SINGLE FACE BARRIER SHOWN; TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC)



FOR 5" DECK SLAB THICKNESS, USE 1'-0" BOLT SPACINGS. FOR NEW DECKS, SLAB THICKNESS MUST BE INCREASED TO 7' TO BE ABLE TO USE ALL INSTALLATION OPTIONS LISTED IN TABLE 1, ON SHEET 1.

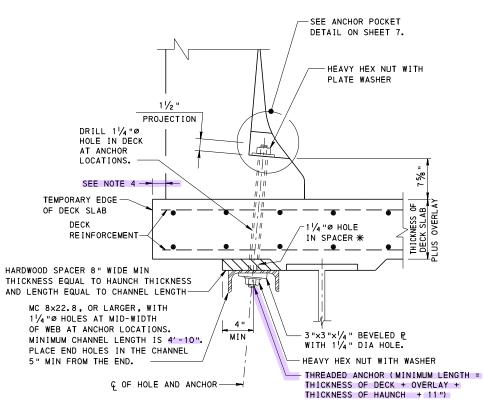
ADHESIVE ANCHOR ON COMPOSITE ADJACENT BOX BEAMS

(TEMPORARY SINGLE FACE BARRIER SHOWN;
TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC)



TYPICAL BOLT THROUGH ANCHOR

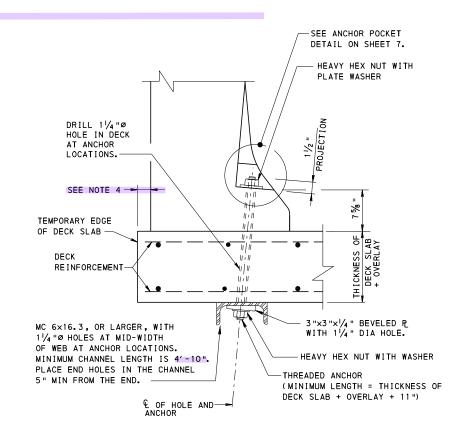
(TEMPORARY SINGLE FACE BARRIER SHOWN;
TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC)



ALTERNATE CONNECTION DETAIL WITH SPACER

FOR HAUNCH CLEARANCE LESS THAN 2"

TEMPORARY MEDIAN BARRIER SIMILAR AT FACE NEAR HAUNCH) **THE ENTIRE CIRCUMFERENCE OF THE HOLE IN THE HARDWOOD SPACER MUST BE $\frac{1}{2}$ " MIN FROM THE EDGES OF THE SPACER.



ALTERNATE BOLT THROUGH ANCHOR

(TEMPORARY SINGLE FACE BARRIER SHOWN;
TEMPORARY MEDIAN BARRIER SIMILAR AT FACE(S) ADJACENT TO TRAFFIC)

NOTE: USE THE ALTERNATE BOLT THROUGH ANCHOR INSTALLATION FOR DECKS WITH METAL DECK PANS AND WHEN THE EXISTING DECK IS DETERIORATED OR THE ALTERNATE INSTALLATION IS REQUIRED BY THE DISTRICT BRIDGE ENGINEER.

NOTES:

- 1. FOR GENERAL NOTES, SEE SHEET 1.
- 2. FOR CONSTRUCTION NOTES, SEE SHEET 2.
- FOR SPACING AND MINIMUM REQUIRED ADHESIVE ANCHOR ULTIMATE CAPACITY SEE TABLE 1, SHEET 1.
- 4. REFER TO CONSTRUCTION NOTE 7 ON SHEET 2.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD
TEMPORARY CONCRETE BARRIER,
STRUCTURE MOUNTED
ADHESIVE AND BOLT THROUGH
ANCHOR DETAILS

RECOMMENDED MAR. 27, 202

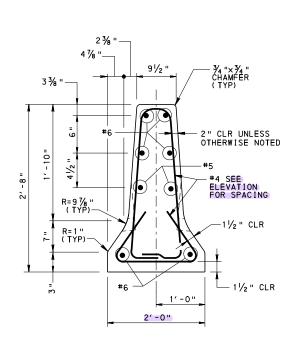
RECOMMENDED MAR. 27, 2024

Clitt L ACTING CHIEF ENGINEER, HIGHWAY ADMIN.

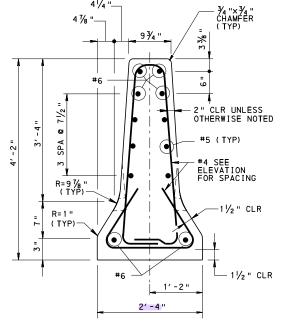
BC-719M

14'-0" MIN ANCHOR POCKETS AND HOLES @ 1'-0" MAX & TO & 4 " _4 "_ |< >| 8" (TYP) FOR REINFORCEMENT DETAILS SEE SECTION A-A, THIS SHEET ΙВ Ç ANCHOR POCKET ⊢ 1 ½ " (TYP) AND ¢ 11/4 "x2" Ø SLOT. SEE ANCHOR POCKET DETAIL ON SHEET 7. В FOR REINFORCEMENT AT SLOTTED PLATE CONNECTION, SHEET 5 (TYP) BARRIER DRAINAGE OPENING (TYP) (SEE DETAIL THIS SHEET) SLOTTED PLATE **ELEVATION** CONNECTION (TYP)

TRAFFIC FACE OF SINGLE FACE TEMPORARY CONCRETE BARRIER AND BOTH FACES OF TEMPORARY CONCRETE MEDIAN BARRIER

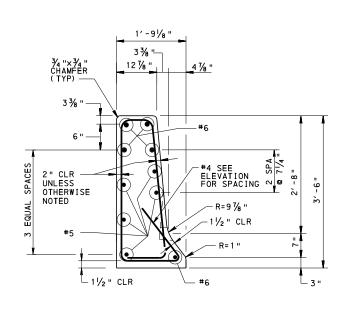


32" TEMPORARY CONCRETE MEDIAN BARRIER

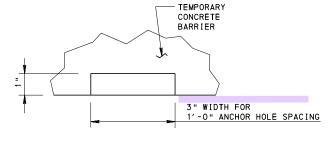


50" TEMPORARY CONCRETE MEDIAN BARRIER

SECTION A-A



42" TEMPORARY SINGLE FACE CONCRETE BARRIER



BARRIER DRAINAGE OPENING DETAIL

NOTE: USE BARRIER DRAINAGE OPENINGS FOR ALL TEMPORARY CONCRETE BARRIERS.

BARRIER DRAINAGE NOTES:

- 1. LOCATE OPENINGS MID-WAY BETWEEN ANCHOR POCKETS.
- 2. PROVIDE A MINIMUM OF SIX (6) OPENINGS PER 14'-0" SEGMENT LENGTH.

NOTES:

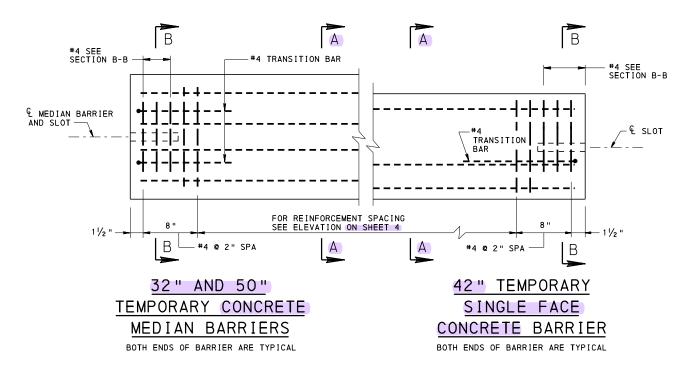
- 1. FOR GENERAL NOTES, SEE SHEET 1.
- 2. FOR CONSTRUCTION NOTES, SEE SHEET 2.
- 3. FOR SECTION B-B, SEE SHEET 5.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD TEMPORARY CONCRETE BARRIER, STRUCTURE MOUNTED REINFORCEMENT DETAILS - 1

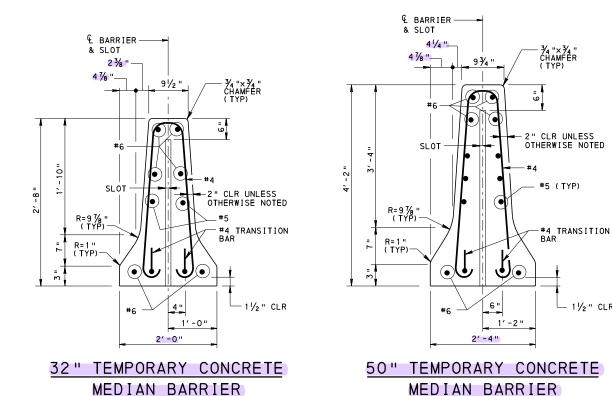
RECOMMENDED MAR. 27, 2024 Clint & BC-719M

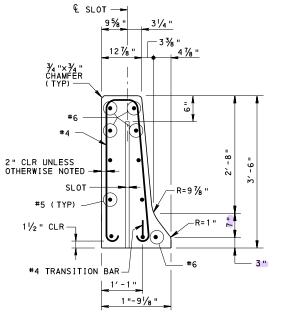
ACTING CHIEF ENGINEER, HIGHRAY ADMIN.



REINFORCEMENT AT SLOTTED PLATE CONNECTION - PLAN

(ANCHOR POCKETS NOT SHOWN)





42" TEMPORARY SINGLE FACE CONCRETE BARRIER

SECTION B-B

└ 1½" CLR

NOTES:

- 1. FOR GENERAL NOTES, SEE SHEET 1.
- 2. FOR CONSTRUCTION NOTES, SEE SHEET 2.
- 3. FOR BARRIER ELEVATION AND SECTION A-A, SEE SHEET 4.

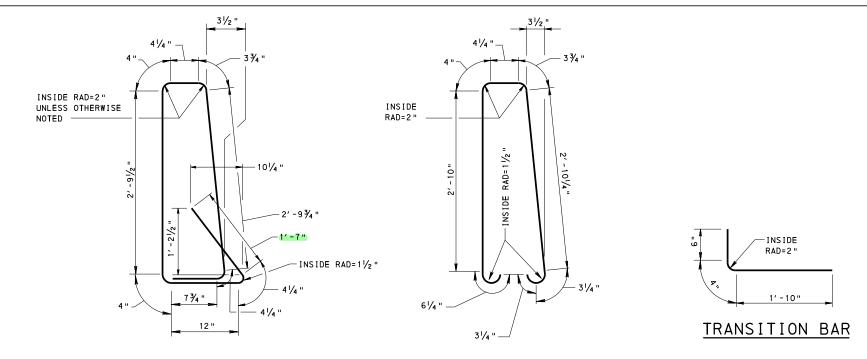
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

STANDARD TEMPORARY CONCRETE BARRIER, STRUCTURE MOUNTED REINFORCEMENT DETAILS - 2

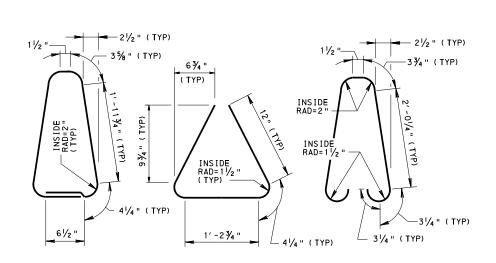
RECOMMENDED MAR. 27, 2024 SHEET 5 OF 7

Clitt L
ACTING CHIEF ENGINEER, HIGHWAY ADMIN.

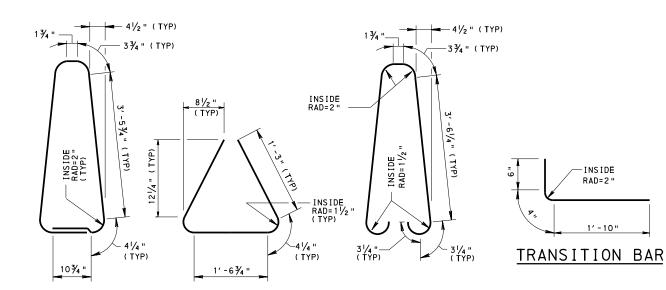
BC-719M



42" TEMPORARY SINGLE FACE BARRIER REINFORCEMENT BARS







50" TEMPORARY MEDIAN BARRIER

TEMPORARY MEDIAN BARRIER REINFORCEMENT BARS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

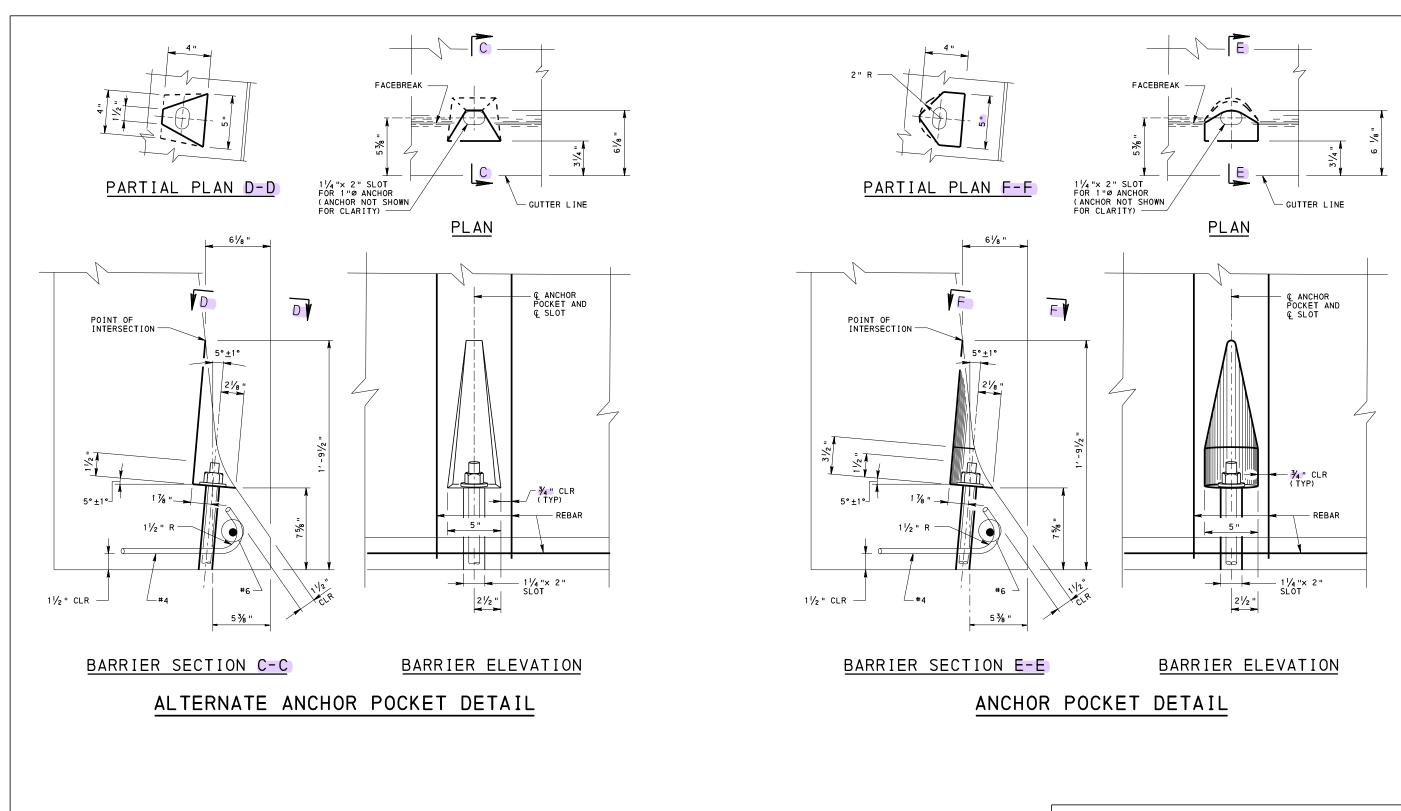
STANDARD TEMPORARY CONCRETE BARRIER, STRUCTURE MOUNTED BAR BENDING DIAGRAMS

-INSIDE RAD=2"

RECOMMENDED MAR. 27, 2024 SHEET 6 OF 7

Clitt L SHEET 6 OF 7

ACTING CHIEF ENGINEER, HIGHWAY ADMIN. BC-719M



NOTES:

- 1. FOR GENERAL NOTES, SEE SHEET 1.
- 2. FOR CONSTRUCTION NOTES, SEE SHEET 2.
- 3. FOR LOCATION OF ANCHOR POCKET, SEE SHEET 4.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BURBAU OF BRIDGE

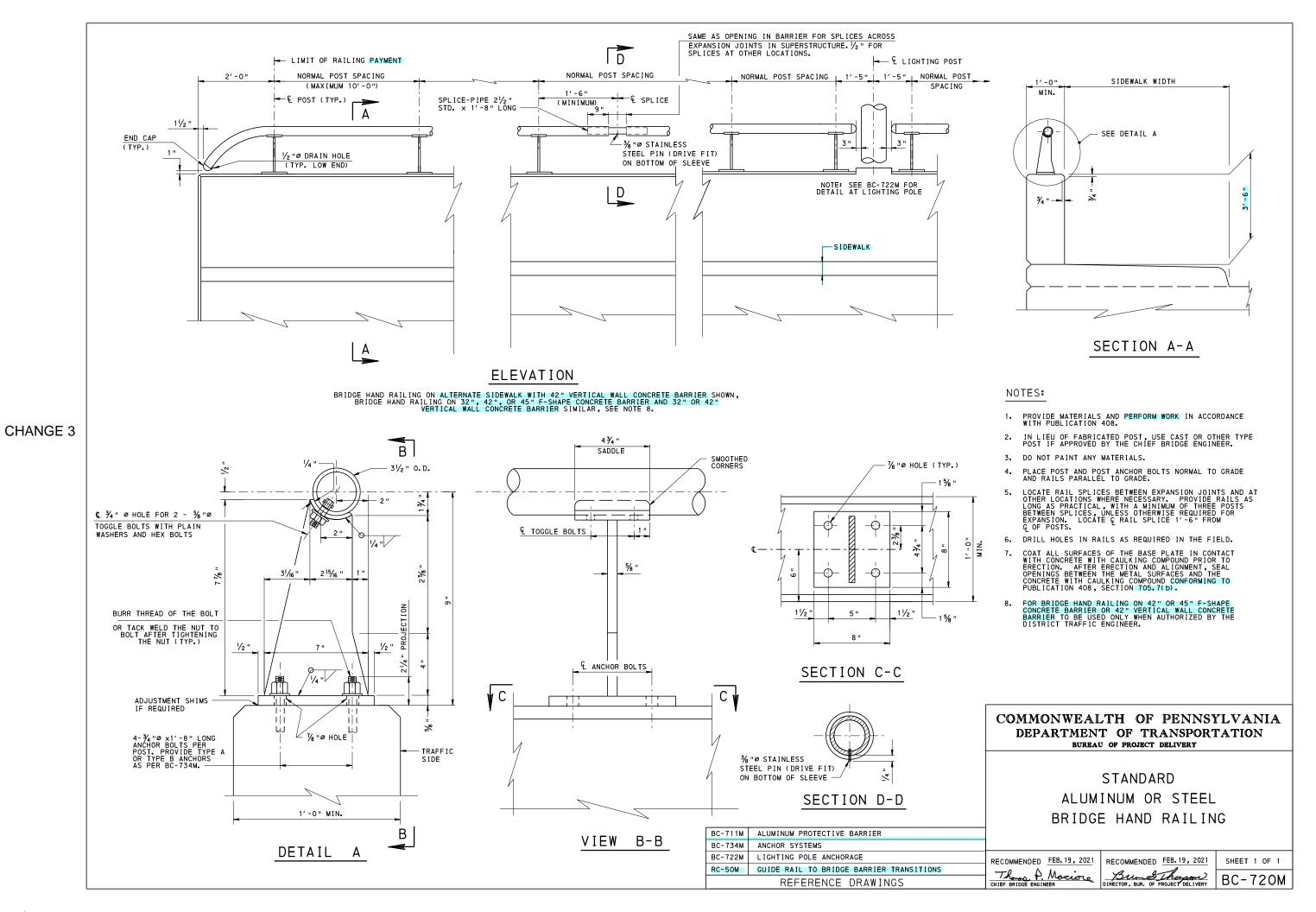
STANDARD
TEMPORARY CONCRETE BARRIER,
STRUCTURE MOUNTED
ANCHOR POCKET DETAILS

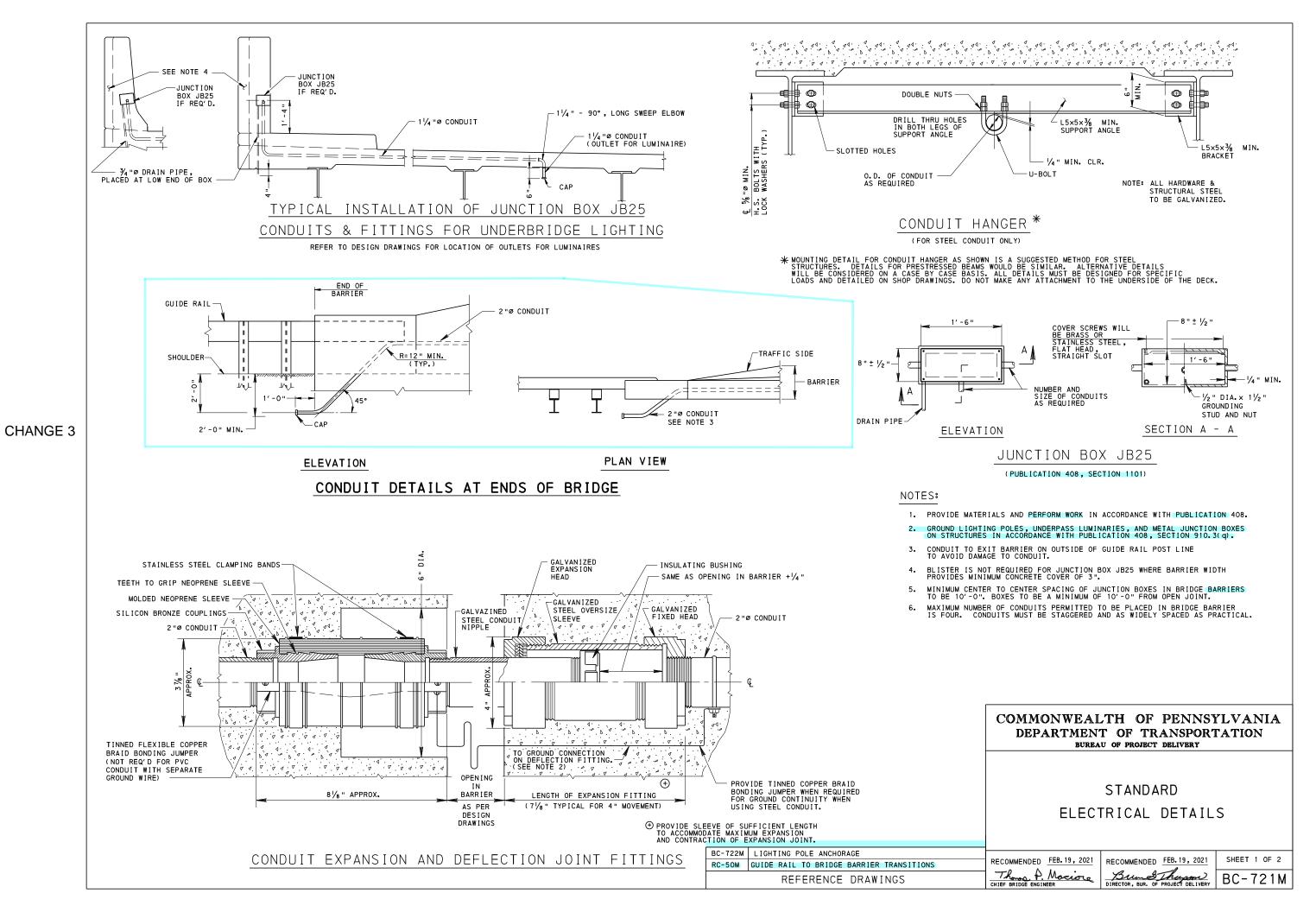
RECOMMENDED MAR. 27, 2024

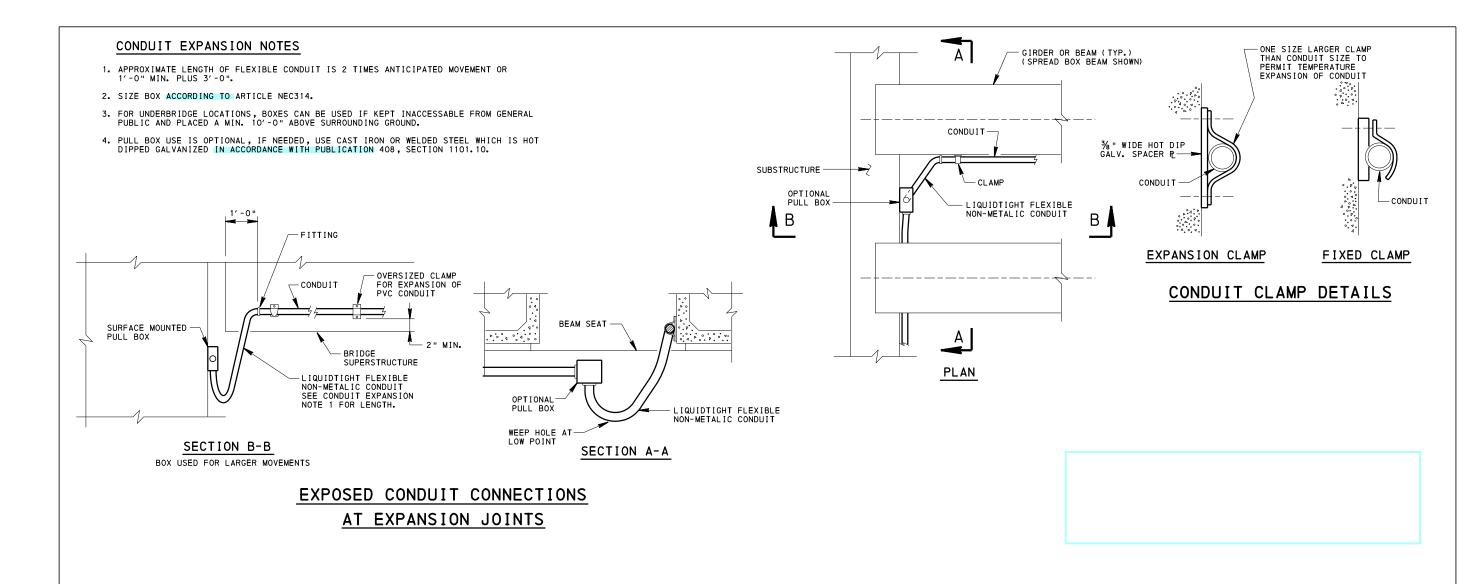
RECOMMENDED MAR. 27, 2024 SHEET 7 OF 7

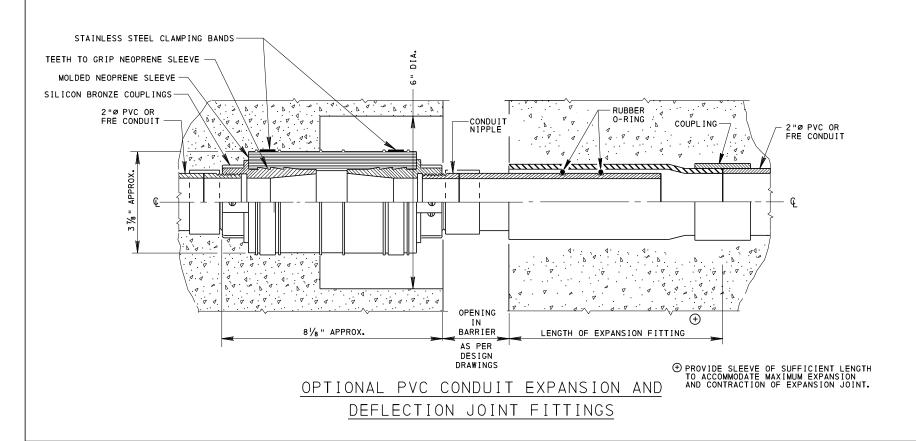
Clitt L SHEET 7 OF 7

ACTING CHIEF ENGINEER, HIGHWAY ADMIN. BC-719M









COMMONWEALTH OF PENNSYLVANIA
DEPARTMENT OF TRANSPORTATION
BUREAU OF PROJECT DELIVERY

STANDARD
ELECTRICAL DETAILS

RECOMMENDED FEB. 19, 2021

Thomas D. Macione
CHIEF BRIDGE ENGINEER

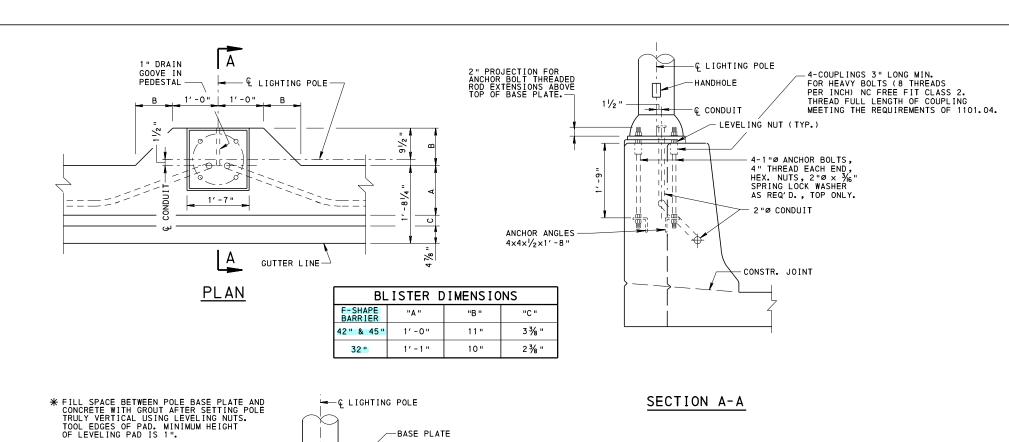
RECOMMENDED FEB. 19, 2021

Bund Thurst

DIRECTOR, BUR. OF PROJECT DELIVERY

SHEET 2 OF 2

BC-721M



CHANGE 3

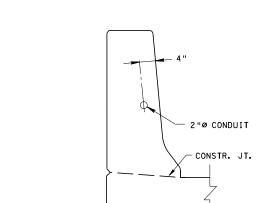
-2" DIA. CONDUIT (TYP.)

EXPANSION

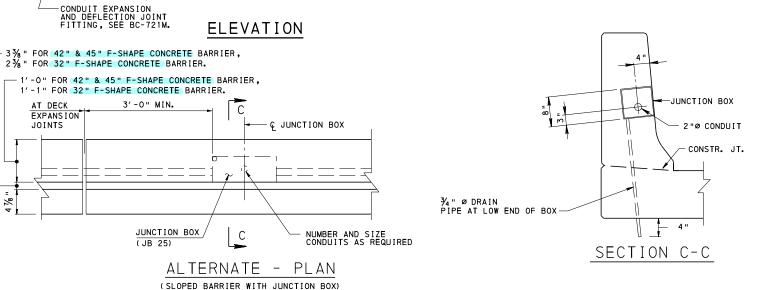
JOINTS

B

R= 9" MIN. (TYP.)



SECTION B-B



- CONCRETE DECK

-BASE PLATE ─[₩] GROUT

GUTTER LINE

NOTES:

- 1. PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH PUBLICATION 408.
- 2. SET ANCHOR BOLTS ACCURATELY BY THE TEMPLATE FURNISHED BY THE MANUFACTURER, TO THE CORRECT ELEVATION AND ALIGNMENT AND SECURELY BRACE AGAINST DISPLACEMENT BEFORE THE SURROUNDING CONCRETE IS PLACED. ANCHOR BOLT DIAMETER AS REQUIRED BY LIGHTING POLE MANUFACTURER. (FOR FUTURE LIGHTING PROVISIONS, SEE CHART ON THIS SHEET.)
- LOCATE JUNCTION BOX ON SIDEWALK SIDE OF BARRIER WHEN APPLICABLE AND PROVIDE TAMPER RESISTANT SCREWS.
- 4. ORIENT HAND HOLES FOR BARRIER MOUNTED POLES TOWARD THE ROADWAY; EXCEPT WHEN THERE IS SIDEWALK ACCESS, ORIENT TOWARD SIDEWALK.
- 5. SEAL CONDUIT AND PROTECT THREADS FOR FUTURE LIGHTING INSTALLATIONS.
- 6. CONFORM ANCHOR MATERIALS TO SECTION 1104.04 OF PUB. 408. ANCHOR ANGLES ARE PERMITTED TO BE GALVANIZED.
- 7. SET LIGHTING POLES TRULY VERTICAL WITH BASES LEVEL USING LEVELING NUTS.
- 8. PROVIDE 2" CLEAR ON ALL REINFORCEMENT UNLESS NOTED.
- 9. PROVIDE A MINIMUM OF 21/2" CONCRETE COVER

PROVISIONS	FOR FUTURE	LIGHTING	
MOUNTING HEIGHT	ANCHOR BOLT CIRCLE DIA.	ANCHOR BOLT DIAMETER	
50'-0" MAX.	15 "	1 "	

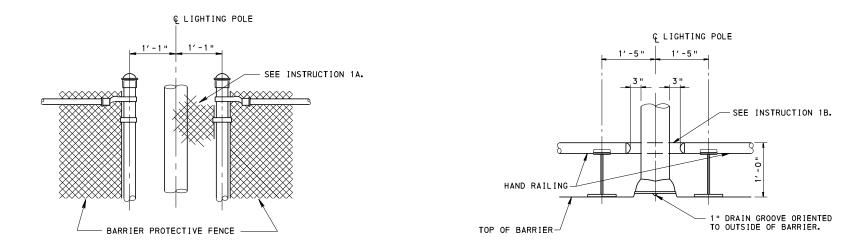
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD LIGHTING POLE ANCHORAGE

BC-721M ELECTRICAL DETAILS Thomas P. Macione
CHIEF BRIDGE ENGINEER REFERENCE DRAWINGS

Bund Theyard
DIRECTOR, BUR. OF PROJECT DELIVERY

RECOMMENDED FEB. 19, 2021 RECOMMENDED FEB. 19, 2021 BC-722M



PEDESTRIAN RAILING / FENCE / HAND RAILING AT LIGHTING POLE

INSTRUCTIONS FOR FUTURE LIGHTING

- 1. IF LIGHTING POLES ARE TO BE INSTALLED AT A FUTURE TIME.
 - A. PLACE RAILING POSTS AS SHOWN AND CLOSE GAPS WITH A SEPARATE PIECE OF FABRIC.

B. PLACE RAILING POSTS AS SHOWN BUT DO NOT INTERRUPT RAILING.

NOTE:

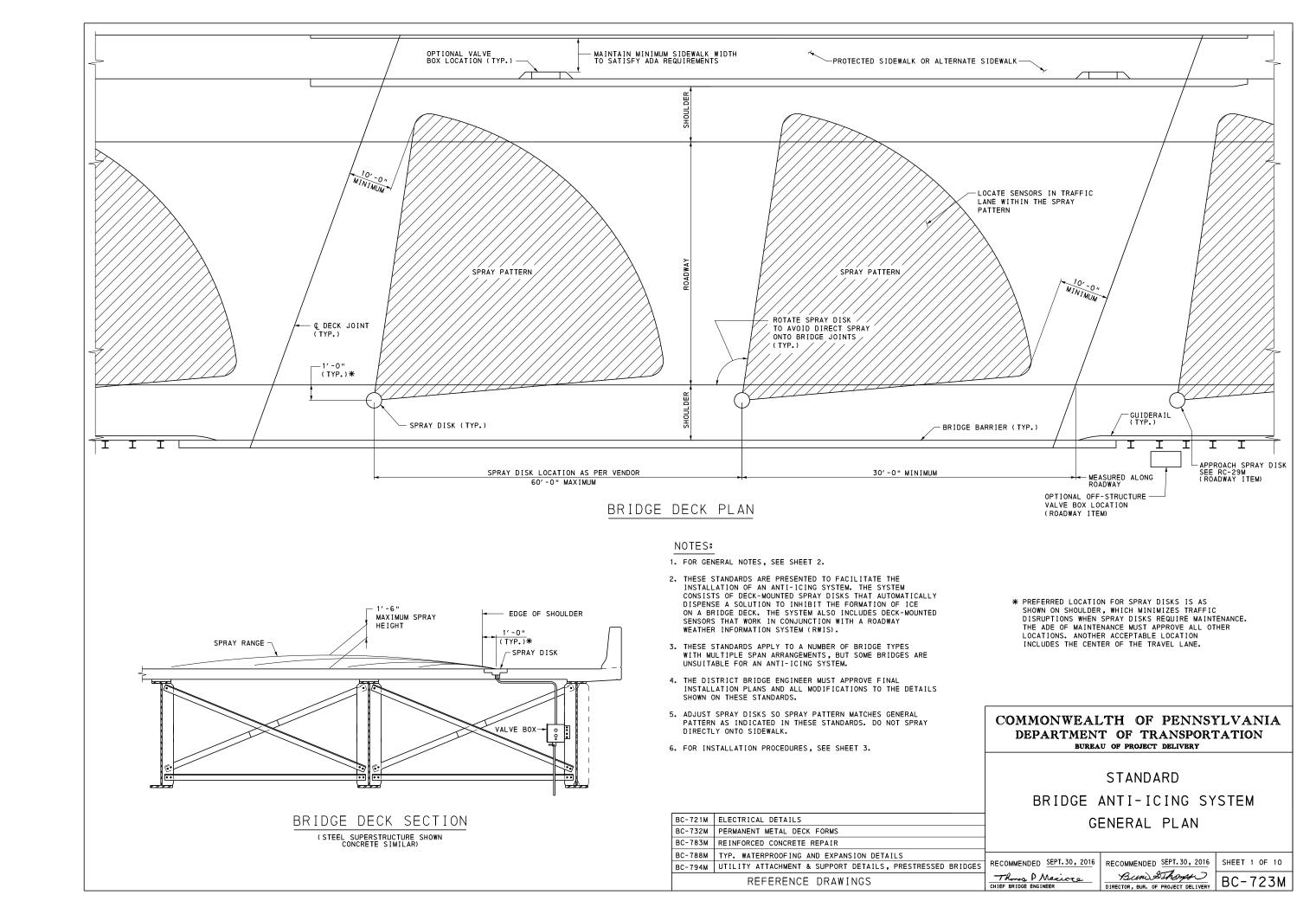
SEE SHEET 1 FOR NOTES.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

> STANDARD LIGHTING POLE ANCHORAGE

RECOMMENDED FEB. 19, 2021 RECOMMENDED FEB. 19, 2021 Thomas A. Macione
CHIEF BRIDGE ENGINEER

Bund Thurson BC-722M



GENERAL NOTES:

- 1. PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH PUB. 408 AND AASHTO/AWS D1.5 SPECIFICATIONS.
- 2. IT IS THE RESPONSIBILITY OF THE DESIGNER TO VERIFY THAT THE BRIDGE MEETS ALL CLEARANCE AND COVER REQUIREMENTS STATED IN THESE STANDARDS PRIOR TO DESIGNING THE SYSTEM. IF THE REQUIRED COVER TO THE TOP MAT OF DECK REINFORCEMENT IS NOT AVAILABLE, ADDITIONAL COVER CAN BE ACHIEVED WITH AN OVERLAY. IF THE BRIDGE CANNOT BE OVERLAYED, THE BRIDGE IS NOT SUITABLE FOR AN ANTI-ICING SYSTEM INSTALLATION.
- 3. PROVIDE SUPPORT HARDWARE IN ACCORDANCE WITH AASHTO M 270 (ASTM A 709) GRADE 50. PROVIDE 5/8" DIAMETER OR LARGER AASHTO M 164 (ASTM A 325) H.S. BOLTS FOR ALL ATTACHMENTS TO STRUCTURAL STEEL. DO NOT EXCEED BOLT SPACING OF 5½". PROVIDE A MINIMUM OF TWO FASTENERS PER ATTACHMENT UNLESS INDICATED OTHERWISE AND APPROVED BY THE DISTRICT BRIDGE ENGINEER. PROVIDE MINIMUM EDGE DISTANCES IN ACCORDANCE WITH DESIGN MANUAL, PART 4, SECTION 6.13.3.10P.
- 4. GALVANIZE ALL SUPPORT HARDWARE (AFTER FABRICATION) IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s). GALVANIZE SUPPORT ANGLES IN ACCORDANCE WITH ASTM A 123. GALVANIZE THREADED RODS AND BOLTS IN ACCORDANCE WITH ASTM A 153 OR ASTM A 695. PAINT ALL HANGERS, SUPPORTS, AND ASSOCIATED ATTACHMENT HARDWARE IN ACCORDANCE WITH PUBLICATION 408. PROVIDE TOP COAT OF PAINT TO MATCH THE BRIDGE SUPERSTRUCTURE.
- 5. SUBMIT SHOP DRAWINGS /INSTALLATION PLANS SHOWING ALL CONDUIT, VALVE BOX, SPRAY DISKS AND SENSOR LOCATIONS; HARDWARE DETAILS; AND ATTACHMENT METHODS.
- 6. PROVIDE DRAINS FOR VALVE BOXES AND CARRIER CONDUIT/PIPE CONTAINING LEVER OPERATED BRASS BALL VALVES WITH STAINLESS STEEL (AISI-410) BALL AND TEFLON (P.T.F.E.) BODY SEAT RINGS AND SEALS. OMIT LEVERS AT LOCATIONS THAT ARE EASILY ACCESSIBLE TO PREVENT TAMPERING. MINIMUM DRAIN VALVE SIZE IS 1/2 ".
- 7. LOCATE ALL SOLUTION CONDUIT AND VALVE BOX DRAINS SUCH THAT THEY ARE NOT DIRECTLY ABOVE AND WITHIN 12" HORIZONTALLY OF ANY EXISTING OR PROPOSED UTILITY.
- 8. PROVIDE DESIGN DRAWINGS THAT IDENTIFY REQUIRED PLACEMENT LOCATIONS AND ANY LIMITATIONS ON PLACEMENT OF ALL SYSTEM COMPONENTS.
- 9. WHEN ANTI-ICING SYSTEMS ARE INSTALLED WITH A NEW CONCRETE DECK SLAB, SEAL THE DECK WITH SILANE SEALER IN LIEU OF CONCRETE SEALANT.

VALVE BOX NOTES:

- 1. ATTACH VALVE BOXES ON ABUTMENTS OR UTILIZE INSPECTION WALKWAYS AND OTHER NON-STRUCTURAL COMPONENTS. WHEN NON-STRUCTURAL COMPONENTS ARE NOT AVAILABLE, UTILIZE SECONDARY MEMBERS (STIFFENERS AND CROSS FRAMES) TO MINIMIZE ADDITIONAL ATTACHMENTS TO THE BRIDGE WHERE POSSIBLE.
- 2. INSTALL VALVE BOX DRAINS SO ANY ERRANT ACCUMULATED SOLUTION CAN BE DRAINED PRIOR TO OPENING THE VALVE BOX DOOR.
- 3. CONSTRUCT VALVE BOXES, PULL BOXES, AND ANY OTHER BOXES IN ACCORDANCE WITH NEMA 4X REQUIREMENTS. CONSTRUCT ALL BOXES WITH AISI 316 STAINLESS STEEL (MINIMUM THICKNESS = 14 GAGE) WITH WATERTIGHT GASKETS ON THE BOX DOOR.
- 4. THE DISTANCE BETWEEN VALVE BOXES AND SPRAY DISKS IS LIMITED TO 50'-0" TO 150'-0". MULTIPLE VALVE BOX LOCATIONS ARE REQUIRED FOR MOST BRIDGES. APPROVED LOCATIONS FOR ATTACHING VALVE BOXES ARE AS FOLLOWS:
 - A.) PREFERRED LOCATION IS AT ABUTMENTS FOR BRIDGES WITH SHORT SPANS. B.) LONGER SPANS MAY REQUIRE VALVE BOXES AT PIERS AND AT BRIDGE DIAPHRAGMS.
- 5. LOCATE VALVE BOXES TO ALLOW FOR SIMPLIFIED ACCESS BY DEPARTMENT MAINTENANCE STAFF, BUT ALSO TO DETER VANDALISM AND PUBLIC ACCESS. CONSIDER LOCATIONS THAT ARE AWAY FROM ACTIVE TRAFFIC (RAIL AND VEHICLE) AND ARE ACCESSIBLE USING EXISTING CATWALKS OR LADDERS RATHER THAN SPECIAL EQUIPMENT. PROVIDE VALVE BOXES WITH LOCKS TO PREVENT VANDALISM PER DIRECTION FROM THE ASSISTANT DISTRICT ENGINEER OF MAINTENANCE. KEY ALL LOCKS THE SAME.
- 6. ATTACH VALVE BOXES NO MORE THAN 10'-0" BELOW THE SPRAY DISKS THAT ARE CONTROLLED BY THAT VALVE BOX. THIS IS TO LIMIT THE PRESSURE HEAD DIFFERENTIAL TO 10'-0".

SPRAY DISK AND SENSOR NOTES:

- 1. CONSTRUCT SPRAY DISKS AND SENSOR USING STAINLESS STEEL OR OTHER DURABLE MATERIALS THAT ARE UV RESISTANT. PROVIDE SPRAY DISKS THAT WILL ACCOMMODATE ADJUSTMENTS TO THE SPRAY PATTERN AFTER INSTALLATION. ADJUSTMENT CHOICES INCLUDE NOZZIE ROTATION AND NOZZIE REPLACEMENT.
- 2. THE LAYOUT AND SPACING OF DISKS IS SITE SPECIFIC AND ANTI-ICING SOLUTION SPECIFIC. DESIGN DISK LAYOUT FOR BRINE SOLUTION.
- 3. MANUFACTURE SPRAY DISKS AND SENSORS TO SUSTAIN A PHL-93 LOADING.
- 4. INSTALL SPRAY DISKS SO THE TOP SURFACE IS 1/8" BELOW THE ROADWAY SURFACE. INSTALL SENSORS SO THE TOP SURFACE IS FLUSH WITH THE ROADWAY SURFACE. PROVIDE SPRAY DISKS AND SENSORS THAT ARE A MAXIMUM OF 2" THICK SO THAT INSTALLATION DOES NOT INTERFERE WITH THE BRIDGE DECK REINFORCING STEEL AND SUCH THAT MINIMUM GROUT THICKNESSES ARE SATISFIED.
- 5. LOCATE SPRAY DISKS AND SENSORS SUCH THAT NO TOP MAT REINFORCEMENT BAR LIES TANGENT TO THE PERIMETER OF THE CORE HOLES, SEE DETAIL THIS SHEET.

CARRIER CONDUIT/PIPE NOTES:

- 1. CONTAIN CONCRETE ENCASED SOLUTION SUPPLY LINES AND ELECTRICAL WIRING IN EITHER RIGID STEEL CONDUIT OR PVC CONDUIT (SCHEDULE 40) IN ACCORDANCE WITH PUBLICATION 408, SECTION 1101.09(c). CONTAIN SOLUTION SUPPLY LINES AND ELECTRICAL WIRING THAT ARE NOT ENCASED IN CONCRETE, IN EITHER RIGID STEEL CONDUIT OR GALVANIZED STEEL PIPE IN ACCORDANCE WITH PUBLICATION 408, SECTIONS 1101.09(c) AND 1105.02(i AND s).
- 2. PROVIDE SEPARATE CARRIER CONDUIT/PIPE FOR ELECTRICAL WIRING AND SOLUTION SUPPLY LINES. INSTALL DRAINS ON ALL CARRIER CONDUIT/PIPE AT THE LOW POINT TO CHECK FOR SOLUTION LEAKS.
- 3. PROPERLY GROUND ALL ELECTRICAL CONDUIT IN ACCORDANCE WITH PUBLICATION 408, SECTION 910.3(g) "GROUNDING ON STRUCTURES". SIZE AND INSTALL ALL WIRING AND CARRIER CONDUIT/PIPE PER THE NATIONAL ELECTRICAL CODE (NEC).
- 4. THE FOLLOWING ARE MAXIMUM UNSUPPORTED LENGTH OF CARRIER CONDUIT/PIPE:
 - " L " = DISTANCE BETWEEN POINTS OF SUPPORT
 - L = 30'-0" FOR 3" GALVANIZED STEEL PIPE L = 22'-0" FOR 2" GALVANIZED STEEL PIPE
 - L = 17'-0" FOR 1" GALVANIZED STEEL PIPE
 - = 10'-0" FOR ALL RIGID STEEL CONDUIT (NOTE: FOR BENT CONDUIT, MAXIMUM UNSUPPORTED LENGTH IS MEASURED ALONG CONDUIT BETWEEN SUPPORTS).
- 5. THE FOLLOWING ARE MINIMUM BEND RADII FOR CARRIER/CONDUIT PIPE:
- 13" FOR 3" A53 STEEL PIPE 91/2" FOR 2" A53 STEEL PIPE
- 5¾ " FOR 1" A53 STEEL PIPE OR STEEL CONDUIT
- 6. PROVIDE THREADED COUPLERS FOR ALL CARRIER CONDUIT/PIPES THAT REQUIRE SPLICING. LOCATE COUPLINGS APPROXIMATELY 0.25*L AWAY FROM A SUPPORT POINT, WHERE "L" IS THE SPAN LENGTH OF THE CONDUIT/PIPE
- 7. INSTALL ALL CARRIER CONDUIT/PIPE BETWEEN BEAMS AT LEAST 15" ABOVE THE BOTTOM OF THE BEAM. LOCATE ALL OTHER HARDWARE (VALVE BOXES, ETC.) AT LEAST 3" ABOVE THE BOTTOM OF THE BEAM.
- 8. INSTALL ANTI-ICING SYSTEM COMPONENTS SUCH THAT LOADING ON THE BRIDGE FASCIA BEAMS IS NOT INCREASED WHENEVER POSSIBLE.
- 9. NOTE THAT DIAPHRAGMS AND OTHER BRIDGE COMPONENTS MAY RESULT IN LIMITED ACCEPTABLE "BETWEEN THE BEAM" LOCATIONS FOR CARRIER CONDUIT/PIPE. THIS SITUATION MAY RESULT IN EITHER NON-STANDARD SPRAY DISK LOCATIONS, OR, THE BRIDGE MAY BE UNSUITABLE FOR AN ANTI-ICING SYSTEM INSTALLATION. ALL NON STANDARD APPLICATIONS ARE SUBJECT TO APPROVAL BY THE DISTRICT BRIDGE ENGINEER.

STEEL BRIDGE COMPONENT NOTES:

- 1. THESE ANTI-ICING STANDARDS ARE NOT APPLICABLE FOR USE WITH EXISTING OR NEW UNPAINTED WEATHERING STEEL BRIDGES.
- 2. CARRIER CONDUIT/PIPE HANGER SHOWN ON BC-721M IS PERMITTED ON NEW CONSTRUCTION ONLY WHEN INCLUDED IN THE DESIGN OF THE GIRDER.
- 3. FOR EXISTING BRIDGES, DRILLING HOLES FOR BOLTS IN STIFFENERS AND DIAPHRAGM MEMBERS IS PERMITTED ONLY AS SHOWN IN STANDARD DRAWINGS. NO OTHER DRILLING, CORING, CUTTING, OR WELDING IS PERMITTED UNLESS DETAILED ON THE SHOP DRAWINGS AND INSTALLATION PLANS, AND APPROVED BY THE DISTRICT BRIDGE ENGINEER.

CONCRETE BRIDGE COMPONENT NOTES:

- 1. PROVIDE ALL ATTACHMENTS TO CONCRETE USING THREADED ROD IN ACCORDANCE WITH AASHTO M 270 (ASTM A 709) GRADE 50. FOR ATTACHMENT TO CONCRETE STRUCTURES WHERE A BOLT-THROUGH OR CAST-IN-PLACE THREADED INSERT CONNECTION IS NOT FEASIBLE, GROUTED ANCHORS MAY BE USED WITH THE APPROVAL OF THE DISTRICT BRIDGE ENGINEER, GROUTED ANCHORS ARE PROHIBITED FOR USE IN VERTICAL OVERHEAD APPLICATIONS.
- 2. FOR EXISTING BRIDGES, INSTALLING SUPPORT BRACKETS BETWEEN THE TOP FLANGES OF ADJACENT BEAMS AS SHOWN ON THE CONCRETE DETAILS IS ONLY VALID FOR P/S I-BEAMS WITH TOP FLANGES THAT ARE 5" OR GREATER IN DEPTH. THIS REQUIREMENT WILL ELIMINATE INSTALLATION ON THE PENNSYLVANIA BULB TEE BEAMS. NO OTHER BRACKETS OR INSTALLATIONS WILL BE PERMITTED UNLESS APPROVED BY THE DISTRICT BRIDGE ENGINEER.
- 3. FOR EXISTING CONCRETE BRIDGE SUPERSTRUCTURES, IT IS PREFERRED TO ATTACH VALVE BOXES ON THE SUBSTRUCTURE UNITS. HOWEVER, IT IS PERMISSIBLE TO LOCATE VALVE BOXES ON INTERMEDIATE DIAPHRAGMS AS SHOWN ON THE CONCRETE BEAM DETAILS, AND OVER PIERS ON FULL DEPTH DIAPHRAGMS AS SHOWN ON THE DIAPHRAGM ATTACHMENT
- 4. CORING IS NOT PERMITTED IN CONTINUITY DIAPHRAGMS OVER THE BRIDGE PIERS.
- 5. FOR NEW CONCRETE SUPERSTRUCTURES, VALVE BOXES AS WELL AS CARRIER CONDUIT/PIPE CAN BE INSTALLED DIRECTLY ON THE P/S BEAMS, PROVIDED THAT ADEQUATE THREADED INSERTS ARE INCLUDED AT THE TIME OF BEAM FABRICATION AND DETAILED AND APPROVED IN BEAM SHOP DRAWINGS. NO DRILLING WILL BE PERMITTED ON NEW P/S BEAMS. ADHESIVE ANCHORS ARE ONLY PERMITTED IN THE TOP FLANGE ON EXISTING BRIDGES AS SHOWN ON THE CONCRETE BEAM DETAILS.

ROADWAY WEATHER INFORMATION SYSTEM (RWIS)

(ROADWAY ITEM):

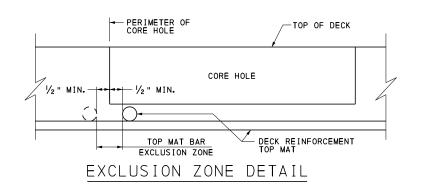
- 1. LOCATE RWIS WITHIN 100' OF THE PAVEMENT SENSORS THAT ARE EMBEDDED IN THE BRIDGE DECK.
- 2. MOUNT RWIS EITHER BEHIND THE TRAFFIC BARRIER OR ADJACENT TO THE BRIDGE. ENCASE ALL WIRING FOR THE BRIDGE MOUNTED RWIS IN EITHER BARRIER OR DECK CONDUIT, OR BURY IF OFF OF THE BRIDGE. NO EXPOSED CONDUIT IS PERMITTED ON THE BRIDGE FOR THE RWIS.
- 3. DESIGN ALL RWIS TOWERS LOCATED ON THE BRIDGE SUPERSTRUCTURE TO CONFORM TO THE VIBRATION REQUIREMENTS OF DESIGN MANUAL, PART 4, A.3.6.1. PREFERRED LOCATIONS FOR RWIS TOWERS ARE OFF OF THE STRUCTURE. THE RWIS TOWER CAN BE LOCATED ON THE SUBSTRUCTURE UNITS WITH ADEQUATE SET BACK SO COLLISION DAMAGE IS MINIMIZED.
- 4. LOCATION AND ATTACHMENT METHODS (IF STRUCTURE MOUNTED) OF THE RWIS SYSTEM COMPONENTS MUST BE DETAILED ON THE SHOP DRAWINGS AND INSTALLATION PLANS AND ARE SUBJECT TO THE APPROVAL OF THE DISTRICT BRIDGE ENGINEER.

DEAD LOAD CALCULATIONS:

USE THE FOLLOWING VALUES TO CALCULATE DESIGN LOADS:

1" STEEL CARRIER CONDUIT/PIPE 2.1 LB./FT. 2" STEEL CARRIER CONDUIT/PIPE 5.2 LB./FT. 3" STEEL CARRIER CONDUIT/PIPE 10.8 LB./FT.

(ALL CARRIER CONDUIT/PIPE LOADS ASSUME 100% FLUID CARRIER IN CONDUIT/PIPE



COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

BRIDGE ANTI-ICING SYSTEM GENERAL NOTES

RECOMMENDED SEPT. 30, 2016 Thoma P Macioca CHIEF BRIDGE ENGINEER

RECOMMENDED SEPT. 30, 2016 SHEET 2 OF 10

Buni SThomps DIRECTOR, BUR. OF PROJECT DELIVERY | BC-723M

PROCEDURE FOR INSTALLING ANTI-ICING SYSTEM IN A NEW BRIDGE:

- PROVIDE DESIGN PLANS THAT SHOW ALL PROPOSED LOCATIONS FOR VALVE BOXES, CONDUIT/PIPE HANGERS, SPRAY DISKS, SENSORS, ETC. AS WELL AS LOCATIONS WHERE ATTACHMENTS FOR THE ANTI-ICING SYSTEM COMPONENTS ARE PROHIBITED. THE FINAL LOCATIONS WILL BE APPROVED BY THE DISTRICT BRIDGE ENGINEER.
- 2. INSTALL BLOCKOUTS IN NEW CONCRETE DECKS WITH SHAPES THAT MATCH THE PROPOSED SENSOR OR SPRAY DISK AND PROVIDE AN ACCESS HOLE FOR CONNECTION TO THE SOLUTION SUPPLY LINE PROVIDE BLOCKOUTS MANUFACTURED OF SOFT WOOD OR SIMILAR MATERIAL.
- 3. INSTALL BLOCKOUTS $\frac{1}{2}$ " BELOW FINAL DECK ELEVATION TO PREVENT CONTACT WITH DECK PLACEMENT MACHINERY. CABLE TIES ARE PERMITTED TO MARK THE BLOCKOUT LOCATION.
- 4. LOCATE ALL CONDUIT AND BLOCKOUTS FOR SPRAY DISKS AND SENSORS TO PROVIDE INDICATED CLEARANCES TO REINFORCEMENT AND SUCH THAT MINIMUM GROUT THICKNESSES ARE SATISFIED. SECURELY FASTEN BLOCKOUTS TO ENSURE POSITION AND ALIGNMENT IS MAINTAINED DURING DECK PLACEMENT OPERATIONS.
- PROVIDE REMOVABLE DECK FORMS AT ALL CONDUIT DECK PENETRATIONS SUCH THAT MINIMUM DISTANCE FROM CONDUIT TO STAY-IN-PLACE FORMS IS 3'-0".
- 6. AFTER THE DECK IS CURED AND ANY GRINDING OR GROOVING IS COMPLETED, REMOVE THE BLOCKOUTS AND INSTALL THE ANTI-ICING
- 7. SUSPEND SPRAY DISKS AND SENSORS OVER BLOCKOUT VOID DURING CEMENTING/GROUTING OPERATIONS SO FINAL EMBEDMENT RELATIVE TO THE FINISHED DECK IS AS INDICATED.
- 8. AFTER SPRAY DISK/SENSOR IS PROPERLY LOCATED, SEAL UNIT IN DECK WITH A PREMIXED FLOWABLE NON-SHRINK GROUT IDENTIFIED IN BULLETIN 15.
- 9. SEAL THE CONCRETE DECK WITH SILANE SEALER.

PROCEDURE FOR INSTALLING ANTI-ICING SYSTEM IN AN EXISTING BRIDGE DECK WITH OR WITHOUT EXISTING OVERLAYS:

- PROVIDE SHOP DRAWINGS THAT SHOW ALL PROPOSED LOCATIONS FOR VALVE BOXES, CONDUIT/PIPE HANGERS, SPRAY DISKS, SENSORS, ETC. AS WELL AS LOCATIONS WHERE ATTACHMENTS FOR THE ANTI-ICING SYSTEM COMPONENTS ARE PROHIBITED. THE FINAL LOCATIONS WILL BE APPROVED BY THE DISTRICT BRIDGE ENGINEER.
- 2. LOCATE ALL REINFORCEMENT PRIOR TO CORING, CUTTING, OR DRILLING INTO THE DECK. VERIFY THAT A MINIMUM 3 3/8 SQUARE SPACE EXISTS BETWEEN REINFORCEMENT STEEL FOR THE 2% " DIAMETER HOLE.
- 3. LOCATE REBARS USING A NON-DESTRUCTIVE MAGNETIC DEVICE WITH THE CAPABILITY TO "AUTOMATICALLY" MEASURE COVER AND REBAR SIZE. PERFORM ALL TESTING, CORING, CUTTING, AND DRILLING OF CONCRETE UNDER THE SUPERVISION OF THE ENGINEER.
- 4. MAKE ALL NEW HOLES IN CONCRETE USING CONCRETE-CORING BITS. HAMMER DRILLS ARE NOT ALLOWED. SAW CUTTING IS PERMITTED FOR FORMING RECTANGULAR RECESSES.
- 5. CORE INTO DECK USING METHODS THAT WILL NOT SHATTER/DAMAGE THE CONCRETE SURFACE ADJACENT TO THE HOLES OR RESULT IN SPALLING AT THE UNDERSIDE OF THE DECK. NOTIFY THE ENGINEER IF SPALLING OCCURS AND PATCH THE SPALLED DECK IMMEDIATELY IN ACCORDANCE WITH THE ENGINEER'S DIRECTION. PERFORM ALL PATCHING IN ACCORDANCE WITH BC-783M.
- INSTALL 1" CONDUIT THROUGH DECK AND CEMENT WITH PREMIXED FLOWABLE NON-SHRINK GROUT USING AN ELASTOMERIC GASKET AT THE BOTTOM OF THE PENETRATION TO PREVENT GROUT LEAKAGE.
- 7. AFTER NON-SHRINK GROUT HARDENS, SEAL BOTTOM OF DECK PENETRATION WITH SILICONE CAULKING.
- 8. SUSPEND SPRAY DISKS AND SENSORS OVER BLOCKOUT VOID DURING CEMENTING/GROUTING OPERATIONS SO FINAL EMBEDMENT RELATIVE TO THE FINISHED DECK IS AS INDICATED. (ALTERNATIVELY, DISKS AND SENSORS MAY BE PLACED INTO WET GROUT PROVIDED MINIMUM GROUT THICKNESSES ARE SATISFIED AND FINAL EMBEDMENT IS AS INDICATED.)
- 9. FOR INSTALLATIONS IN AN EXISTING ASPHALT OVERLAY, INSTALL TEMPORARY NEOPRENE SPONGE AROUND VOID PERIMETER, AS INDICATED.
- 10. AFTER SPRAY DISK/SENSOR IS PROPERLY LOCATED, SEAL UNIT IN DECK WITH A PREMIXED FLOWABLE NON-SHRINK GROUT IDENTIFIED IN BULLETIN 15 TO THE TOP OF THE EXISTING CONCRETE DECK OR TO THE TOP OF ASPHALT, FOR ASPHALT OVERLAYS. INSTALL SO THAT THE FINAL SURFACE IS AS INDICATED ON THE DRAWINGS. FOR EXISTING ASPHALT OVERLAY APPLICATIONS, REMOVE TEMPORARY NEOPRENE SPONGE AND INSTALL PERMANENT BACKER ROD AND JOINT SEALING MATERIAL AS INDICATED.

PROCEDURE FOR INSTALLING AN ASPHALT OVERLAY ON A BRIDGE WITH AN EXISTING ANTI-ICING SYSTEM:

- PROVIDE SHOP DRAWINGS THAT SHOW ALL PROPOSED LOCATIONS FOR NEW VALVE BOXES, CONDUIT/PIPE HANGERS, SPRAY DISKS, SENSORS, ETC. AS WELL AS LOCATIONS WHERE NEW ATTACHMENTS FOR THE ANTI-ICING SYSTEM COMPONENTS ARE PROHIBITED. THE FINAL LOCATIONS WILL BE APPROVED BY THE DISTRICT BRIDGE ENGINEER.
- 2. THE FOLLOWING COMPONENTS OF THE ANTI-ICING SYSTEM ARE ANTICIPATED TO BE SALVAGEABLE: NOZZLES, INSIDE COMPONENTS OF THE SPRAY DISKS, AND SOLUTION/ELECTRICAL SUPPLY LINES.
- 3. REMOVE ALL SALVAGEABLE PARTS OF THE ANTI-ICING SYSTEM.
- 4. REMOVE EXISTING SPRAY DISK SHELL AND SENSOR BY EITHER MILLING OR CORING AROUND ITS PERIMETER AND THEN BY USING A PNEUMATIC HAMMER WITH MAXIMUM NOMINAL MASS OF 30-LB. DO NOT OPERATE PNEUMATIC HAMMERS OR MECHANICAL CHIPPING TOOLS AT AN ANGLE IN EXCESS OF 45 DEGREES RELATIVE TO THE SURFACE OF THE SLAB. ENTIRELY REMOVE EXISTING GROUT.
- 5. INSTALL BLOCKOUTS IN DECK, AND/OR OVERLAY. PROVIDE BLOCKOUTS IN SHAPES THAT ACCOMMODATE AND PROTECT THE EXISTING COMPONENTS AS WELL AS MATCH THE PROPOSED SENSOR OR SPRAY DISK AND PROVIDE AN ACCESS HOLE FOR CONNECTION TO THE SOLUTION/ELECTRICAL SUPPLY LINE. PROVIDE BLOCKOUTS MANUFACTURED OF SOFT WOOD OR SIMILAR MATERIAL.
- 6. INSTALL BLOCKOUTS ½ " BELOW FINAL DECK ELEVATION TO PREVENT CONTACT WITH PAVER. CABLE TIES ARE PERMITTED TO MARK THE BLOCKOUT LOCATION.
- 7. INSTALL NEW OVERLAY.
- 8. REMOVE BLOCKOUTS AND INSTALL TEMPORARY NEOPRENE SPONGE AROUND VOID PERIMETER, AS INDICATED.
- 9. AFTER OVERLAY IS PLACED, SUSPEND SPRAY DISKS AND SENSORS OVER BLOCKOUT DURING CEMENTING/GROUTING OPERATIONS SO FINAL EMBEDMENT RELATIVE TO THE FINISHED DECK IS AS INDICATED. (ALTERNATIVELY, DISKS AND SENSORS MAY BE PLACED INTO WET GROUT PROVIDED MINIMUM GROUT THICKNESSES ARE SATISFIED AND FINAL EMBEDMENT IS AS INDICATED.)
- 10. AFTER SPRAY DISK/SENSOR IS PROPERLY LOCATED, SEAL UNIT IN DECK WITH A PREMIXED FLOWABLE NON-SHRINK GROUT IDENTIFIED IN BUILLETIN 15 TO THE TOP OF THE ASPHALT OVERLAY, INSTALL SO THAT THE FINAL SURFACE IS AS INDICATED ON THE DRAWINGS.
- REMOVE TEMPORARY NEOPRENE SPONGE AND INSTALL PERMANENT BACKER ROD AND JOINT SEALING MATERIAL AS INDICATED.

PROCEDURE FOR INSTALLING A LATEX MODIFIED CONCRETE OVERLAY ON A BRIDGE WITH AN EXISTING ANTI-ICING SYSTEM:

- PROVIDE SHOP DRAWINGS THAT SHOW ALL PROPOSED LOCATIONS FOR NEW VALVE BOXES, CONDUIT/PIPE HANGERS, SPRAY DISKS, SENSORS, ETC. AS WELL AS LOCATIONS WHERE NEW ATTACHMENTS FOR THE ANTI-ICING SYSTEM COMPONENTS ARE PROHIBITED. THE FINAL LOCATIONS WILL BE APPROVED BY THE DISTRICT BRIDGE
- 2. THE FOLLOWING COMPONENTS OF THE ANTI-ICING SYSTEM ARE ANTICIPATED TO BE SALVAGEABLE: NOZZLES, INSIDE COMPONENTS OF THE SPRAY DISKS, AND SOLUTION/ELECTRICAL SUPPLY
- 3. REMOVE ALL SALVAGEABLE PARTS OF THE ANTI-ICING SYSTEM.
- 4. REMOVE EXISTING SPRAY DISK SHELL AND SENSOR BY EITHER MILLING OR CORING AROUND ITS PERIMETER AND THEN BY USING A PNEUMATIC HAMMER WITH MAXIMUM NOMINAL MASS OF 30-LB. DO NOT OPERATE PNEUMATIC HAMMERS OR MECHANICAL CHIPPING TOOLS AT AN ANGLE IN EXCESS OF 45 DEGREES RELATIVE TO THE SURFACE OF THE SLAB. ENTIRELY REMOVE EXISTING GROUT.
- 5. SCARIFY DECK IN ACCORDANCE WITH BC-783M.
- 6. INSTALL BLOCKOUTS IN DECK, AND/OR OVERLAY. PROVIDE BLOCKOUTS IN SHAPES THAT ACCOMMODATE AND PROTECT THE EXISTING COMPONENTS AS WELL AS MATCH THE PROPOSED SENSOR OR SPRAY DISK AND PROVIDE AN ACCESS HOLE FOR CONNECTION TO THE SOLUTION/ELECTRICAL SUPPLY LINE. PROVIDE BLOCKOUTS MANUFACTURED OF SOFT WOOD OR SIMILAR MATERIAL.
- 7. INSTALL BLOCKOUTS ½" BELOW FINAL DECK ELEVATION TO PREVENT CONTACT WITH DECK FINISHING MACHINE. CABLE TIES ARE PERMITTED TO MARK THE BLOCKOUT LOCATION.
- 8. INSTALL NEW OVERLAY.
- 9. AFTER OVERLAY IS CURED AND ANY GRINDING OR GROOVING IS COMPLETED. REMOVE THE BLOCKOUTS.
- 10. SUSPEND SPRAY DISKS AND SENSORS OVER BLOCKOUT DURING CEMENTING/GROUTING OPERATIONS SO FINAL EMBEDMENT RELATIVE TO THE FINISHED DECK IS AS INDICATED. (ALTERNATIVELY, DISKS AND SENSORS MAY BE PLACED INTO WET GROUT PROVIDED MINIMUM GROUT THICKNESSES ARE SATISFIED AND FINAL EMBEDMENT IS AS INDICATED.)
- 11. AFTER SPRAY DISK/SENSOR IS PROPERLY LOCATED, SEAL UNIT IN DECK WITH A PREMIXED FLOWABLE NON-SHRINK GROUT IDENTIFIED IN BULLETIN 15 TO THE TOP OF THE OVERLAY.

NOTES:

1. FOR GENERAL NOTES. SEE SHEET 2.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

BUREAU OF PROJECT DELIVERY

STANDARD

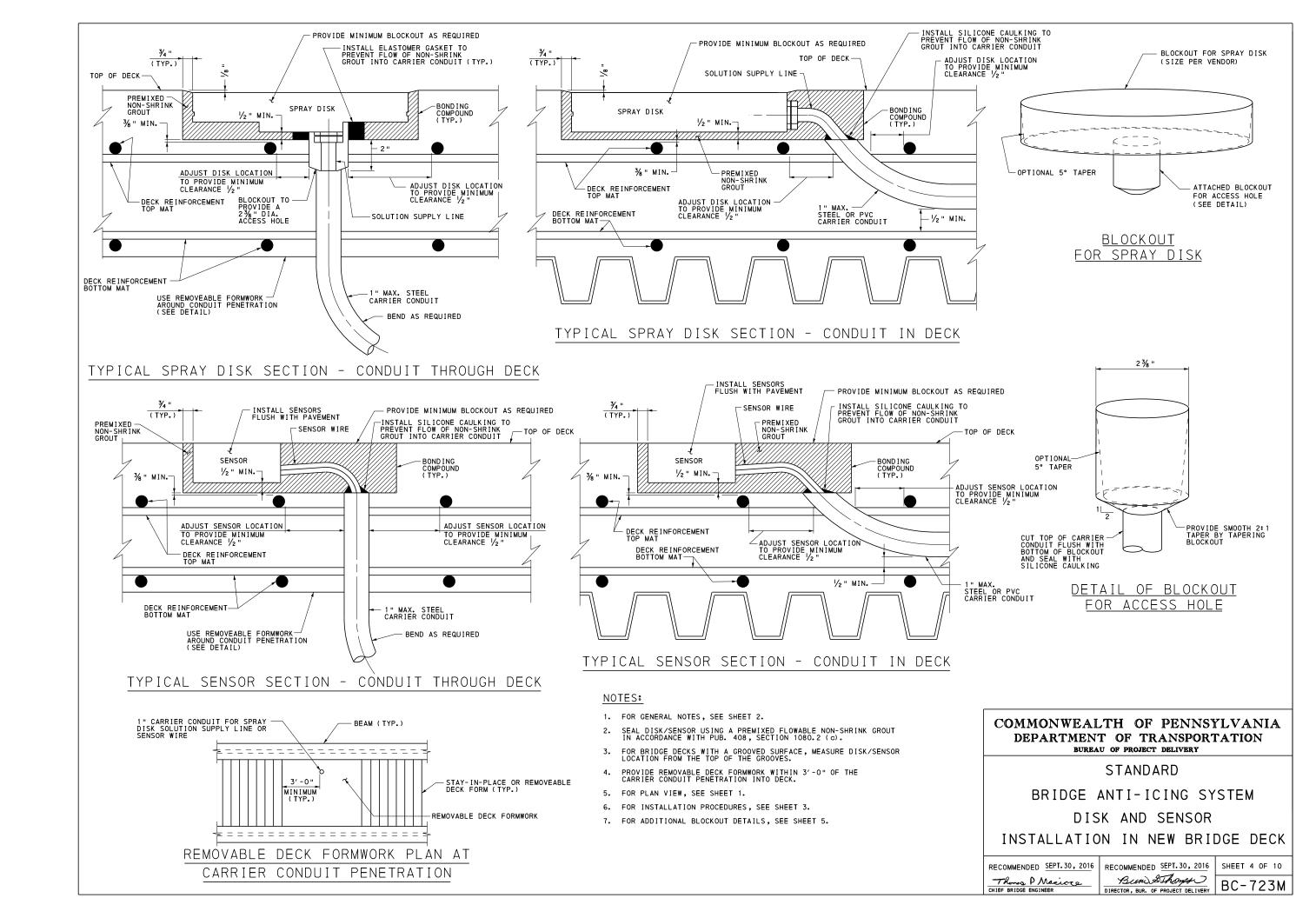
BRIDGE ANTI-ICING SYSTEM INSTALLATION PROCEDURES

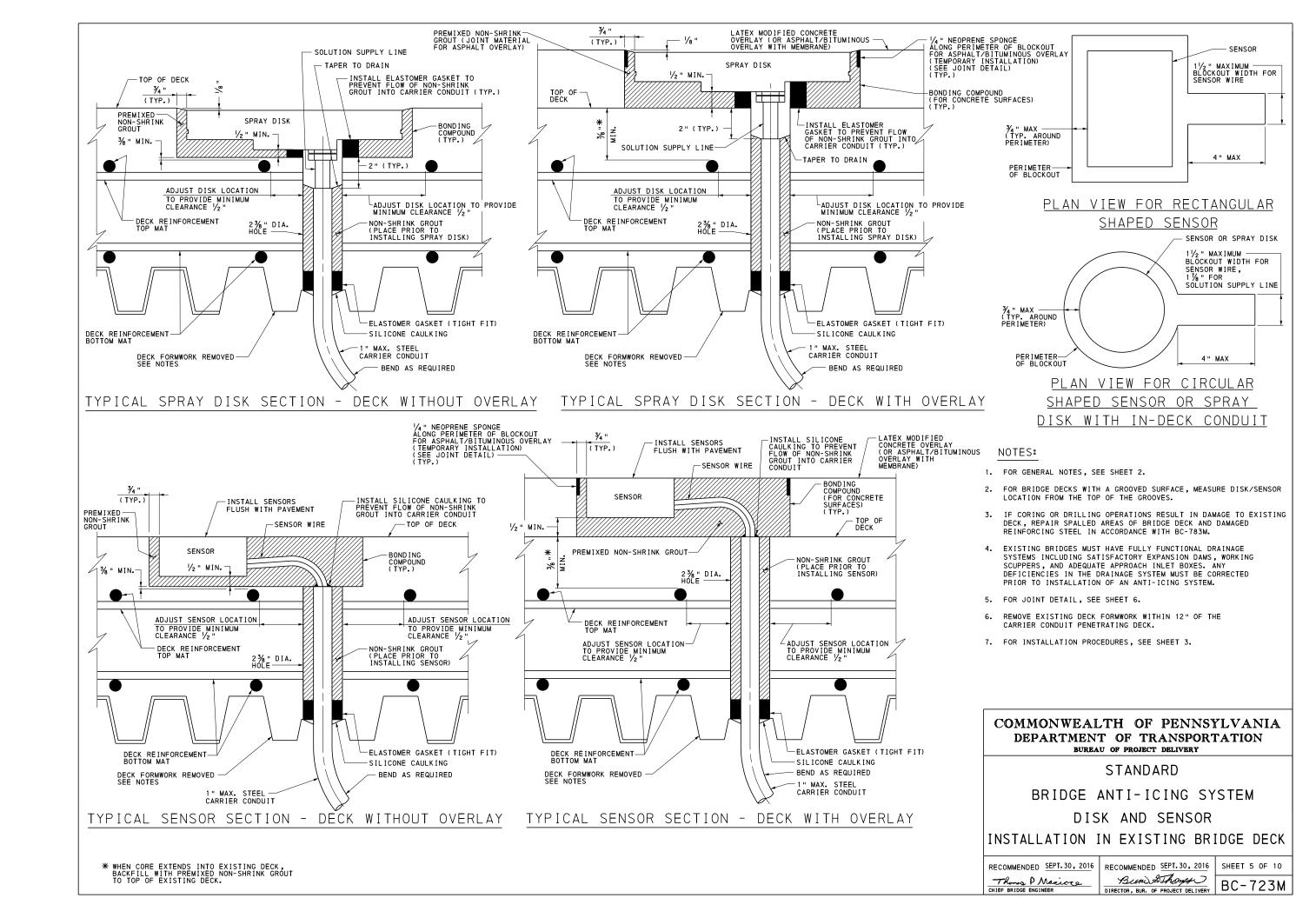
RECOMMENDED SEPT. 30, 2016 Thomas P Macioca

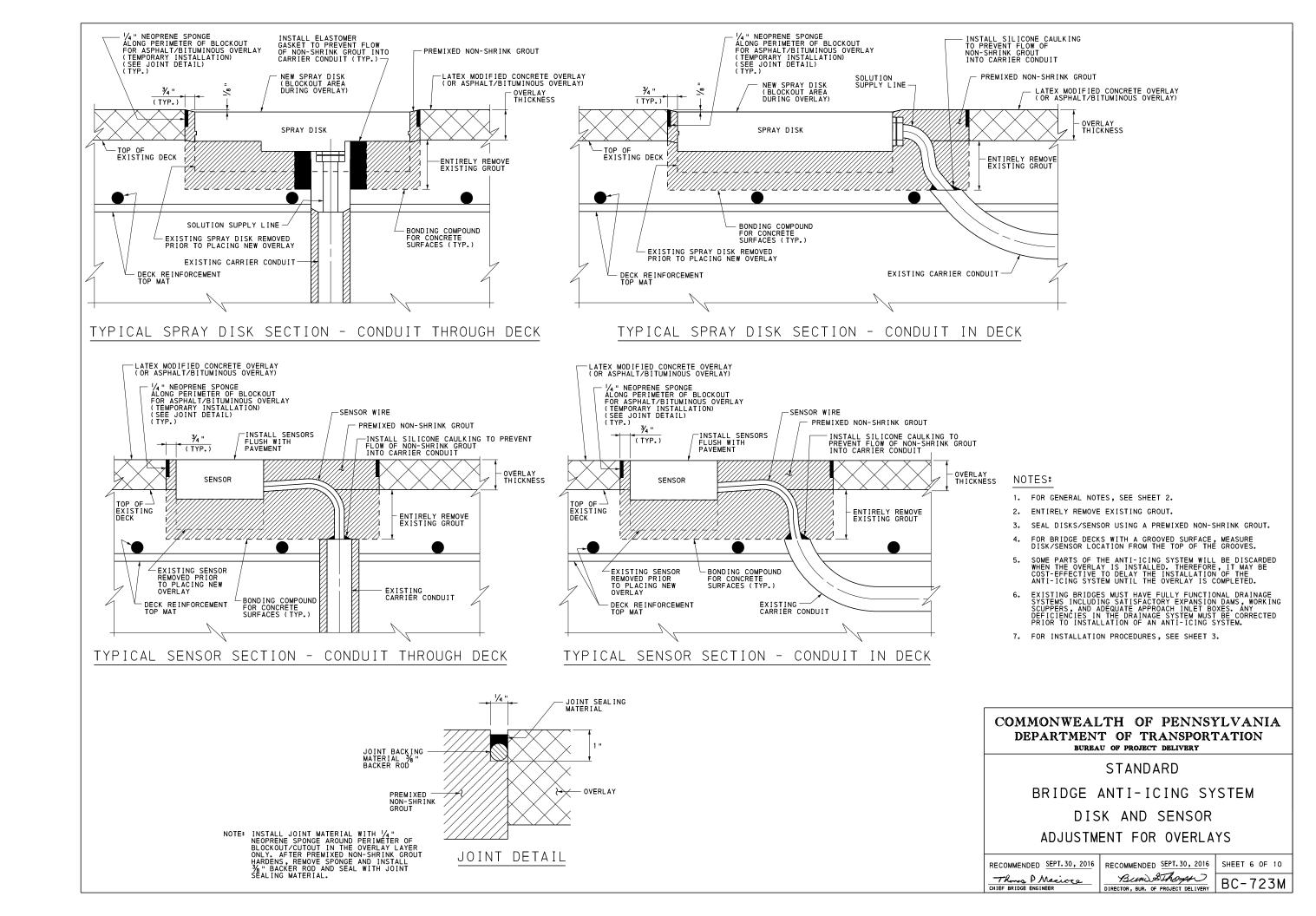
RECOMMENDED SEPT. 30, 2016

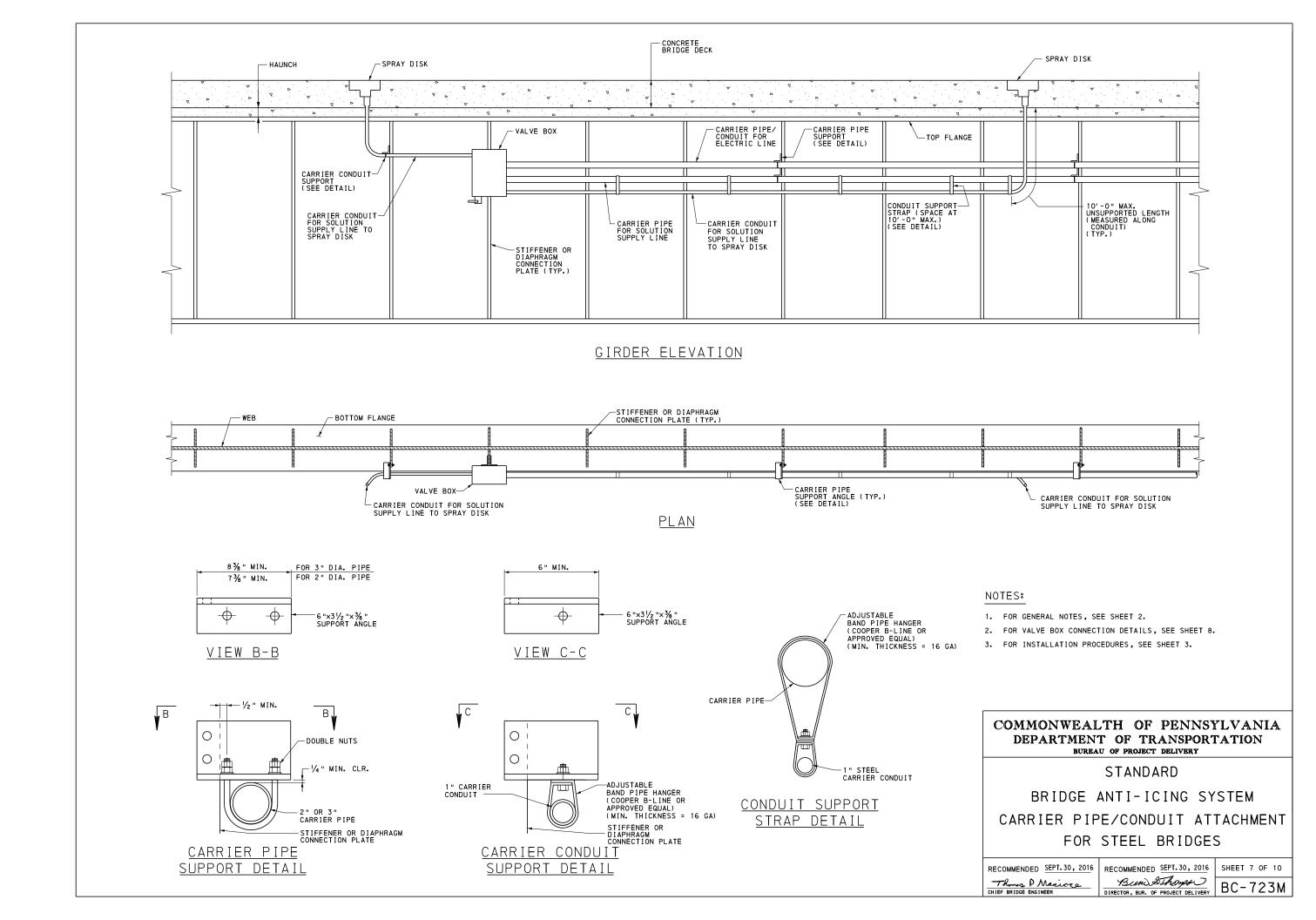
Buni SThomps DIRECTOR, BUR. OF PROJECT DELIVERY BC-723M

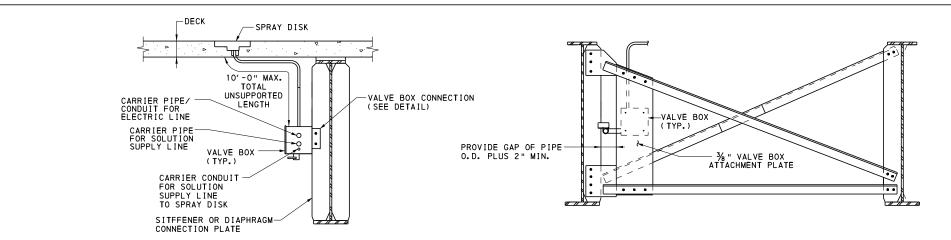
SHEET 3 OF 10





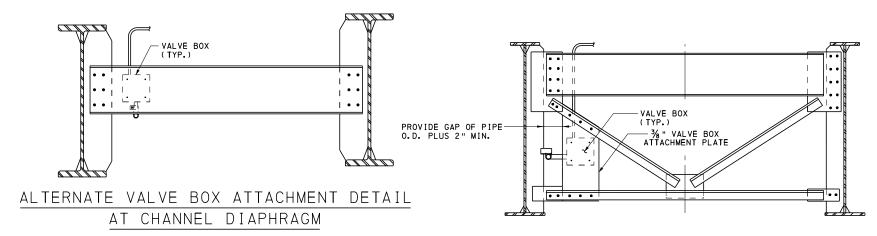






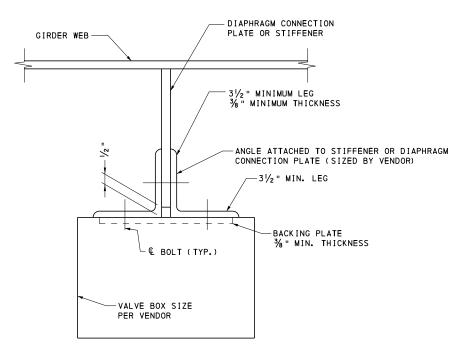
PREFERRED VALVE BOX ATTACHMENT DETAIL AT STIFFENER OR DIAPHRAGM CONNECTION PLATE

ALTERNATE VALVE BOX ATTACHMENT DETAIL AT INTERMEDIATE DIAPHRAGM



NOTE: DO NOT PLACE VALVE BOX ON JACKING DIAPHRAGM

ALTERNATE VALVE BOX ATTACHMENT DETAIL AT END DIAPHRAGM



PREFERRED VALVE BOX CONNECTION DETAIL - PLAN

NOTES:

- 1. FOR GENERAL NOTES, SEE SHEET 2.
- 2. FOR INSTALLATION PROCEDURES, SEE SHEET 3.
- 3. PREFERRED VALVE BOX MOUNTING METHOD IS USING THE DOUBLE ANGLE ATTACHMENT ON THE STIFFENERS OR DIAPHRAGM CONNECTION PLATES. USE THE ALTERNATE DIAPHRAGM CONNECTION ON BRIDGES THAT USE TANGENT BEAM ALIGNMENTS IF BRACING OR OTHER ATTACHMENTS INTERFERE WITH THE VALVE BOX DOORS. THE ALTERNATE DIAPHRAGM CONNECTION IS NOT PERMITTED ON BRIDGES WITH CURVED GIRDERS OR CHORDED STRAIGHT GIRDERS THAT MIMIC A CURVE UNLESS APPROVED BY THE DISTRICT BRIDGE ENGINEER.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

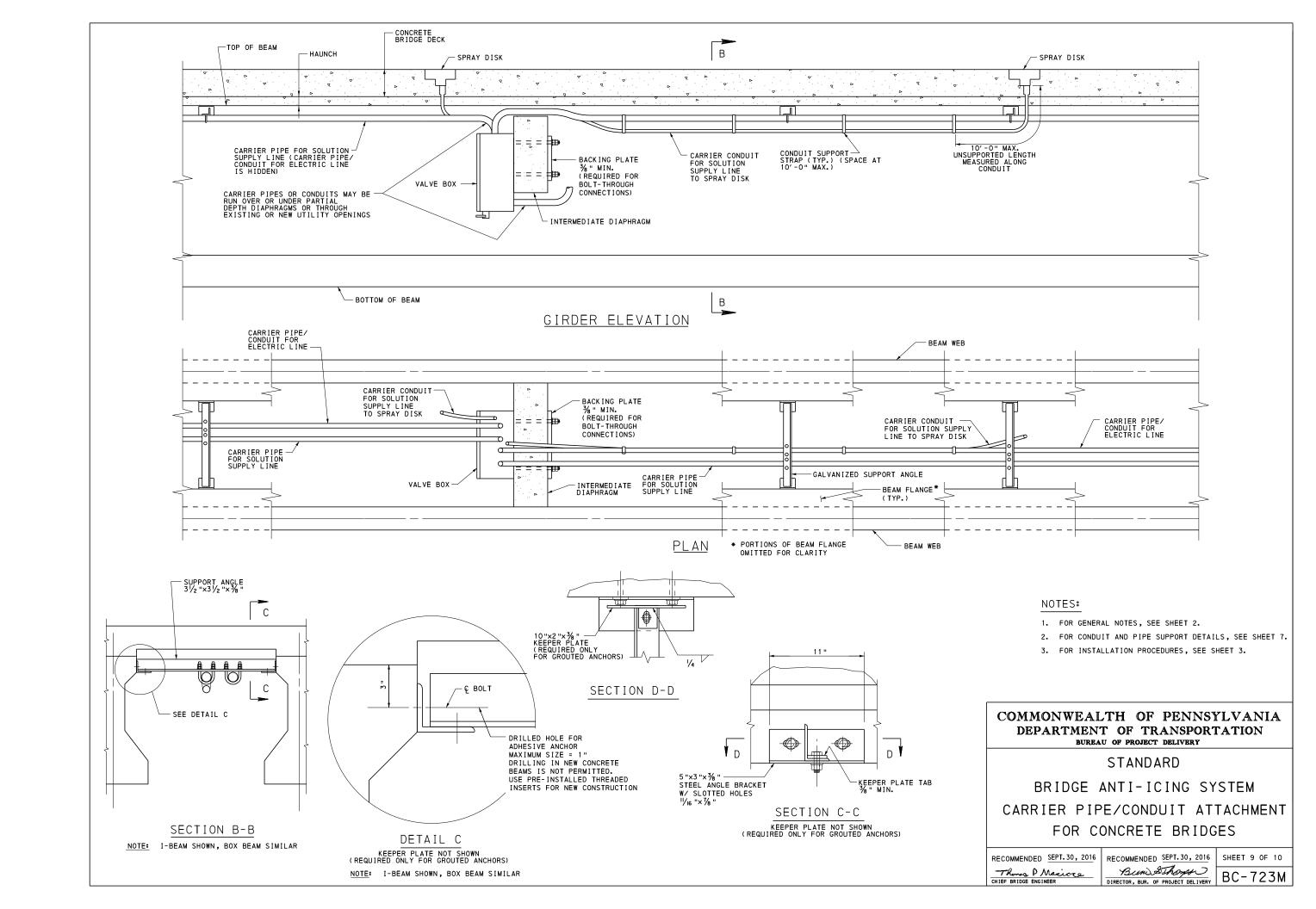
STANDARD

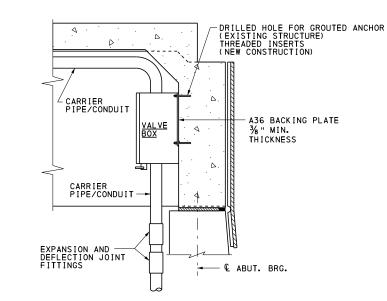
BRIDGE ANTI-ICING SYSTEM VALVE BOX ATTACHMENT FOR STEEL BRIDGES

RECOMMENDED SEPT. 30, 2016 | RECOMMENDED SEPT. 30, 2016 | SHEET 8 OF 10 Thomas P Macioca
CHIEF BRIDGE ENGINEER

Bun & Thomps

DIRECTOR, BUR. OF PROJECT DELIVERY BC-723M





VALVE BOX ATTACHMENT AT END DIAPHRAGM

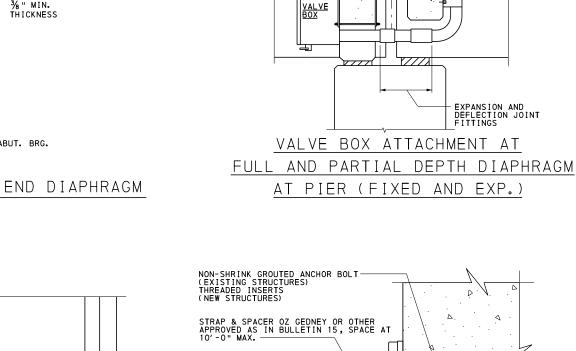
-VALVE BOX

SET TO MATCH VALVE BOX DEPTH

8" MAX.

BARRIER

≬ E



ALTERNATE DIRECTION OF STRAP OPENING (NEXT PAIR OPPOSITE DIRECTION)

CARRIER PIPE/CONDUIT

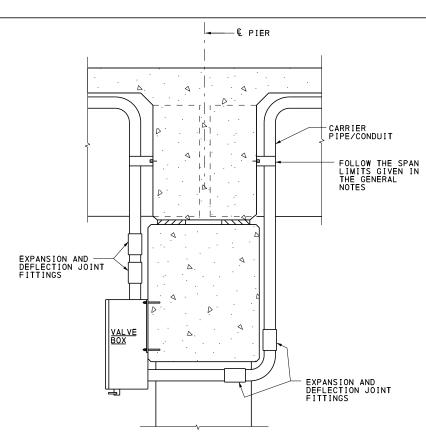
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CARRIER PIPE / CONDUIT ATTACHMENT TO CONCRETE SUBSTRUCTURES AND DIAPHRAGMS

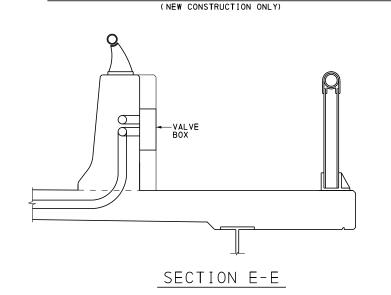
- © PIER AND EXP. DAM

CARRIER PIPE/CONDUIT



VALVE BOX ATTACHMENT AT CONTINUITY DIAPHRAGM AT PIER (FIXED AND EXP.)

WHERE CROSSING AN EXISTING CONTINUITY DIAPHRAGM (WITHOUT UTILITY OPENINGS) IS REQUIRED, ATTACH VALVE BOXES TO PIER, AND RUN CONDUIT BELOW PIER CAPS.



BARRIER MOUNTED VALVE BOX DETAIL

NOTES:

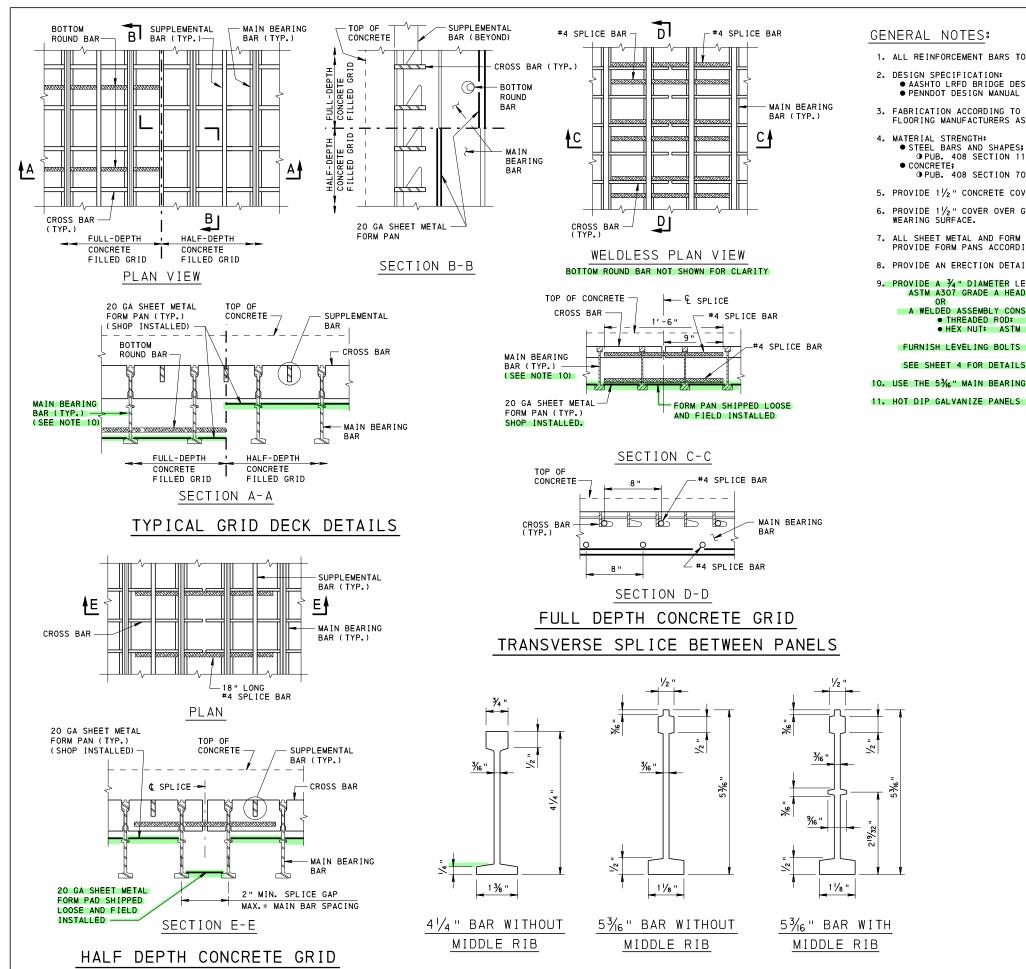
- 1. FOR GENERAL NOTES, SEE SHEET 2.
- 2. FOR INSTALLATION PROCEDURES, SEE SHEET 3.
- 3. THE USE OF BARRIER BLISTERS IS STRONGLY DISCOURAGED AND SUBJECT TO DEPARTMENT APPROVAL. OTHER LOCATIONS MUST BE USED IF POSSIBLE.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD BRIDGE ANTI-ICING SYSTEM GENERAL DETAILS

RECOMMENDED SEPT. 30, 2016 Thoma P Macioca CHIEF BRIDGE ENGINEER

RECOMMENDED SEPT. 30, 2016 | SHEET 10 OF 10 Bun & Thomps DIRECTOR, BUR. OF PROJECT DELIVERY BC-723M



MAIN BEARING BAR

SUBJECT TO MILL TOLERANCE

CHANGE 2

TRANSVERSE SPLICE BETWEEN PANELS

1. ALL REINFORCEMENT BARS TO MEET THE REQUIREMENTS OF PUB. 408 SECTION 709.1.

2. DESIGN SPECIFICATION: • AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AND COMMENTARY

PENNDOT DESIGN MANUAL PART 4

3. FABRICATION ACCORDING TO AASHTO/AWS D1.5 (DATE AS NOTED IN PUB. 408 SECTION 1105), BRIDGE GRID FLOORING MANUFACTURERS ASSOCIATION AND APPROVED SHOP DRAWINGS.

● PUB. 408 SECTION 1105.2 ● CONCRETE;

●PUB. 408 SECTION 704.1 (b), CLASS AAAP

5. PROVIDE 1 1/2 " CONCRETE COVER ON REINFORCEMENT BARS UNLESS OTHERWISE NOTED.

6. PROVIDE 11/2" COVER OVER GRID. THE TOP 1/2" OF OVERFILL/OVERLAY IS CONSIDERED SACRIFICIAL

7. ALL SHEET METAL AND FORM PANS TO MEET PUB. 408, SECTION 1001.2 GALVANIZING REQUIREMENTS. PROVIDE FORM PANS ACCORDING TO PUB. 408, SECTION 1001.2(h) 2.

8. PROVIDE AN ERECTION DETAIL COMPLETE WITH PIECE MARKS WITH THE SHOP DRAWING SUBMISSION.

9. PROVIDE A 3/4" DIAMETER LEVELING BOLT THAT IS EITHER: ASTM A307 GRADE A HEADED BOLT OR EQUIVALENT.

A WELDED ASSEMBLY CONSISTING OF THREADED ROD AND HEX NUT.

• THREADED ROD: ASTM A307, ASTM F1554 GRADE 36, OR EQUIVALENT

• HEX NUT: ASTM A194 OR ASTM A563.

FURNISH LEVELING BOLTS UNCOATED UNLESS REQUIRED TO BE GALVANIZED

SEE SHEET 4 FOR DETAILS

10. USE THE 5 1/4 " MAIN BEARING BAR WITH OR WITHOUT THE MIDDLE RIB FOR FULL DEPTH CONCRETE DECKS.

11. HOT DIP GALVANIZE PANELS PER PUB. 408. SECTION 1105.02(S).

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

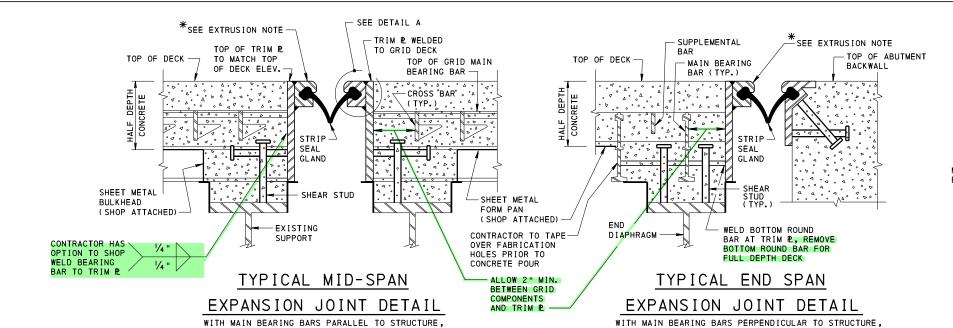
STANDARD

STEEL GRID REINFORCED CONCRETE BRIDGE DECK DETAILS FOR BEAM BRIDGES CAST-IN-PLACE OR PRECAST DECKS

RECOMMENDED JAN. 31, 2019 Thomas P. Marioca CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019 Allin Houtre ACTING DIR. BUR. OF PROJECT DELIVERY BC-726M

SHEET 1 OF 5



- 20 GAGE MIN. BULKHEAD PAN, WELD TO BEARING BARS WITH SUPPLEMENTAL -1/8 "x1/2" LONG FILLET WELD -CROSS BAR MAIN BEARING BULKHEAD PAN FLUSH WITH BOTTOM OF GRID

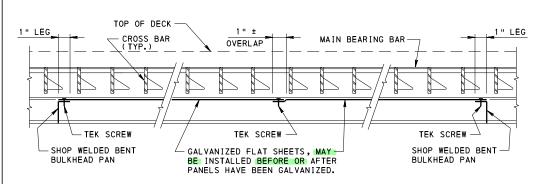
BULKHEAD PAN FIT-UP DETAIL

HALF DEPTH ONLY

* ONE PIECE EXTRUSION IN LIEU OF TWO PIECE MEMBER (EXTRUSION AND PLATE COMBINATION) IS PERMITTED. WELD IN ACCORDANCE WITH AASHTO/AWS D1.5M SPECIFICATIONS. (FULL PENETRATION WELD AND N.D.T. REQUIRED)

BOTTOM FLANGE OF MAIN BEARING BAR.

HALF DEPTH CONCRETE GRID IS SHOWN. FULL DEPTH CONCRETE GRID SIMILAR WITH FORM PANS LOCATED AT

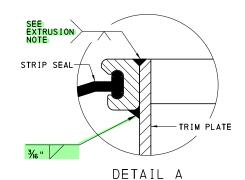


SECTION VIEW

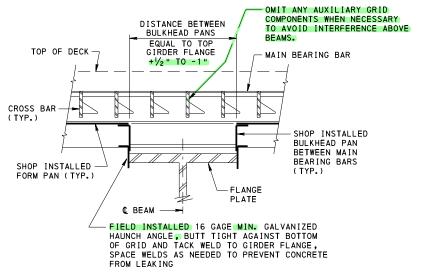
HALF DEPTH CONCRETE GRID

FORM PAN INSTALLATION DETAIL

FULL DEPTH GRID SIMILAR BUT WITHOUT THE BULKHEADS



EXTRUSION NOTE: ONE PIECE EXTRUSION IN LIEU OF TWO PIECE MEMBER (EXTRUSION AND PLATE COMBINATION) IS PERMITTED. WELD IN ACCORDANCE WITH AASHTO/AWS D1.5M SPECIFICATIONS.



HALF DEPTH CONCRETE GRID IS SHOWN. FULL DEPTH CONCRETE GRID SIMILAR WITH FORM PANS LOCATED AT

BOTTOM FLANGE OF MAIN BEARING BAR.

SECTION VIEW

TYPICAL HAUNCH FORM DETAIL

INSTALLATION NOTES:

- 1. DURING PLACEMENT OF THE GRID PANELS THE CONTRACTOR MUST PLACE EACH PANEL IN ITS PROPER POSITION AND VERIFY ITS LOCATION FROM A COMMON FIXED POINT. DOING SO WILL MINIMIZE CUMULATIVE PLACEMENT ERRORS. CUMULATIVE ERRORS CAN RESULT IN A TOTAL DECK AREA LARGER OR SMALLER THAN THE ACTUAL AREA TO BE FILLED.
- 2. PANELS WITH THE SAME ERECTION MARK ARE INTERCHANGEABLE.
- AS WITH OTHER DECKS THIS IS NOT A LEAK PROOF BRIDGE DECK SYSTEM AND MINOR CONCRETE AND GROUT SEEPAGE MAY OCCUR. FIELD CAULKING BY THE DECK ERECTOR MAY BE REQUIRED TO PREVENT EXCESSIVE CONCRETE AND GROUT LEAKAGE.
- 4. PANEL WIDTHS SHOWN ARE NOMINAL. ADJUST DIMENSION BETWEEN BEARING BARS AT FIELD JOINT TO ACCOUNT FOR TEMPERATURE AND ANY OTHER CONDITIONS AT THE TIME OF INSTALLATION.
- 5. FIELD INSTALL SHEAR STUDS AFTER PANELS ARE PLACED TO AVOID INTERFERENCE WITH GRID COMPONENTS.
- 6. HAVE AN EXPERIENCED REPRESENTATIVE OF MANUFACTURER PRESENT DURING INITIAL INSTALLATION OF GRID DECKING AND AT SUCH OTHER TIMES AS THE ENGINEER MAY REQUEST.

DEPARTMENT OF TRANSPORTATION

STEEL GRID REINFORCED FOR BEAM BRIDGES

RECOMMENDED JAN. 31, 2019 Thomas P. Mariora CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019

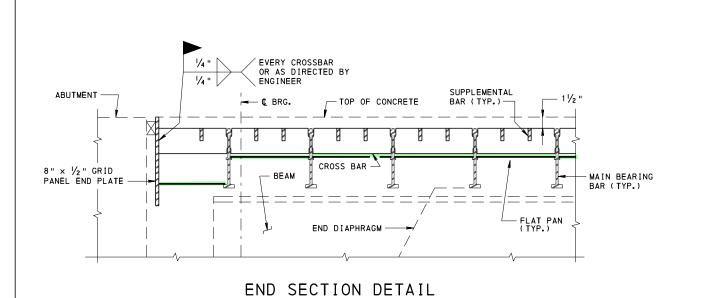
Milwin Harture ACTING DIR. BUR. OF PROJECT DELIVERY BC-726M

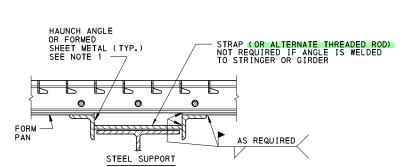
SHEET 2 OF 5



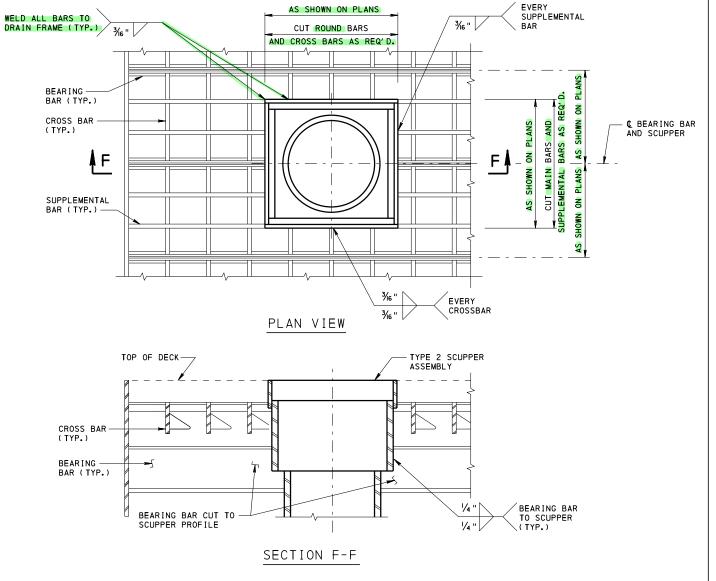
STANDARD

CONCRETE BRIDGE DECK DETAILS CAST-IN-PLACE OR PRECAST DECKS

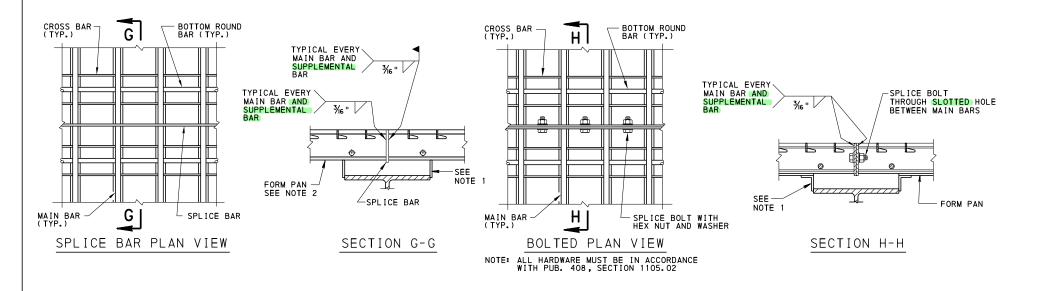




FORMED ANGLE - WELDED STRAP



SCUPPER INSTALLATION DETAILS



- HAUNCH NOTES:
 1. HAUNCH ANGLES NOT TO BE WELDED TO TENSION MEMBERS. USE TIE STRAPS WELDED BETWEEN HAUNCH ANGLES.
- 2. OMIT CONCRETE FORM PAN OVER SUPPORT MEMBERS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

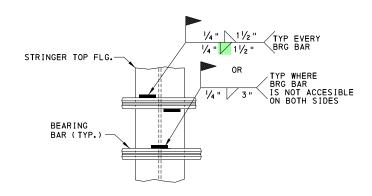
STEEL GRID REINFORCED CONCRETE BRIDGE DECK DETAILS FOR BEAM BRIDGES CAST-IN-PLACE OR PRECAST DECKS

RECOMMENDED JAN. 31, 2019 Thomas P. Mariora CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019
Millia House

SHEET 3 OF 5 ACTING DIR. BUR. OF PROJECT DELIVERY BC-726M

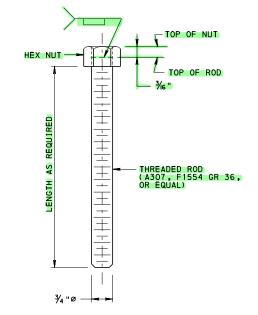
MAIN BAR SPLICE AT PANEL ENDS



OPTIONAL FIELD WELD DETAIL

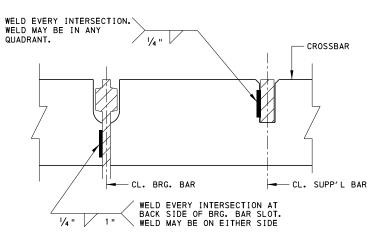
WITHOUT HAUNCH

FIELD NOTE; AFTER FIELD WELDING OF DECK, REPAIR ANY DAMAGE TO GALVANIZING, BY APPLYING A ZINC RICH COLD APPLIED COATING TO

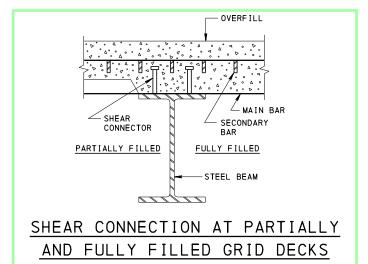


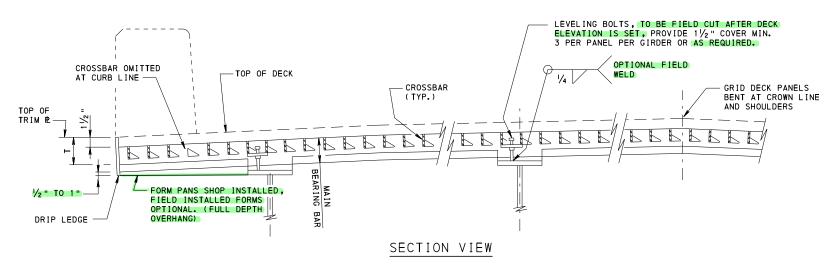
LEVELING BOLT DETAIL

- HEADED BOLT IS PERMITTED IN LIEU OF NUT TO
- LEVELING BOLTS MAY BE FURNISHED UNCOATED.



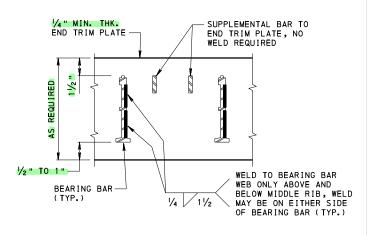
GRID COMPONENT WELD DETAILS



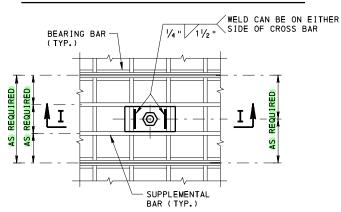


PARTIAL TRANSVERSE SECTION THRU GRID DECK

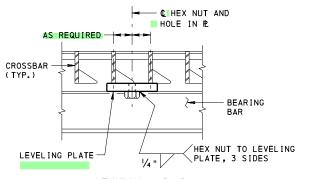
MAIN BAR CAMBERING AS PERMITTED BY AWS D1.5 PUB. 408



END TRIM PLATE WELD DETAIL



PLAN VIEW



SECTION I-I

LEVELING PLATE WELD DETAIL

- HEX NUT CAN BE TAPPED OVERSIZE FOR GALVANIZING.
- THE LEVELING NUT MAY BE PLACED UNDER THE MAIN BAR WHEN CONDITIONS PERMIT. ALTERNATE LEVELING DETAILS
 PERMITTED AS APPROVED BY THE DISTRICT BRIDGE ENGINEER.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

STEEL GRID REINFORCED CONCRETE BRIDGE DECK DETAILS FOR BEAM BRIDGES CAST-IN-PLACE OR PRECAST DECKS

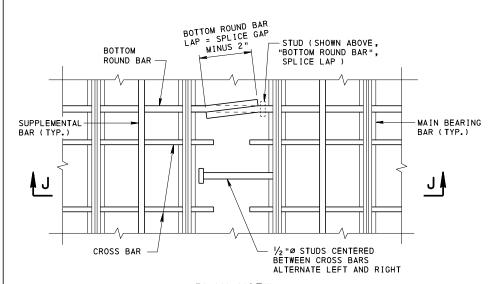
RECOMMENDED JAN. 31, 2019 Thomas P. Mariora

CHIEF BRIDGE ENGINEER

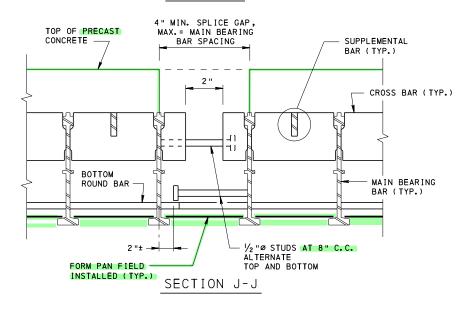
RECOMMENDED JAN. 31, 2019

Millia Halia

ACTING DIR. BUR. OF PROJECT DELIVERY BC-726M



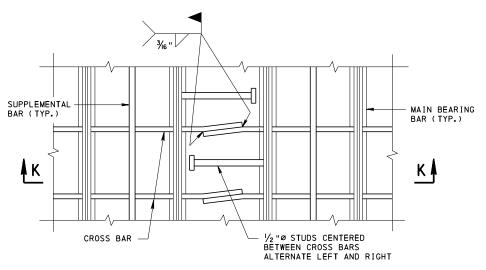
PLAN VIEW



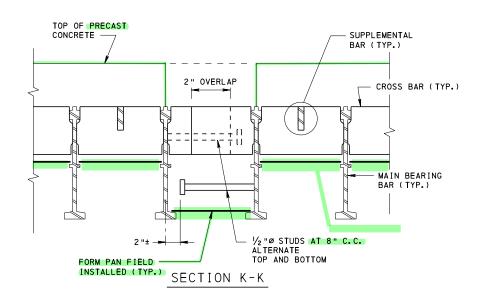
FULL DEPTH CONCRETE GRID

TRANSVERSE SPLICE BETWEEN PANELS

OPTIONAL BOLTED SPLICE PERMITTED AS APPROVED BY THE DISTRICT BRIDGE ENGINEER



PLAN VIEW



HALF DEPTH CONCRETE GRID

TRANSVERSE SPLICE BETWEEN PANELS

OPTIONAL BOLTED SPLICE PERMITTED AS APPROVED BY THE DISTRICT BRIDGE ENGINEER

NOTE: SPLICE DETAILS CAN ALSO BE USED FOR CAST-IN-PLACE WITHOUT BLOCKOUT CLOSURE POURS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

STEEL GRID REINFORCED CONCRETE BRIDGE DECK DETAILS FOR BEAM BRIDGES PRECAST DETAILS

CHIEF BRIDGE ENGINEER

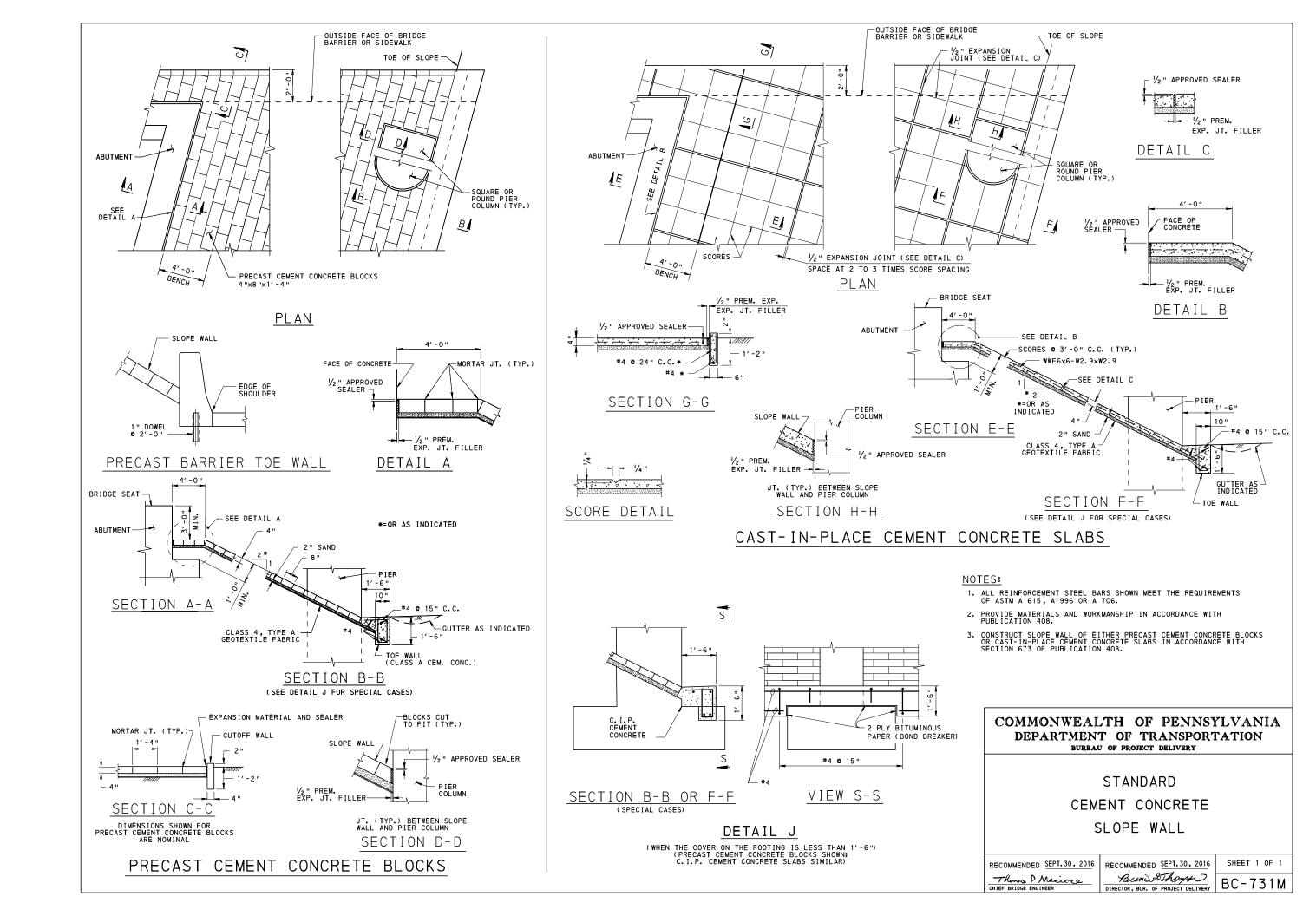
RECOMMENDED JAN. 31, 2019

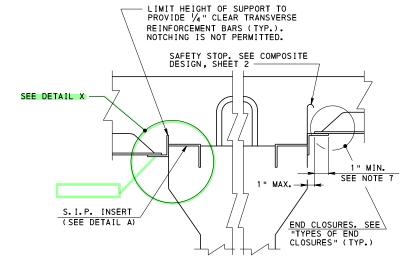
RECOMMENDED JAN. 31, 2019

RECOMMENDED JAN. 31, 2019

Milling House ACTING DIR. BUR. OF PROJECT DELIVERY BC-726M

SHEET 5 OF 5





SUPPORT AT P/S CONCRETE BEAM

DETAIL SHOWN FOR P/S I-BEAM BRIDGE, DETAIL SIMILAR FOR P/S BOX BEAM BRIDGE

NOTE:
THE REQUIREMENT FOR SAFETY STOPS CAN BE
WAIVED IF IT IS SPECIFIED ON THE SHOP DETAIL
DRAWINGS THAT EACH SHEET BE SCREWED DOWN
IMMEDIATELY UPON PLACEMENT. THIS SPECIFICATION
SHOULD BE MADE HIGHLY VISIBLE ON THE DRAWINGS.

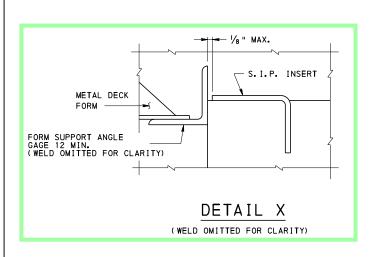
DIRECTION OF PLACING FORMS SLAB TH SHOWN DRAWIN DIRECTION OF PLACING CONCRETE (PREFERRED) METAL DECK FORM — 18" MAX. C-C SPA. — OF FASTENERS OR LESS AS MAX I MUM NOM I NAL PER MANUFACTURER'S
RECOMMENDATION. PROVIDE ADDITIONAL FASTENERS
AS NEEDED TO INSURE A TIGHT FIT. SEE NOTE 11.

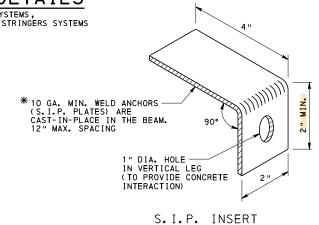
TYPICAL LONGITUDINAL SECTION

- THE MAXIMUM CORRUGATION DEPTH AND WIDTH SHALL BE SUCH THAT THE TOTAL DEAD LOAD OF THE FORM AND THE CONCRETE IN THE FORM DOES NOT EXCEED 15 LB/FT?
- FOR DECK TOP REINFORCEMENT MAT: TRANSVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.

SUGGESTED SUPPORT DETAILS

FOR STEEL BEAM SYSTEMS, STEEL GIRDER SYSTEMS, GIRDER-FLOOR BEAM SYSTEMS AND GIRDER-FLOOR BEAM-STRINGERS SYSTEMS





DETAIL A

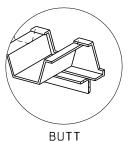
* FOR LARGE BEAM SPACINGS THE DESIGN OF THE INSERT MAY REQUIRE THAT THE GAGE BE INCREASED AND/OR THE SPACING BE DECREASED



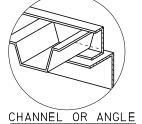
CHANGE 2

CHANGE 4

PRECLOSED DECK FORM END CLOSURE







THIS DETAIL NOT PERMITTED

ANGLE OR CHANNEL

TYPES OF END CLOSURES

ALTERNATE DECK FORM END CLOSURES

NOTES:

- 1. PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH PUB. 408.
- USE THIS STANDARD AS A GUIDE IN THE PREPARATION OF SHOP DETAIL DRAWINGS.
- SHOW THICKNESS AND SIZE OF SUPPORTING ELEMENTS AND METAL DECK FORMS ON THE SHOP DRAWINGS ALONG WITH LENGTH, SIZE AND SPACING OF WELDS.
- METAL DECK FORM CLOSURES AND STYROFOAM FILLERS MAY BE USED AS SHOWN ON DESIGN DRAWINGS TO REDUCE DEAD LOAD. SEAL END CLOSURES TO ENSURE THAT BLEED WATER DOES NOT DRAIN OUT.
- DESIGN ALL METAL DECK FORM SUPPORTS AND THEIR ATTACHMENTS TO CARRY DEAD LOAD OF DECK SLAB (INCLUDES CONCRETE IN CORRUGATIONS) PLUS 50 LBS./SQ. FT. FOR CONSTRUCTION LOADS.
- ALSO INCLUDE ALL RESULTANT HORIZONTAL LOADS DUE TO FORMING OF CANTILEVER OVERHANGS IN THE DESIGN OF METAL DECK FORM SUPPORTS AND ATTACHMENT DETAILS.
- SECURELY FASTEN ALL METAL DECK FORMS TO FORM SUPPORT ANGLES AND PROVIDE A MINIMUM BEARING LENGTH OF 1" AT EACH END.
- ATTACH METAL DECK FORM SHEETS PROPERLY TO AVOID HAZARDS THAT CAN RESULT FROM LATERAL MOVEMENT OR SUDDEN UPLIFT. PROVIDE SAFETY STOPS WHERE
- 9. CONNECT ADJOINING HAUNCH ANGLE OR CHANNEL BY WELDING.
- ALL METAL DECK FORMS MUST HAVE FACTORY CLOSED ENDS.
- USE $\frac{3}{6}$ " HWH × $\frac{1}{4}$ "- 14 THREADS/INCH SCREW FASTENER TO CONNECT METAL DECK FORMS.
- METAL DECK FORMS TO BE DESIGNED FOR MAXIMUM DEPTH OF CONCRETE IN THE BAY TO ACCOUNT FOR A SUPERELEVATION CROWN FALLING BETWEEN BEAMS ADDING SIGNIFICANT ADDITIONAL DECK THICKNESS.
- 13. FOR WELD DETAILS SEE SHEET 2.
- FOR STAGED CONSTRUCTION, DETAIL DECK FORMS SPANNING BETWEEN BEAMS OF DIFFERENT STAGES TO ACCOMMODATE THE VERTICAL AND LATERAL MOVEMENTS DURING CONSTRUCTION INCLUDING DECK PLACEMENT.
- DURING STAGED CONSTRUCTION, DO NOT USE DECK FORMS SPANNING BETWEEN GIRDERS OF DIFFERENT STAGES AS A WORK PLATFORM.
- DESIGN COMPUTATIONS AND SHOP DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE ARE REQUIRED TO BE SUBMITTED FOR CONDITIONS THAT EXCEED THE LIMITATIONS PROVIDED IN

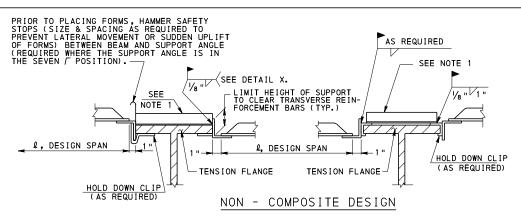
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

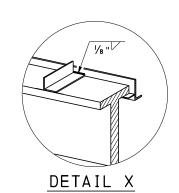
STANDARD PERMANENT METAL DECK FORMS

Havin E. Hray HIEF ENGINEER, HIGHWAY ADMIN

BC-732M

RECOMMENDED NOV. 23, 2022 SHEET 1 OF 3





1" MAX.

→ 1" MIN.

COMPRESSION FLANGE

TOP FLANGE - TENSION

NOTE 1:

TENSION BAR, ANGLE OR CHANNEL, SIZE/SPACING AS REQUIRED. MINIMUM BAR GALVANIZED 2"x 8 GAGE MAX. SPACING 1'-3", ANGLE OR CHANNEL GAGE 12, WITH MAXIMUM SPACING 24". WELDING TYPICAL EACH SIDE, UNLESS NOTED.

PRIOR TO PLACING FORMS, HAMMER SAFETY STOPS (SIZE & SPACING AS REQUIRED TO PREVENT LATERAL MOVEMENT OR SUDDEN UPLIFT OF FORMS) BETWEEN BEAM AND SUPPORT ANGLE (REQUIRED WHERE THE SUPPORT ANGLE IS IN THE SEVEN / POSITION). SEE NOTE 2 SEE NOTE 2 SEE NOTE 2 FULL LENGTH //8 " / 1/8 " 1 " AS REQUIRED 4 AS REQUIRED FULL LENGTH AS REQUIRED /8 " 1"-12" AS REQUIRED GAGE 12-SUPPORT ANGLE GAGE 12 COMPRESSION FLANGE

COMPRESSION FLANGE

SHEET 1 (TYP.)

Q, DESIGN SPAN

COMPOSITE DESIGN CONDITION 1

-COMPRESSION FLANGE

NON - COMPOSITE DESIGN CONDITION 2

ℓ , DESIGN SPAN

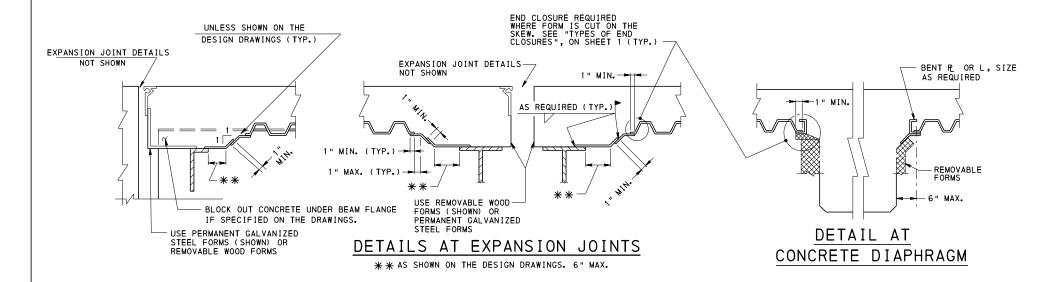
∠
★ CONTINUOUS HAUNCH ANGLE (TYP.)

* FOR REHABILITATIONS, STEEL MUST BE PREPAINTED.

IN NON-COMPOSITE COMPRESSION FLANGES THE HAUNCH ANGLE MAY BE ELIMINATED WHENEVER THE BOTTOM OF THE METAL DECK FORM IS AT OR BELOW THE BOTTOM OF THE TOP FLANGE.

NOTE 2:

HAUNCH ANGLE OR CHANNEL, SIZE/SPACING AS REQUIRED. MINIMUM GAGE 12, WITH MAXIMUM SPACING 24". WELDING TYPICAL EACH SIDE, UNLESS NOTED.



COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD PERMANENT METAL DECK FORMS

RECOMMENDED NOV. 23, 2022 SHEET 2 OF 3 Havin E. Hray BC-732M

T SLAB

METAL FORM. SEE LONGITUDINAL SECTION.

TYPICAL LONGITUDINAL SECTION OF DECK SLAB WITH METAL FORM

FOR DECK TOP REINFORCEMENT MAT: TRANSVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP

DESIGN SPECIFICATIONS

DESIGN THE METAL FORMS ON THE BASIS OF THE DEAD LOAD OF THE FORM, REINFORCEMENT AND PLASTIC CONCRETE PLUS 50 POUNDS PER SQUARE FOOT FOR CONSTRUCTION LOADS. USE UNIT WORKING STRESS IN THE STEEL SHEET AS FOLLOWS:

MATERIAL GRADE ASTM A 653/A 653M OR A 924/A 924M	YIELD (PSI)	ALLOWABLE STRESS (PSI)
33	33,000	23,900
37	37.000	26,800
40	40,000	29,000
50	50,000	36,000
80	80,000	36,000

DO NOT ALLOW DEFLECTION UNDER THE WEIGHT OF THE FORMS, PLASTIC CONCRETE AND REINFORCEMENT TO EXCEED \$\(\extstyle{\ell}\) 180 OF THE FORM SPAN OR 1/2 INCH WHICHEVER IS LESS. IN NO CASE IS THE LOADING FOR DEFLECTION CALCULATION TO BE LESS THAN 120 PSF TOTAL. FOR SPANS IN EXCESS OF TEN FEET THE PERMISSIABLE DEFLECTION IS NOT TO EXCEED 3/4 INCH OR 2/240 WHICHEVER IS LESS.
BASE THE PERMISSIBLE FORM CAMBER ON THE ACTUAL DEAD LOAD CONDITION. DO NOT USE CAMBER TO COMPENSATE FOR DEFLECTION IN EXCESS OF THE FOREGOING LIMITS.

EXAMPLES

EXAMPLE #1

STEEL I-BEAM BRIDGE, COMPOSITE DESIGN BEAM SPACING = 7'-10" C. TO C. BEAM FLANGE WIDTH = 12" SLAB THICKNESS T = 8.5" (2½" CL.) GIVEN:

SECTION MODULUS S, AND MOMENT OF INERTIA I FOR METAL FORM SECTION, GRADE 40 MATERIAL. REQUIRED:

SOLUTION: DESIGN SPAN $\frac{1}{2}$ (FROM SHEET 2 FOR CONDITION 1) = (7'-10") - (12") - (2") = 6'-8"FROM TABLE, FOR $\frac{1}{2} = 6'-8"$ AND $\frac{1}{2} = 8\frac{1}{2}$ (S = .3181 IN. $\frac{3}{2}$ /FT. AND I = .4131 IN. $\frac{4}{2}$ /FT. FOR GRADE 40 MATERIAL, MULTIPLY S BY 1.24 i.e. S REQD. = 1.24 X .3181 = .3944 IN. $\frac{3}{2}$ /FT.

I REQD. = .4131 IN. 4 /FT.

EXAMPLE #2

GIVEN:

STEEL I-BEAM BRIDGE, NON-COMPOSITE DESIGN BEAM SPACING = 7'-10" C. TO C. BEAM FLANGE WIDTH = 12" SLAB THICKNESS T = 8.5" (2½" CL.)

SECTION MODULUS S, AND MOMENT OF INERTIA I FOR METAL FORM SECTION, GRADE 50 MATERIAL, SUPPORT DETAIL AS PER TOP FLANGE-COMPRESSION, REQUIRED:

AS SHOWN ON SHEET 2.

DESIGN SPAN & (FROM SHEET 2 FOR CONDITION 2) = (7'-10") - (12") - (6") = 6'-4"FROM TABLE, FOR Q = 6'-4" AND T = $8\frac{1}{2}$ " S = .2871 IN.³ /FT. AND I = .3542 IN.⁴ /FT. FOR GRADE 50 MATERIAL, MULTIPLY S BY 1.00 i.e. S REQD. = 1.00 X .2871 = .2871 IN.³ /FT. I REQD. = .3542 IN. 4 /FT.

LEGEND

- * MULTIPLY VALUES OF S SHOWN IN TABLE BY:
 - 1.00 FOR GRADES 50 AND 80

 - 1.24 FOR GRADE 40 1.34 FOR GRADE 37 1.51 FOR GRADE 33
- \bigotimes ℓ = DESIGN SPAN MEASURED PARALLEL TO THE FORM FLUTE, FOR STEEL STRUCTURES AS SHOWN ON SHEET 1, AND FOR (PRESTRESSED) CONCRETE STRUCTURES TAKE DESIGN SPAN AS CLEAR
 DISTANCE BETWEEN (THE FLANGES OF) THE SUPPORTING BEAMS MINUS TWO INCHES FOR CONDITION 1 AND SIX INCHES FOR
- ▲ = COMPUTE PHYSICAL DESIGN PROPERTIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE AMERICAN IRON AND STEEL INSTITUTE SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURALMEMBERS, LATEST PUBLISHED EDITION.

REQUIRED SECTION MODULUS AND MOMENT OF INERTIA OF FORMS

*	S. SE	CTION	MODIII I	JS IN. ³	/FT.		⊗	1. M	OMENT O	F INFRT	1Δ IN. ⁴	/FT. ▲
				S, INCH			l,		I, MOMENT OF INERTIA IN. 4 /FT. ▲ T, SLAB THICKNESS, INCHES			
7	71/2	8	8½	9	91/2	10	DESIGN SPAN	<u><</u> 8		9	91/2	10
.0574	.0597	.0621	.0644	.0667	.0691	.0714	3'-0"	.0371	8½ .0376	.0395	.0415	.0434
.0607	.0632	.0656	.0681	.0706	.0730	.0755	3'-1"	.0404	.0410	.0431	.0452	.0473
.0640	.0666	.0693	.0719	.0745	.0771	.0797	3'-2"	.0438	.0444	.0467	.0490	.0512
.0673	.0701	.0728	.0756	.0784	.0811	.0838	3'-4"	.0471	.0478	.0502	.0527	.0551
.0744	.0775	.0805	.0836	.0866	.0897	.0927	3′ -5"	.0550	.0558	.0587	.0615	.0644
.0780	.0812	.0844	.0876	.0908	.0940	.0972	3'-6" 3'-7"	.0589	.0598	.0629	.0659	.0690
.0858	.0893	.0928	.0963	.0998	.1033	.1068	3′ -8"	.0679	.0689	.0725	.0760	.0795
.0897	.0934	.0970	.1006	.1043	.1080	.1116	3'-9" 3'-10"	.0724	.0735	.0773	.0810	.0848
.0979	.1019	.1059	.1099	.1139	.1179	.1219	3'-11"	.0776	.0787	.0828	.0868	.0969
.1020	.1062	.1103	.1145	.1187	. 1228	.1270	4'-0"	.0879	.0892	.0938	.0983	.1029
.1064	.1108	.1151	.1194	.1238	.1282	.1325	4'-1"	.0937	.1010	.1000	.1048	.1097
.1152	.1199	.1246	.1293	.1340	.1387	.1434	4′ - 3 "	.1054	.1069	.1124	.1179	. 1234
.1198	.1247	.1296	.1345	.1394	.1443	.1492	4' - 4 " 4' - 5 "	.1120	.1136	.1194	.1253	.1311
.1291	.1344	.1396	.1449	.1502	. 1554	.1607	4'-6"	.1251	.1269	.1334	.1400	.1465
.1340	. 1395	.1449	.1504	. 1559	. 1613	.1668	4'-7"	.1324	. 1343	.1412	.1482	. 1551
.1390	.1447	.1503	.1560	.1616	.1673	.1730	4'-8"	.1398	.1418	.1491	.1563	.1636
.1491	. 1552	.1612	.1673	.1733	.1794	.1855	4' -10"	.1553	. 1575	.1656	. 1737	.1818
.1542	.1605	.1668	.1731	.1794	.1857	.1920	4'-11" 5'-0"	.1634	.1652	.1743	.1828	.1913
.1648	.1716	.1783	.1851	.1917	.1985	.2052	5′ - 1 "	.1806	.1833	.1926	.2021	.2115
.1702	.1772	.1842	.1912	.1981	.2051	.2120	5'-2" 5'-3"	.1897	.1924	.2023	.2122	.2220
.1757	.1829	.1963	.1972	.2044	.2185	.2188	5'-3"	.1987	.2016	.2119	.2223	.2326
.1871	.1947	.2024	.2100	.2177	. 2254	.2330	5′-5"	.2185	.2217	.2330	.2444	.2558
.1928	.2007	.2086	.2164	.2243	.2323	.2401	5′-6" 5′-7"	.2284	.2317	.2436	.2555	.2674
.2048	.2137	.2226	.2315	.2404	.2494	.2583	5′-8"	. 2501	2538	.2668	.2798	.2929
.2108	.2202	.2297	.2391	.2485	.2580	.2674	5'-9" 5'-10"	.2610	.2648	.2784	.2920	.3056
.2232	.2264	.2420	.2453	.2608	.2703	.2735	5'-11"	.2729	.2768	.3037	.3053	.3334
.2294	.2388	.2482	.2576	.2670	.2764	.2858	6′-0"	.2966	.3009	.3164	.3318	.3473
.2359	.2456	.2553	.2649	.2746	.2842	.2939	6'-1" 6'-2"	.3095	.3140	.3301	.3462	.3624
.2490	.2592	.2694	.2795	.2897	.2999	.3101	6′ - 3 "	. 3352	.3401	.3576	.3750	.3925
.2558	.2663	.2767	.2871	.3055	.3081	.3185	6' - 4 " 6' - 5 "	.3491	.3542	.3724	.3906	.4088
.2694	.2804	.2914	.3024	.3134	.3244	.3354	6'-6"	.3770	.3825	.4021	.4218	.4414
.2764	.2877	.2990	.3103	.3216	. 3329	.3442	6′ -7"	.3921	.3978	.4182	. 4386	. 4590
.2834	.2950	.3066	.3181	.3379	.3413	.3529	6′-8" 6′-9"	.4071	.4131	. 4343	.4555	.4767
.2977	.3099	.3221	.3342	.3464	. 3586	.3708	6' -10"	. 4384	.4449	. 4677	. 4905	.5133
.3051	.3176	.3300	.3425	.3549	.3674	.3798	6' -11" 7' -0"	.4546	.4613	.4850 .5023	.5086 .5268	.5323
.3200	.3331	.3461	.3592	.3722	.3853	.3983	7' - 1 "	.4883	. 4955	.5209	.5463	.5717
.3275	.3409	.3543	.3676	.3810	. 3944	.4078	7'-2"	.5058 .5232	.5131	.5395 .5581	.5658 .5853	.5922 .6126
.3429	.3570	.3710	.3849	.3989	.4130	.4270	7'-4"	.5419	.5497	.5780	.6062	.6344
.3508	. 3651	.3794	.3938	. 4081	. 4224	. 4367	7′ -5"	.5605	.5687	.5979	.6270	. 6563
.3586	.3733	.3879	.4026	.4172	.4319	.4465	7′-6"	.5792 .6063	.5876 .6151	.6178	.6479	.6781
.3747	.3901	. 4053	. 4207	. 4360	. 4514	. 4666	7′-8"	. 6333	.6425	. 6755	.7085	.7415
.3828	.3985	.4141	.4298	.4454	.4611	.4767	7′-9" 7′-10"	.6604 .6902	.7002	.7044	.7388	.7732
.3996	.4159	. 4322	.4486	. 4649	. 4813	.4976	7'-11"	.7200	.7305	.7680	.8055	.8430
.4080	.4247	.4413	.4580	.4746	.4913 .5017	.5080 .5187	8'-0" 8'-1"	.7498	.7607	.7998	.8388	.8779 .9162
. 4253	. 4427	.4600	.4774	. 4948	.5121	.5295	8'-2"	.8153	.8272	.8697	.9121	.9546
.4339	. 4516 . 4609	.4693	.4871	.5048 .5151	.5225 .5332	.5402	8'-3" 8'-4"	.8480	.8604	.9046	.9487	.9929
.4428	.4702	.4790	.4971 .5071	.5255	.5332	.5513 .5624	8'-4"	.8839 .9197	.8968 .9331	.9428	.9888 1.0289	1.0348
. 4606	. 4794	. 4982	.5171	.5359	. 5547	.5735	8'-6"	. 9556	.9695	1.0193	1.0690	1.1188
.4698	.4890	.5082 .5180	.5274 .5376	.5465 .5572	.5657 .5767	.5849	8'-7" 8'-8"	.9947 1.0339	1.0092	1.0610	1.1128 1.1566	1.1646
. 4881	.5080	.5280	.5479	.5678	.5878	.6077	8'-9"	1.0730	1.0886	1.1445	1.2004	1.2563
.4975	.5178	.5381	• 5585 5691	.5788 .5898	.5991 .6105	.6194	8'-10" 8'-11"	1.1157 1.1583	1.1319	1.1900	1.2482	1.3063 1.3562
.5164	.5277 .5375	.5586	.5691 .5797	.6007	.6218	.6429	9'-0"	1.2010	1.2185	1.2356	1.3436	1.4062
.5261	.5476	.5691	.5906	.6120	. 6335	.6550	9'-1"	1.2474	1.2656	1.3306	1.3956	1.4606
.5358	.5577 .5678	.5796 .5901	.6015	.6233	.6452	.6671 .6792	9'-2"	1.2938	1.3126	1.3801	1.4475	1.5149
. 5555	.5782	.6009	.6236	.6462	.6689	.6916	9′ - 4 "	1.3905	1.4107	1.4832	1.5556	1.6281
.5654	.5885 .5989	.6116	.6347	.6578	.6809	.7040	9'-5"	1.4407	1.4617	1.5368	1.6118	1.6869
.5856	.6095	.6334	.6574	.6813	.7052	.7291	9'-7"	1.5454	1.5679	1.6485	1.7289	1.8094
.5958	.6202	.6445	.6689	.6932	.7176	.7419	9′-8"	1.5999	1.6232	1.7065	1.7898	1.8732
.6060	.6308	.6555	.6803 .6921	.7051	.7298	.7546	9'-9" 9'-10"	1.6543		1.7646	1.8507	1.9369 2.0057
.6270	.6526	.6782	.7039	.7295	. 7551	.7807	9'-11"	1.7718	1.7977	1.8900	1.9822	2.0745
.6375	.6636	.6896	.7157	.7417	.7678	.7938	10'-0"	1.8306	1.8573	1.9526	2.0480	2.1433

	S, SECTION MODULUS IN.³/FT.* ▲						₩	I, MO	MENT OF	INERTI	A IN. 4	FT. ▲
	Т	, SLAB	THICKNE	SS, INC	HES		l,		T, SLAB	THICKN	ESS, IN	CHES
7	71/2	8	81/2	9	91/2	10	DESIGN SPAN	<u><</u> 8	81/2	9	91/2	10
.6461	. 6725	.6990	. 7255	.7520	.7784	.8049	10' -1"	1.8295	1.9289	2.0283	2.1277	2.2272
. 6568	.6837	.7106	.7375	.7644	.7914	.8183	10' -2"	1.8752	1.9771	2.0790	2.1809	2.2829
.6676	.6949	.7223	.7497	.7770	.8044	.8317	10' -3"	1.9217	2.0261	2.1306	2.2350	2.3395
.6785	. 7063	. 7341	.7619	.7897	.8175	.8453	10' -4"	1.9689	2.0760	2.1830	2.2900	2.3970
. 6895	.7177	.7460	.7742	.8025	.8308	.8590	10' -5"	2.0170	2.1266	2.2362	2.3458	2.4554
.7005	.7293	.7580	.7867	.8154	.8441	.8728	10'-6"	2.0658	2.1780	2.2903	2.4026	2.5148
.7117	. 7409	.7700	.7992	.8154	.8441	.8867	10' -7"	2.1153	2.2303	2.3453	2.4602	2.5752
.7230	.7526	.7822	.8119	.8415	.8711	.9007	10' -8"	2.1657	2.2834	2.4011	2.5188	2.6365
. 7343	.7644	. 7945	.8246	.8547	.8848	.9149	10'-9"	2.2169	2.3373	2.4578	2.5783	2.6988
.7457	.7763	.8069	.8374	.8680	.8985	.9291	10' -10"	2.2688	2.3921	2.5154	2.6387	2.7620
.7572	.7883	.8193	.8504	.8814	.9124	.9435	10' -11"	2.3219	2.4477	2.5739	2.7001	2.8263
.7689	. 8004	.8319	.8634	.8949	.9264	. 9579	11'-0"	2.3751	2.5042	2.6333	2.7624	2.8915
. 7805	.8125	.8445	.8765	. 9085	.9405	.9725	11'-1"	2.4295	2.5616	2.6936	2.8257	2.9577
.7923	.8248	.8573	.8897	.9222	.9547	.9872	11'-2"	2.4848	2.6198	2.7548	2.8899	3.0249
.8042	.8372	.8701	.9031	.9360	.9690	1.0020	11'-3"	2.5408	2.6789	2.8170	2.9551	3.0931
.8162	.8496	.8831	.9165	.9500	.9834	10169	11'-4"	2.5977	2.7389	2.8800	3.0212	3.1624
.8282	.8621	.8961	.9300	.9640	.9979	1.0319	11'-5"	2.6554	2.7997	2.9440	3.0883	3.2327
.8403	.8748	.9092	.9437	.9781	1.0125	1.0470	11'-6"	2.7140	2.8615	3.0090	3.1565	3.3040
.8526	. 8875	. 9224	.9574	.9923	1.0273	1.0622	11'-7"	2.7734	2.9241	3.0749	3.2256	3.3763
.8649	. 9003	. 9358	.9712	1.0067	1.0421	1.0775	11'-8"	2.8337	2.9877	3.1417	3.2957	3.4497
.8773	.9132	.9492	.9851	1.0211	1.0570	1.0930	11'-9"	2.8948	3.0522	3.2095	3.3668	3.5242
.8898	. 9262	.9781	.9992	1.0356	1.0721	1.1086	11'-10"	2.9569	3.1176	3.2783	3.4390	3.5997
.9023	. 9393	.9763	1.0133	1.0503	1.0872	1.1242	11' - 11"	3.0198	3.1839	3.3480	3.5121	3.6763
.9150	. 9525	.9900	1.0275.	1.0650	1.1025	1.1400	12'-0"	3.0836	3.2512	3.4188	3.5863	3.7539

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD PERMANENT METAL DECK FORMS

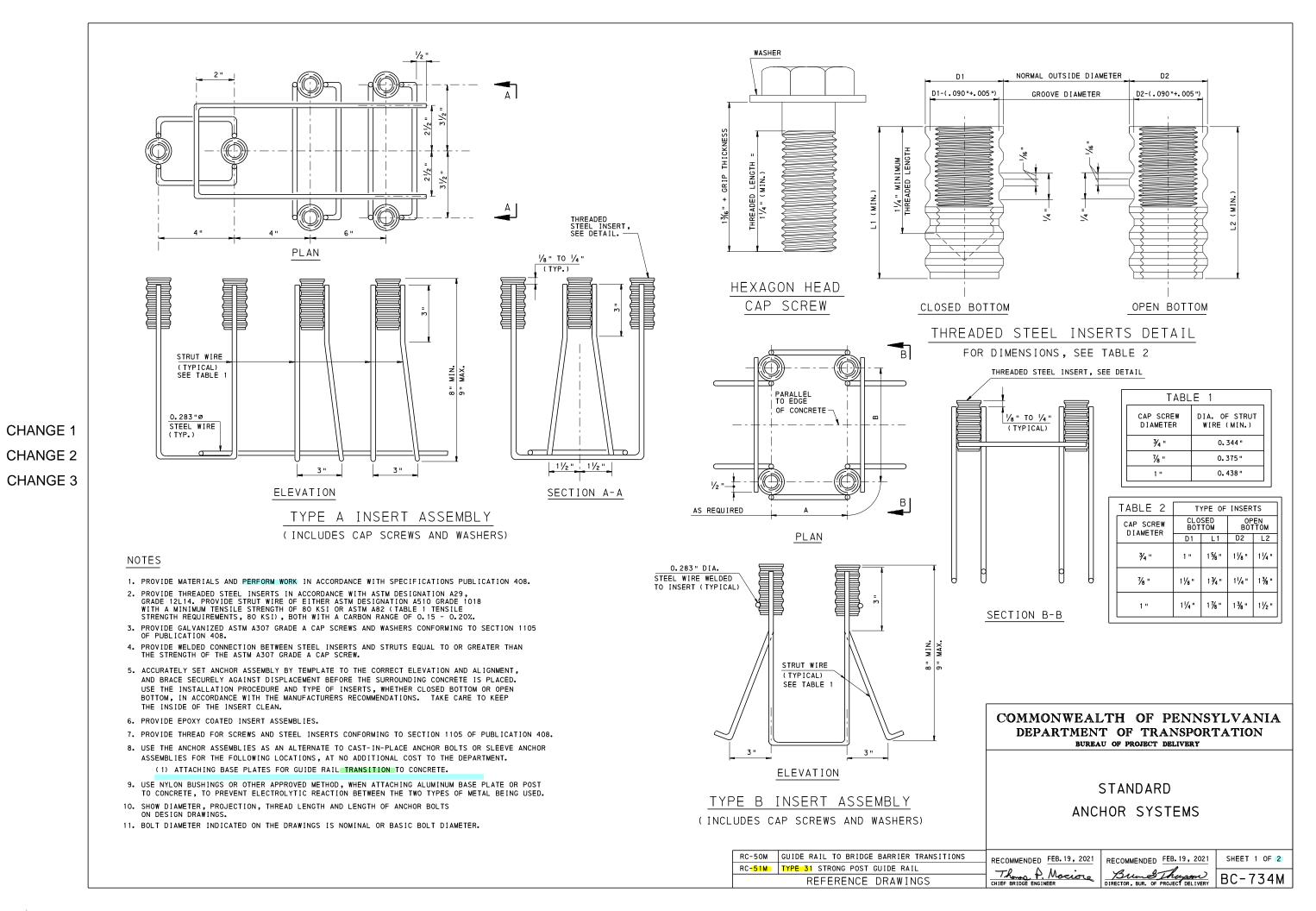
RECOMMENDED NOV. 23, 2022

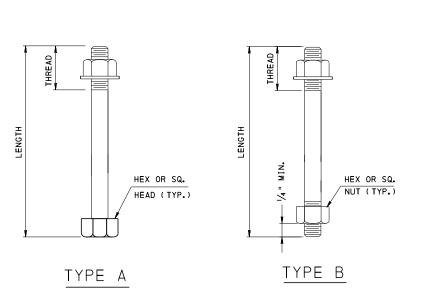
RECOMMENDED NOV. 23, 2022

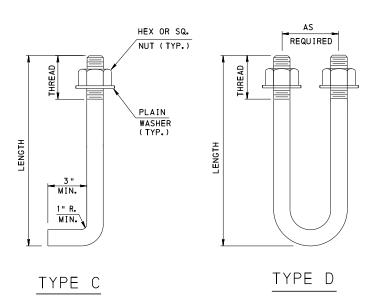
SHEET 3 OF 3

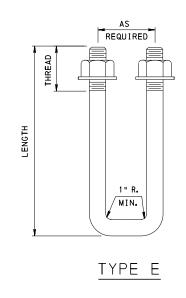
Havin E. Hray

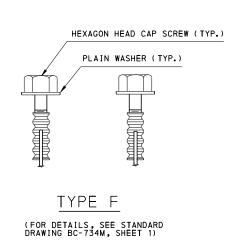
CHIEF ENGINEER, HIGHWAY ADMINI BC-732M



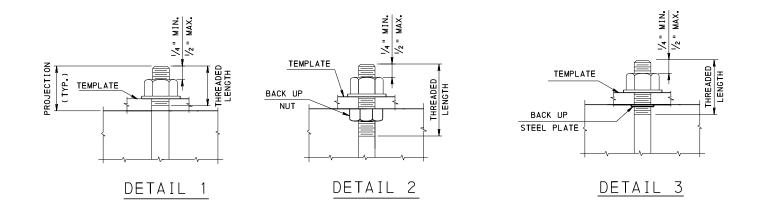








TYPES OF ANCHORS



CONSTRUCTION NOTES

- SET ANCHOR ASSEMBLY BY TEMPLATE TO THE CORRECT ELEVATION AND ALIGNMENT, AND BRACE SECURELY AGAINST DISPLACEMENT BEFORE THE SURROUNDING CONCRETE IS PLACED.
- 2. THE USE OF A BACK UP NUT OR PLATE, AS SHOWN IN DETAIL 2 OR 3, WILL FACILITATE SETTING OF ANCHOR BOLTS TO THEIR CORRECT ELEVATION AND ALIGNMENT. THREADED LENGTH OF ANCHOR BOLTS DEPENDS ON THE METHOD OF INSTALLATION CHOSEN BY THE CONTRACTOR.
- 3. TEMPLATE THICKNESS = BASE OR BASE PLATE THICKNESS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD ANCHOR SYSTEMS

RECOMMENDED FEB. 19, 2021 Thomas A. Macione
CHIEF BRIDGE ENGINEER

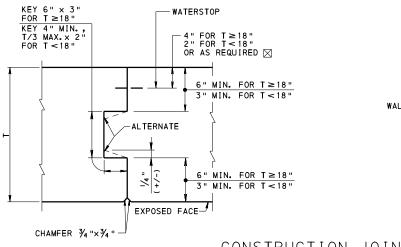
RECOMMENDED FEB. 19, 2021

SHEET 2 OF 2 Bund Thurson BC-734M

1/2 " PREM. EXP. JT. FILLER _ KEY 6" x 3" FOR T ≥18" -WATERSTOP KEY 4" MIN., -4" FOR T≥18" 2" FOR T<18" OR AS REQUIRED ⊠ T/3 MAX. x 2 6" MIN. FOR T≥18" 3" MIN. FOR T < 18" ALTERNATE 6" MIN. FOR T≥18" 3" MIN. FOR T < 18" 1/2 " CAULKING -EXPOSED FACE-COMPOUND CHAMFER 3/4 "×3/4"

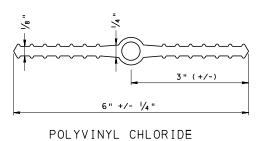
KEYED EXPANSION JOINT

(FLUSH EXPANSION JOINT SIMILAR EXCEPT OMIT KEY)
CHAMFER DIMENSIONS ARE NOMINAL AND MAY VARY 1/4" (+/-).



CONSTRUCTION JOINTS

- 1. KEY DIMENSIONS ARE NOMINAL AND MAY VARY 1/2 " (+/-).
- 2. STOP KEYED JOINTS IN TOP OF EXPOSED WALL FLUSH TO A DEPTH OF 12".
- 3. STOP WATERSTOP 12" FROM TOP OF WALL.



3" (+/-) 6" +/- 1/4"

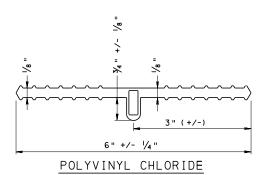
POLYVINYL CHLORIDE

TYPE C1

TYPE C2

WATERSTOPS FOR CONSTRUCTION JOINTS

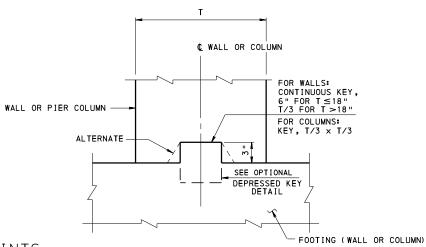
PROVIDE HOLES OR SLOTS IN WATERSTOP, AS REQUIRED, WHEN NECESSARY TO ACCOMMODATE REINFORCEMENT BARS, BUT DO NOT COMPROMISE SEAL.



TYPE E1

WATERSTOPS FOR EXPANSION JOINTS

PROVIDE HOLES OR SLOTS IN WATERSTOP, AS REQUIRED, WHEN NECESSARY TO ACCOMMODATE REINFORCEMENT BARS, BUT DO NOT COMPROMISE SEAL.



3" (+/-) 6" +/- 1/4"

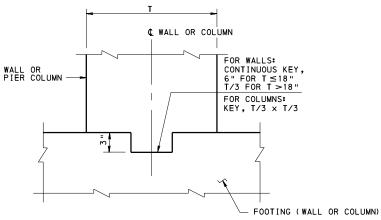
POLYVINYL CHLORIDE

TYPE E2

GENERAL NOTES:

- 1. PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH PUB. 408.
- WHEN TYPE C2 WATERSTOP IS USED TO FACILITATE INSTALLATION, JOIN THE SPLIT-FLANGE TOGETHER USING PVC SOLVENT CEMENT MEETING THE REQUIREMENTS OF ASTM D 2564, AFTER THE FORMS FOR THE FIRST PLACEMENT OF CONCRETE ARE REMOVED.
- 3. CONTRACTOR HAS THE OPTION TO SELECT ANY OF THE WATERSTOPS SHOWN.

- □ = PLACE WATERSTOP INSIDE OF REINFORCEMENT BARS.
- T = THICKNESS OF WALL OR DIAMETER OF COLUMN,



OPTIONAL DEPRESSED KEY DETAIL

OPTIONAL DEPRESSED KEY DETAIL IS TO BE USED ONLY WHEN SHOWN ON CONTRACT DRAWINGS. SURFACES OF DEPRESSED KEY MUST BE ROUGHENED TO ENSURE ADEQUATE BOND WITH NEW CONCRETE.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

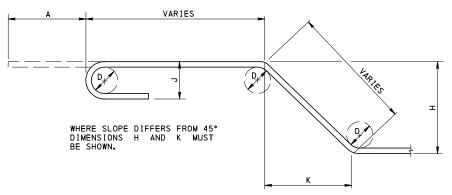
STANDARD WALL CONSTRUCTION & EXPANSION JOINT DETAILS

RECOMMENDED SEPT. 30, 2016 Thoma P Macioca

RECOMMENDED SEPT. 30, 2016 Bun SThomps

DIRECTOR, BUR. OF PROJECT DELIVERY BC-735M

SHEET 1 OF 1



BAR BENDING DETAILS

UNLESS OTHERWISE NOTED, DIAMETER D IS THE SAME FOR ALL BENDS AND HOOKS ON A BAR.

> *6dь 0R 21/2" MIN.

STANDARD REINFORCEMENT BARS

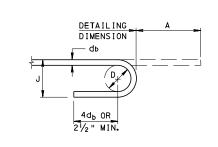
BAR SIZE	WEIGHT (LBS./FT.)	NOMINAL DIAMETER (INCHES)	NOMINAL CROSS SECTIONS AREA (SQ. IN.)	NOMINAL PERIMETER (INCHES)
#3	.376	. 375	.11	1.178
#4	. 668	. 500	.20	1.571
#5	1.043	. 625	.31	1.963
#6	1.502	. 750	. 44	2.356
#7	2.044	.875	.60	2.749
#8	2.670	1.000	.79	3.142
#9	3.400	1.128	1.00	3.544
#10	4.303	1.270	1.27	3.990
#11	5.313	1.410	1.56	4. 430
#14	7.650	1.693	2.25	5.320
#18	13.600	2.257	4.00	7.090

BAR	90° I	HOOKS	1	80° HOOK	S
SIZE	D	Α	D	Α	J
#3	21/4"	6"	21/4"	5 "	3 "
#4	3 "	8 "	3 "	6"	4 "
#5	3 3/4 "	10"	3 3/4 "	7"	5 "
#6	41/2"	1'-0"	41/2 "	8 "	6"
#7	51/4"	1′-2"	51/4"	10"	7 "
#8	6"	1'-4"	6"	11"	8 "
#9	91/2"	1'-7"	91/2"	1′-3"	113/4"
#10	10¾"	1'-10"	10¾"	1′-5"	1'-11/4"
#11	12"	2'-0"	12"	1′-7"	1'-23/4"
#14	181/4"	2′-7"	181/4"	2'-3"	1′-9¾"
#18	24"	3′ -5 "	24"	3′-0"	2'-41/2"

RECOMMENDED END HOOK DIMENSIONS

GRADE 60

BAR	90° I	HOOKS	1	80° HOOK	S
SIZE	D	Α	D	Α	J
#3	21/4"	6"	21/4"	5 "	3 "
#4	3 "	8 "	3 "	6"	4 "
#5	3 3/4 "	10"	3 3/4 "	7 "	5 "
#6	41/2"	1'-0"	41/2 "	8 "	6"
#7	51/4"	1′-2"	51/4"	10"	7"
#8	6"	1′-4"	6"	11"	8 "
#9	91/2"	1'-7"	91/2"	1′-3"	113/4"
#10	10¾"	1'-10"	10¾"	1′-5"	1'-11/4"
#11	12 "	2'-0"	12"	1′-7"	1'-23/4"
#14	181/4"	2′-7"	181/4"	2′-3"	1'-9¾"
#18	24"	3′ -5 "	24"	3′-0"	2'-41/2"



90° HOOKS

DETAILING DIMENSION

180° HOOKS

D = 6db FOR #3 THROUGH #8 D = 8db FOR #9, #10, AND #11

#4	15 FT.	#8	80 FT.
•			
#5	25 FT.	#9	110 FT.
#6	40 FT.	#10	130 FT.
#7	60 FT.	#11	150 FT.

#14 & #18 - ALL BENDING PREFABRICATED.

D = 10db FOR #14 AND #18

BARS THAT ARE REQUIRED TO BE BENT TO A LARGER RADIUS THAN SHOWN IN THE FOLLOWING TABLE MAY BE BENT IN THE FIELD.

#4	15 FT.	#8	80 FT.
•			
#5	25 FT.	#9	110 FT.
#6	40 FT.	#10	130 FT.
#7	60 FT.	#11	150 FT.

GENERAL NOTES:

- 1. PROVIDE MATERIAL AND WORKMANSHIP IN ACCORDANCE WITH THE APPROPRIATE SPECIFICATIONS AS OUTLINED IN THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408 SECTION 1002.
- 2. DESIGNATE REINFORCEMENT BARS AS FOLLOWS TO AVOID ANY MISINTERPRETATION.
 - (A) REFER DEFORMED REINFORCEMENT BAR SIZES BY NUMBER, FOR EXAMPLE,
 - (B) INDICATE PLAIN REINFORCEMENT BAR SIZES BY DIAMETER IN FRACTIONS OF AN INCH, FOR EXAMPLE, $\frac{3}{8}$ "Ø, $\frac{1}{2}$ "Ø, $\frac{5}{8}$ "Ø, ETC.
 - (C) INDICATE STEEL WIRE FABRIC BY A LETTER (W FOR SMOOTH WIRE, D FOR DEFORMED WIRE) FOLLOWED BY A NUMBER WHICH INDICATES THE AREA OF THE WIRE IN HUNDREDTHS OF A SQUARE INCH, FOR EXAMPLE, W16 OR D16.
 - (D) INDICATE SMOOTH STEEL WIRE FABRIC THUS: WWF6x9-W10xW12 (DENOTES LONGITUDINAL WIRES ARE 6 INCHES ON CENTERS, TRANSVERSE WIRES ARE 9 INCHES ON CENTERS). THE AREA OF THE LONGITUDINAL WIRE IS 0.10 SQUARE INCHES AND THE AREA OF THE TRANSVERSE WIRE IS 0.12 SQUARE INCHES.)
 FOR DEFORMED WELDED WIRE FABRIC, DESIGNATE WWF6x9-D10xD12.
- (E) ALL REINFORCEMENT DIMENSIONS ARE MEASURED OUT-TO-OUT OF THE BAR EXCEPT THE "A" DIMENSION ON STANDARD 180° AND 135° HOOKS.
- 3. SPLICING & LAPPING:
- (A) SPLICE BARS ONLY AS SHOWN ON THE DESIGN DRAWINGS OR AS AUTHORIZED BY THE ENGINEER. WHEN LAP SPLICING IS SHOWN ON THE DESIGN DRAWINGS, LAP THE REINFORCEMENT BARS FOR A LENGTH ACCORDING TO AASHTO LRFD ARTICLE 5.10.8.4 AND SECURELY WIRE TOGETHER.
- (B) SPLICE WELDED WIRE FABRIC IN ACCORDANCE WITH AASHTO LRFD ARTICLE 5.10.8.5.
- (C) INCREASE THE BAR LAPS BY 20% FOR A THREE BAR BUNDLE. ADD 33% FOR A FOUR BAR BUNDLE. DO NOT OVERLAP INDIVIDUAL BAR SPLICES WITHIN THE BUNDLE.
- 4. CONFORM REINFORCEMENT BARS TO THE DIMENSIONS SHOWN ON THE DRAWINGS AND WITHIN THE FABRICATION TOLERANCES AS SHOWN IN THE CURRENT "MANUAL OF STANDARD PRACTICE FOR REINFORCED CONCRETE CONSTRUCTION" AS PUBLISHED BY THE CONCRETE REINFORCING

COMMON STOCK STYLES OF WELDED WIRE FABRIC

STYLE DESIGNATION	STEEL SQ. IN.		WEIGHT APPROX.
DESTONATION	LONGIT.	TRANS.	LBS. PER 100 S.F.
ROLLS			
6×6-W1.4×W1.4	.028	.028	21
6×6-W2.1×W2.1	.040	.040	29
6×6-W2.9×W2.9	. 058	.058	42
6×6-W4.0×W4.0	.080	.080	58
4×4-W1.4×W1.4	.042	.042	31
4×4-W2.1×W2.1	.060	.060	43
4×4-W2.9×W2.9	.087	.087	62
4×4-W4.0×W4.0	. 120	.120	85
SHEETS		-	-
6×6-W2.9×W2.9	. 058	.058	42
6×6-W4.0×W4.0	.080	.080	58
6×6-W5.5×W5.5	.110	.110	80
4×4-W4.0×W4.0	. 120	.120	85

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD REINFORCEMENT BAR FABRICATION DETAILS

RECOMMENDED NOV. 23, 2022 Havin E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 1 OF 3 BC-736M

CHANGE 2 CHANGE 4

90° HOOKS

STIRRUPS (TIES SIMILAR)

D = 4db FOR #3 THROUGH #5 BARS

D = 6db FOR #6 BARS RECOMMENDED STIRRUP AND TIE HOOK DIMENSIONS

- * 6db FOR #3, #4, AND #5 12d_b FOR #6.
- ** FOR SEISMIC HOOKS USE 6db OR 3" MIN.

	GRADE 60									
		90° HOOK 135°		ноок						
BAR SIZE	D	А	А	APPROXIMATE H						
#3	1 1/2 "	4"	4"	21/2"						
#4	2 "	41/2"	41/2"	3 "						
#5	21/2"	6"	51/2"	3 3/4 "						
#6	4½"	1′-0"	8"	41/2 "						

GRADE 60

135° HOOKS

**6dь OR

21/2" MIN.

	SEISMIC STIRRUP AND TIE							
		135° HOOK						
BAR SIZE	D	А	APPROXIMATE H					
#3	1 1/2 "	4 1/4 "	3 "					
#4	2 "	41/2"	3 "					
#5	21/2"	51/2 "	3 3/4 "					
#6	41/2"	8 "	41/2"					

DEVELOPMENT LENGTH AND LAP SPLICE LENGTH OF DEFORMED BARS IN COMPRESSION

FOR REINFORCING STEEL GRADE 60
AASHTO LRFD SPECIFICATIONS, ARTICLES 5.10.8.2.20 AND 5.10.8.4.50

		00		J. 1. J. 1. 1.					
BAR	BAR DIA.	f' _C =3,000 PSI (CLASS A)		f' _C =3,500 PSI (CLASS AA)		f' _C =4,000 PSI (CLASS AAA OR AAAP)		f' _C =4,500 PSI	
SIZE	OF BAR db (IN.)	DEVELOP. LENGTH (IN.)	SPLICE LENGTH (IN.)	DEVELOP. LENGTH (IN.)	SPLICE LENGTH (IN.)	DEVELOP. LENGTH (IN.)	SPLICE LENGTH (IN.)	DEVELOP. LENGTH (IN.)	SPLICE LENGTH (IN.)
#3	0.375	9	12	8	12	8	12	8	12
#4	0.500	11	15	11	15	10	15	9	15
#5	0.625	14	19	13	19	12	19	12	19
#6	0.750	17	23	16	23	15	23	14	23
#7	0.875	20	27	18	27	17	27	16	27
#8	1.000	22	30	21	30	19	30	18	30
#9	1.128	25	34	23	34	22	34	21	34
#10	1.270	28	39	26	39	24	39	23	39
#11	1.410	31	43	29	43	27	43	26	43
#14	1.693	37	51	35	51	32	51	31	51
#18	2.257	50	68	46	68	43	68	41	68

NOTE: A FACTOR OF 0.75 CAN BE APPLIED IF THE REINFORCEMENT IS ENCLOSED WITHIN A SPIRAL COMPOSED OF BARS NOT LESS THAN 1/4" IN DIAMETER AND SPACED AT NOT MORE THAN 4" PITCH. BUT THE DEVELOPMENT LENGTH CANNOT BE LESS THAN 8" AND SPLICE LENGTH CANNOT BE LESS THAN 12".

DEVELOPMENT LENGTH OF STANDARD HOOKS IN TENSION

FOR REINFORCING STEEL GRADE 60
AASHTO LRFD SPECIFICATION, ARTICLE 5.10.8.2.40

		AASHIU LKE	D SECTET	DATION, AN	LICEE 3. 10	. 0. 2. Tu
BAD	DIA	f' _C =3,000 PSI	f' _C =3,500 PSI	f' _C =4,000 PSI	f' _C =4,500 PSI	□ d _b □
BAR SIZE	DIA. OF BAR d _b (IN.)	ldh(IN.)	ldh(IN.)	ldh(IN₊)	ldh(IN.)	CRITICAL SECTION
#3	0.375	9	8	8	7	rd _P □ T
#4	0.500	11	11	10	9	
#5	0.625	14	13	12	12	1 1 1 ())
#6	0.750	17	16	15	14	1 44
#7	0.875	20	18	17	16	4db OR 21/2" MIN.
#8	1.000	22	21	19	18	4d _b #3 THROUGH #8
#9	1.128	25	23	22	21	5d _b #9,#10 AND #11
#10	1.270	28	26	25	23	ldh
#11	1.410	31	29	27	26	Kan =

NOTE:

- 1. A FACTOR OF 0.8 CAN BE APPLIED, IF THE SIDE COVER
 (NORMAL TO PLANE OF HOOK) IS NOT LESS THAN 2 ½ ", AND FOR 90° HOOK,
 COVER ON BAR EXTENSION BEYOND HOOK IS NOT LESS THAN 2".
- 2. INCREASE THE DEVELOPMENT LENGTH, ldh, BY 1.2 FOR EPOXY COATED HOOKS IN TENSION.

COMMONWEALTH OF PENNSYLVANIA
DEPARTMENT OF TRANSPORTATION
BRIDGE OFFICE

STANDARD
REINFORCEMENT BAR
FABRICATION DETAILS

RECOMMENDED NOV. 23, 2022

RECOMMENDED NOV. 23, 2022

Haw E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 2 OF 3
BC-736M

DEVELOPMENT LENGTH AND LAP SPLICE LENGTH OF DEFORMED BARS IN TENSION

AASHTO LRFD SPECIFICATIONS, ARTICLES 5.10.8.2.1, 5.10.8.2.2 AND 5.10.8.4.5a

TABLE A

f'c = 3000 PSI (CLASS A) GRADE 60

SIZE	00SS 0N 0. IN.)	HOR	BARS E ZONTAL SEE NOT	BARS		IZONTAL SEE NOT	
BAR S	AR CR SECTION EA (SO	CTH CTH IN.)	LENGTH SPLICE	OF LAP	CTH STH IN.)	LENGTH SPLICE	
	P BA) PY PEN Sq. (A	В	SCEVE ACEN ACEN	A	В
#3	0.11	13	13	17	17	17	22
#4	0.20	17	17	22	22	22	29
#5	0.31	21	21	28	28	28	36
#6	0.44	25	25	33	33	33	43
#7	0.60	30	30	38	38	38	50
#8	0.79	34	34	44	44	44	57
#9	1.00	42	42	54	54	54	70
#10	1.27	51	51	67	67	67	87
#11	1.56	62	62	80	80	80	104

f'c = 3500 PSI (CLASS AA) GRADE 60

$\overline{}$							
SIZE	COSS ON IN.)	HOR	BARS E ZONTAL SEE NOT	BARS		IZONTAL SEE NOT	
BAR S	SECTI FA (S	GTHON.	LENGTH SPLICE	OF LAP	EVELOP. ENGTH I (IN.)	LENGTH SPLICE	OF LAP
1	P ARI	SEVE PENE SEVE	A	В	DEVE PLEN	A	В
#3	0.11	12	12	16	16	16	21
#4	0.20	16	16	21	21	21	27
#5	0.31	20	20	26	26	26	33
#6	0.44	24	24	31	31	31	40
#7	0.60	27	27	36	36	36	46
#8	0.79	31	31	41	41	41	53
#9	1.00	39	39	50	50	50	65
#10	1.27	48	48	62	62	62	80
#11	1.56	57	57	74	74	74	96

f'c = 4000 PSI (CLASS AAA OR AAAP) GRADE 60

SIZE	00SS ON Q. IN.)	HOR	BARS E ZONTAL SEE NOT	BARS		IZONTAL SEE NOT	
BAR S	AR CR SECTION SA (SO	CTH IN.)		OF LAP	CTH.	LENGTH SPLICE	
	G ARE	DEVE LEN	A	В		Α	В
#3	0.11	12	12	16	16	16	21
#4	0.20	15	15	19	19	19	25
#5	0.31	18	18	24	24	24	31
#6	0.44	22	22	29	29	29	37
#7	0.60	26	26	33	33	33	43
#8	0.79	29	29	38	38	38	49
#9	1.00	36	36	47	47	47	61
#10	1.27	45	45	58	58	58	75
#11	1.56	53	53	69	69	69	90
			,	,		,	

f'c = 4500 PSI GRADE 60

SIZE	ROSS ION SQ. IN.)		BARS I IZONTAL SEE NOT			IZONTAL SEE NOT	
BAR S	AECTO	STOP.		OF LAP	COP.	LENGTH SPLICE	OF LA
	P BAF		A	В	DEVELO LENGI QG (IN	Α	В
#3	0.11	12	12	16	16	16	21
#4	0.20	14	14	18	18	18	23
#5	0.31	17	17	23	23	23	29
#6	0.44	21	21	27	27	27	35
#7	0.60	24	24	31	31	31	41
#8	0.79	28	28	36	36	36	46
#9	1.00	34	34	44	44	44	57
#10	1.27	42	42	55	55	55	71
#11	1.56	50	50	65	65	65	85

TABLE B

f'c = 3000 PSI (CLASS A) GRADE 60

SIZE	ROSS ON O. IN.)	HOR	BARS E ZONTAL SEE NOT	BARS		IZONTAL SEE NOT	
BAR S	AR CRI SECTION EA (SO	CTH IN.	LENGTH SPLICE		GTH.	LENGTH SPLICE	
	P B.	SPA SPA SPA SPA SPA SPA SPA SPA SPA SPA	A	В	DEVEL RENG RA (II	A	В
#3	0.11	19	19	25	22	22	28
#4	0.20	25	25	33	29	29	37
#5	0.31	32	32	41	36	36	46
#6	0.44	38	38	49	43	43	56
#7	0.60	44	44	57	50	50	65
#8	0.79	50	50	65	57	57	74
#9	1.00	62	62	81	71	71	92
#10	1.27	77	77	100	87	87	113
#11	1.56	92	92	120	104	104	136

f'c = 3500 PSI (CLASS AA) GRADE 60

SIZE	CROSS TION SQ. IN.)	HOR	BARS E ZONTAL EE NOT	BARS		IZONTAL EE NOT	
BAR S	SEC.	STON N.	LENGTH SPLICE	(IN.)	ĞĖŽ.	LENGTH SPLICE	
	P BA	DEVE SLEN	A	В	DEVE PLENC Q d (A	В
#3	0.11	18	18	23	20	20	26
#4	0.20	24	24	31	27	27	35
#5	0.31	29	29	38	33	33	43
#6	0.44	35	35	46	40	40	52
#7	0.60	41	41	53	46	46	60
#8	0.79	47	47	61	53	53	69
#9	1.00	58	58	75	65	65	85
#10	1.27	71	71	92	81	81	105
#11	1.56	85	85	111	97	97	126

f'c = 4000 PSI (CLASS AAA OR AAAP) GRADE 60

SIZE	SIZE CROSS TION SQ. IN.)		BARS E IZONTAL SEE NOT	BARS	HORIZONTAL BARS		
BAR S	SECT C	STO N.	LENGTH SPLICE	OF LAP	CTH.	LENGTH SPLICE	OF LAP
	q _b		A	В	DEVEL PLENG Pd II	A	В
#3	0.11	17	17	22	19	19	24
#4	0.20	22	22	29	25	25	32
#5	0.31	27	27	36	31	31	40
#6	0.44	33	33	43	37	37	48
#7	0.60	38	38	50	43	43	56
#8	0.79	44	44	57	49	49	64
#9	1.00	54	54	70	61	61	79
#10	1.27	67	67	86	75	75	98
#11	1.56	80	80	104	90	90	117

f'c = 4500 PSI GRADE 60

SIZE	20SS ON Q. IN.)	HOR	BARS E IZONTAL SEE NOT	BARS		IZONTAL EE NOT	
BAR S	BAR CRO SECTIC REA (SQ	CTH IN.	LENGTH SPLICE	OF LAP	CTH.	LENGTH SPLICE	OF LAP
	P B	SPA SPA SPA SPA SPA SPA SPA SPA SPA SPA	A	В	DEVEL PLENG 11	A	В
#3	0.11	16	16	20	18	18	23
#4	0.20	21	21	27	24	24	31
#5	0.31	26	26	34	29	29	38
#6	0.44	31	31	40	35	35	46
#7	0.60	36	36	47	41	41	53
#8	0.79	41	41	53	47	47	61
#9	1.00	51	51	66	58	58	75
#10	1.27	63	63	82	71	71	92
#11	1.56	75	75	98	85	85	111

TABLE C

f'c = 3000 PSI (CLASS A) GRADE 60

SIZE	ROSS ON O. IN.		BARS E ZONTAL SEE NOT			IZONTAL SEE NOT	
BAR S	AR CR SECTION EA (SO	CTH IN.)	LENGTH SPLICE	OF LAP	LOP. GTH IN.)	LENGTH SPLICE	OF LAP
	A _P	SCENE SCENE	A	В	DEVE PLENC	A	В
#3	0.11	15	15	20	20	20	26
#4	0.20	20	20	26	26	26	34
#5	0.31	25	25	33	33	33	43
#6	0.44	30	30	39	39	39	51
#7	0.60	35	35	46	46	46	60
#8	0.79	40	40	52	52	52	68
#9	1.00	50	50	65	65	65	84
#10	1.27	62	62	80	80	80	104
#11	1.56	74	74	96	96	96	124

f'c = 3500 PSI (CLASS AA) GRADE 60

SIZE	CROSS FION SQ. IN.)	HOR	BARS E ZONTAL EE NOT	BARS		IZONTAL EE NOT	
BAR S	SEC	CTH IN.)	LENGTH SPLICE	OF LAP	CTHON.	LENGTH SPLICE	OF LAI
	P BA	DEVE LEN	A	В	DEVE PCEN	A	В
#3	0.11	14	14	19	19	19	24
#4	0.20	19	19	25	25	25	32
#5	0.31	24	24	31	31	31	40
#6	0.44	28	28	37	37	37	47
#7	0.60	33	33	43	43	43	55
#8	0.79	37	37	49	49	49	63
#9	1.00	46	46	60	60	60	78
#10	1.27	57	57	74	74	74	96
#11	1.56	68	68	89	89	89	115

f'c = 4000 PSI (CLASS AAA OR AAAP) GRADE 60

SIZE	OSS ON IN.)		BARS E ZONTAL SEE NOT			IZONTAL EE NOT							
BAR S	SECTION (S							STHO IN: US:	LENGTH SPLICE	OF LAP	COP.	LENGTH SPLICE	OF LAP
	P BA		A	В		A	В						
#3	0.11	13	13	17	17	17	22						
#4	0.20	18	18	23	23	23	30						
#5	0.31	22	22	29	29	29	37						
#6	0.44	26	26	34	34	34	44						
#7	0.60	31	31	40	40	40	52						
#8	0.79	35	35	45	45	45	59						
#9	1.00	43	43	56	56	56	73						
#10	1.27	53	53	69	69	69	90						
#11	1.56	64	64	83	83	83	108						

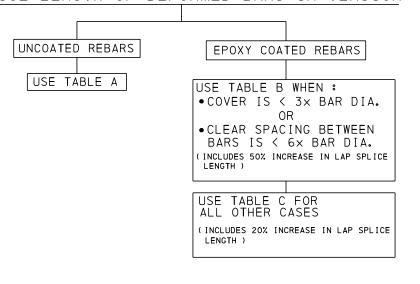
f'c = 4500 PSI GRADE 60

SIZE	ROSS ION IN.)		BARS E ZONTAL EE NOT			IZONTAL EE NOT			
BAR S	AR CF SECTI EA (S	OTION N. N. N.	LENGTH SPLICE		CTH.	LENGTH SPLICE	OF LAP		
	P B		A	В	DEVE PLENC Ad (A	В		
#3	0.11	13	13	16	16	16	21		
#4	0.20	17	17	22	22	22	28		
#5	0.31	21	21	27	27	27	35		
#6	0.44	25	25	32	32	32	42		
#7	0.60	29	29	38	38	38	49		
#8	0.79	33	33	43	43	43	56		
#9	1.00	41	41	53	53	53	69		
#10	1.27	50	50	65	65	65	85		
#11	1.56	60	60	78	78	78	102		

NOTES FOR DEFORMED BARS IN TENSION (TABLES A, B AND C):

- 1. DEVELOPMENT LENGTH:
- REFER TO AASHTO/LRFD FOR APPLICABLE MODIFICATION FACTORS.
- TABLES ARE BASED ON NORMAL-WEIGHT CONCRETE.
- DEVELOPMENT LENGTHS ARE BASED ON 2 INCHES OF COVER. THE DEPTH OF COVER AND HALF THE THICKNESS OF REINFORCEMENT IS ASSUMED TO BE LESS THAN HALF THE CENTER TO CENTER SPACING OF THE REINFORCEMENT.
- 2. LAP SPLICE LENGTH:
- LAP SPLICE LENGTHS ARE BASED ON 2 INCHES OF COVER. THE DEPTH OF COVER AND HALF THE THICKNESS OF REINFORCEMENT IS ASSUMED TO BE LESS THAN HALF THE CENTER TO CENTER SPACING OF THE REINFORCEMENT.
- CLASS A SPLICES MAY BE USED WHERE THE AREA OF REINFORCEMENT PROVIDED IS AT LEAST TWICE THAT REQUIRED BY ANALYSIS OVER THE ENTIRE LENGTH OF THE LAP SPLICE AND ONE-HALF OR LESS OF THE TOTAL REINFORCEMENT IS SPLICED WITHIN THE REQUIRED LAP SPLICE LENGTH.
- ASSUME CLASS B SPLICES IN THE ABSENCE OF DESIGN ANALYSIS.
- 3. HORIZONTAL REINFORCEMENT HAS MORE THAN 12 INCHES OF FRESH CONCRETE CAST BELOW THE REINFORCEMENT.

GUIDELINES FOR USE OF DEVELOPMENT LENGTH AND SPLICE LENGTH OF DEFORMED BARS IN TENSION



COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD
REINFORCEMENT BAR
FABRICATION DETAILS

RECOMMENDED NOV. 23, 202

RECOMMENDED NOV. 23, 2022

LAWN E. HANNEL CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 3 OF 3

BC-736M

INFORMATION CONTAINED IN THE BD-641M DESIGN TABLES

- DESIGN TABLES ON STANDARD DRAWING BD-641M WERE DEVELOPED USING A COMPUTER PROGRAM AND ARE BASED ON THE DESIGN CRITERIA SHOWN ON THIS SHEET.
- THE MEMBER SIZES INDICATED IN THE DESIGN TABLES MEET THE FATIGUE REQUIREMENTS FOR FATIGUE CATEGORY II. THE DESIGNER MUST CHECK THE ADEQUACY OF THE MEMBER SIZES INDICATED WHEN THE FATIGUE CATEGORY IS SPECIFIED TO BE I FOR THE PROJECT.
- THE SPAN RANGE INCLUDED ON STANDARD DRAWING BD-641M IS AS FOLLOWS:

BD-641M: CANTILEVER AND CENTER-MOUNT STRUCTURES, STRUT LENGTHS UP TO 40'

THE DESIGN TABLES INCLUDE MEMBER SIZES FOR THE STRUCTURES FOR VARIOUS COMBINATIONS OF COLUMN HEIGHT, SPAN LENGTH, AND SIGN AREA. THEY ALSO INCLUDE SPREAD FOOTING DESIGNS. ALTERNATE CAISSON FOUNDATIONS ARE PERMITTED, HOWEVER, THE REQUIRED CAISSON EMBEDMENT AND REINFORCEMENT MUST BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PENNSYLVANIA. THE DESIGN COMPUTATIONS MUST BE SUBMITTED TO THE DISTRICT BRIDGE ENGINEER FOR REVIEW AND APPROVAL. THE CORRESPONDING FABRICATION AND CONSTRUCTION DETAILS ARE CONTAINED IN THIS STANDARD

GENERAL NOTES

- 1. PROVIDE 3-INCH CONCRETE COVER ON REINFORCEMENT BARS, EXCEPT AS NOTED.
- 2. USE CLASS A CEMENT CONCRETE f'c = 3000 PSI IN PEDESTALS, FOOTINGS AND CAISSONS.
- 3. PROVIDE GRADE 60 REINFORCING STEEL BARS THAT MEET THE REQUIREMENTS OF ASTM A615 FOR CONCRETE REINFORCEMENT. DO NOT WELD REINFORCING STEEL BARS.
- 4. RAKE-FINISH ALL HORIZONTAL CONSTRUCTION JOINTS, EXCEPT AS INDICATED.
- 5. VERIFY ALL DIMENSIONS AND GEOMETRY OF THE EXISTING STRUCTURES IN THE FIELD AS NECESSARY FOR PROPER FIT OF THE PROPOSED CONSTRUCTION.
- 6. CHAMFER EXPOSED CONCRETE EDGES 1 INCH BY 1 INCH.

CHANGE 1

- 7. ALL DIMENSIONS SHOWN ARE HORIZONTAL, EXCEPT AS NOTED.
- 8. DIMENSIONS ARE BASED ON A NORMAL TEMPERATURE OF 68 DEGREES F.
- SPREAD FOOTINGS MAY BE ORDERED BY THE ENGINEER TO BE AT ANY ELEVATION OR OF ANY DIMENSIONS NECESSARY TO PROVIDE A PROPER FOUNDATION.
- 10. GALVANIZE ALL STRUCTURAL STEEL, BOLTS, NUTS & WASHERS IN ACCORDANCE WITH PUB. 408, UNLESS STAINLESS STEEL OR OTHERWISE INDICATED.
- 11. PIPE DIAMETERS SHOWN UP TO AND INCLUDING 12 INCHES ARE NOMINAL DIAMETERS. PIPE DIAMETERS SHOWN FROM 14 INCHES AND UP ARE ACTUAL DIAMETERS.
- 12. USE STANDARD SIZE HOLE. THE STANDARD HOLE DIAMETER FOR BOLTS SMALLER THAN
 1" DIAMETER SHALL BE THE NOMINAL DIAMETER OF THE BOLT PLUS 1/6". FOR BOLTS
 1" DIAMETER AND LARGER, THE WIDTH OF EACH STANDARD HOLE SHALL BE THE NOMINAL
- 13. CLEAR DISTANCE BETWEEN BOLT HOLES OR BETWEEN THE BOLT HOLE AND THE END OF THE MEMBER IN THE DIRECTION OF THE APPLIED BEARING FORCE SHALL BE CHECKED.
- 14. PROVIDE ANCHOR BOLT HOLES 1/4" LARGER THAN BOLT DIAMETER.
- 15. PROVIDE A MINIMUM ANCHOR BOLT EMBEDMENT LENGTH OF 20 ANCHOR BOLT DIAMETERS.
- 16. PROVIDE DOUBLE NUTS AND WASHER FOR EACH ANCHOR BOLT.
- 17. STEEL MEMBER COMPONENTS REQUIRING CHARPY V-NOTCH TESTING ARE DESIGNATED ON THE PLANS BY (CVN), PROVIDE STEEL CONFORMING TO THE CVN REQUIREMENTS FOR ZONE 2, NON FRACTURE CRITICAL AS GIVEN IN THE AASHTO MATERIAL SPECIFICATIONS.

DESIGN CRITERIA FOR PENNDOT SIGN STRUCTURES

 DEAD LOADS PENNDOT STD. DWGS. (U.N.O.) * SIGN PANELS LIGHT FIXTURES SIGN SUPPORT BEA COLUMNS, STRUTS TC-8701E OR TC-8701S BC-741M, SHT. 6 BC-741M, SHT. 6 CALCULATED INTERNALLY WITHIN PROGRAM • EXTERNAL LOADS AASHTO SIGN SPECS. ICE LOAD WIND LOAD 3.7
APPENDIX C, SECTION C.3,
EQ. C-1, WITH 80 MPH
WIND AND 30% GUST FACTOR

 GROUP LOADS AASHTO SIGN SPECS. 3.4

STEEL CRITERIA

AASHTO SIGN SPECS.

SECTION PROPERTIES FOR TUBULAR SHAPES MAXIMUM STRESSES FOR TUBULAR SHAPES ALLOWABLE STRESSES FOR TUBULAR SHAPES ALLOWABLE STRESSES FOR SIGN SUPPORTS ALLOWABLE STRESSES FOR BASE PLATES APPENDIX B, TABLE B-1
APPENDIX B, TABLE B-2 5.6 (TABLE 5-3) & 5.11 5. 12 5. 8 ALLOWABLE STRESSES FOR COMBINED STEEL STRESS FATIGUE REQUIREMENTS (FATIGUE CATEGORY II) 5.12 SECTION 11 ALLOWABLE DEFLECTION PERMANENT CAMBER PERMANENT CAMBER
ALLOWABLE STRESSES FOR STRUCTURAL STEEL 10.5 SECTION 5

BOLT CRITERIA AASHTO HIGHWAY BRIDGES (U.N.O.)

ALLOWABLE BOLT STRESSES
SLIP-CRITICAL BOLT ALLOWABLE
BOLT PRYING ACTION
COMBINED BOLT SHEAR AND TENSION
BOLT DESIGN CRITERIA
ALLOWABLE ANCHOR BOLT STRESSES TABLE 10.32.3B 10.32.3.2.1 10.32.3.3.2 10.32.3.3.3 AASHTO SIGN SPECS. 5.16 AASHTO SIGN SPECS. 5.17

 CONCRETE CRITERIA AASHTO HIGHWAY BRIDGES (U.N.O.)

ALLOWABLE BEARING STRESS
REINFORCEMENT TENSILE STRESS
SHEAR CAPACITY OF FOOTINGS
SHEAR STRESS IN FOOTINGS
ALLOWABLE SHEAR STRESS
ALLOWABLE SHEAR STRESS
MINIMUM REINF. OF FLEXURAL MEMBERS
SPACING LIMITS FOR REINFORCEMENT
MINIMUM CONCRETE COVER
PRESSURES FOR ECCENTRICALLY LOADED FOOTINGS
DISTRIBUTION OF REINFORCEMENT
FOOTING STABILITY REQUIREMENTS
TORSION
COLUMN DESIGN (PEDESTALS) 8. 15. 2. 1. 3 8. 15. 2. 2 8. 15. 5. 6. 1 8. 15. 5. 6. 2 8.17.1 8.21 DM4 D8.22.1* FIG. 4.4.7.1.1.1C 4.4.11.2.2 DM4 D5.5.5 ACI SECTION A.7.3* COLUMN DESIGN (PEDESTALS)

SPREAD FOOTINGS

MAXIMUM DESIGN PRESSURE MINIMUM AREA IN BEARING UNIT WEIGHT OF SOIL

1.5 TONS PER SQUARE FOOT 100 POUNDS PER CUBIC FOOT

10.0 POUNDS PER CUBIC INCH

100 POUNDS PER CUBIC FOOT

O KIPS PER SQUARE FOOT

• DRILLED SHAFTS (CAISSONS) DM4 SEC. 4.6, PENNDOT COM624 COMPUTER PROGRAM .5 TONS PER SQUARE FOOT

MAXIMUM DESIGN PRESSURE
MAXIMUM DESIGN LATERAL DISPLACEMENT
MODULUS OF SUBGRADE REACTION
UNIT WEIGHT OF SOIL
ANGLE OF INTERNAL FRICTION

• SEISMIC DESIGN CRITERIA

STRUCTURES ARE DESIGNED FOR A SEISMIC ACCELERATION COEFFICIENT = 0.15

CONSTRUCTION GENERAL NOTES

MATERIALS AND WORKMANSHIP

PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH THE CURRENT VERSIONS OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, AASHTO/AWS WELDING CODE D1.5, CONTRACT SPECIAL PROVISIONS, AND AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS". USE AASHTO/AWS D1.1 FOR WELDING NOT COVERED IN

• PROVIDE STRUCTURAL STEEL CONFORMING TO THE FOLLOWING:

COLUMNS & PIPE STRUTS:

SEE PUBLICATION 408, SECTION 948.2.

AASHTO M270, GRADE 36 ASTM A709, GRADE 36 ANGLES. SHAPES. AND PLATES:

ALTERNATE PRESS-BREAK MEMBERS:

ALTERNATE PRESS-BREAK MEMBERS MUST HAVE THE EQUIVALENT STRENGTH OF THE MEMBER ALTERNATE PRESS-BREAK MEMBERS MUST HAVE THE EQUIVALENT STRENGTH OF THE MEMBER THY ARE REPLACING. EQUIVALENT RADIUS FOR PRESS-BREAK MEMBERS IS MEASURED FROM THE CENTER OF THE MEMBER TO THE MID-POINT OF ANY CHORD OF THE MEMBER. MINIMUM THICKNESS OF PRESS-BREAK MEMBERS TO BE \$\\^6\]. PENNDOT SIGN STRUCTURE COMPUTER PROGRAM OR AN APPROVED FINITE ELEMENT ANALYSIS COMPUTER PROGRAM MUST BE RUN TO VERIFY THE ADEQUACY OF PRESS-BREAK MEMBERS FOR STRENGTH AND FATIGUE. ALTERNATE PRESS-BREAK MEMBERS ARE ONLY PERMITTED FOR COLUMNS. PRESS-BREAK MEMBERS ARE NOT PERMITTED FOR STRUTS.

PROVIDE BOLTS CONFORMING TO THE FOLLOWING:

ANCHOR BOLTS: ASTM, F1554 GRADE 55 PER PUBLICATION 408 SECTION 1105.02(c) 3.

AASHTO M164 (ASTM A325) H.S. BOLTS EXCEPT AS NOTED

BOLTS: DESIGN SPECIFICATIONS:

AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS", 2001 WITH CURRENT INTERIMS (UNLESS NOTED OTHERWISE); AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 1996 WITH INTERIMS THROUGH AND INCLUDING 2000; PENNDOT DESIGN MANUAL - PART 4, AUGUST 1993 EDITION (INCLUDING AUGUST 1995 REVISIONS)

• ALL FILLET WELDS SHOWN ARE MINIMUM SIZE UNLESS NOTED OTHERWISE.

NOTES TO FABRICATOR

- CENTER-MOUNT STRUCTURE TYPES AS PRESENTED IN THESE STANDARDS ARE RECOMMENDED TO CARRY DYNAMIC\VARIABLE MESSAGE SIGNS (DMS\VMS). DMS\VMS ARE PROHIBITED ON OVERHEAD CANTILEVER STRUCTURE TYPES AS PRESENTED IN THESE STANDARDS. OVERHEAD SIGN STRUCTURES INTENDED TO CARRY DMS\VMS, NOT REPRESENTED BY BD-649M, MUST BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PENNSYLVANIA AND SUBMITTED TO THE CHIEF BRIDGE ENGINEER FOR REVIEW AND APPROVAL.
- DESIGN COMPUTATIONS ARE REQUIRED FOR ANY PORTION OF A STRUCTURE FOR WHICH THE INFORMATION IS NOT TAKEN DIRECTLY FROM THE CONTRACT DRAWINGS OR THE DETAILS CONTAINED IN THIS STANDARD. DO NOT VIOLATE CRITERIA USED FOR THE DEVELOPMENT OF THE DESIGN TABLES ON STANDARD DRAWING BD-641M AND THE DETAILS IN THIS STANDARD.

* LEGEND:

CVN:

AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS" AASHTO SIGN SPEC:

• AASHTO HIGHWAY BRIDGES: AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES"

DM4: PENNSYLVANIA DEPARTMENT OF TRANSPORTATION, DESIGN MANUAL PART 4,

UNLESS NOTED OTHERWISE • U. N. O. :

AMERICAN CONCRETE INSTITUTE - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE WITH COMMENTARY (ACI 318-99).

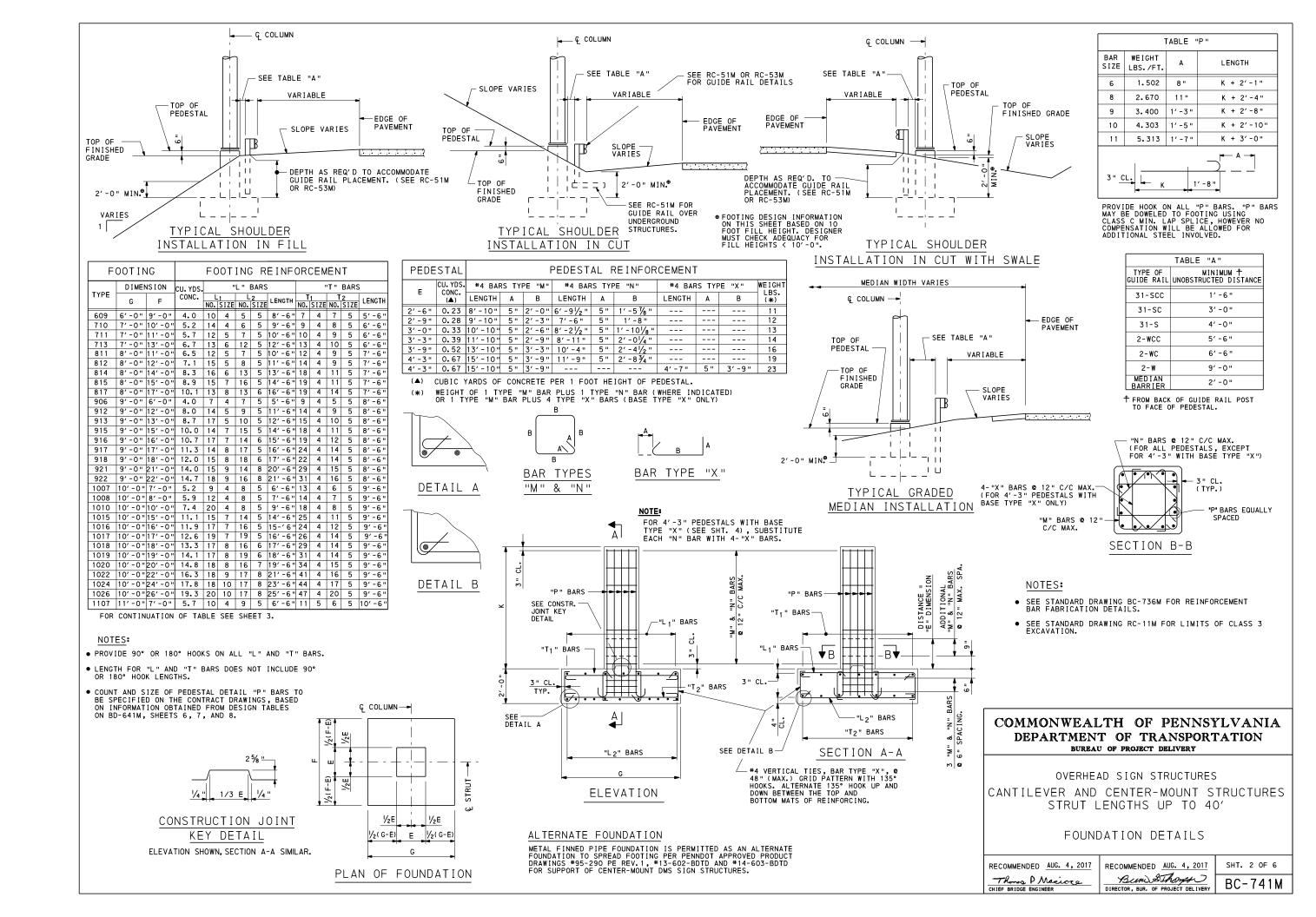
CHARPY V-NOTCH.

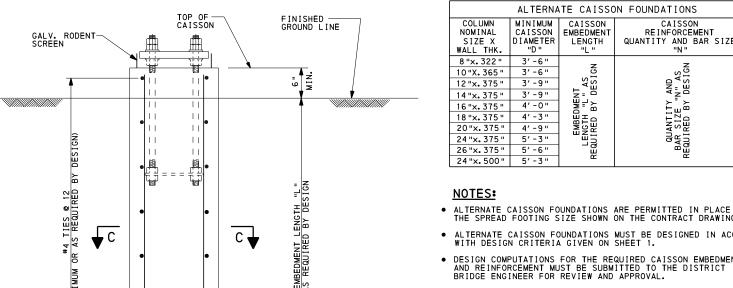
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

CANTILEVER AND CENTER-MOUNT STRUCTURES STRUT LENGTHS UP TO 40'

> BC-741M DIRECTOR, BUR, OF PROJECT DELIVERY

BUREAU OF PROJECT DELIVERY SPACING CHARTS/DIRECT APPLIED LETTERS, NUMERALS, & ARROWS TC-8700C SIGN DETAILS/FREEWAY AND EXPRESSWAY GUIDE SIGNS TC-8701D TC-8701F OVERHEAD SIGN STRUCTURES EXTRUDED ALUMINUM CHANNEL SIGN TC-8701S FLAT SHEET ALUMINUM SIGNS WITH EXTRUDED ALUMINUM STIFFENERS TC-8715 SIGN LIGHTING BC-736M REINFORCEMENT BAR FABRICATION DETAILS RC-11M CLASSIFICATION OF EARTHWORK FOR STRUCTURES NOTES AND DESIGN CRITERIA RC-51M TYPE 31 STRONG POST GUIDE RAIL RC-53M TYPE 2 WEAK POST GUIDE RAIL RC-54M BARRIER PLACEMENT AT OBSTRUCTIONS RECOMMENDED AUG. 4, 2017 RECOMMENDED AUG. 4, 2017 SHT. 1 OF 6 SINGLE FACE CONCRETE BARRIER PLACEMENT AT MEDIAN PIERS RC-58M Bun SThomps Thoma P Macioca REFERENCE DRAWINGS





C 🚽

3" CLEAR

"D "

DIAMETER

(SEE TABLE FOR MINIMUM CAISSON DIAMETER

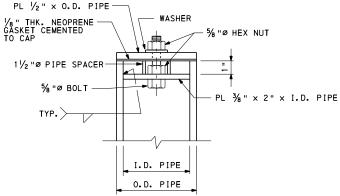
BASED ON COLUMN SIZE)

CAISSON ELEVATION

SECTION C-C

N. T. S.

- ALTERNATE CAISSON FOUNDATIONS ARE PERMITTED IN PLACE OF THE SPREAD FOOTING SIZE SHOWN ON THE CONTRACT DRAWINGS.
- ALTERNATE CAISSON FOUNDATIONS MUST BE DESIGNED IN ACCORDANCE WITH DESIGN CRITERIA GIVEN ON SHEET 1.
- DESIGN COMPUTATIONS FOR THE REQUIRED CAISSON EMBEDMENT AND REINFORCEMENT MUST BE SUBMITTED TO THE DISTRICT BRIDGE ENGINEER FOR REVIEW AND APPROVAL.
- IN PLACE OF #4 TIES AT 12", A #4 BAR SPIRAL WITH A 3" PITCH MAY BE USED. THE #4 TIES AT 12" ARE THE MINIMUM OR AS REQUIRED BY DESIGN.



ALTERNATE PIPE CAP DETAIL

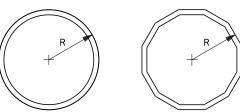
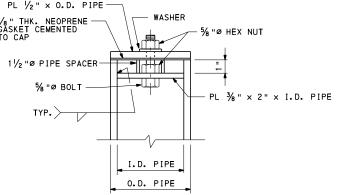
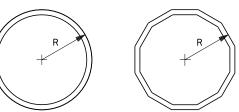


ILLUSTRATION OF DIMENSION "R" FOR CIRCULAR MEMBERS AND

"PRESS-BREAK" NOTE:

ALTERNATE "PRESS-BREAK" MEMBERS ARE PERMITTED FOR COLUMNS. "PRESS-BREAK" MEMBERS MUST HAVE THE EQUIVALENT STRENGTH AND FATIGUE RESISTANCE OF THE CIRCULAR MEMBER BEING REPLACED. A MINIMUM NUMBER OF 12 BREAKS IS REQUIRED. A CHANGE IN STEEL MATERIAL OR WALL THICKNESS REQUIRES A SPECIAL DESIGN TO BE SUBMITTED FOR REVIEW. CONTRACTOR MUST SUBMIT DESIGN CALCULATIONS AND DESIGN DRAWINGS FOR REVIEW AND ACCEPTANCE FOR LONGITUDINAL SEAM WELDS INDICATING TYPE OF WELD, WELD PENETRATION, EFFECTIVE DEPTH AND LENGTH OF EACH WELD TYPE. LONGITUDINAL SEAM WELDS SHALL HAVE 60 PERCENT MINIMUM PENETRATION, EXCEPT LONGITUDINAL SEAM WELDS WITHIN 6" OF THE ENDS OF THE PRESS BREAK MEMBER OR LENGTH SHOWN ON DETAILS SHALL BE COMPLETE PENETRATION WELDS. COMPLETE PENETRATION LONGITUDINAL SEAM WELDS MUST BE 100X RADIOGRAPHICALLY INSPECTED. FOR THE COLUMN CONNECTION TO BASE PLATE, AND AT COLUMN CONNECTION SPLICE PLATE LOCATIONS, WELD SHALL START AND STOP IN THE MIDDLE THIRD REGION OF FLAT SECTIONS BETWEEN BREAK POINTS.





#4 TIES @ 12" (MINIMUM OR AS REQUIRED BY DESIGN) EQUIVALENT "PRESS-BREAK" MEMBERS

ALTERNATE FOUNDATION

"N" BARS @ EQUAL SPACES (QUANTITY AND BAR SIZE AS REQUIRED BY DESIGN)

LAP LENGTH, 30 BAR — DIA. MIN. (ALTERNATE SPLICE LOCATIONS)

METAL FINNED PIPE FOUNDATION IS PERMITTED AS AN ALTERNATE FOUNDATION TO CAISSON PER PENNDOT APPROVED PRODUCT DRAWINGS #95-290 PE REV. 1, #13-602-BDTD AND #14-603-BDTD FOR SUPPORT OF CENTER-MOUNT DMS SIGN STRUCTURES.

DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

CANTILEVER AND CENTER-MOUNT STRUCTURES STRUT LENGTHS UP TO 40'

> FOUNDATION DETAILS AND ALTERNATE CAISSON FOUNDATION

RECOMMENDED AUG. 4, 2017 Thomas P Macioca CHIEF BRIDGE ENGINEER

RECOMMENDED AUG. 4, 2017 Bund SThomps DIRECTOR, BUR. OF PROJECT DELIVERY

SHT. 3 OF 6 BC-741M

COMMONWEALTH OF PENNSYLVANIA

TABLE CONTINUED FROM SHEET 2.

1108 11'-0" 8'-0" 6.5 13 4 9 5 7'-6" 12 5 7 5 10'-6"

1109 11'-0" 9'-0" 7.3 16 4 9 5 8'-6" 13 5 7 5 10'-6" 1111 11'-0" 11'-0" 9.0 15 5 9 5 10'-6" 22 4 9 5 10'-6" 1112 11'-0" 12'-0" 9.8 22 5 11 5 11'-6" 17 5 9 5 10'-6" 1114 11'-0" 14'-0" 11.4 21 6 15 5 13'-6" 21 5 11 5 10'-6" 1210 12'-0" 10'-0" 8.9 17 4 9 5 9'-6" 14 5 8 5 11'-6"

1210 12'-0" 10'-0" 8.9 17 4 9 5 9'-6" 14 5 8 5 11'-6" 1212 12'-0" 12'-0" 10.7 19 5 11 5 11'-6" 19 5 9 5 11'-6" 1213 12'-0" 13'-0" 11.6 24 5 14 5 12'-6" 21 5 10 5 11'-6" 1215 12'-0" 15'-0" 13.3 19 7 21 5 14'-6" 26 5 12 5 11'-6" 1218 12'-0" 18'-0" 16.0 20 8 19 7 17'-6" 36 5 18 5 11'-6" 1219 12'-0" 19'-0" 16.9 20 8 19 7 18'-6" 35 5 18 5 11'-6"

1219 12'-0" 19'-0" 16.9 20 8 19 7 18'-6" 35 5 18 5 11'-6" 1221 12'-0" 21'-0" 18.7 20 9 19 8 20'-6" 39 5 20 5 11'-6" 1314 13'-0" 14'-0" 13.5 21 6 16 5 13'-6" 26 5 11 5 12'-6" 1315 13'-0" 15'-0" 14.4 24 6 18 5 14'-6" 28 5 11 5 12'-6" 1316 13'-0" 16'-0" 15.4 24 7 26 5 15'-6" 24 6 12 5 12'-6" 1317 13'-0" 17'-0" 16.4 21 8 21 6 16'-6" 26 6 14 5 12'-6" 1320 13'-0" 20'-0" 19.3 26 8 25 7 19'-6" 35 6 23 5 12'-6" 1321 13'-0" 21'-0" 20.2 21 9 26 7 20'-6" 35 6 15 5 12'-6" 1323 13'-0" 21'-0" 20.2 21 26 9 23 8 22'-6" 38 6 21 5 12'-6"

| 1321 | 13' - 0" | 21' - 0" | 20. 2 | 21 | 9 | 26 | 7 | 20' - 6" | 35 | 6 | 15 | 5 | 12' - 6" | 1323 | 13' - 0" | 23' - 0" | 22. 1 | 26 | 9 | 23 | 8 | 22' - 6" | 38 | 6 | 21 | 5 | 12' - 6" | 1414 | 14' - 0" | 14' - 0" | 14. 5 | 25 | 6 | 21 | 5 | 13' - 6" | 23 | 6 | 16 | 5 | 13' - 6" | 1418 | 14' - 0" | 18' - 0" | 18. 7 | 23 | 8 | 27 | 6 | 17' - 6" | 34 | 6 | 23 | 5 | 13' - 6" | 1420 | 14' - 0" | 20' - 0" | 20. 7 | 27 | 8 | 26 | 7 | 19' - 6" | 38 | 6 | 27 | 5 | 13' - 6" | 1422 | 14' - 0" | 22' - 0" | 22. 8 | 26 | 9 | 28 | 7 | 21' - 6" | 44 | 6 | 26 | 5 | 13' - 6" | 1516 | 15' - 0" | 16' - 0" | 17. 8 | 27 | 7 | 30 | 5 | 15' - 6" | 32 | 6 | 14 | 5 | 14' - 6" | 1520 | 15' - 0" | 20' - 0" | 22. 2 | 27 | 8 | 26 | 7 | 19' - 6" | 32 | 7 | 19 | 5 | 14' - 6" | 1520 | 15' - 0" | 20' - 0" | 22. 2 | 27 | 8 | 26 | 7 | 19' - 6" | 32 | 7 | 19 | 5 | 14' - 6" | 1520 | 15' - 0" | 20' - 0" | 22. 2 | 27 | 8 | 26 | 7 | 19' - 6" | 32 | 7 | 19 | 5 | 14' - 6" | 1520 | 15' - 0" | 20' - 0" | 22. 2 | 27 | 8 | 26 | 7 | 20' - 6" | 32 | 7 | 19 | 5 | 14' - 6" | 1520 | 15' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20' - 0" | 20

1522 | 15' - 0" | 22' - 0" | 24.4 | 26 | 9 | 30 | 7 | 21' - 6" | 36 | 7 | 17 | 5 | 14' - 6"

• PROVIDE 90° OR 180° HOOKS ON ALL "L" AND "T" BARS.

• LENGTH FOR "L" AND "T" BARS DOES NOT INCLUDE 90°

• COUNT AND SIZE OF PEDESTAL DETAIL "P" BARS TO

ON INFORMATION OBTAINED FROM DESIGN TABLES

• FOR ADDITIONAL FOUNDATION NOTES, SEE SHEET 2.

ON BD-641M. SHEETS 6. 7. AND 8.

BE SPECIFIED ON THE CONTRACT DRAWINGS, BASED

"L" BARS

FOOTING REINFORCEMENT

L1 L2 LENGTH T1 T2 LENGTH N0. SIZE NO. SIZE LENGTH

"T" BARS

FOOTING

G

NOTES:

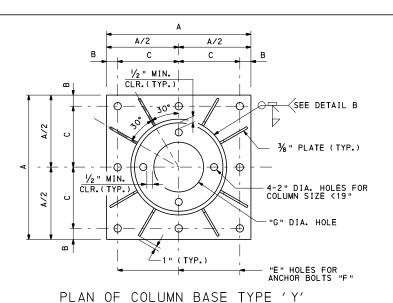
OR 180° HOOK LENGTHS.

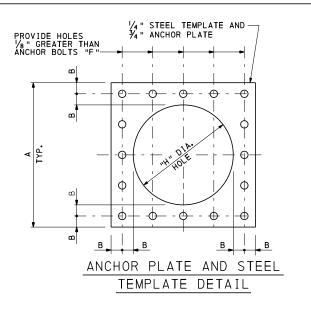
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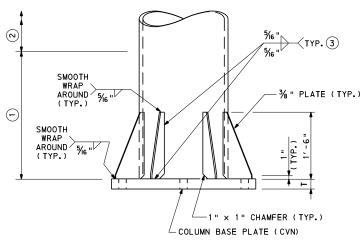
DIMENSION

F

II. YDS.

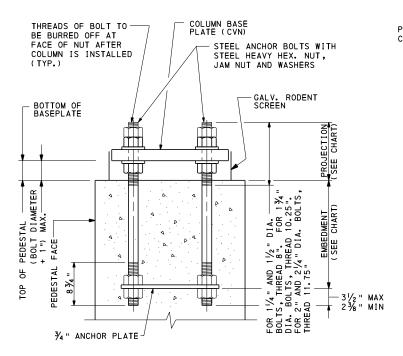






ELEVATION - TYPE Y
(TYPE - X SIMILAR)

- 1) FOR PRESS BREAK COLUMN, 2'-6" LENGTH OF SEAM WELD TO BE COMPLETE PENETRATION GROOVE WELD.
- (2) SEAM WELD TO HAVE 60% MIN. PENETRATION.
- 3 TERMINATE WELDS 1/4" SHORT OF STIFFENER CHAMFER.

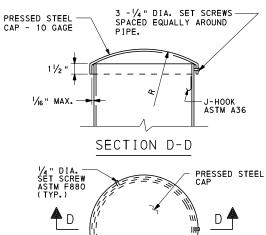


ANCHOR BOLT DETAIL

COLUMN, STIFFENERS, AND REINF. OMITTED FOR CLARITY

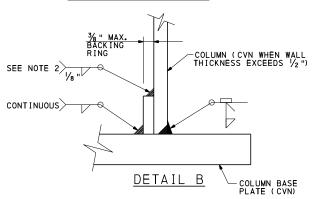
NOTES:

- ANCHOR BOLTS SHALL BE PROVIDED WITH FOUR HEAVY HEXAGON NUTS, ONE JAM NUT AND TWO WASHERS AS SHOWN ON THE ANCHOR BOLT DETAIL.
- ANCHOR BOLTS SHALL BE GALVANIZED AFTER THREADING.
- USE STEEL TEMPLATE TO SET ANCHOR BOLTS IN ACCORDANCE WITH PUBLICATION 408, SECTION 948.3(b).
- STEEL TEMPLATE AND ANCHOR PLATE TO BE PROVIDED BY SIGN FABRICATOR.
- STEEL TEMPLATE PLATE WITH NUTS ON BOTH SIDES SHALL BE USED TO MAINTAIN THE SPACING AND ALIGNMENT OF ANCHOR BOLTS.
- FOR EQUIVALENT "PRESS BREAK" MEMBER DETAILS AND NOTES, SEE SHEET 3.
- FOR ALTERNATE PIPE CAP DETAIL, SEE SHEET 3.
- SEAL BASE PLATE TO FOUNDATION GAP WITH GALVANIZED STEEL SCREEN, 1/2" BY 1/2" MESH AND 0.063" DIAMETER WIRES. SCREEN IS TO PREVENT ENTRY OF RODENTS WHILE PERMITTING DRAINAGE. SCREEN IS TO BE REMOVABLE AND ATTACHED TO BASEPLATE WITH STAINLESS STEEL HARDWARE.



PIPE SIZE (NOMINAL)	R
2" DIA.	9"
3" DIA.	9"
3½" DIA.	9"
4" DIA.	9"
5" DIA.	9"
6" DIA.	9"
8" DIA.	9"
10" DIA.	9"
12" DIA.	1′-6"
14" DIA.	1′-6"
16" DIA.	1′-6"
18" DIA.	1′-6"
20" DIA.	2′-6"
24" DIA.	2′-6"

PIPE CAPS



PIPE CAP DETAILS

DETAIL B NOTES:

- BACKING RING MUST BE FITTED/SIZED TO THE PIPE COLUMN AND CONTINUOUSLY FILLET WELDED TO THE BASE PLATE BEFORE THE FULL PENETRATION GROOVE WELD IS MADE. BACKING RING MUST BE FABRICATED AS A CONTINUOUS RING.
- 2. FOR COLUMNS LESS THAN 19" DIA., THIS FILLET WELD IS NOT REQUIRED BUT SHOP IS TO APPLY SILICON CAULKING TO THIS LOCATION AFTER POLE ASSEMBLY IS GALVANIZED.

	COLUMN BASES											
COLUMN NOMINAL SIZE X WALL THK.*	BASE TYPE	А	В	С	E	F	G	Н	т	WASHER SIZE	PRO- JECTION	EMBED- MENT
8 "x. 322 "	Y	1′-8"	21/2"	71/2"	1 ½ "D	1 1/4 "D	2 "	10"	2 "	3½ "D×¾ "	7 3/4 "	2′-1"
10 "x. 365 "	Y	1′-8"	21/2"	71/2"	1 ½ "D	1 1/4 "D	31/4"	10"	2 "	3½ "D×¾ "	7 3/4 "	2′-1"
12 "x• 375 "	Y	1′-10"	21/2"	8 1/2 "	1¾ "D	1 ½ "D	51/4"	1′-0"	2 "	3½ "D×¾ "	8 1/2 "	2′-6"
14"ו 375"	Y	2'-0"	21/2"	91/2"	1 ¾ "D	1 ½ "D	6½"	1′-2"	2 "	3½ "D×¾ "	8 1/2 "	2′-6"
16 "x• 375 "	Y	2′ -2"	21/2"	101/2 "	2 "D	1 ¾ "D	8"	1′-4"	2 "	4 "D×3% "	91/4"	2' -11"
18 "x. 375 "	Y	2'-4"	21/2"	111/2"	2 "D	1 ¾ "D	91/4"	1′-6"	2 "	4 "D×3/8 "	91/4"	2'-11"
20 "x• 375 "	Y	2′-7"	3 "	1′-01/2 "	2 1/4 "D	2 "D	1′-5"	1′-7"	3 "	5 "D×3/8 "	11"	3′ - 4 "
24 "x. 375 "	Y	2'-11"	3 "	1'-21/2"	2 1/4 "D	2 "D	1′-6"	1'-11"	3 "	5 "D×3/8 "	11"	3′ - 4 "
24 "ו 500 "	Y	3′-0"	3½"	1'-21/2"	2 ½ "D	2 1/4 "D	1′-6"	1'-10"	3 "	5 "D×3/8 "	113/4"	3′-9"

NOTE: D DENOTES DIAMETER * CVN REQUIRED FOR WALL THICKNESSES EXCEEDING 1/2 " (0.500").

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

CANTILEVER AND CENTER-MOUNT STRUCTURES

STRUT LENGTHS UP TO 40'

COLUMN BASE

RECOMMENDED AUG. 4, 2017

There P Macioca

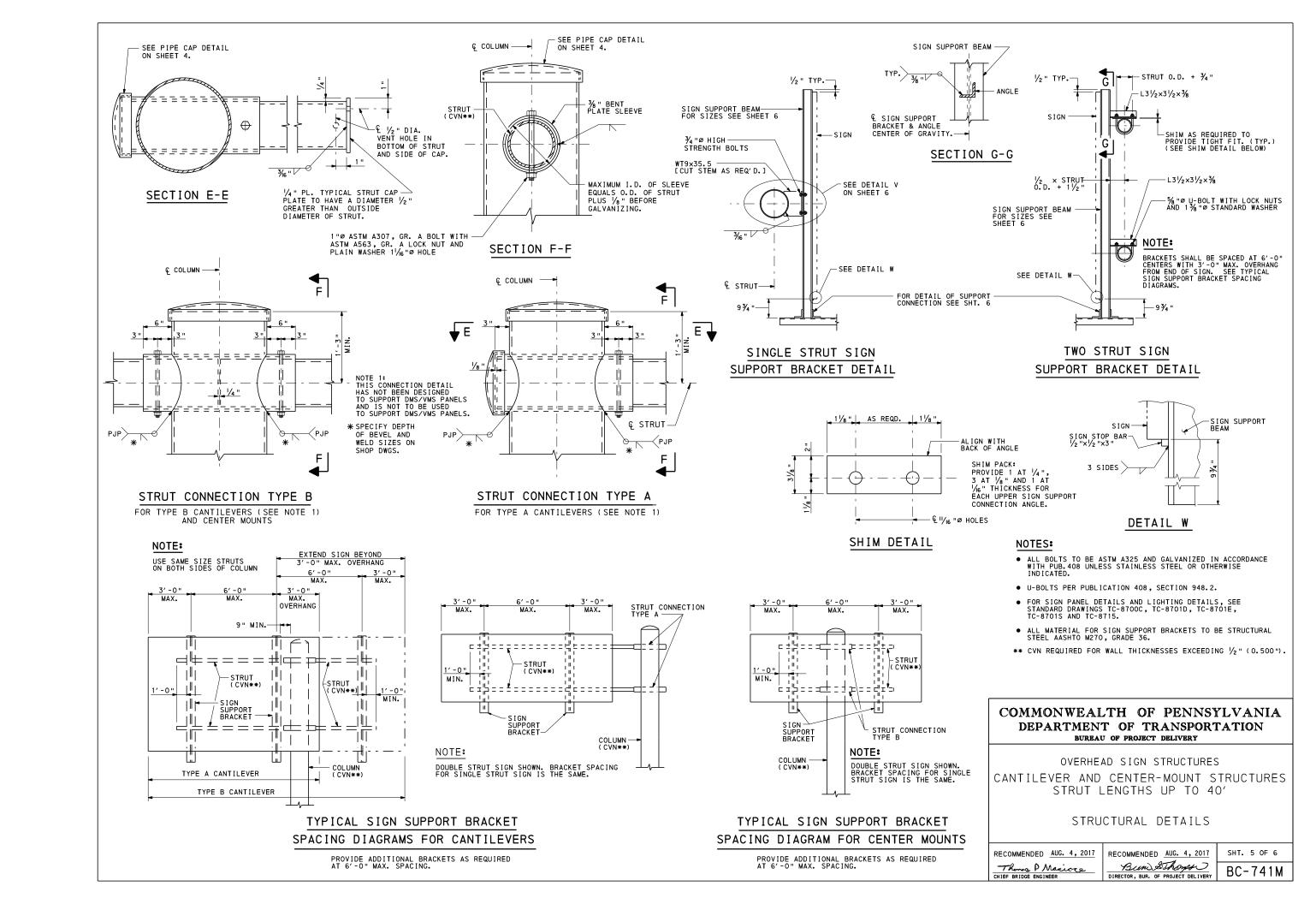
CHIEF BRIDGE ENGINEER

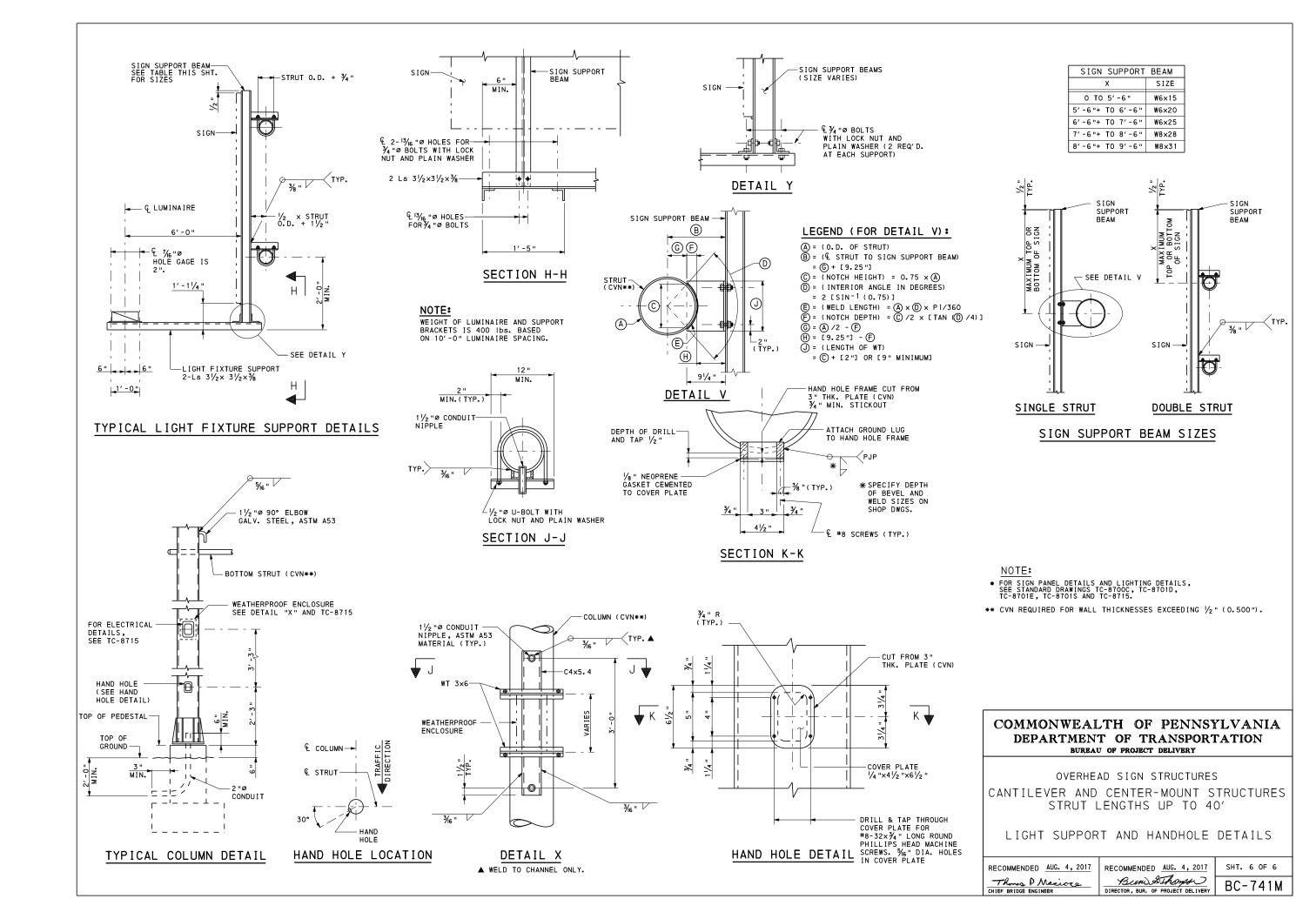
RECOMMENDED AUG. 4, 2017

Bund Sthongs

DIRECTOR, BUR. OF PROJECT DELIVERY

BC-741M





INFORMATION CONTAINED IN THE BD-643M DESIGN TABLES

- DESIGN TABLES ON STANDARD DRAWING BD-643M WERE DEVELOPED USING A COMPUTER PROGRAM AND ARE BASED ON THE DESIGN CRITERIA SHOWN ON THIS SHEET, EXCEPT, THE MEMBER SIZES INDICATED DO NOT INCLUDE THE FATIGUE REQUIREMENTS INDICATED IN THE DESIGN CRITERIA SHOWN ON THE PROGRAM AND AREA OF THE PROGRAM AND THE PROGRAM AND
- THE MEMBER SIZES INDICATED IN THE DESIGN TABLES MEET THE FATIGUE REQUIREMENTS FOR FATIGUE CATEGORY II. THE DESIGNER MUST CHECK THE ADEQUACY OF THE MEMBER SIZES INDICATED WHEN THE FATIGUE CATEGORY IS SPECIFIED TO BE I FOR THE PROJECT.
- THE SPAN RANGE INCLUDED ON STANDARD DRAWING BD-643M IS AS FOLLOWS:

TWO-POST PLANAR TRUSS, SPANS FROM BD-643M:

• THE DESIGN TABLES INCLUDE MEMBER SIZES FOR THE STRUCTURES FOR VARIOUS COMBINATIONS OF COLUMN HEIGHT, SPAN LENGTH, AND SIGN AREA. THEY ALSO INCLUDE SPREAD FOOTING DESIGNS. ALTERNATE CAISSON FOUNDATIONS ARE PERMITTED, HOWEVER, THE REQUIRED CAISSON EMBEDMENT AND REINFORCEMENT MUST BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PENNSYLVANIA. THE DESIGN COMPUTATIONS MUST BE SUBMITTED TO THE DISTRICT BRIDGE ENGINEER FOR REVIEW AND APPROVAL. THE CORRESPONDING FABRICATION AND CONSTRUCTION DETAILS ARE CONTAINED IN THIS STANDARD.

GENERAL NOTES

- 1. PROVIDE 3-INCH CONCRETE COVER ON REINFORCEMENT BARS, EXCEPT AS NOTED.
- USE CLASS A CEMENT CONCRETE f'c = 3000 PSI IN PEDESTALS, FOOTINGS AND CAISSONS.
- 3. PROVIDE GRADE 60 REINFORCING STEEL BARS THAT MEET THE REQUIREMENTS OF ASTM A615 FOR CONCRETE REINFORCEMENT. DO NOT WELD REINFORCING STEEL BARS.
- 4. RAKE-FINISH ALL HORIZONTAL CONSTRUCTION JOINTS, EXCEPT AS INDICATED.
- 5. VERIFY ALL DIMENSIONS AND GEOMETRY OF THE EXISTING STRUCTURES IN THE FIELD AS NECESSARY FOR PROPER FIT OF THE PROPOSED CONSTRUCTION.
- 6. CHAMFER EXPOSED CONCRETE EDGES 1 INCH BY 1 INCH

CHANGE 1

- 7. ALL DIMENSIONS SHOWN ARE HORIZONTAL, EXCEPT AS NOTED.
- 8. DIMENSIONS ARE BASED ON A NORMAL TEMPERATURE OF 68 DEGREES F.
- SPREAD FOOTINGS MAY BE ORDERED BY THE ENGINEER TO BE AT ANY ELEVATION OR OF ANY DIMENSIONS NECESSARY TO PROVIDE A PROPER FOUNDATION.
- 10. GALVANIZE ALL STRUCTURAL STEEL, BOLTS, NUTS & WASHERS IN ACCORDANCE WITH PUB. 408 UNLESS STAINLESS STEEL OR OTHERWISE INDICATED.
- 11. PIPE DIAMETERS SHOWN UP TO AND INCLUDING 12 INCHES ARE NOMINAL DIAMETERS. PIPE DIAMETERS SHOWN FROM 14 INCHES AND UP ARE ACTUAL DIAMETERS.
- 12. USE STANDARD SIZE HOLE. THE STANDARD HOLE DIAMETER FOR BOLTS SMALLER THAN
 1" DIAMETER SHALL BE THE NOMINAL DIAMETER OF THE BOLT PLUS 1/6". FOR BOLTS
 1" DIAMETER AND LARGER, THE WIDTH OF EACH STANDARD HOLE SHALL BE THE NOMINAL DIAMETER OF THE BOLT PLUS 1/6".
- 13. CLEAR DISTANCE BETWEEN BOLT HOLES OR BETWEEN THE BOLT HOLE AND THE END OF THE MEMBER IN THE DIRECTION OF THE APPLIED BEARING FORCE SHALL BE CHECKED.
- 14. PROVIDE ANCHOR BOLT HOLES 1/4" LARGER THAN BOLT DIAMETER.
- 15. PROVIDE A MINIMUM ANCHOR BOLT EMBEDMENT LENGTH OF 20 ANCHOR BOLT DIAMETERS.
- 16. PROVIDE DOUBLE NUTS AND WASHER FOR EACH ANCHOR BOLT.
- 17. STEEL MEMBER COMPONENTS REQUIRING CHARPY V-NOTCH TESTING ARE DESIGNATED ON THE PLANS BY (CVN), PROVIDE STEEL CONFORMING TO THE CVN REQUIREMENTS FOR ZONE 2, NON FRACTURE CRITICAL AS GIVEN IN THE AASHTO MATERIAL SPECIFICATIONS.

NOTES TO FABRICATOR

- DYNAMIC/VARIABLE MESSAGE SIGNS (DMS/VMS) ARE PROHIBITED ON 2-POST PLANAR TRUSS STRUCTURE TYPES AS PRESENTED IN THESE STANDARDS. OVERHEAD SIGN STRUCTURES INTENDED TO CARRY DMS/VMS MUST BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PENNSYLVANIA AND SUBMITTED TO THE CHIEF BRIDGE ENGINEER FOR REVIEW
- DESIGN COMPUTATIONS ARE REQUIRED FOR ANY PORTION OF A STRUCTURE FOR WHICH THE INFORMATION IS NOT TAKEN DIRECTLY FROM THE CONTRACT DRAWINGS OR THE DETAILS CONTAINED IN THIS STANDARD. DO NOT VIOLATE CRITERIA USED FOR THE DEVELOPMENT OF THE DESIGN TABLES ON STANDARD DRAWING BD-643M AND THE DETAILS IN THIS STANDARD.
- FABRICATOR TO SELECT PANEL POINT CONNECTION DETAIL TYPE BASED ON MEMBER SIZE AND TRUSS CONFIGURATION TO ENSURE FIT-UP, FABRICATION, GALVANIZING AND ERECTION.

DESIGN CRITERIA FOR PENNDOT SIGN STRUCTURES

 DEAD LOADS PENNDOT STD. DWGS. (U.N.O.) * TC-8701E OR TC-8701S BC-743M, SHT. 9 BC-743M, SHT. 10 CALCULATED INTERNALLY WITHIN PROGRAM SIGN PANELS LIGHT FIXTURES SIGN SUPPORT BEAM COLUMNS, CHORDS

• EXTERNAL LOADS AASHTO SIGN SPECS. ICE LOAD WIND LOAD APPENDIX C, SECTION C.3, EQ. C-1, WITH 80 MPH WIND AND 30% GUST FACTOR

AASHTO SIGN SPECS, 3,4 GROUP LOADS

• STEEL CRITERIA

SECTION PROPERTIES FOR TUBULAR SHAPES
MAXIMUM STRESSES IN TUBULAR SHAPES
ALLOWABLE STRESSES FOR TUBULAR SHAPES
ALLOWABLE STRESSES FOR SIGN SUPPORTS
ALLOWABLE STRESSES FOR BASE PLATES
ALLOWABLE STRESSES FOR BASE PLATES
FATIGUE REQUIREMENTS (FATIGUE CATEGORY II) APPENDIX B, TABLE B-1 APPENDIX B, TABLE B-2 5.6 (TABLE 5-3) & 5.11 5.12 SECTION 11

ALLOWABLE DEFLECTION
PERMANENT CAMBER
ALLOWABLE STRESSES FOR STRUCTURAL STEEL SECTION 5

 BOLT CRITERIA AASHTO HIGHWAY BRIDGES (U.N.O.)

ALLOWABLE BOLT STRESSES SLIP-CRITICAL BOLT ALLOWABLE BOLT PRYING ACTION COMBINED BOLT SHEAR AND TENSION BOLT DESIGN CRITERIA ALLOWABLE ANCHOR BOLT STRESSES TABLE 10.32.3B 10.32.3.2.1 10.32.3.3.2 10.32.3.3.2 10.32.3.3.3 AASHTO SIGN SPECS. 5.16 AASHTO SIGN SPECS. 5.17

CONCRETE CRITERIA

AASHTO HIGHWAY BRIDGES (U.N.O.) ALLOWABLE BEARING STRESS
REINFORCEMENT TENSILE STRESS
SHEAR CAPACITY OF FOOTINGS
SHEAR STRESS IN FOOTINGS
ALLOWABLE SHEAR STRESS
ALLOWABLE SHEAR STRESS
SLENDERNESS OF COLUMNS
MINIMUM REINF. OF FLEXURAL MEMBERS
SPACING LIMITS FOR REINFORCEMENT
MINIMUM CONCRETE COVER
PRESSURES FOR ECCENTRICALLY LOADED FOOTINGS
DISTRIBUTION OF REINFORCEMENT
FOOTING STABILITY REQUIREMENTS
TORSION
COLUMN DESIGN (PEDESTALS) 8. 15. 2. 1. 3 8. 15. 2. 2 8. 15. 5. 6. 1 8. 15. 5. 6. 2 8. 15. 5. 6. 4 8. 16. 5. 2 8.21 DM4 D8.22.1* FIG. 4.4.7.1.1.1C 4.4.11.2.2 DM4 D5.5.5 ACI SECTION A.7.3* 8.15.4

• SPREAD FOOTINGS

MAXIMUM DESIGN PRESSURE MINIMUM AREA IN BEARING UNIT WEIGHT OF SOIL 1.5 TONS PER SQUARE FOOT 95% 100 POUNDS PER CUBIC FOOT

• DRILLED SHAFTS (CAISSONS)

DM4 SEC. 4.6, PENNDOT COM624 COMPUTER PROGRAM

TC-8700C | SPACING CHARTS/DIRECT APPLIED LETTERS, NUMERALS, & ARROWS

FLAT SHEET ALUMINUM SIGNS WITH EXTRUDED ALUMINUM STIFFENERS

SINGLE FACE CONCRETE BARRIER PLACEMENT AT MEDIAN PIERS

SIGN DETAILS/FREEWAY AND EXPRESSWAY GUIDE SIGNS

EXTRUDED ALUMINUM CHANNEL SIGN

TYPE 31 STRONG POST GUIDE RAIL

BARRIER PLACEMENT AT OBSTRUCTIONS

TYPE 2 WEAK POST GUIDE RAIL

REINFORCEMENT BAR FABRICATION DETAILS

CLASSIFICATION OF EARTHWORK FOR STRUCTURES

REFERENCE DRAWINGS

SIGN LIGHTING

1.5 TONS PER SQUARE FOOT

O KIPS PER SQUARE FOOT

10.0 POUNDS PER CUBIC INCH 100 POUNDS PER CUBIC FOOT 25°

AASHTO SIGN SPECS.

MAXIMUM DESIGN PRESSURE
MAXIMUM DESIGN LATERAL DISPLACEMENT
MODULUS OF SUBGRADE REACTION
UNIT WEIGHT OF SOIL
ANGLE OF INTERNAL FRICTION
COHESION

COLUMN DESIGN (PEDESTALS)

SEISMIC DESIGN CRITERIA

STRUCTURES ARE DESIGNED FOR A SEISMIC ACCELERATION COEFFICIENT = 0.15

TC-8701D

TC-8701S

TC-8715

BC-736M

RC-11M

RC-51M

RC-53M

RC-54M

RC-58M

CONSTRUCTION GENERAL NOTES

MATERIALS AND WORKMANSHIPs

PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH THE CURRENT VERSIONS OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, AASHTO/AWS WELDING CODE D1.5, CONTRACT SPECIAL PROVISIONS, AND AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS". USE AASHTO/AWS D1.1 FOR WELDING NOT COVERED IN

• PROVIDE STRUCTURAL STEEL CONFORMING TO THE FOLLOWING:

COLUMNS & PIPE CHORDS:

SEE PUBLICATION 408. SECTION 948.2.

AASHTO M270, GRADE 36 ASTM A709, GRADE 36 ANGLES, SHAPES, AND PLATES:

ALTERNATE PRESS-BREAK MEMBERS:

ALTERNATE PRESS-BREAK MEMBERS MUST HAVE THE EQUIVALENT STRENGTH OF THE MEMBER THEY ARE REPLACING. EQUIVALENT RADIUS FOR PRESS-BREAK MEMBERS IS MEASURED FROM THE CENTER OF THE MEMBER TO THE MID-POINT OF ANY CHORD OF THE MEMBER. MINIMUM THICKNESS OF PRESS-BREAK MEMBERS TO BE \$\frac{\pi}{6}\$". PENNDOT SIGN STRUCTURE COMPUTER PROGRAM OR AN APPROVED FINITE ELEMENT ANALYSIS COMPUTER PROGRAM MUST BE RUN TO VERIFY THE ADEQUACY OF PRESS-BREAK MEMBERS FOR STRENGTH AND FATIGUE. ALTERNATE PRESS-BREAK MEMBERS ARE ONLY PERMITTED FOR COLUMNS. PRESS-BREAK MEMBERS ARE NOT PERMITTED FOR CHORDS.

PROVIDE BOLTS CONFORMING TO THE FOLLOWING:

ANCHOR BOLTS:

ASTM, F1554 GRADE 55 PER PUBLICATION 408 SECTION 1105.02(c)3.

AASHTO M164 (ASTM A325) H.S. BOLTS EXCEPT AS NOTED

• DESIGN SPECIFICATIONS:

AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS", 2001 WITH CURRENT INTERIMS (UNLESS NOTED OTHERWISE); AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 1996 WITH INTERIMS THROUGH AND INCLUDING 2000; PENNDOT DESIGN MANUAL - PART 4, AUGUST 1993 EDITION (INCLUDING AUGUST 1995 REVISIONS)

ALL FILLET WELDS SHOWN ARE MINIMUM SIZE UNLESS NOTED OTHERWISE.

* LEGEND:

AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS" • AASHTO SIGN SPEC:

AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" AASHTO HIGHWAY BRIDGES:

DM4: PENNSYLVANIA DEPARTMENT OF TRANSPORTATION, DESIGN MANUAL PART 4, STRUCTURES

UNLESS NOTED OTHERWISE

AMERICAN CONCRETE INSTITUTE - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE WITH COMMENTARY (ACI 318-99). ACI:

CVN: CHARPY V-NOTCH.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

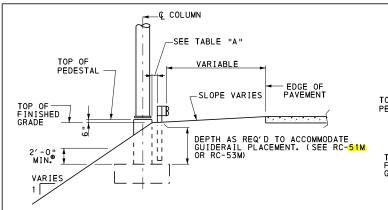
2 POST PLANAR TRUSS SPANS FROM 30' TO 100'

NOTES AND DESIGN CRITERIA

RECOMMENDED AUG. 4, 2017 RECOMMENDED AUG. 4, 2017 Thoma P Macioca

SHT. 1 OF 10 Bun SThomps DIRECTOR, BUR, OF PROJECT DELIVERY

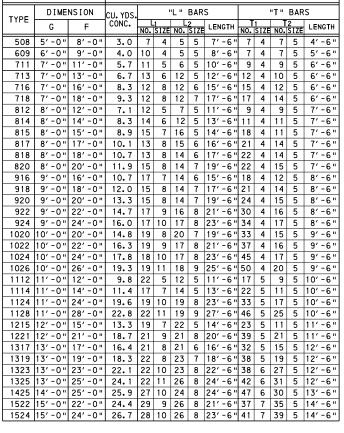
BC-743M



FOOTING REINFORCEMENT

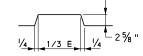
TYPICAL SHOULDER INSTALLATION IN FILL

FOOTING



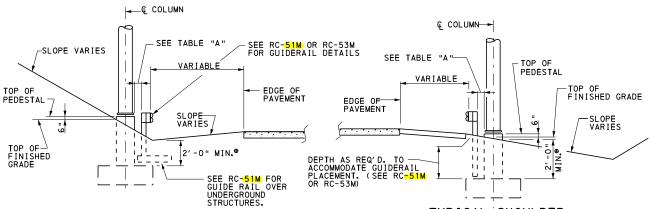
NOTES:

- PROVIDE 90° OR 180° HOOKS ON ALL "L" AND "T" BARS.
- LENGTH FOR "L" AND "T" BARS DOES NOT INCLUDE 90° OR 180° HOOK LENGTHS.
- COUNT AND SIZE OF PEDESTAL "P" BARS TO BE SPECIFIED ON THE CONTRACT DRAWINGS, BASED ON INFORMATION OBTAINED FROM DESIGN TABLES ON BD-643M, SHTS. 5 AND 6.



CONSTRUCTION JOINT KEY DETAIL

ELEVATION SHOWN, SECTION A-A SIMILAR.



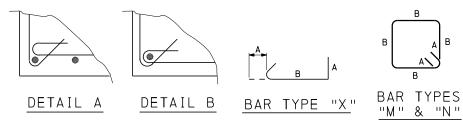
TYPICAL SHOULDER INSTALLATION IN CUT TYPICAL SHOULDER

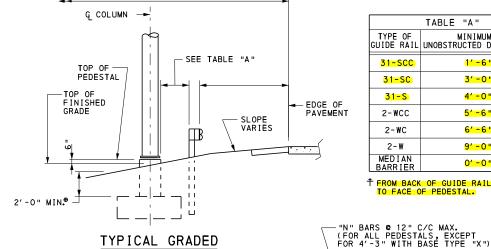
**FOOTING DESIGN INFORMATION ON THIS SHEET BASED ON 10 FOOT FILL HEIGHT. DESIGNER MUST CHECK ADEQUACY FOR FILL HEIGHTS < 10'-0".

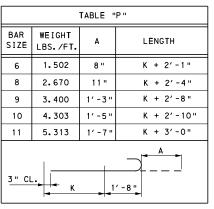
MEDIAN WIDT MEDIAN WIDTH VARIES

PE	DE	STAL		PEDESTAL REINFORCEMENT								
E		CU. YDS.	#4 BAF	RS TY	PE "M"	#4 BAF	RS TYF	PE "N"	#4 BAF	RS TYF	E "X"	WEIGHT
		CONC.	LENGTH	Α	В	LENGTH	Α	В	LENGTH	Α	В	LBS. (*)
2'-6	6"	0.23	8'-10"	5 "	2'-0"	6'-91/2"	5"	1′-5 1/8 "				11
2'-9	9 "	0.28	9'-10"	5 "	2'-3"	7'-6"	5 "	1′-8"				12
3'-0	٥.	0.33	10' - 10"	5 "	2'-6"	8'-21/2"	5 "	1'-101/8"				13
3'-3	3 "	0.39	11'-10"	5 "	2'-9"	8'-11"	5 "	2' -01/4"				14
3' - 9	9"	0.52	13' - 10"	5 "	3'-3"	10' -4"	5 "	2'-41/2"				16
4'-3	3 "	0.67	15' - 10"	5 "	3'-9"	11'-9"	5 "	2'-83/4"				19
4'-3	3 "	0.67	15' -10"	5 "	3'-9"				4'-7"	5 "	3′-9"	23

- () CUBIC YARDS OF CONCRETE PER 1 FOOT HEIGHT OF PEDESTAL.
- WEIGHT OF 1 TYPE "M" BAR PLUS 1 TYPE "N" BAR (WHERE INDICATED) OR 1 TYPE "M" BAR PLUS 4 TYPE "X" BARS (BASE TYPE "X" ONLY)



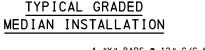




PROVIDE HOOK ON ALL "P" BARS. "P" BA MAY BE DOWELED TO FOOTING USING CLASS C MIN. LAP SPLICE, HOWEVER NO COMPENSATION WILL BE ALLOWED FOR ADDITIONAL STEEL INVOLVED. "P" BARS

TABLE "A"							
TYPE OF GUIDE RAIL	MINIMUM † UNOBSTRUCTED DISTANCE						
31-SCC	1′ -6"						
31-SC	3′ -0"						
31-S	4′ -0"						
2-WCC	5′ -6"						
2-WC	6′ -6"						
2-W	9′ -0"						
MEDIAN BARRIER	0′ -0"						

T FROM BACK OF GUIDE RAIL POST



4-"X" BARS © 12" C/C MAX.-(FOR 4'-3" PEDESTALS WITH BASE TYPE "X" ONLY) "P" BARS EQUALLY SPACED "M" BARS @ 12 C/C MAX.

SECTION B-B

SEE STANDARD DRAWING BC-736M FOR REINFORCING BAR FABRICATION DETAILS.

SEE STANDARD DRAWING RC-11M FOR LIMITS OF ● CLASS 3 EXCAVATION.

NOTES:

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

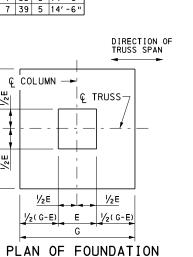
2 POST PLANAR TRUSS SPANS FROM 30' TO 100'

FOUNDATION DETAILS

RECOMMENDED AUG. 4, 2017 Thomas P Macioca

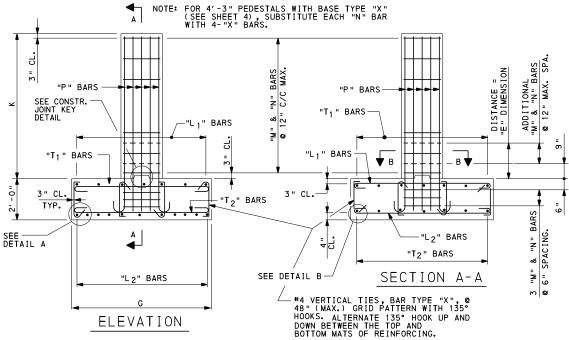
RECOMMENDED AUG. 4, 2017 Bun SThomps

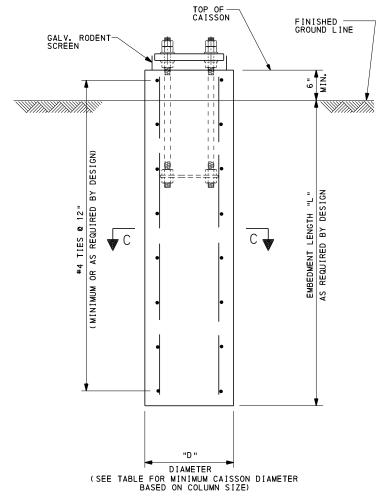
SHT. 2 OF 10 BC-743M DIRECTOR, BUR. OF PROJECT DELIVERY



1/2E

F-E)





CAISSON ELEVATION

LAP LENGTH, 30 BAR — DIA. MIN. (ALTERNATE SPLICE LOCATIONS) 3" CLEAR #4 TIES @ 12" (MINIMUM OR AS REQUIRED BY DESIGN)

SECTION C-C

	ALTERNATE CAISSON FOUNDATIONS								
COLUMN NOMINAL SIZE X WALL THK.	MINIMUM CAISSON DIAMETER "D"	CAISSON EMBEDMENT LENGTH "L"	CAISSON REINFORCEMENT QUANTITY AND BAR SIZE "N"						
8 "x. 322" 10 "x. 365" 12 "x. 375" 14 "x. 375" 16 "x. 375" 18 "x. 375" 20 "x. 375" 24 "x. 375"	3'-6" 3'-6" 3'-9" 3'-9" 4'-0" 4'-3" 4'-9"	EMBEDMENT LENGTH "L" AS REQUIRED BY DESIGN	QUANTITY AND BAR SIZE "N" AS REQUIRED BY DESIGN						
24"x.500"	5′-3"	ш	-						

NOTES:

- ALTERNATE CAISSON FOUNDATIONS ARE PERMITTED IN PLACE OF THE SPREAD FOOTING SIZE SHOWN ON THE CONTRACT DRAWINGS.
- ALTERNATE CAISSON FOUNDATIONS MUST BE DESIGNED IN ACCORDANCE WITH DESIGN CRITERIA GIVEN ON SHEET 1.
- DESIGN COMPUTATIONS FOR THE REQUIRED CAISSON EMBEDMENT AND REINFORCEMENT MUST BE SUBMITTED TO THE DISTRICT BRIDGE ENGINEER FOR REVIEW AND APPROVAL.
- IN PLACE OF #4 TIES AT 12", A #4 BAR SPIRAL WITH A 3" PITCH MAY BE USED. THE #4 TIES AT 12" ARE THE MINIMUM OR AS REQUIRED BY DESIGN.

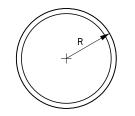




ILLUSTRATION OF DIMENSION "R" FOR CIRCULAR MEMBERS AND EQUIVALENT "PRESS-BREAK" MEMBERS

"PRESS-BREAK" NOTE:

ALTERNATE "PRESS-BREAK" MEMBERS ARE PERMITTED FOR COLUMNS. "PRESS-BREAK" MEMBERS MUST HAVE THE EQUIVALENT STRENGTH AND FATIGUE RESISTANCE OF THE CIRCULAR MEMBER BEING REPLACED. A MINIMUM NUMBER OF 12 BREAKS IS REQUIRED. A CHANGE IN STEEL MATERIAL OR WALL THICKNESS REQUIRES A SPECIAL DESIGN TO BE SUBMITTED FOR REVIEW. CONTRACTOR MUST SUBMIT DESIGN CALCULATIONS AND DESIGN DRAWINGS FOR REVIEW AND ACCEPTANCE FOR LONGITUDINAL SEAM WELDS INDICATING TYPE OF WELD, WELD PENETRATION, EFFECTIVE DEPTH AND LENGTH OF EACH WELD TYPE. LONGITUDINAL SEAM WELDS SHALL HAVE 60 PERCENT MINIMUM PENETRATION, EXCEPT LONGITUDINAL SEAM WELDS WITHIN 6" OF THE ENDS OF THE PRESS BREAK MEMBER OR LENGTH SHOWN ON DETAILS SHALL BE COMPLETE PENETRATION WELDS. COMPLETE MEMBER OR LENGTH SHOWN ON DETAILS SHALL BE COMPLETE PENETRATION WELDS. COMPLETE PENETRATION LONGITUDINAL SEAM WELDS MUST BE 100% RADIOGRAPHICALLY INSPECTED. FOR THE COLUMN CONNECTION TO BASE PLATE, AND AT COLUMN CONNECTION SPLICE PLATE LOCATIONS, WELD SHALL START AND STOP IN THE MIDDLE THIRD REGION OF FLAT SECTIONS BETWEEN BREAK POINTS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

2 POST PLANAR TRUSS SPANS FROM 30' TO 100'

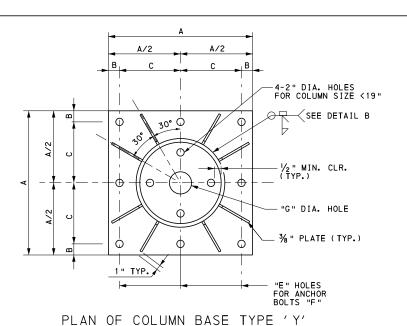
ALTERNATE CAISSON FOUNDATION

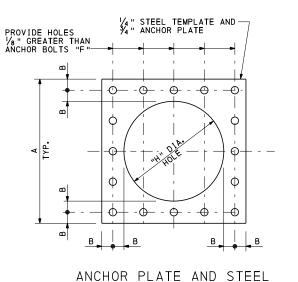
RECOMMENDED AUG. 4, 2017

SHT. 3 OF 10 Bun SThomps BC-743M

RECOMMENDED AUG. 4, 2017

Thoma P Macioca
CHIEF BRIDGE ENGINEER DIRECTOR, BUR. OF PROJECT DELIVERY



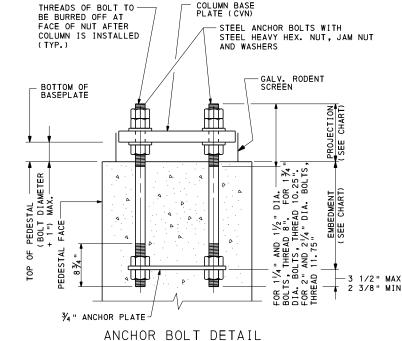


TEMPLATE DETAIL

ELEVATION - TYPE Y
(TYPE - X SIMILAR)

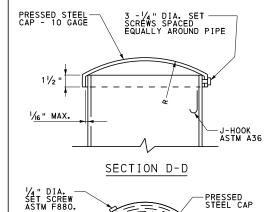
- 1) FOR PRESS BREAK COLUMN, 2'-6" LENGTH OF SEAM WELD TO BE COMPLETE PENETRATION GROOVE WELD.
- 2 SEAM WELD TO HAVE 60% MIN. PENETRATION.
- 3 TERMINATE WELDS 1/4" SHORT OF STIFFENER CHAMFER.

PIPE (CAPS
PIPE SIZE (NOMINAL)	R
8" DIA.	9"
10" DIA.	9"
12" DIA.	1′-6"
14" DIA.	1′-6"
16" DIA.	1′-6"
18" DIA.	1′-6"
20" DIA.	2′-6"
24" DIA.	2′-6"



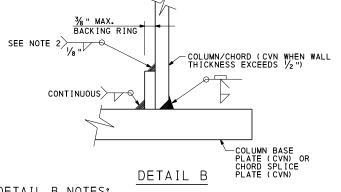
	COLUMN BASES											
COLUMN NOMINAL SIZE X VALL THK.*	BASE TYPE	А	В	С	Е	F	G	Н	Т	WASHER SIZE	PROJECTION	EMBEDMENT
8 "x. 322"	Y	1′-8"	2 1/2 "	7.5"	1 ½ "D	1 1/4 "D	2 "	10"	2 "	3½ "D×¾ "	7¾"	2′-1"
10"x• 365"	Υ	1′-8"	2 1/2 "	7.5"	1 ½ "D	1 1/4 "D	31/4"	10"	2 "	3½ "D×¾ "	7 3⁄4 "	2′-1"
12 "x• 375 "	Υ	1′-10"	2 1/2 "	8.5"	1 ¾ ''D	1 ½ "D	51/4"	1'-0"	2 "	3½ "D×¾ "	8 1/2 "	2′-6"
14"x•375"	Υ	2′-0"	2 1/2 "	9.5"	1 ¾ "D	1 ½ "D	6½"	1′-2"	2 "	3½ "D×¾ "	8 1/2 "	2′-6"
16 "x. 375 "	Y	2′ -2"	21/2"	10.5"	2 "D	1 ¾ "D	8 "	1'-4"	2 "	4 "D×¾ "	91/4"	2'-11"
18 "x. 375 "	Y	2′-4"	2 1/2 "	11.5"	2 "D	1 ¾ "D	91/4"	1′-6"	2 "	4 "D×¾ "	91/4"	2'-11"
20 "x• 375 "	Y	2′-7"	3 "	1′-01/2"	2	2 "D	1′-5"	1′-7"	3 "	5 "D×¾ "	11"	3′ -4"
24 "x• 375 "	Υ	2'-11"	3 "	1'-21/2"	2 1/4 "D	2 "D	1′-6"	1'-11"	3 "	5 "D×3/8 "	11"	3′ - 4 "
24"x.500"	Y	3′-0"	3½"	1'-21/2"	2 ½ "D	2 1/4 "D	1′-6"	1′-10"	3 "	5 "D×¾ "	113/4"	3′-9"

NOTE: D DENOTES DIAMETER * CVN REQUIRED FOR WALL THICKNESSES EXCEEDING 1/2 " (.500").



PIPE CAP DETAILS

D 📤



COLUMN, STIFFENERS, AND REINF. OMITTED FOR CLARITY

- <u>DETAIL B NOTES</u>:
- 1. BACKING RING MUST BE FITTED/SIZED TO THE PIPE COLUMN AND CONTINUOUSLY FILLET WELDED TO THE BASE PLATE BEFORE THE FULL PENETRATION GROOVE WELD IS MADE. BACKING RING MUST BE FABRICATED AS A CONTINUOUS RING.
- 2. FOR COLUMNS AND CHORDS LESS THAN 19" DIA., THIS FILLET WELD IS NOT REQUIRED BUT SHOP IS TO APPLY SILICON CAULKING TO THIS LOCATION AFTER POLE ASSEMBLY IS GALVANIIZED.

NOTES

- ANCHOR BOLTS SHALL BE PROVIDED WITH FOUR HEAVY HEXAGON NUTS, ONE JAM NUT AND TWO WASHERS AS SHOWN ON THE ANCHOR BOLT DETAIL.
- ullet ANCHOR BOLTS SHALL BE GALVANIZED AFTER THREADING.
- USE STEEL TEMPLATE TO SET ANCHOR BOLTS IN ACCORDANCE WITH PUBLICATION 408, SECTION 948.3(b).
- STEEL TEMPLATE AND ANCHOR PLATE TO BE PROVIDED BY SIGN FABRICATOR.
- STEEL TEMPLATE PLATE WITH NUTS ON BOTH SIDES SHALL BE USED TO MAINTAIN THE SPACING AND ALIGNMENT OF ANCHOR BOLTS.
- FOR EQUIVALENT "PRESS BREAK" MEMBER DETAILS AND NOTES, SEE SHEET 3
- FOR ALTERNATE PIPE CAP DETAIL, SEE SHEET 10.
- SEAL BASE PLATE TO FOUNDATION GAP WITH GALVANIZED STEEL SCREEN, 1/2" BY 1/2" MESH AND 0.063" DIAMETER WIRES. SCREEN IS TO PREVENT ENTRY OF RODENTS WHILE PERMITTING DRAINAGE. SCREEN IS TO BE REMOVABLE AND ATTACHED TO BASEPLATE WITH STAINLESS STEEL HARDWARE.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

2 POST PLANAR TRUSS SPANS FROM 30' TO 100'

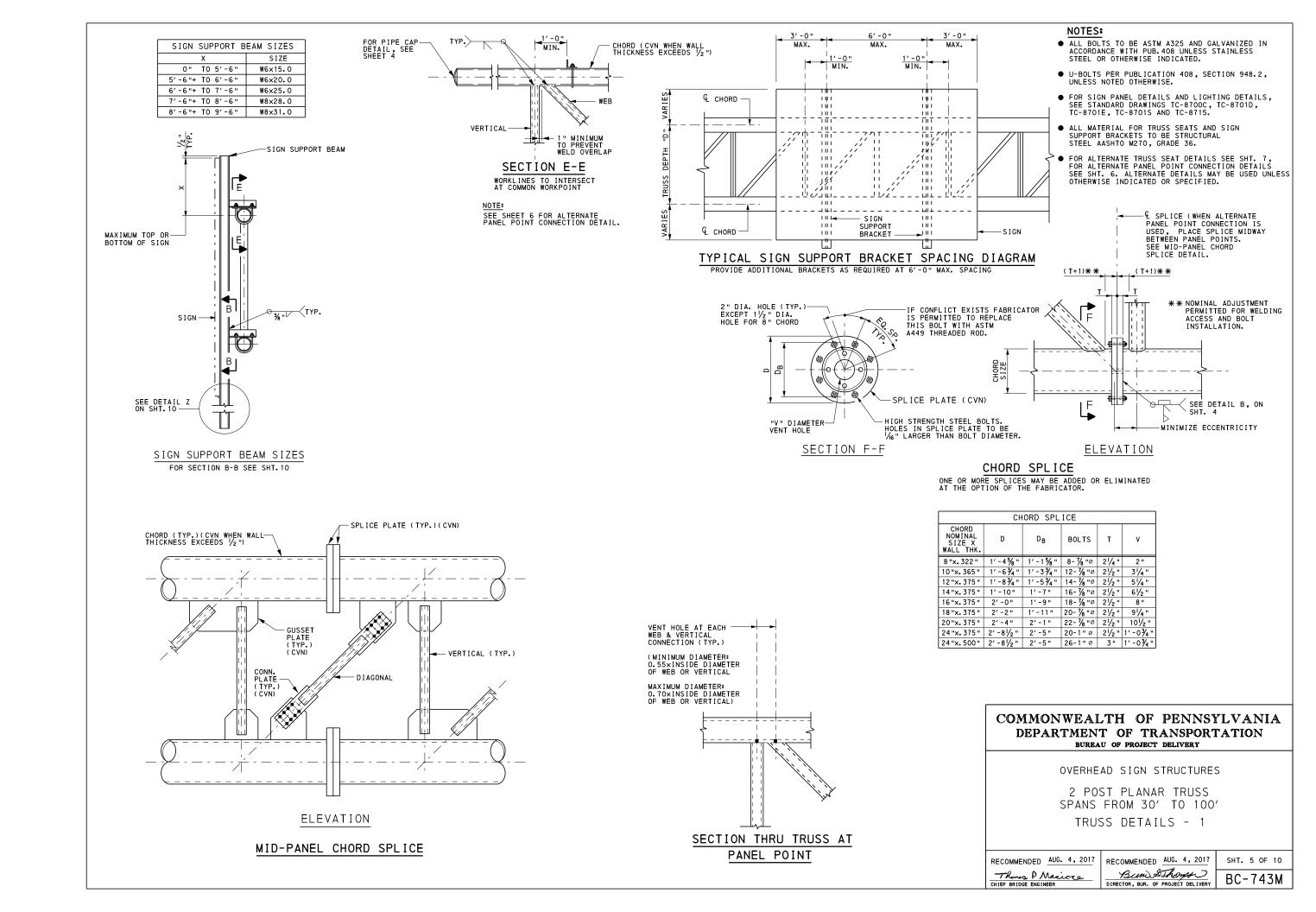
COLUMN BASE

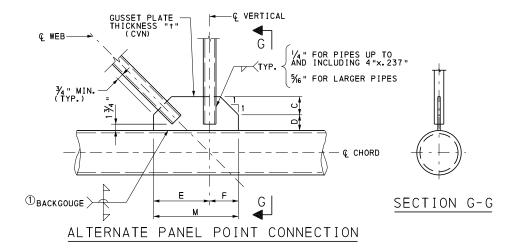
RECOMMENDED AUG. 4, 2017	R
Thomas P. Macioca	_
CHIEF BRIDGE ENGINEER	D

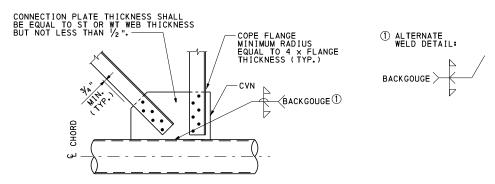
RECOMMENDED AUG. 4, 2017 SHT. 4 OF 10

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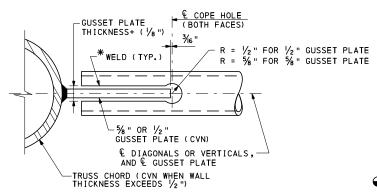
BC-743M







ST/WT ALTERNATE PANEL POINT CONNECTION DETAIL



COPE HOLE DETAIL (TYP.) * PROVIDE A WELD 'HOLDBACK' AT THE EDGE OF THE GUSSET PLATE IN THE BRACING MEMBERS EQUAL TO THE MINIMUM WELD SIZE REQUIRED.

WEB AND VERTICAL MEMBER SUBSTITUTION TABLE ⊖								
PIPE SECTION	ST/WT SUBSTITUTION SECTION							
2½ "x. 203"	ST4×11.5							
3"x.216"	ST6×15.9							
3½ "x. 226"	ST6×17.5							
4 "x. 237 "	ST6×20.4							
5 "x. 258 "	ST7.5×25							
6 "x. 280 "	WT10.5×41.5							

₩EB AND/OR VERTICAL PIPE SECTIONS MAY BE REPLACED WITH THE ST OR WT SUBSTITUTION SECTION SHOWN IN THIS TABLE. IF ST OR WT SUBSTITUTION SECTIONS ARE USED, USE THE "ST/WT ALTERNATE PANEL POINT CONNECTION DETAIL". IF THIS SUBSTITUTION IS MADE, IT WILL BE AT NO ADDITIONAL COST TO THE DEPARTMENT.

ALTERNATE PANEL POINT CONNECTION GUSSET PLATE DIMENSIONS									
CHORD NOMINAL SIZE X WALL THK.	WEB AND VERTICAL SIZE	С	D	E	F	М	t	MIN. VERT. WELD LENGTH	MIN. WEB WELD LENGTH
6 "x. 280 "	2½ "x. 203"	31/4"	4"	101/2 "	5¾"	1'-41/4"	1/2 "	51/4"	3 5/8 "
8 "x. 322 "	2½ "x. 203"	31/4"	4 "	111/2"	5¾"	1′-51/4"	1/2 "	51/4"	3 5/8 "
10 "x• 365 "	2½ "x. 203"	31/4"	4 "	1'-01/2"	5¾"	1'-61/4"	1/2 "	51/4"	3 5/8 "
12 "x• 375 "	2½ "x. 203"	31/4"	4 "	1'-11/2"	5¾"	1'-71/4"	1/2 "	51/4"	3 5/8 "
14"x. 375"	3"x. 216"	3 5/8 "	4 5/8 "	1′-31/4"	6¾"	1′-9%"	1/2 "	61/4"	41/2 "
	2½ "x. 203"	31/4"	4 "	1′-31/8"	5¾"	1′-8	1/2 "	51/4"	3 5% "
16 "x. 375 "	3"x. 216"	3 5/8 "	4 5/8 "	1'-41/4"	63/8"	1′-10%"	1/2 "	61/4"	41/2 "
	3½ "x. 226"	4"	51/4"	1′-51/4"	7 "	2' -01/4 "	5/8 "	71/4"	5 ½"
10 11 775 11	3"x. 216"	3 5/8 "	4 5/8 "	1′-51/4"	6¾"	1′-115/8"	1/2 "	61/4"	41/2 "
18 "x. 375 "	3½ "x. 226"	4"	51/4"	1′-61/4"	7"	2'-11/4"	5/8 "	71/4"	5 1/16"
	3 "x. 216 "	3 5/8 "	4 5/8 "	1′-61/4"	63/8"	2′-0%"	1/2 "	61/4"	41/2 "
20 "x• 375 "	3½ "x. 226"	4"	51/4"	1'-71/4"	7"	2' -21/4"	5/8 "	71/4"	5 1/16"
	4 "x• 237 "	41/4"	5 1/8 "	1′-81/8"	71/2"	2′-35/8"	5/8 "	81/8"	61/4"
	3½ "x. 226"	4"	51/4"	1'-91/4"	7 "	2'-41/4"	5/8 "	71/4"	5 1/16"
24"x. 375"	4 "x• 237 "	41/4"	5 % "	1′-101/8"	7½"	2′ -5 5/8 "	5/8 "	81/8"	61/4"
	5 "x. 258 "	5 "	61/4"	1'-111/4"	8	2'-81/8"	5/8 "	91/4"	6¾"
0411 50011	5 "x. 258 "	5 "	61/4"	1′-111/4"	8 % "	2'-81/8"	5/8 "	91/4"	6¾"
24"x.500"	6 "x. 280"	5¾"	7 3/4 "	2'-11/2"	101/8"	2′-115/8"	5/8 "	111/2"	8

BOLTS REQUIRED FOR ST/WT ALTERNATE PANEL POINT CONNECTION DETAIL										
MEMBER	QTY.	DIA.								
ST4×11.5	5	7∕8 "								
ST6x15.9	5	1 "								
ST6×17.5	6	1 "								
ST6×20.4	7	1 "								
ST7.5x25	8	1 "								
WT10.5×41.5	10	1 1/8 "								

NOTE: MINIMUM GUSSET PLATE SIZES PROVIDED AS A GUIDE. FABRICATOR MUST PROVIDE PLATES OF ADEQUATE SIZE TO PROVIDE MINIMUM WELD LENGTHS SPECIFIED.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

2 POST PLANAR TRUSS SPANS FROM 30' TO 100'

TRUSS DETAILS - 2

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CHIEF BRIDGE ENGINEER

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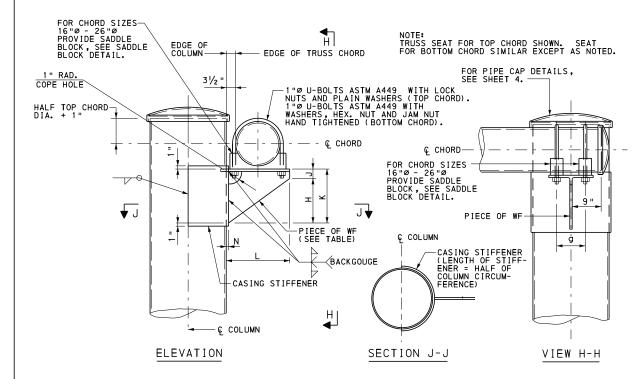
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4, 2017 SHT. 6 OF 10

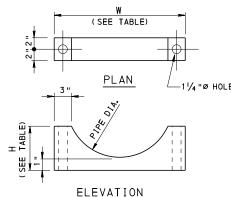
BC-743M

ALT	ALTERNATE TRUSS SEAT DETAILS SEAT DIMENSIONS												
CDAN LENGTH	WE 6175			DIMENS	IONS								
SPAN LENGTH	WF SIZE	Н	J	К	L	g	▲ N						
30′	W16×36	9¾"	3"	1'-03/4"	CHORD Ø +61/2 "	31/2"	1 "						
40′	W18×35	115/8"	3 "	1'-25/8"	CHORD Ø +61/2 "	31/2"	1 1/8 "						
50′	W21×44	1'-21/2"	3"	1'-51/2"	CHORD Ø +61/2 "	31/2"	1 1/4 "						
60′	W27×84	1′-85/8"	3 "	1′-11%"	CHORD Ø +61/2 "	5 1/2 "	1 1/4 "						
70′	W30×90	1′-9¾"	3"	2'-23/8"	CHORD Ø +61/2 "	51/2"	1 3/8 "						
80′	W33×118	2'-23/4"	3"	2' -5 3/4 "	CHORD Ø +61/2 "	5½"	1 5/8 "						
90′	W36×135	2′-5¾"	3 "	2′-83/8"	CHORD Ø +61/2 "	51/2"	1 5/8 "						
100′	W36×135	2′-5¾"	3"	2′-8¾"	CHORD Ø +61/2 "	51/2"	1 3/4 "						

▲ "N" IS TOTAL THICKNESS OF COLUMN AND CASING STIFFENER



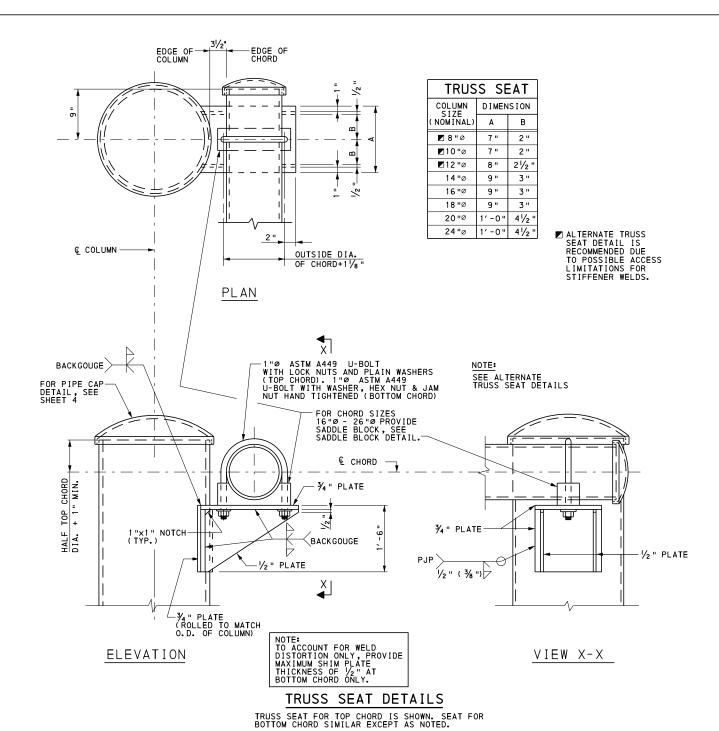
ALTERNATE TRUSS SEAT DETAILS



DIME	DDLE BL NSIONS NOMINAL	TABLE			
PIPE	1" U-B0	LT DIA.			
DIA.	NOMINAL HEIGHT "H"	NOMINAL WIDTH "W"			
16"	51/4"	1′-81/8"			
18 "	6"	1'-101/8			
20"	6¾"	2' -01/8"			
24"	85⁄16"	2'-41/8'			

SADDLE BLOCK DETAIL

4" THICK PLATE, MATERIAL SHALL BE ASTM A36, GALVANIZED PER ASTM A123.



COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

2 POST PLANAR TRUSS SPANS FROM 30' TO 100'

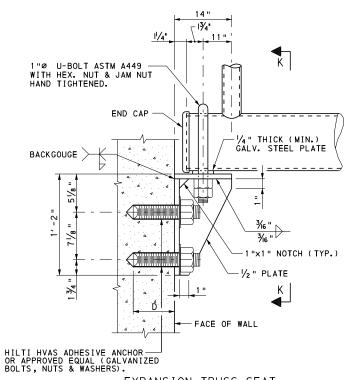
TRUSS SEAT DETAILS

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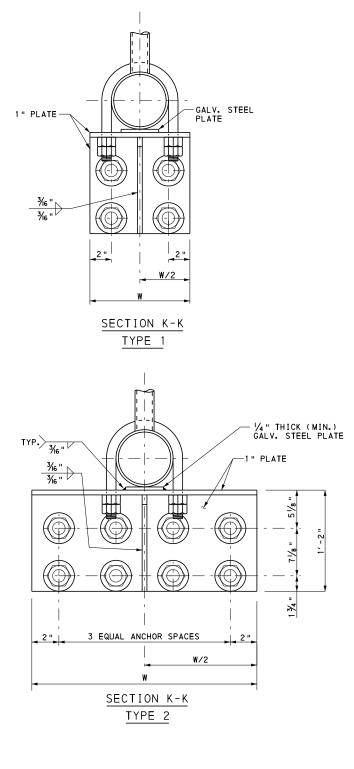
SHT. 7 OF 10 BC-743M

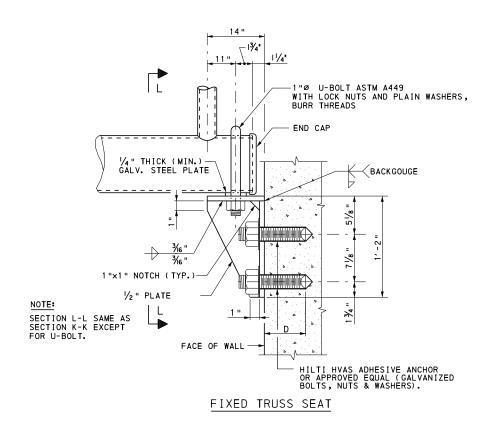
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EXPANSION TRUSS SEAT

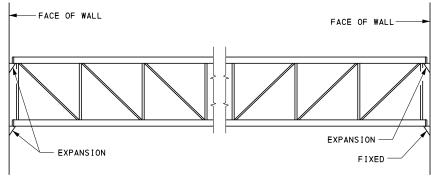
NOTE: TO SEAL ENDS OF CHORD MEMBERS, USE ALTERNATE PIPE CAP DETAIL AS SHOWN ON SHEET 10.





NOTE: ANCHOR DESIGN BASED ON CLASS A CEMENT CONCRETE (f'c = 3000 PSI).

	TRUSS S	SEATS		
SPAN LENGTH	ANCHOR DIA.	TYPE	W	D
30′	1/2 "	1	1′-33/8"	8 1/2 "
40′	5/8 "	1	1′-5¾"	10"
50′	5⁄8 "	1	1′-65/8"	10"
60′	5⁄8 "	2	1′-10%"	10"
70′	7/8 "	2	2′-05/8"	1'-11/4"
80′	⅓ "	2	2' -4 1/8 "	1'-11/4"
90′	7∕8 "	2	2′-65/8"	1'-11/4"
100′	1 "	2	2′-65%"	1'-41/2"



ELEVATION OF TYPICAL TRUSS SHOWING FIXED AND EXPANSION SEATS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

2 POST PLANAR TRUSS SPANS FROM 30' TO 100'

WALL MOUNTED TRUSS BEARING DETAILS

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CHIEF BRIDGE ENGINEER

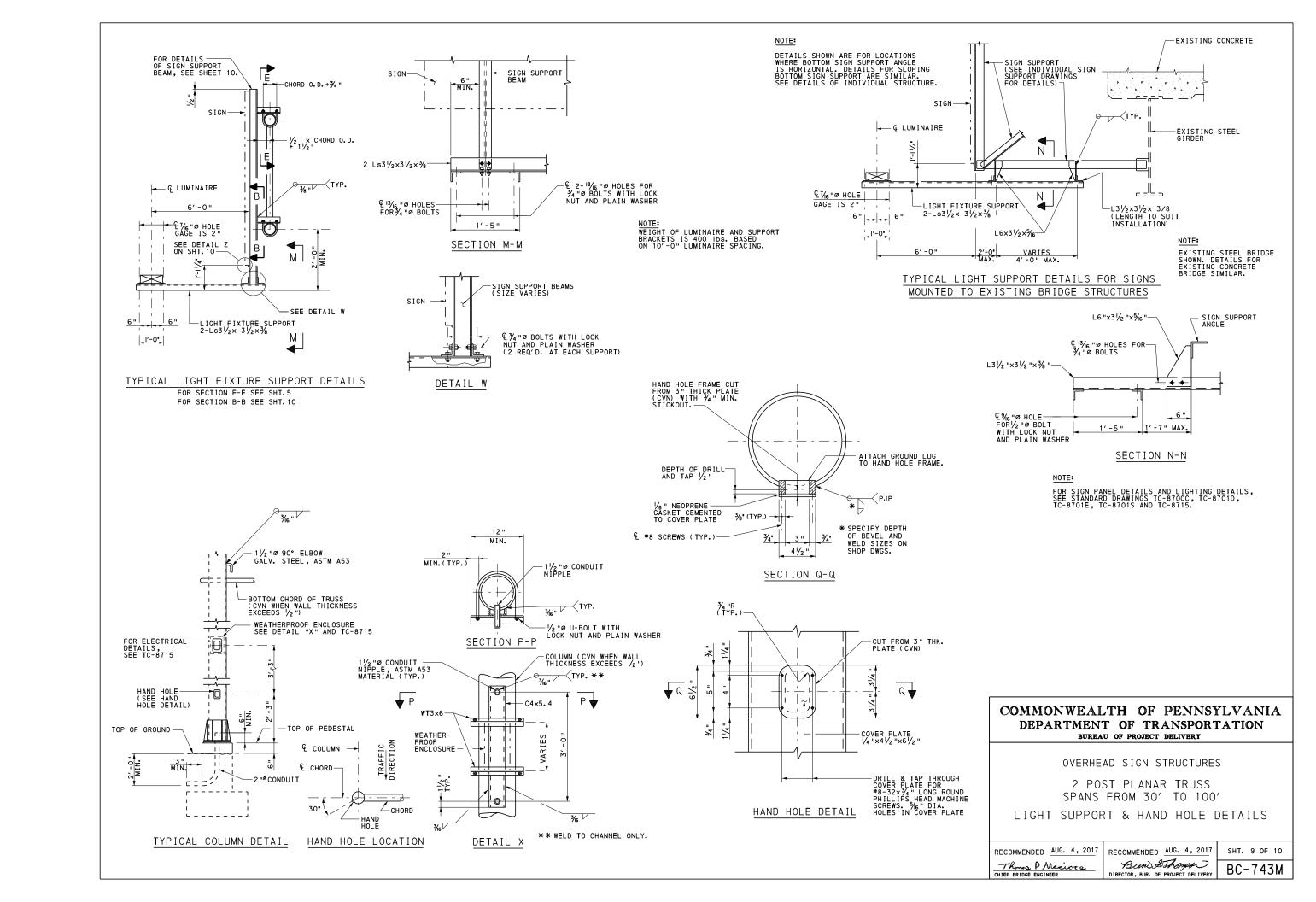
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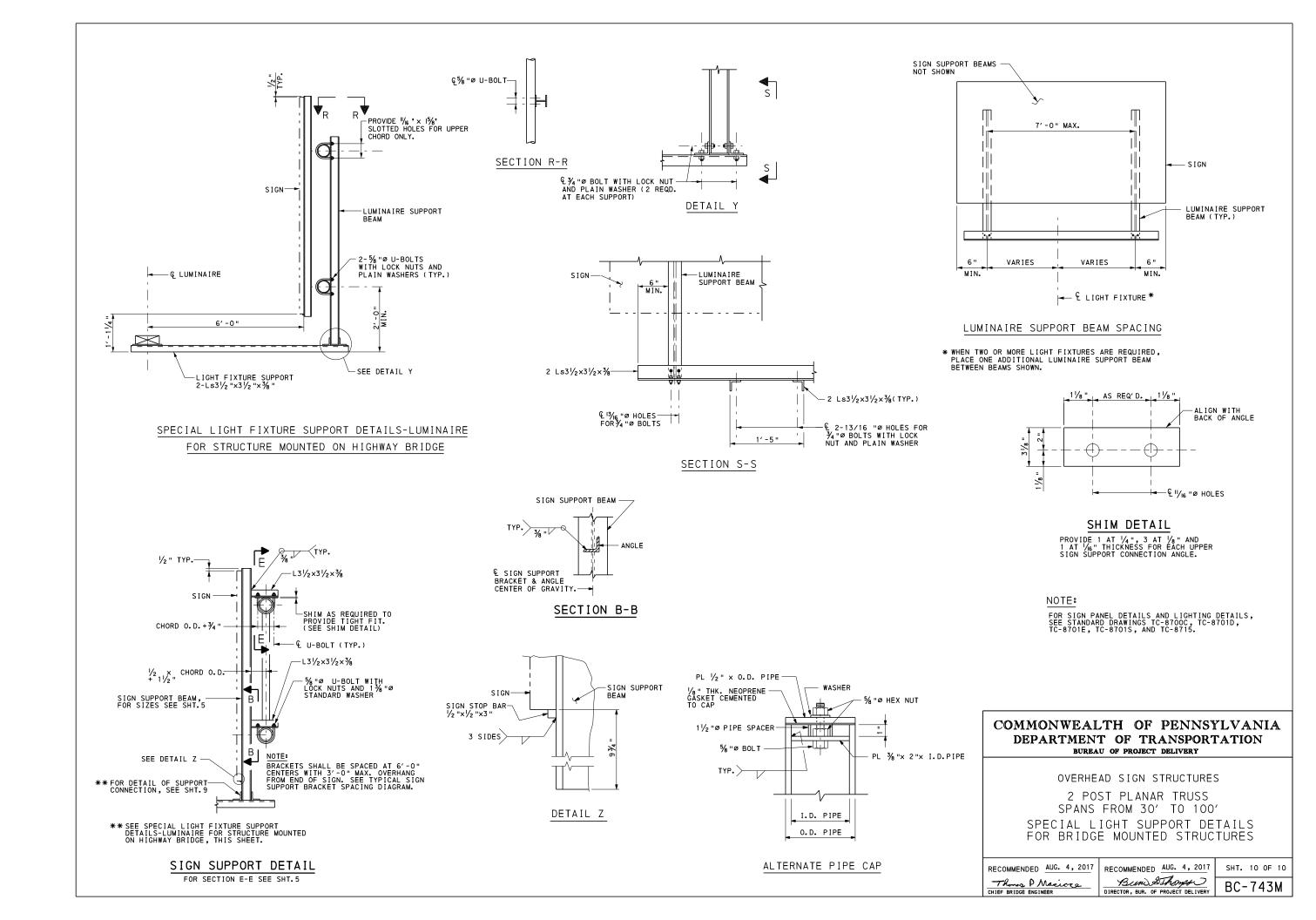
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BC-743M

SHT. 8 OF 10





INFORMATION CONTAINED IN THE BD-644M DESIGN TABLES

- DESIGN TABLES ON STANDARD DRAWING BD-644M WERE DEVELOPED USING A COMPUTER PROGRAM AND ARE BASED ON THE DESIGN CRITERIA SHOWN ON THIS SHEET, EXCEPT, THE MEMBER SIZES INDICATED DO NOT INCLUDE THE FATIGUE REQUIREMENTS INDICATED IN THE DESIGN CRITERIA SHOWN ON THE PROGRAM AND AREA OF THE PROGRAM AND THE PROGRAM AND
- THE MEMBER SIZES INDICATED IN THE DESIGN TABLES SHOULD MEET THE FATIGUE REQUIREMENTS FOR FATIGUE CATEGORY III. THE DESIGNER MUST CHECK THE ADEQUACY OF THE MEMBER SIZES INDICATED WHEN THE FATIGUE CATEGORY IS SPECIFIED TO BE I OR II FOR THE PROJECT.
- THE SPAN RANGES INCLUDED ON STANDARD DRAWING BD-644M ARE AS FOLLOWS:

TWO-POST TRI-CHORD TRUSS, SPANS FROM 60' TO 100'. FOUR-POST TRI-CHORD TRUSS, SPANS FROM 60' TO 200'.

THE DESIGN TABLES INCLUDE MEMBER SIZES FOR THE STRUCTURES FOR VARIOUS COMBINATIONS OF COLUMN HEIGHT, SPAN LENGTH, AND SIGN AREA. THEY ALSO INCLUDE SPREAD FOOTING DESIGNS. ALTERNATE CAISSON FOUNDATIONS ARE PERMITTED, HOWEVER, THE REQUIRED CAISSON EMBEDMENT AND REINFORCEMENT MUST BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PENNSYLVANIA. THE DESIGN COMPUTATIONS MUST BE SUBMITTED TO THE DISTRICT BRIDGE ENGINEER FOR REVIEW AND APPROVAL. THE CORRESPONDING FABRICATION AND CONSTRUCTION DETAILS ARE CONTAINED IN THIS STANDARD.

GENERAL NOTES

- 1. PROVIDE 3-INCH CONCRETE COVER ON REINFORCEMENT BARS, EXCEPT AS NOTED.
- USE CLASS A CEMENT CONCRETE f'c = 3000 PSI IN PEDESTALS, FOOTINGS AND CAISSONS.
- 3. PROVIDE GRADE 60 REINFORCING STEEL BARS THAT MEET THE REQUIREMENTS OF ASTM A615 FOR CONCRETE REINFORCEMENT. DO NOT WELD REINFORCING STEEL BARS.
- 4. RAKE-FINISH ALL HORIZONTAL CONSTRUCTION JOINTS, EXCEPT AS INDICATED.
- 5. VERIFY ALL DIMENSIONS AND GEOMETRY OF THE EXISTING STRUCTURES IN THE FIELD AS NECESSARY FOR PROPER FIT OF THE PROPOSED CONSTRUCTION.
- 6. CHAMFER EXPOSED CONCRETE EDGES 1 INCH BY 1 INCH

CHANGE 1

- 7. ALL DIMENSIONS SHOWN ARE HORIZONTAL, EXCEPT AS NOTED.
- 8. DIMENSIONS ARE BASED ON A NORMAL TEMPERATURE OF 68 DEGREES F.
- SPREAD FOOTINGS MAY BE ORDERED BY THE ENGINEER TO BE AT ANY ELEVATION OR OF ANY DIMENSIONS NECESSARY TO PROVIDE A PROPER FOUNDATION.
- 10. GALVANIZE ALL STRUCTURAL STEEL, BOLTS, NUTS & WASHERS IN ACCORDANCE WITH PUB. 408 UNLESS STAINLESS STEEL OR OTHERWISE INDICATED.
- 11. PIPE DIAMETERS SHOWN UP TO AND INCLUDING 12 INCHES ARE NOMINAL DIAMETERS. PIPE DIAMETERS SHOWN FROM 14 INCHES AND UP ARE ACTUAL DIAMETERS.
- 12. USE STANDARD SIZE HOLE. THE STANDARD HOLE DIAMETER FOR BOLTS SMALLER THAN 1" DIAMETER SHALL BE THE NOMINAL DIAMETER OF THE BOLT PLUS 1/6". FOR BOLTS 1" DIAMETER AND LARGER, THE WIDTH OF EACH STANDARD HOLE SHALL BE THE NOMINAL DIAMETER OF THE BOLT PLUS 1/8".
- 13. CLEAR DISTANCE BETWEEN BOLT HOLES OR BETWEEN THE BOLT HOLE AND THE END OF THE MEMBER IN THE DIRECTION OF THE APPLIED BEARING FORCE SHALL BE CHECKED.
- 14. PROVIDE ANCHOR BOLT HOLES 1/4" LARGER THAN BOLT DIAMETER.
- 15. PROVIDE A MINIMUM ANCHOR BOLT EMBEDMENT LENGTH OF 20 ANCHOR BOLT DIAMETERS.
- 16. PROVIDE DOUBLE NUTS AND WASHER FOR EACH ANCHOR BOLT.
- 17. STEEL MEMBER COMPONENTS REQUIRING CHARPY V-NOTCH TESTING ARE DESIGNATED ON THE PLANS BY (CVN), PROVIDE STEEL CONFORMING TO THE CVN REQUIREMENTS FOR ZONE 2, NON FRACTURE CRITICAL AS GIVEN IN THE AASHTO MATERIAL SPECIFICATIONS.

NOTES TO FABRICATOR

- DYNAMIC/VARIABLE MESSAGE SIGNS (DMS/VMS) ARE PROHIBITED ON 2-POST AND 4-POST TRI-CHORD TRUSS STRUCTURE TYPES AS PRESENTED IN THESE STANDARDS. OVERHEAD SIGN STRUCTURES INTENDED TO CARRY DMS/VMS MUST BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PENNSYLVANIA AND SUBMITTED TO THE CHIEF BRIDGE ENGINEER FOR REVIEW AND APPROVAL.
- DESIGN COMPUTATIONS ARE REQUIRED FOR ANY PORTION OF A STRUCTURE FOR WHICH THE INFORMATION IS NOT TAKEN DIRECTLY FROM THE CONTRACT DRAWINGS OR THE DETAILS CONTAINED IN THIS STANDARD. DO NOT VIOLATE CRITERIA USED FOR THE DEVELOPMENT OF THE DESIGN TABLES ON STANDARD DRAWING BD-644M AND THE DETAILS IN THIS STANDARD.
- FABRICATOR TO SELECT PANEL POINT CONNECTION DETAIL TYPE BASED ON MEMBER SIZE AND TRUSS CONFIGURATION TO ENSURE FIT-UP, FABRICATION, GALVANIZING AND ERECTION.

DESIGN CRITERIA FOR PENNDOT SIGN STRUCTURES

 DEAD LOADS PENNDOT STD. DWGS. (U.N.O.) * TC-8701E OR TC-8701S BC-744M, SHT. 12 BC-744M, SHT. 8 CALCULATED INTERNALLY WITHIN PROGRAM SIGN PANELS LIGHT FIXTURES SIGN SUPPORT BEAM COLUMNS, CHORDS • EXTERNAL LOADS AASHTO SIGN SPECS. ICE LOAD WIND LOAD APPENDIX C, SECTION C.3, EQ. C-1, WITH 80 MPH WIND AND 30% GUST FACTOR

 GROUP LOADS AASHTO SIGN SPECS, 3,4

STEEL CRITERIA

AASHTO SIGN SPECS.

SECTION PROPERTIES FOR TUBULAR SHAPES
MAXIMUM STRESSES IN TUBULAR SHAPES
ALLOWABLE STRESSES FOR TUBULAR SHAPES
ALLOWABLE STRESSES FOR SIGN SUPPORTS
ALLOWABLE STRESSES FOR BASE PLATES
ALLOWABLE STRESSES FOR BASE PLATES
FATIGUE REQUIREMENTS (FATIGUE CATEGORY II) APPENDIX B, TABLE B-1 APPENDIX B, TABLE B-2 5.6 (TABLE 5-3) & 5.11 5.12 SECTION 11 ALLOWABLE DEFLECTION
PERMANENT CAMBER
ALLOWABLE STRESSES FOR STRUCTURAL STEEL SECTION 5

AASHTO HIGHWAY BRIDGES (U.N.O.) BOLT CRITERIA ALLOWABLE BOLT STRESSES SLIP-CRITICAL BOLT ALLOWABLE BOLT PRYING ACTION COMBINED BOLT SHEAR AND TENSION BOLT DESIGN CRITERIA ALLOWABLE ANCHOR BOLT STRESSES TABLE 10.32.3B 10.32.3.2.1 10.32.3.3.2 10.32.3.3.2 10.32.3.3.3 AASHTO SIGN SPECS. 5.16 AASHTO SIGN SPECS. 5.17

 CONCRETE CRITERIA AASHTO HIGHWAY BRIDGES (U.N.O.)

ALLOWABLE BEARING STRESS
REINFORCEMENT TENSILE STRESS
SHEAR CAPACITY OF FOOTINGS
SHEAR STRESS IN FOOTINGS
ALLOWABLE SHEAR STRESS
ALLOWABLE SHEAR STRESS
SLENDERNESS OF COLUMNS
MINIMUM REINF. OF FLEXURAL MEMBERS
SPACING LIMITS FOR REINFORCEMENT
MINIMUM CONCRETE COVER
PRESSURES FOR ECCENTRICALLY LOADED FOOTINGS
DISTRIBUTION OF REINFORCEMENT
FOOTING STABILITY REQUIREMENTS
TORSION
COLUMN DESIGN (PEDESTALS) 8.15.2.1.3 8. 15. 2. 2 8. 15. 5. 6. 1 8. 15. 5. 6. 2 8. 15. 5. 6. 4 8. 16. 5. 2 8.21 DM4 D8.22.1* FIG. 4.4.7.1.1.1C 4.4.11.2.2 DM4 D5.5.5 ACI SECTION A.7.3* 8.15.4

SPREAD FOOTINGS

MAXIMUM DESIGN PRESSURE MINIMUM AREA IN BEARING UNIT WEIGHT OF SOIL 1.5 TONS PER SQUARE FOOT 95% 100 POUNDS PER CUBIC FOOT

• DRILLED SHAFTS (CAISSONS)

COLUMN DESIGN (PEDESTALS)

DM4 SEC. 4.6, PENNDOT COM624 COMPUTER PROGRAM

BARRIER PLACEMENT AT OBSTRUCTIONS

SINGLE FACE CONCRETE BARRIER PLACEMENT AT MEDIAN PIERS

REFERENCE DRAWINGS

MAXIMUM DESIGN PRESSURE
MAXIMUM DESIGN LATERAL DISPLACEMENT
MODULUS OF SUBGRADE REACTION
UNIT WEIGHT OF SOIL
ANGLE OF INTERNAL FRICTION
COHESION

1.5 TONS PER SQUARE FOOT 10.0 POUNDS PER CUBIC INCH 100 POUNDS PER CUBIC FOOT 25° O KIPS PER SQUARE FOOT

SEISMIC DESIGN CRITERIA

STRUCTURES ARE DESIGNED FOR A SEISMIC ACCELERATION COEFFICIENT = 0.15

RC-54M

RC-58M

CONSTRUCTION GENERAL NOTES

MATERIALS AND WORKMANSHIPs

PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH THE CURRENT VERSIONS OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, AASHTO/AWS WELDING CODE D1.5, CONTRACT SPECIAL PROVISIONS, AND AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS". USE AASHTO/AWS D1.1 FOR WELDING NOT COVERED IN

• PROVIDE STRUCTURAL STEEL CONFORMING TO THE FOLLOWING:

COLUMNS & PIPE CHORDS:

SEE PUBLICATION 408. SECTION 948.2.

ANGLES, SHAPES, AND PLATES: AASHTO M270, GRADE 36 ASTM A709, GRADE 36

ALTERNATE PRESS-BREAK MEMBERS:

ALTERNATE PRESS-BREAK MEMBERS MUST HAVE THE EQUIVALENT STRENGTH OF THE MEMBER THEY ARE REPLACING. EQUIVALENT RADIUS FOR PRESS-BREAK MEMBERS IS MEASURED FROM THE CENTER OF THE MEMBER TO THE MID-POINT OF ANY CHORD OF THE MEMBER. MINIMUM THICKNESS OF PRESS-BREAK MEMBERS TO BE \$6". PENNDOT SIGN STRUCTURE COMPUTER PROGRAM OR AN APPROVED FINITE ELEMENT ANALYSIS COMPUTER PROGRAM MUST BE RUN TO VERIFY THE ADEQUACY OF PRESS-BREAK MEMBERS FOR STRENGTH AND FATIGUE. ALTERNATE PRESS-BREAK MEMBERS ARE ONLY PERMITTED FOR COLUMNS. PRESS-BREAK MEMBERS ARE NOT PERMITTED FOR CHORDS.

PROVIDE BOLTS CONFORMING TO THE FOLLOWING:

ANCHOR BOLTS:

ASTM, F1554 GRADE 55 PER PUBLICATION 408 SECTION 1105.02(c)3.

AASHTO M164 (ASTM A325) H.S. BOLTS EXCEPT AS NOTED

• DESIGN SPECIFICATIONS:

AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS", 2001 WITH CURRENT INTERIMS (UNLESS NOTED OTHERWISE); AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 1996 WITH INTERIMS THROUGH AND INCLUDING 2000; PENNDOT DESIGN MANUAL - PART 4, AUGUST 1993 EDITION (INCLUDING AUGUST 1995 REVISIONS)

ALL FILLET WELDS SHOWN ARE MINIMUM SIZE UNLESS NOTED OTHERWISE.

* LEGEND:

AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS" • AASHTO SIGN SPEC:

AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" AASHTO HIGHWAY BRIDGES:

DM4: PENNSYLVANIA DEPARTMENT OF TRANSPORTATION, DESIGN MANUAL PART 4, STRUCTURES

UNLESS NOTED OTHERWISE

AMERICAN CONCRETE INSTITUTE - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE WITH COMMENTARY (ACI 318-99). ACI:

Thoma P Macioca

CVN: CHARPY V-NOTCH.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

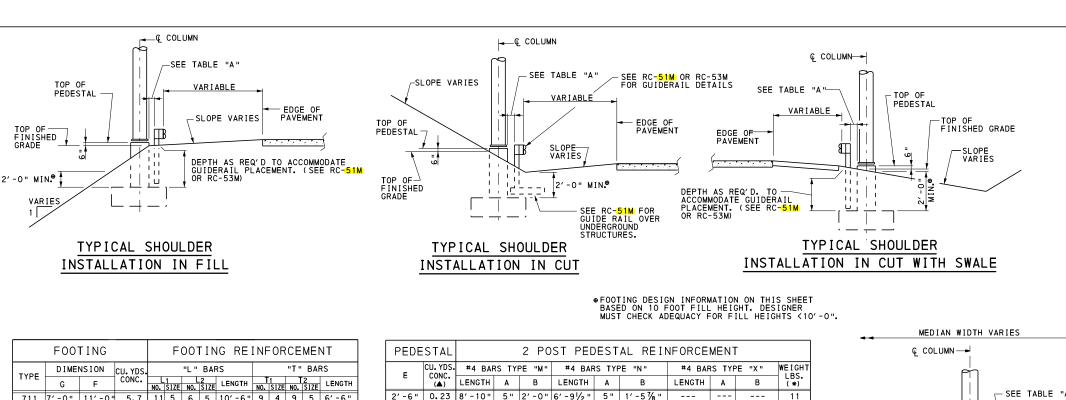
RECOMMENDED AUG. 4, 2017 RECOMMENDED AUG. 4, 2017

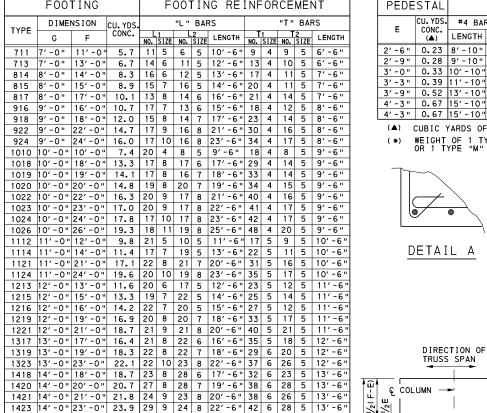
Bun SThomps DIRECTOR, BUR, OF PROJECT DELIVERY

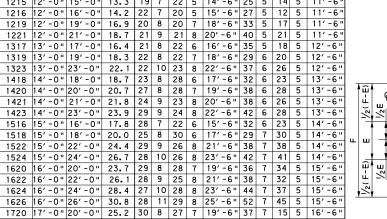
BC-744M

SHT. 1 OF 12

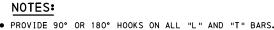
BUREAU OF PROJECT DELIVERY SPACING CHARTS/DIRECT APPLIED LETTERS, NUMERALS, & ARROWS TC-8700C SIGN DETAILS/FREEWAY AND EXPRESSWAY GUIDE SIGNS TC-8701D OVERHEAD SIGN STRUCTURES TC-8701E EXTRUDED ALUMINUM CHANNEL SIGN TC-8701S FLAT SHEET ALUMINUM SIGNS WITH EXTRUDED ALUMINUM STIFFENERS TC-8715 NOTES AND DESIGN CRITERIA BC-736M REINFORCEMENT BAR FABRICATION DETAILS RC-11M CLASSIFICATION OF EARTHWORK FOR STRUCTURES 2 POST AND 4 POST TRI-CHORD TRUSS RC-51M TYPE 31 STRONG POST GUIDE RAIL SPANS FROM 60' TO 240' RC-53M TYPE 2 WEAK POST GUIDE RAIL







ELEVATION SHOWN. SECTION A-A SIMILAR.

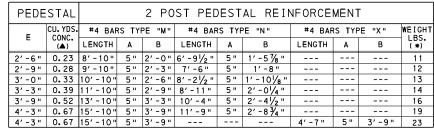


• LENGTH FOR "L" AND "T" BARS DOES NOT INCLUDE 90° OR 180° HOOK LENGTHS.

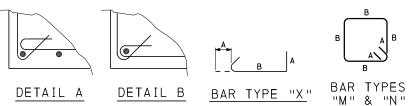
1821 | 18' - 0 " | 21' - 0 " | 28. 0 | 30 | 9 | 28 | 8 | 20' - 6 " | 37 | 8 | 42 | 5 | 17' - 6 "

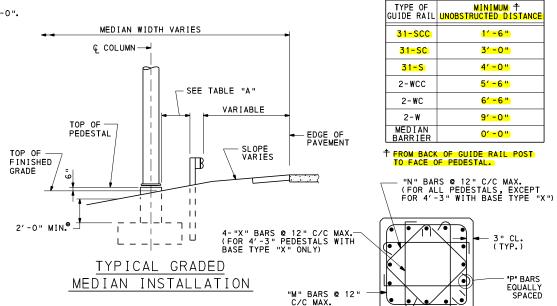
1917 | 19' - 0 | | 17' - 0 | | 23.9 | 32 | 8 | 29 | 6 | 16' - 6 | | 28 | 8 | 14 | 5 | 18' - 6 |

• COUNT AND SIZE OF PEDESTAL DETAIL "P" BARS TO BE SPECIFIED ON THE CONTRACT DRAWINGS, BASED ON INFORMATION OBTAINED FROM DESIGN TABLES ON BD-644M, SHEETS 5, 6, AND 7.



(A) CUBIC YARDS OF CONCRETE PER 1 FOOT HEIGHT OF PEDESTAL. WEIGHT OF 1 TYPE "M" BAR PLUS 1 TYPE "N" BAR (WHERE INDICATED) OR 1 TYPE "M" BAR PLUS 4 TYPE "X" BARS (BASE TYPE "X" ONLY)





NOTES:

- SEE STANDARD DRAWING BC-736M FOR REINFORCEMENT BAR FABRICATION DETAILS.
- SEE STANDARD DRAWING RC-11M FOR LIMITS OF CLASS 3 EXCAVATION.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

SECTION B-B

TABLE "P"

1'-3"

PROVIDE HOOK ON ALL "P" BARS. "P" BA MAY BE DOWELED TO FOOTING USING CLASS C MIN. LAP SPLICE, HOWEVER NO COMPENSATION WILL BE ALLOWED FOR

TABLE "A"

4.303 1'-5"

5.313 1'-7"

LENGTH

K + 2'-1"

K + 2'-4"

K + 2'-8"

K + 2'-10"

K + 3'-0"

"P" BARS, "P" BARS

BAR | WEIGHT

LBS./FT.

1.502

2.670

3.400

SIZE

6

10

11

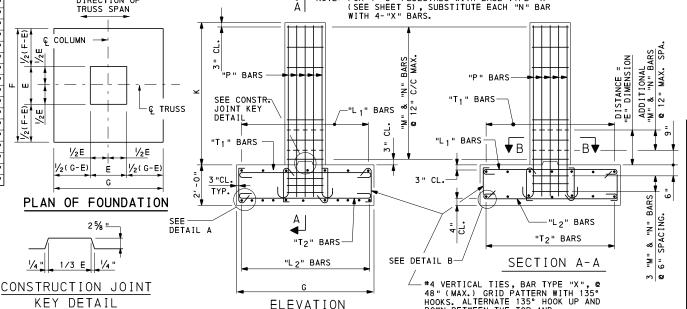
OVERHEAD SIGN STRUCTURES 2 POST TRI-CHORD TRUSS SPANS FROM 60' TO 100'

FOUNDATION DETAILS

RECOMMENDED AUG. 4, 2017 Thomas P Macioca

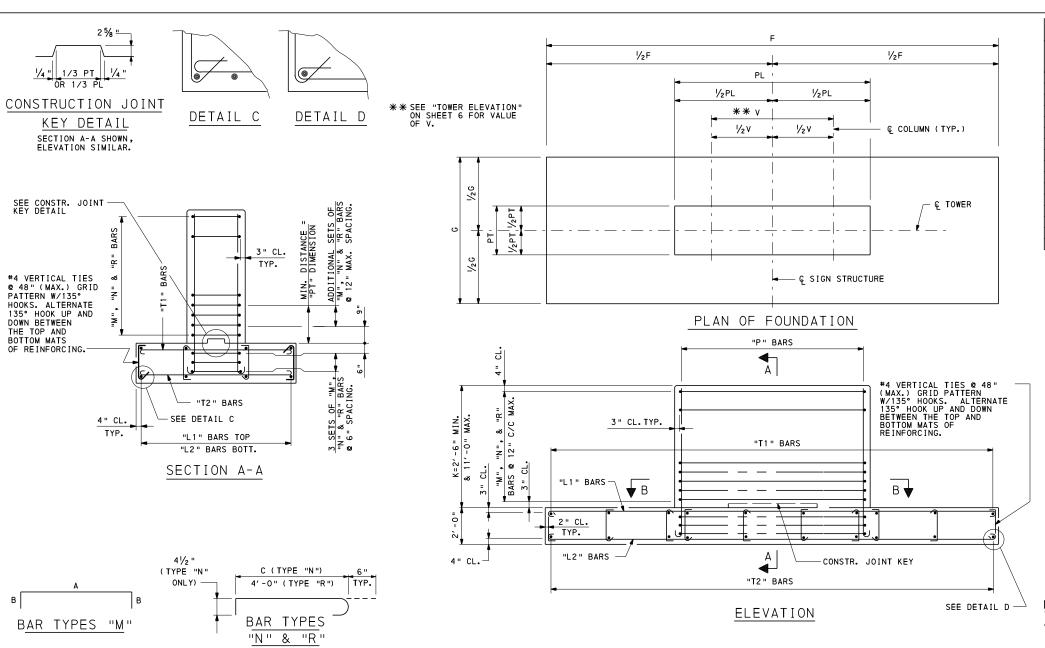
RECOMMENDED AUG. 4, 2017 Bun SThomps DIRECTOR, BUR. OF PROJECT DELIVERY

SHT. 2 OF 12 BC-744M



NOTE: FOR 4'-3" PEDESTALS WITH BASE TYPE "X'

DOWN BETWEEN THE TOP AND BOTTOM MATS OF REINFORCING.



TVDE	DIME	NSION	CU. YDS.			"L "	BAR	S	"T" BARS				
TYPE	G	F	CONC.	NO.	1 SIZE		2 SIZE	LENGTH		1 SIZE		2 SIZE	LENGTH
612	6'-0"	12'-0"	5.3	5	4	5	5	11'-6"	9	4	9	5	5′-6"
613	6'-0"	13'-0"	5.8	6	4	5	5	12'-6"	10	4	10	5	5′-6"
614	6'-0"	14'-0"	6.2	9	4	5	5	13′-6"	11	4	11	5	5′-6"
615	6'-0"	15'-0"	6.7	10	4	5	5	14'-6"	11	4	11	5	5′-6"
616	6'-0"	16'-0"	7.1	9	4	5	5	15'-6"	12	4	12	5	5′-6"
714	7'-0"	14'-0"	7.3	6	4	6	5	13′-6"	11	4	11	5	6'-6"
715	7′-0"	15'-0"	7.8	6	4	6	5	14'-6"	11	4	11	5	6'-6"
716	7′-0"	16'-0"	8.3	11	5	8	5	15'-6"	14	4	12	5	6'-6"
717	7'-0"	17'-0"	8.8	14	5	9	5	16' -6"	16	4	14	5	6' -6"
718	7'-0"	18'-0"	9.3	12	6	11	5	17'-6"	17	4	14	5	6'-6"
719	7′-0"	19'-0"	9.9	11	7	14	5	18'-6"	19	4	14	5	6′-6"
720	7′-0"	20' -0"	10.4	13	6	13	5	19'-6"	20	4	15	5	6′ -6"
721	7′-0"	21'-0"	10.9	13	6	13	5	20′ -6"	21	4	15	5	6' -6"
722	7′-0"	22'-0"	11.4	14	6	14	5	21'-6"	23	4	16	5	6′ -6"
818	8'-0"	18'-0"	10.7	13	5	10	5	17'-6"	14	4	14	5	7′ -6"
819	8'-0"	19'-0"	11.3	13	6	11	5	18'-6"	24	4	14	5	7′ -6"
• LE		OR "L" 80° HOO	AND "T K LENG	" B THS	•	DOE	S N	L " AND		:	RS.		

FOOTING REINFORCEMENT

• FOR CONTINUATION OF FOOTING TABLE, SEE SHEET 4.

FOOTING

	TABLE "P"												
BAR SIZE	WEIGHT LBS./FT.	A	LENGTH										
9	3.400	1′-3"	K + 2'-8"										
10	4.303	1′-5"	K + 2'-10"										
11	5.313	1′-7"	K + 3'-0"										
<u>3" CI</u>	К	1	/ - 8 " A										

PROVIDE HOOK ON ALL "P" BARS. "P" BARS MAY BE DOWELED TO FOOTING USING CLASS C MIN. LAP SPLICE, HOWEVER NO COMPENSATION WILL BE ALLOWED FOR ADDITIONAL STEEL INVOLVED.

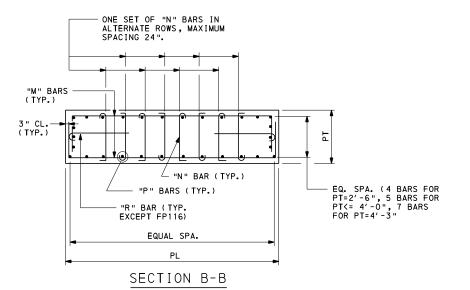
NOTES:

- PEDESTAL TYPE AND FOOTING TYPE INDICATED ON CONTRACT DRAWINGS OBTAINED FROM BD-644M, SHEETS 9, 10, 11, 12, AND 13.
- FOR INSTALLATION DETAILS, SEE SHEET 2.
- FOOTING DESIGN INFORMATION ON THIS SHEET BASED ON 10 FT. FILL HEIGHT. DESIGNER MUST CHECK ADEQUACY FOR FILL HEIGHTS <10'-0".
- SEE STANDARD DRAWING BC-736M FOR REINFORCING BAR FABRICATION DETAILS.
- SEE STANDARD DRAWING RC-11M FOR LIMITS OF CLASS 3 EXCAVATION.

			CU. YDS.			HOR	ZONTAL F	REINFORCE	MENT		WEIGHT
PEDESTAL TYPE	PT	PL	CONC.	"P" BARS	#4 B	ARS TYPE	"M "	#4 BA	RS TYPE	"N "	PER >
11176			(▲)	DAKS	LENGTH	Α	В	LENGTH	С	NO.	(LBS.
FP116	2′-6"	7′-9"	0.72	28-#9	11'-9"	7′ -3 "	2'-3"	2'-101/2 1	2'-0"	3	21.5
FP122	2'-9"	8'-9"	0.89	36-#9	12'-11"	8′-3"	2'-4"	3'-11/2"	2'-3"	4	31.6
FP222	2'-9"	10' -3"	1.04	42-#9	14'-5"	9'-9"	2'-4"	3'-11/2"	2'-3"	5	35.7
FP126	3′ -3 "	9'-3"	1.11	36-#10	13'-11"	8′-9"	2'-7"	3' - 7 1/2 "	2'-9"	4	34.3
FP226	3′-3"	11'-0"	1.32	42-#10	15′-8"	10'-6"	2'-7"	3' -71/2 "	2'-9"	5	39.1
FP326	3′-3"	12'-6"	1.50	48-#10	17'-2"	12'-0"	2'-7"	3'-71/2"	2'-9"	6	43.5
FP426	3′-3"	14'-3"	1.72	54-#10	18' - 11"	13'-9"	2'-7"	3'-71/2"	2'-9"	7	48.2
FP526	3′-3"	15'-9"	1.90	60-#10	20′ -5 "	15'-3"	2'-7"	3'-71/2"	2'-9"	8	52.7
FP131	3′-9"	10'-0"	1.39	36-#11	15'-2"	9'-6"	2'-10"	4'-11/2"	3′-3"	4	37.3
FP231	3′-9"	11'-9"	1.63	42-#11	16'-11"	11'-3"	2'-10"	4' - 1 1/2 "	3′-3"	5	42.4
FP331	3′-9"	13' -3"	1.84	46-#11	18'-5"	12'-9"	2'-10"	4' - 1 1/2 "	3′-3"	6	47.1
FP431	3′-9"	15'-0"	2.08	52-#11	20′ -2 "	14'-6"	2'-10"	4'-11/2"	3′ -3"	7	52.2
FP531	3′-9"	17'-0"	2.36	60-#11	22'-2"	16'-6"	2'-10"	4'-11/2"	3′ -3 "	8	57.7
FP136	4'-0"	11'-3"	1.67	42-#11	16'-9"	10'-9"	3'-0"	4'-41/2"	3′-6"	5	43.0
FP236	4'-0"	12'-9"	1.89	48-#11	18'-3"	12'-3"	3'-0"	4'-41/2"	3′-6"	6	47.9
FP336	4'-0"	13'-6"	2.00	50-#11	19'-0"	13'-0"	3′-0"	4'-41/2"	3′-6"	7	51.9
FP436	4'-0"	15'-0"	2.22	56-#11	20′ -6"	14'-6"	3′-0"	4'-41/2"	3'-6"	8	56.8
FP536	4'-0"	16'-3"	2.41	62-#11	21'-9"	15'-9"	3'-0"	4'-41/2"	3′-6"	9	61.4
FP636	4'-0"	18'-0"	2.67	68-#11	23'-6"	17'-6"	3'-0"	4'-41/2"	3′-6"	10	66.6
FP138	4'-3"	13'-0"	2.05	54-#11	18' -8"	12'-6"	3′-1"	4' -71/2"	3′-9"	7	52.6
FP238	4′ - 3 "	15'-0"	2.36	60-#11	20' -8 "	14' -6"	3′ - 1 "	4'-71/2"	3′-9"	8	58.3
FP338	4' -3"	16'-9"	2.64	66-#11	22′ -5 "	16'-3"	3'-1"	4'-71/2"	3′-9"	9	63.8
FP438	4'-3"	18'-3"	2.87	72-#11	23'-11"	17'-9"	3'-1"	4'-71/2"	3'-9"	10	68.9

* ONE SET INCLUDES 2 "M" BARS, 2 "R" BARS (EXCEPT FOR FP116) AND NO. OF "N" BARS SHOWN IN TABLE.

(▲) CUBIC YARDS OF CONCRETE PER 1 FOOT HEIGHT OF PEDESTAL.



COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES 4 POST TRI-CHORD TRUSS SPANS FROM 60' TO 240'

FOUNDATION DETAILS

RECOMMENDED AUG. 4, 2017 Thomas P Macioca CHIEF BRIDGE ENGINEER

RECOMMENDED AUG. 4, 2017 Bun & Thomps DIRECTOR, BUR. OF PROJECT DELIVERY

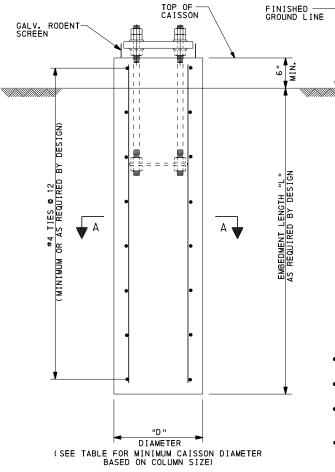
SHT. 3 OF 12 BC-744M

TABLE CONTINUED FROM SHEET 3.														
F	11 TOO	1G		F	FOOTING REINFORCEMENT									
TYPE	DIME	NSION	CU. YDS.	"L" BARS						"T" BARS				
ITE	G	F	CONC.	NO.	1 SIZE	NO.	2 SIZE	LENGTH	NO.	1 SIZE	NO.	2 S 7 F	LENGTH	
820	8'-0"	20′ -0"	11.9	15	7	13	6	19'-6"	29	4	15	5	7′-6"	
821		21'-0"	12.4	13	8	15	6	20' -6"	31	4	15	5	7'-6"	
822		22'-0"	13.0	16	7	15	6	21'-6"	32	4	16	5	7'-6"	
823		23′ -0"	13.6	13	8	13	7	22' -6"	34	4	17	5	7'-6"	
824	8'-0"	24'-0"	14.2	15	7	13	6	23' -6"	32	4	17	5	7′-6"	
825	8'-0"	25'-0"	14.8	13	8	15	7	24' -6"	38	4	18	5	7′-6"	
921	9'-0"	21'-0"	14.0	16	6	14	5	20'-6"	24	4	15	5	8'-6"	
922	9'-0"	22'-0"	14.7	15	8	16	6	21'-6"	40	4	16	5	8'-6"	
923	9'-0"	23′-0"	15.3	15	8	17	6	22' -6"	42	4	17	5	8'-6"	
924	9'-0"	24'-0"	16.0	16	8	16	7	23' -6"	47	4	18	5	8'-6"	
925		25′-0"	16.7	17	8	18	7	24'-6"	50	4	19	5	8'-6"	
926		26′-0"	17.3	16	8	16	7	25'-6"	48	4	21	5	8'-6"	
927		27′-0"	18.0	17	8	18	7	26' -6"	53	4	20	5	8'-6"	
928		28'-0"	18.7	15	8	15	7	27'-6"	56	4	24	5	8'-6"	
929	-	29'-0"	19.3	18	8	16	8	28'-6"	59	4	24	5	8'-6"	
1016		16'-0"	11.9	12	4	8	5	15'-6"	22	5	14	5	9'-6"	
1017		17'-0"	12.6	13	5	9	5	16' -6"	24	5	15	5	9'-6"	
1018		18'-0"	13.3	14	5	9	5	17'-6"	27	5	18	5	9'-6"	
1019		19'-0"	14.1	14	5	10	5	18' -6"	29	5	20	5	9'-6"	
1020		20' -0"	14.8	15	5	11	5	19' -6"	31	5	23	5	9'-6"	
1021		21'-0"	15.6	15	5	12 19	5	20' -6"	33 39	5	17	5	9'-6"	
1024		25'-0"	18.5	17 17	8	17	6 7	24'-6"	44	4	18	5	9'-6"	
1025		26'-0"	19.3	18	8	19	7	25'-6"	47	4	20	5	9'-6"	
1028		27'-0"	20.0	19	8	17	8	26'-6"	52	4	23	5	9'-6"	
1021		28'-0"	20.7	19	8	17	8	27'-6"	54	4	24	5	9'-6"	
1029		29'-0"	21.5	20	8	17	8	28'-6"	58	4	25	5	9'-6"	
1030		30' -0"	22.2	17	9	17	8	29' -6"	62	4	27	5	9'-6"	
1032		32′-0"	23.7	16	9	17	8	31'-6"	63	4	27	5	9'-6"	
1117		17'-0"	13.9	21	5	13	5	16'-6"	28	5	14	5	10′ -6 "	
1118		18'-0"	14.7	19	6	17	5	17'-6"	33	5	18	5	10′ -6"	
1119	11'-0"	19'-0"	15.5	17	7	20	5	18'-6"	35	5	19	5	10′ -6"	
1120	11'-0"	20' -0"	16.3	20	7	17	6	19'-6"	39	5	22	5	10' -6"	
1121	11'-0"	21'-0"	17.1	19	7	16	6	20' -6"	42	5	29	5	10' -6"	
1122		22′-0"	17.9	19	7	17	6	21'-6"	44	5	30	5	10′ -6"	
1123		23′-0"	18.7	20	7	18	6	22'-6"	47	5	30	5	10′-6"	
1124		24'-0"	19.6	16	6	17	5	23′-6"	48	5	33	5	10′ -6"	
1125		25′-0"	20.4	17	8	20	6	24'-6"	51	5	31	5	10′ -6 "	
1127		27′-0"	22.0	18	8	19	7	26'-6"	42	5	25	5	10′ -6 "	
1129		29′-0"	23.6	20	9	19	8	28'-6"	46	5	26	5	10′ -6"	
1130		30′-0"	24.4	19	9	19	8	29'-6"	47	5	29	5	10′ -6"	
1131		31′-0"	25.3	20	9	19	8	30′ -6"	49	5	29	5	10' -6"	
1132		32′-0"	26.1	22	9	19	8	31'-6"	53	5	32	5	10′ -6 "	
1133		33′-0"	26.9	19	8	18	8	32′ -6"	53	5	36	5	10′ -6 "	
1134		34'-0"	27.7	18	10	20	8	33′ -6"	59	5	40	5	10′ -6"	
1135		35′-0"	28.5	19	9	19	8	34' -6"	62	5	47	5	10′ -6"	
1219		19'-0"	16.9	23	5	15	5	18' -6"	28	5	14	5	11'-6"	
1221		21'-0"	18.7	19	8	19	6	20′ -6"	33	6	25	5	11'-6"	
1222		22′-0" 23′-0"	19.6	20	8	23	6	21' -6"	39	6	29	5	11'-6"	
1223	120"	<u> 23 -0"</u>	20.4	20	8	23	6	22 -0"	40	6	37	5	11'-6"	

		F00	TING			F	OOT	ΙN	G REI	NF	0R	CEI	MEN	١T
	TYPE	DIME	NSION	CU. YDS.			"L "		-	_		'T"_		S
	• • • •	G	F	CONC.	NO.	SIZE	NO.	2 SIZE	LENGTH	NO.	1 SIZE	NO.	2 SIZE	LENGTH
	1224	12′ -0"	24′-0"	21.3	20	8	23	6	23′-6"	43	6	40	5	11'-6
ı	1225		25′-0"	22.2	20	8	24	6	24'-6"	44	6	37	5	11'-6
ı	1226		26'-0"	23.1	20	8	23	6	25' -6"	44	6	38	5	11'-6
ı	1227	12'-0"	27'-0"	24.0	18	6	22	5	26' -6"	53	5	41	5	11'-6
ı	1236	12'-0"	36′-0"	32.0	23	10	20	9	35′-6"	57	6	52	5	11'-6
ı	1323	13'-0"	23′-0"	22.1	26	7	21	6	22'-6"	47	5	23	5	12'-6
ı	1324	13'-0"	24'-0"	23.1	21	8	20	7	23' -6"	43	6	39	5	12'-6
Ī	1325	13'-0"	25'-0"	24.1	21	8	20	7	24' -6"	42	6	44	5	12'-6
	1326	13'-0"	26′-0"	25.0	22	8	24	7	25′-6"	46	6	38	6	12'-6
	1327	13'-0"	27′-0"	26.0	23	8	26	7	26'-6"	49	6	48	5	12'-6
	1328	13'-0"	28′-0"	27.0	24	7	25	6	27′-6"	49	6	51	5	12'-6
	1329		29′-0"	27.9	21	8	24	7	28' -6"	52	6	42	6	12'-6
	1330		30′ -0"	28.9	21	8	25	7	29'-6"	54	6	56	5	12'-6
	1331		31'-0"		21	8	24	7	30′ -6"	54	6	57	5	12'-6
	1425	14'-0"	25′-0"	25.9	24	7	23	6	24'-6"	40	7	41	6	13′ -6
	1426	14'-0"	26′-0"	27.0	23	8	23	7	25′-6"	39	7	45	6	13′ -6
	1427		27'-0"		23	8	24	7	26'-6"	44	7	42	6	13′-6
	1428		28'-0"		23	8	26	7	27′-6"	45	7	49	6	13′ -6
	1429		29'-0"	30.1	29	8	23	8	28'-6"	47	7	52	6	13′ -6
	1430		30′ -0"		25	9	24	8	29′-6"	48	7	48	6	13'-6
	1431		31'-0"		27	9	24	8	30′ -6"	51	7	49	6	13'-6
	1432		32′-0"	33.2	27	8	24	8	31'-6"	54	7	57	6	13'-6
	1433		33′-0"		27	9	24	8	32′-6"	55	7	61	6	13′ -6
	1434		34′-0"		27	9	25	8	33′ -6"	57	7	56	6	13'-6
	1435		35′-0"		26	9	28	8	34′ -6"	59	7	63	6	13' -6
	1527		27'-0"		30	5	26	5	26' -6"	48	7	40	7	14'-6
	1528		28'-0"		28	7	28	6	27'-6"	53	7	42	7	14'-6
	1529		29'-0"		30	7	24	7	28′ -6"	54	7	43	7	14'-6
-	1530		30′ -0"		25	8	28	7	29′ -6"	59	7	50	7	14'-6
	1531		31'-0"		24	8	26	7	30′ -6"	56		58	6	14' -6
-	1532		32′ -0"		25	8	24	8	31'-6"	58	7	63	6	14' -6
-	1533		33′ -0"		26	10 9	26 26	8	32′ -6 " 33′ -6 "	62	7	52 65	7	14' -6 14' -6
-	1534 1535		34′-0" 35′-0"	37.8 38.9	27	10	30	8	34'-6"	66 70	7	57	6 7	14'-6
-	1536		36'-0"		28	10	27	9	35'-6"	73	7	70	6	14'-6
-	1537		37'-0"		27	10	28	9	36′ -6"	75	7	60	7	14'-6
-	1538		38'-0"		27	11	25		37'-6"	61	8	77	6	14'-6
-	1623		23' -0"	27.3	30	7	26	6	22′ -6"	44	7	43	5	15'-6
ł	1627		27' -0"		24	7	25	6	26'-6"	45	8	41	7	15' -6
ł	1630		30' -0"		26	8	27	7	29′-6"	51	8	51	7	15'-6
-	1637		37'-0"		27	11	31	9	36'-6"	65	8	57	7	15' -6
ŀ	1638		38'-0"		28	11	31	9	37'-6"	66	8	76	6	15'-6
ŀ	1639		39' -0"		32	10	31	9	38'-6"	65	8	61	7	15'-6
ŀ	1641		41'-0"		32	11	32		40′ -6"	72	8	72	7	15'-6
-	1726		26'-0"		29	5	25	5	25′-6"	44	8	40	8	16' -6
-	1727		27'-0"	34.0	29	6	33	5	26'-6"	46	8	48	7	16'-6
1	1728		28'-0"		28	8	31	7	27'-6"	48	8	52	7	16'-6
1	1732		32′ -0"		28	8	34	7	31'-6"	55	8	64	7	16' -6
	1828		28'-0"		31	6	26	6	27'-6"	48	8	46	8	17'-6
·			,								_			



- PROVIDE 90° OR 180° HOOKS ON ALL "L" AND "T" BARS.
- LENGTH FOR "L" AND "T" BARS DOES NOT INCLUDE 90° OR 180° HOOK LENGTHS.
- FOR ADDITIONAL FOUNDATION NOTES, SEE SHEET 3.

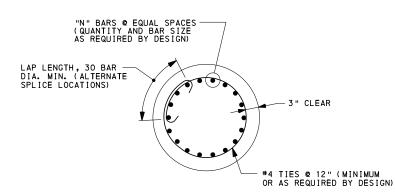


CAISSON ELEVATION N. T. S.

	ALTERNA	TE CAISSO	N FOUNDATIONS
COLUMN NOMINAL SIZE X WALL THK.	MINIMUM CAISSON DIAMETER "D"	CAISSON EMBEDMENT LENGTH "L"	CAISSON REINFORCEMENT QUANTITY AND BAR SIZE "N"
10 "X. 365 "	3'-6"	- Q	Q = Q
14 "x. 375"	3'-9"	ENT "L" IRE	ANE "N" IRED
16"x.375"	4′-0"	EMBEDMEN ENGTH "L S REQUIR BY DESIG	ZE ZE SE
18 "x. 375 "	4'-3"	BE TST O	SI SI DI
20"x. 375"	4′-9"		QUANT BAR S AS RE BY L
24 "x. 375 "	5′-3"	L A	A A
24"x.500"	5′-3"		

NOTES:

- ALTERNATE CAISSON FOUNDATIONS ARE PERMITTED IN PLACE OF THE SPREAD FOOTING SIZE SHOWN ON THE CONTRACT DRAWINGS.
- ALTERNATE CAISSON FOUNDATIONS MUST BE DESIGNED IN ACCORDANCE WITH DESIGN CRITERIA GIVEN ON SHEET 1.
- DESIGN COMPUTATIONS FOR THE REQUIRED CAISSON EMBEDMENT AND REINFORCEMENT MUST BE SUBMITTED TO THE DISTRICT BRIDGE ENGINEER FOR REVIEW AND APPROVAL.
- IN PLACE OF #4 TIES AT 12", A #4 BAR SPIRAL WITH A 3" PITCH MAY BE USED. THE #4 TIES AT 12" ARE THE MINIMUM OR AS REQUIRED BY DESIGN.



SECTION A-A N. T. S.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES 2 POST AND 4 POST TRI-CHORD TRUSS SPANS FROM 60' TO 240'

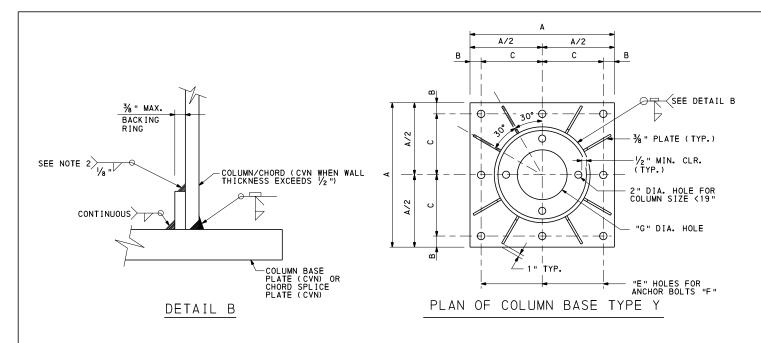
FOUNDATION DETAILS AND ALTERNATE CAISSON FOUNDATION

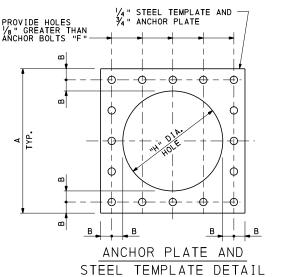
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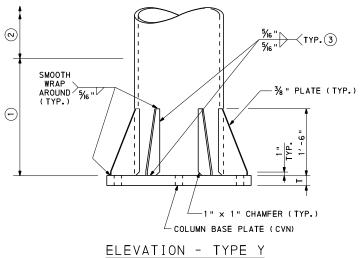
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SHT. 4 OF 12 BC-744M

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- (TYPES X & W SIMILAR) 1) FOR PRESS BREAK COLUMN, 2'-6" LENGTH OF SEAM WELD TO BE COMPLETE PENETRATION GROOVE WELD.
- (2) SEAM WELD TO HAVE 60% MIN. PENETRATION.

THREADS OF BOLT TO -BE BURRED OFF AT FACE OF NUT AFTER

(3) TERMINATE WELDS 1/4" SHORT OF STIFFENER CHAMFER.

STEEL ANCHOR BOLTS WITH

DETAIL B NOTES:

- BACKING RING MUST BE FITTED/SIZED TO THE PIPE COLUMN AND CONTINUOUSLY FILLET WELDED TO THE BASE PLATE BEFORE THE FULL PENETRATION GROOVE WELD IS MADE. BACKING RING MUST BE FABRICATED AS A CONTINUOUS RING.
- FOR COLUMNS AND CHORDS LESS THAN 19", THIS FILLET WELD IS NOT REQUIRED BUT SHOP IS TO APPLY SILICON CAULKING TO THIS LOCATION AFTER POLE ASSEMBLY IS GALVANIZED.

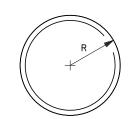
	COLUMN BASES - 2 POST STRUCTURES													
COLUMN NOMINAL SIZE X WALL THK.*	BASE TYPE	А	В	С	E	F	G	Н	Т	WASHER SIZE	PRO- JECTION	EMBED- MENT		
10 "x. 365 "	Y	1′-8"	21/2"	71/2"	1½ "D	1 1/4 "D	31/4"	10"	2 "	3½ "D×¾ "	7 3/4 "	2′-1"		
12 "x. 375 "	Y	1'-10"	21/2"	8 1/2 "	1¾ "D	1 ½ "D	51/4"	1'-0"	2 "	3½ "D×¾ "	8 1/2 "	2′ -6 "		
14 "x. 375 "	Y	2'-0"	21/2"	9½"	1 ¾ "D	1 ½ "D	6½"	1′-2"	2 "	3½ "D×¾ "	8 1/2 "	2′-6"		
16 "x. 375 "	Y	2′-2"	21/2"	101/2 "	2 "D	1 ¾ "D	8 "	1'-4"	2 "	4 "D×3/8 "	91/4"	2' -11"		
18 "x. 375 "	Y	2'-4"	2 1/2 "	111/2"	2 "D	1 ¾ "D	91/4"	1′-6"	2 "	4 "D×3/8 "	91/4"	2'-11"		
20 "x• 375 "	Y	2′ -7"	3 "	1′-0½"	2 1/4 "D	2 "D	1′-5"	1′-7"	3"	5 "D×3/8"	11"	3′ - 4 "		
24 "x. 375 "	Y	2' -11"	3 "	1'-21/2"	2 1/4 "D	2 "D	1′-6"	1′ -11"	3"	5 "D×3/8 "	11"	3′ -4"		
24 "x. 500"	Y	3′-0"	31/2"	1'-21/2"	2 ½ "D	2 1/4 "D	1'-6"	1'-10"	3"	5 "D×3/8 "	113/4"	3′-9"		

NOTE: D DENOTES DIAMETER

* CVN REQUIRED FOR WALL THICKNESSES EXCEEDING 1/2 " (.500").

NOTES:

- ANCHOR BOLTS SHALL BE PROVIDED WITH FOUR HEAVY HEX NUTS, ONE JAM NUT AND TWO WASHERS AS SHOWN ON THE ANCHOR BOLT DETAIL.
- ANCHOR BOLTS SHALL BE GALVANIZED AFTER THREADING.
- USE STEEL TEMPLATE TO SET ANCHOR BOLTS IN ACCORDANCE WITH PUBLICATION 408, SECTION 948.3(b).
- STEEL TEMPLATE AND ANCHOR PLATE TO BE PROVIDED BY SIGN FABRICATOR.
- STEEL TEMPLATE PLATE WITH NUTS ON BOTH SIDES SHALL BE USED TO MAINTAIN THE SPACING AND ALIGNMENT OF ANCHOR BOLTS.
- FOR PIPE CAP DETAILS, SEE SHEET 12.
- FOR ALTERNATE PIPE CAP DETAILS, SEE SHEET 11.
- SEAL BASE PLATE TO FOUNDATION GAP WITH GALVANIZED STEEL SCREEN, 1/2" BY 1/2" MESH AND 0.063" DIAMETER WIRES. SCREEN IS TO PREVENT ENTRY OF RODENTS WHILE PERMITTING DRAINAGE. SCREEN IS TO BE REMOVABLE AND ATTACHED TO BASEPLATE WITH STAINLESS STEEL HARDWARE.



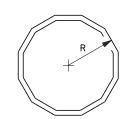


ILLUSTRATION OF DIMENSION "R" FOR

CIRCULAR MEMBERS AND EQUIVALENT "PRESS-BREAK" MEMBERS

"PRESS-BREAK" NOTE:

ALTERNATE "PRESS-BREAK" MEMBERS ARE PERMITTED FOR COLUMNS. "PRESS-BREAK" MEMBERS MUST HAVE THE EQUIVALENT STRENGTH AND FATIGUE RESISTANCE OF THE CIRCULAR MEMBER BEING REPLACED. A MINIMUM NUMBER OF 12 BREAKS IS REQUIRED. A CHANGE IN STEEL MATERIAL OR WALL THICKNESS REQUIRES A SPECIAL DESIGN TO BE SUBMITTED FOR REVIEW. CONTRACTOR MUST SUBMIT DESIGN CALCULATIONS AND DESIGN DRAWINGS FOR REVIEW AND ACCEPTANCE FOR LONGITUDINAL SEAM WELDS INDICATING TYPE OF WELD, WELD PENETRATION, EFFECTIVE DEPTH AND LENGTH OF EACH WELD TYPE. LONGITUDINAL SEAM WELDS SHALL HAVE 60 PERCENT MINIMUM PENETRATION, EXCEPT LONGITUDINAL SEAM WELDS WITHIN 6" OF THE ENDS OF THE PRESS BREAK MEMBER OR LENGTH SHOWN ON DETAILS SHALL BE COMPLETE PENETRATION WELDS. COMPLETE PENETRATION LONGITUDINAL SEAM WELDS MUST BE 100% RADIOGRAPHICALLY INSPECTED. FOR THE COLUMN CONNECTION TO BASE PLATE, AND AT COLUMN CONNECTION SPLICE PLATE LOCATIONS, WELD SHALL START AND STOP IN THE MIDDLE THIRD REGION OF FLAT SECTIONS BETWEEN BREAK POINTS.

COLUMN IS INSTALLED STEEL HEAVY HEX NUTS, JAM NUT AND WASHERS GALV. RODENT BOTTOM OF BASEPLATE AMETER ". FOR 13 EAD 10.25 DIA. BOLT AL/ FOR 1/4" AND 1/2" D BOLTS, THREAD 8". DIA. BOLTS, THREAD FOR 2" AND 2/4" DIA THREAD 11.75" P TOP -3½" MAX. 2¾" MIN. 3/4" ANCHOR PLATE ANCHOR BOLT DETAIL COLUMN, STIFFENERS, AND REINF. OMITTED FOR CLARITY

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

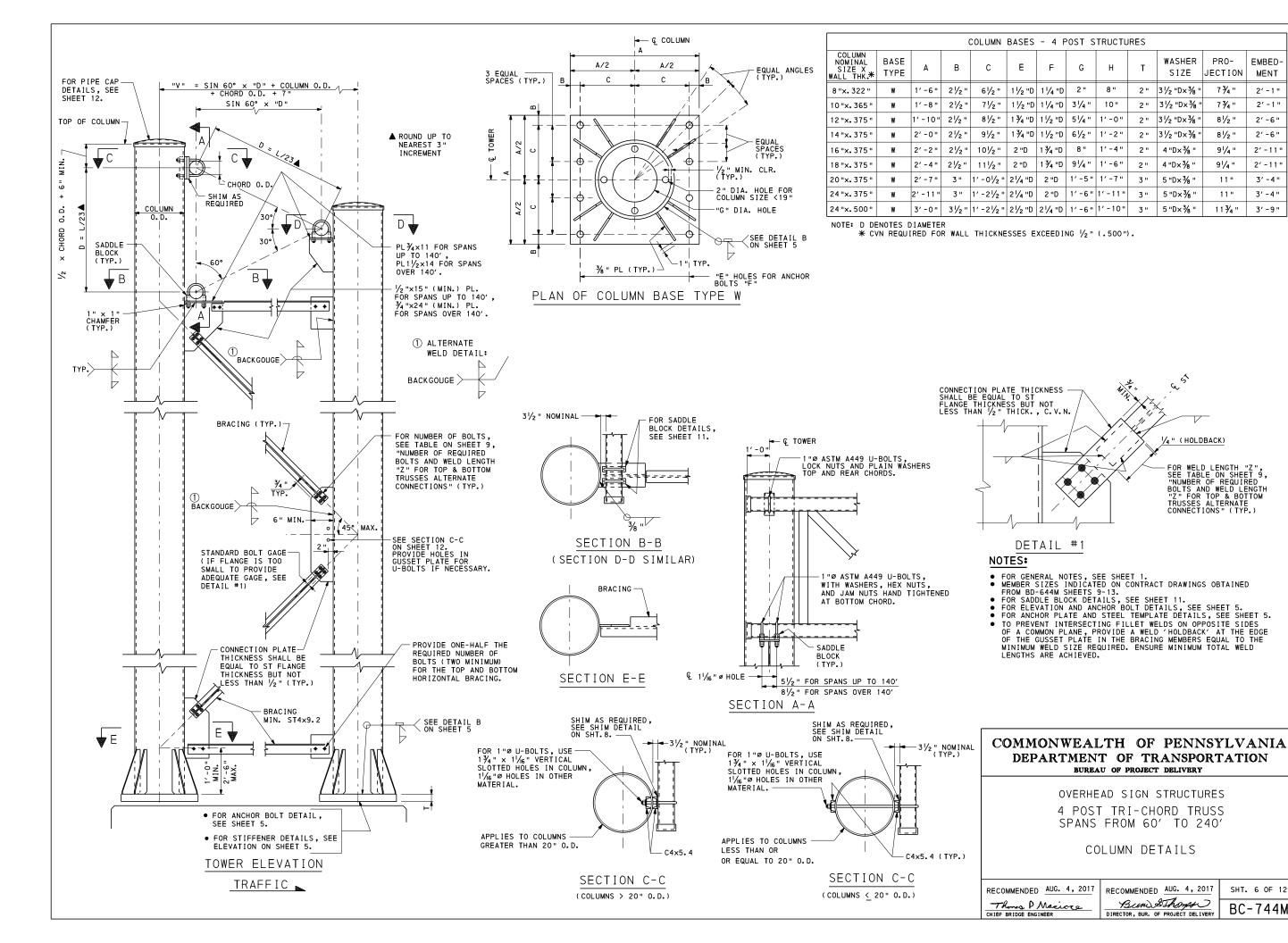
OVERHEAD SIGN STRUCTURES 2 POST AND 4 POST TRI-CHORD TRUSS SPANS FROM 60' TO 240'

COLUMN BASE DETAILS

RECOMMENDED AUG. 4, 2017 Thomas P Macioca CHIEF BRIDGE ENGINEER

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BC-744M



EMBED-

MENT

2'-1"

2'-1"

2'-6"

2'-6"

2'-11

2'-11'

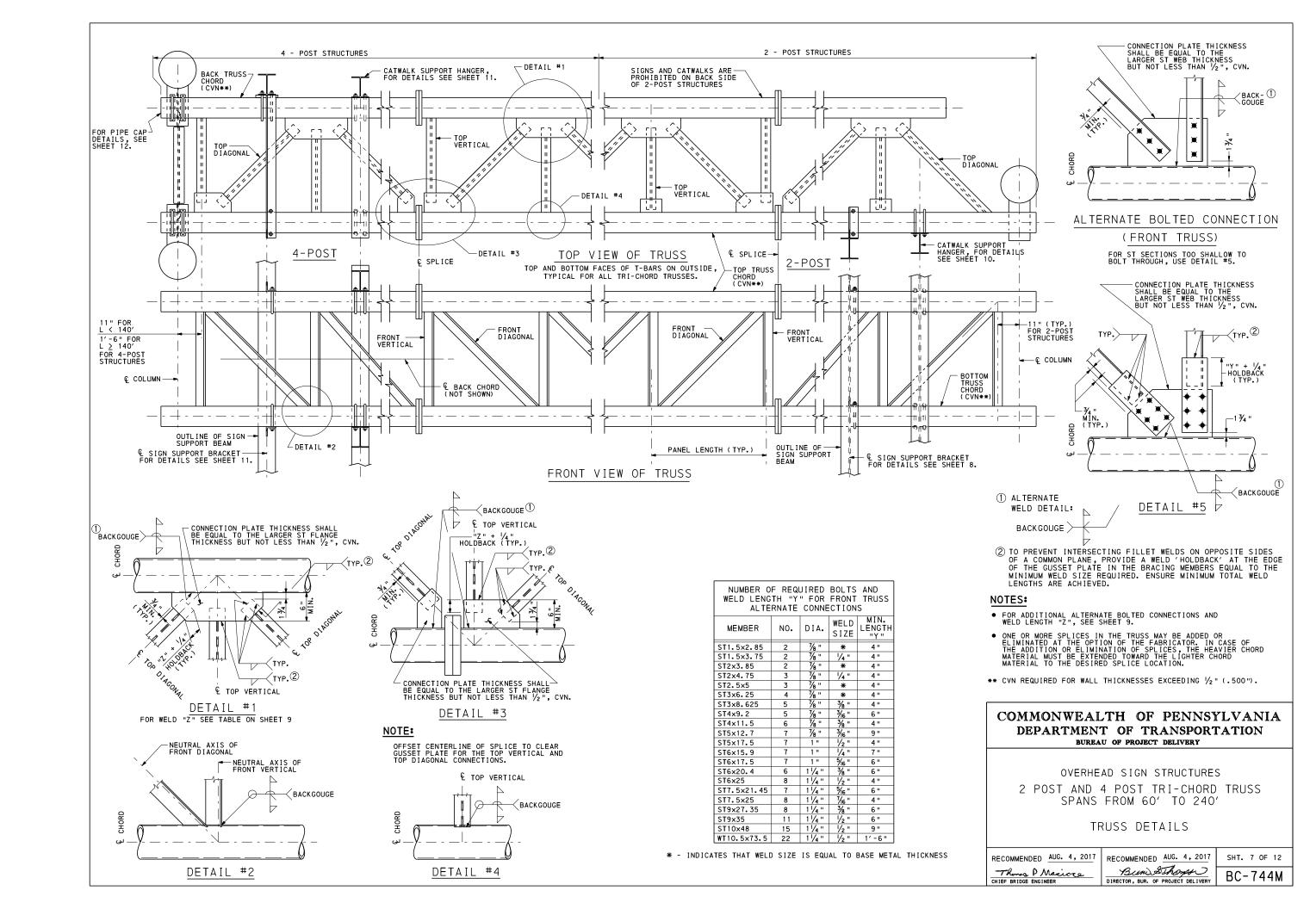
3' - 4"

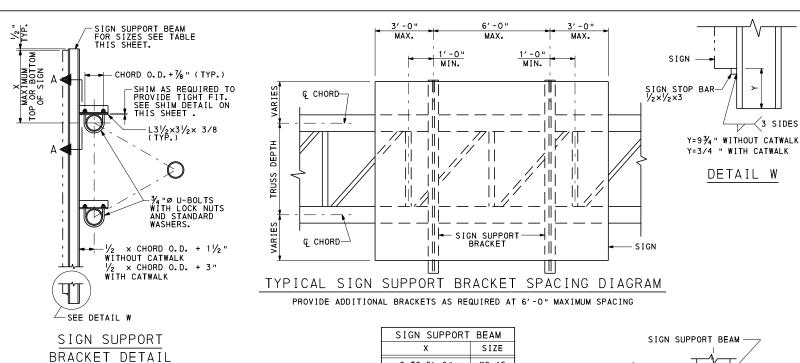
3'-4"

3′-9"

SHT. 6 OF 12

BC-744M





0 TO 5'-6" W6×15 5'-6"+ TO 6'-6" W6×20 6'-6"+ TO 7'-6" W6×25 7'-6"+ TO 8'-6" W8×28 8'-6"+ TO 9'-6"

SIGN SUPPORT BEAM ANGLE € SIGN SUPPORT BRACKET & ANGLE CENTER OF GRAVITY.

SECTION A-A

3 SIDES

CHORD (TYP.)(CVN — WHEN WALL THICKNESS EXCEEDS 1/2 ")

ELEVATION

- SPLICE PLATE (TYP.)

(REAR TRUSS SHOWN, FRONT TRUSS SIMILAR)

MID-PANEL CHORD SPLICE

NOTES:

- ALL BOLTS TO BE ASTM A325 AND GALVANIZED IN ACCORDANCE WITH PUB. 408 UNLESS STAINLESS STEEL OR OTHERWISE INDICATED.
- U-BOLTS PER PUBLICATION 408, SECTION 948.2, UNLESS NOTED OTHERWISE.
- FOR DETAILS OF MOUNTING SIGNS TO SIGN SUPPORT BEAMS, SEE STANDARD DRAWING TC-8701E.

NOTES:

FOR SIGN ATTACHMENT TO BACK OF TRUSS, SEE SHEET 11.

- ALL MATERIALS FOR TRUSS SEATS AND SIGN SUPPORT BRACKETS TO BE STRUCTURAL STEEL AASHTO M270, GRADE 36.
- FOR DETAIL B, SEE SHEET 5.

SPLICE (WHEN ALTERNATE PANEL POINT CONNECTION IS USED, PLACE SPLICE MIDWAY BETWEEN PANEL POINTS. SEE MID-PANEL CHORD SPLICE DETAIL. ** NOMINAL ADJUSTMENT PERMITTED FOR WELDING ACCESS. WELDING ACCESS.
CHORD SPLICE PLATE (CVN) * T+1 " + + + + + + + + + + + + + + + + + +
IF CONFLICT EXISTS, THE FABRICATOR IS PERMITTED TO REPLACE THIS BOLT WITH ASTM A449 THREADED ROD.
"V" DIAMETER VENT HOLE
SEE DETAIL B
HOLES IN SPLICE PLATE SHALL BE //6" LARGER THAN BOLT DIAMETER. MINIMIZE
2" DIA. HOLE (TYP.) EXCEPT 1 /2 " DIA. HOLES FOR 8" CHÔRD AND 1 /4 " DIA. HOLES FOR CHORDS LÉSS THAN 8".
NOTES: CHORD SPLICE

ADJUST BOLT PATTERN AS NECESSARY TO AVOID CHORD BRACING AND GUSSET PLATES. DO NOT VIOLATE MINIMUM BOLT SPACING OF 3 TIME THE BOLT DIAMETER.

CHORD SPLICE NOMINAL SIZE X WALL THK. BOLTS 5"x. 250" 1'-1%6" 4- 1/8 "Ø 2 " 5"x. 375" 6- 1/8 "Ø 21/2 " 6- ½ "Ø 2" 8- ½ "Ø 2½" 6"x.280" 6"x. 432" 8- 1/8 "Ø 2 1/4 " 8 "x. 322 " 1'-1\%" 12-\%"\\ 2\\4\" 8 "x. 500 " 10"x.365" " 12- 1/8 "Ø 21/2 10"x.500" 1'-6¾" 1'-3¾" 16-¾" Ø 2¾" 3¼" 12 "x. 375 " 1'-8¾" 1'-5¾" 14-¾" Ø 2½" 5¼" 12"x.500" 1'-8¾" 1'-5¾" 18-¾" Ø 2¾" 5¼" 12"x, 500" | 1 - 674 | 1 - 77 | 16- 1/8 "Ø 21/2" 61/2" | 14"x, 375" | 1′ - 10" | 1′ - 7" | 16- 1/8 "Ø 21/2" 61/2" | 14"x, 500" | 1′ - 10" | 1′ - 7" | 20- 1/8 "Ø 2 1/4" 61/2" 16"x. 375" 2'-0" 1'-9" 18- \(\frac{7}{8}\) "\(\omega \) 2\\(\frac{7}{2}\)" 16"x.500" 2'-0" 1'-9" 22- 1/8 "Ø 2 3/4 " 18"x.375" 2'-2" 1'-11" 20-7/8 "Ø 21/2" 91/4" 18 "x. 500" 2' -2\frac{1}{2}" 1' -11" 20-1 "\text{ "0} 2\frac{3}{4}" 9\frac{1}{4}" 20" x. 375" 2' -4" 2' -1" 22-\frac{1}{8}"\text{ "0} 2\frac{1}{4}" 9\frac{1}{4}" 20" x. 500" 2' -4\frac{1}{2}" 2' -1" 22-\frac{1}{8}"\text{ "0} 2\frac{1}{4}" 10\frac{1}{2}" 20" x. 500" 2' -4\frac{1}{2}" 2' -1" 22-1 "\text{ "0} 2\frac{3}{4}" 10\frac{1}{2}" 24" x. 375" 2' -8\frac{1}{2}" 2' -5" 20-1 "\text{ "0} 2\frac{1}{2}" 1' -0\frac{3}{4}" 24" x. 500" 2' -8\frac{1}{2}" 2' -5" 26-1 "\text{ "0} 3" 1' -0\frac{3}{4}"

NOTE:
WHERE LARGER CHORD SIZE SPLICES TO SMALLER CHORD SIZE,
USE SPLICE AS SHOWN FOR SMALLER CHORD.

lacktriangle CVN REQUIRED FOR WALL THICKNESSES EXCEEDING 1/2 " (0.500").

	1 1/8 " AS REQ' D. 11/8 "
- 2	ALIGN WITH BACK OF ANGLE
31/8	
= 88	- & 13/16 "Ø HOLES
	SHIM DETAIL
	PROVIDE 1 AT 1/4", 3 AT 1/8" AND 1 AT 1/16" THICKNESS FOR EACH UPPER SIGN SUPPORT CON- NECTION ANGLE.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

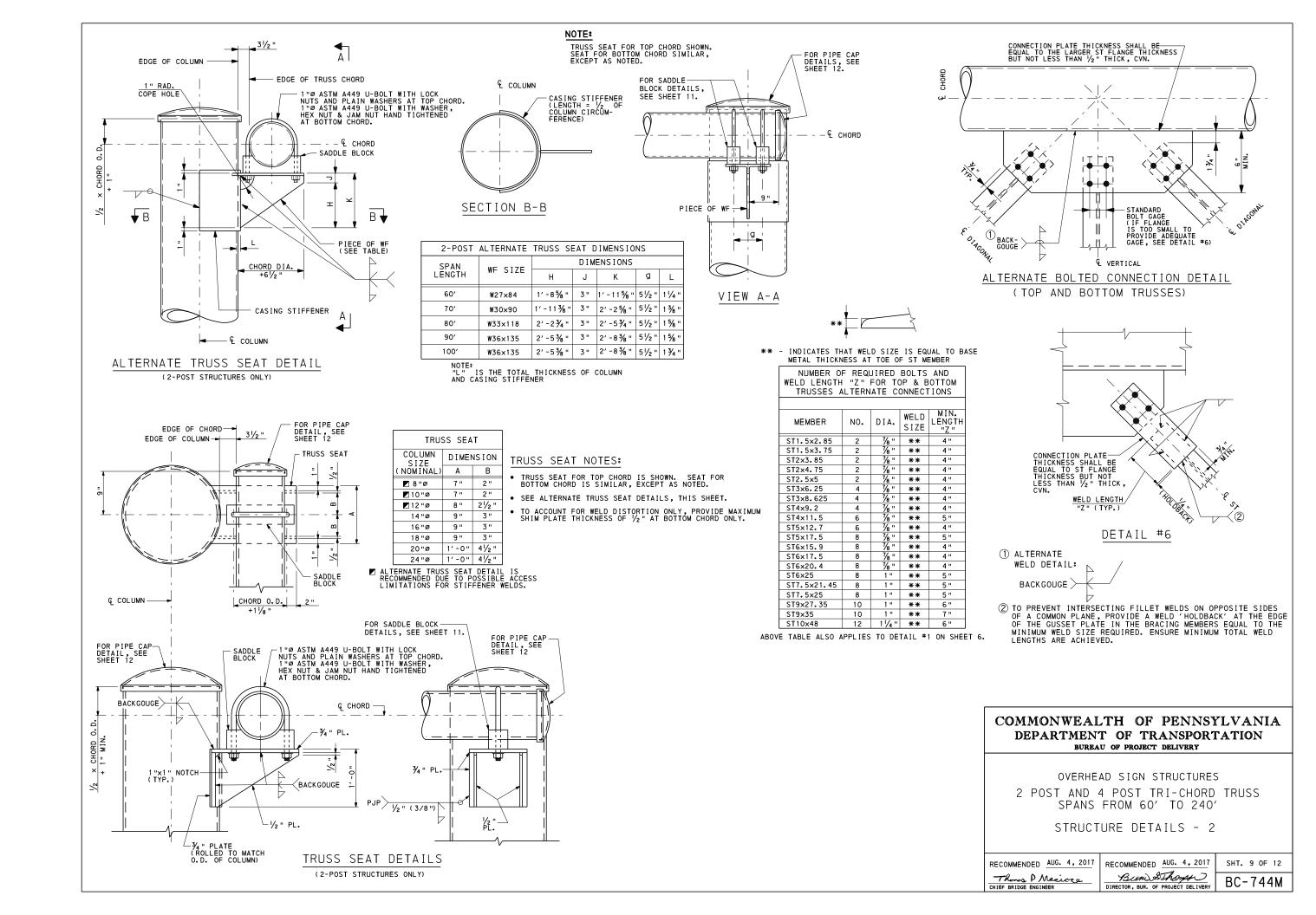
OVERHEAD SIGN STRUCTURES 2 POST AND 4 POST TRI-CHORD TRUSS SPANS FROM 60' TO 240'

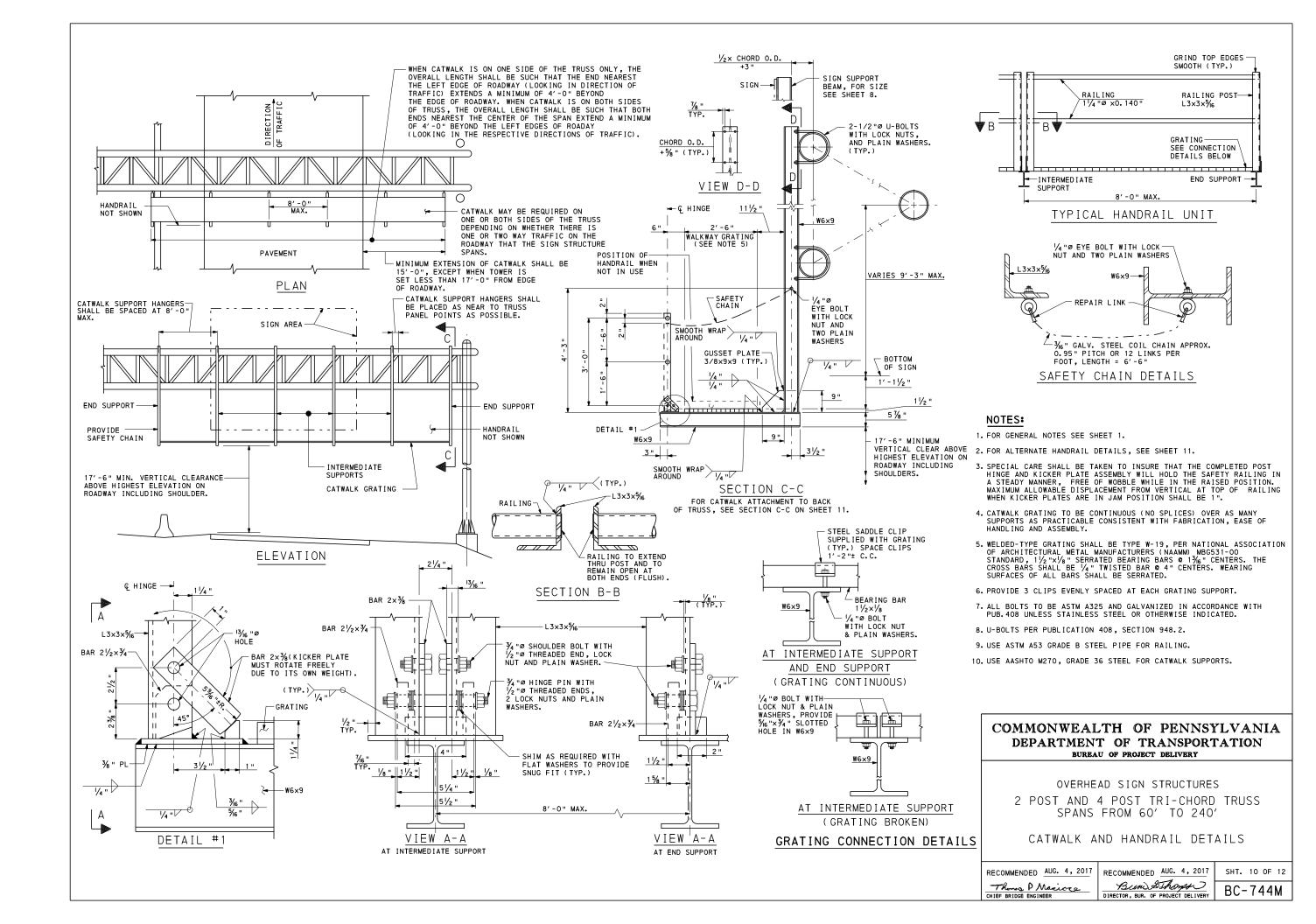
STRUCTURAL DETAILS-1

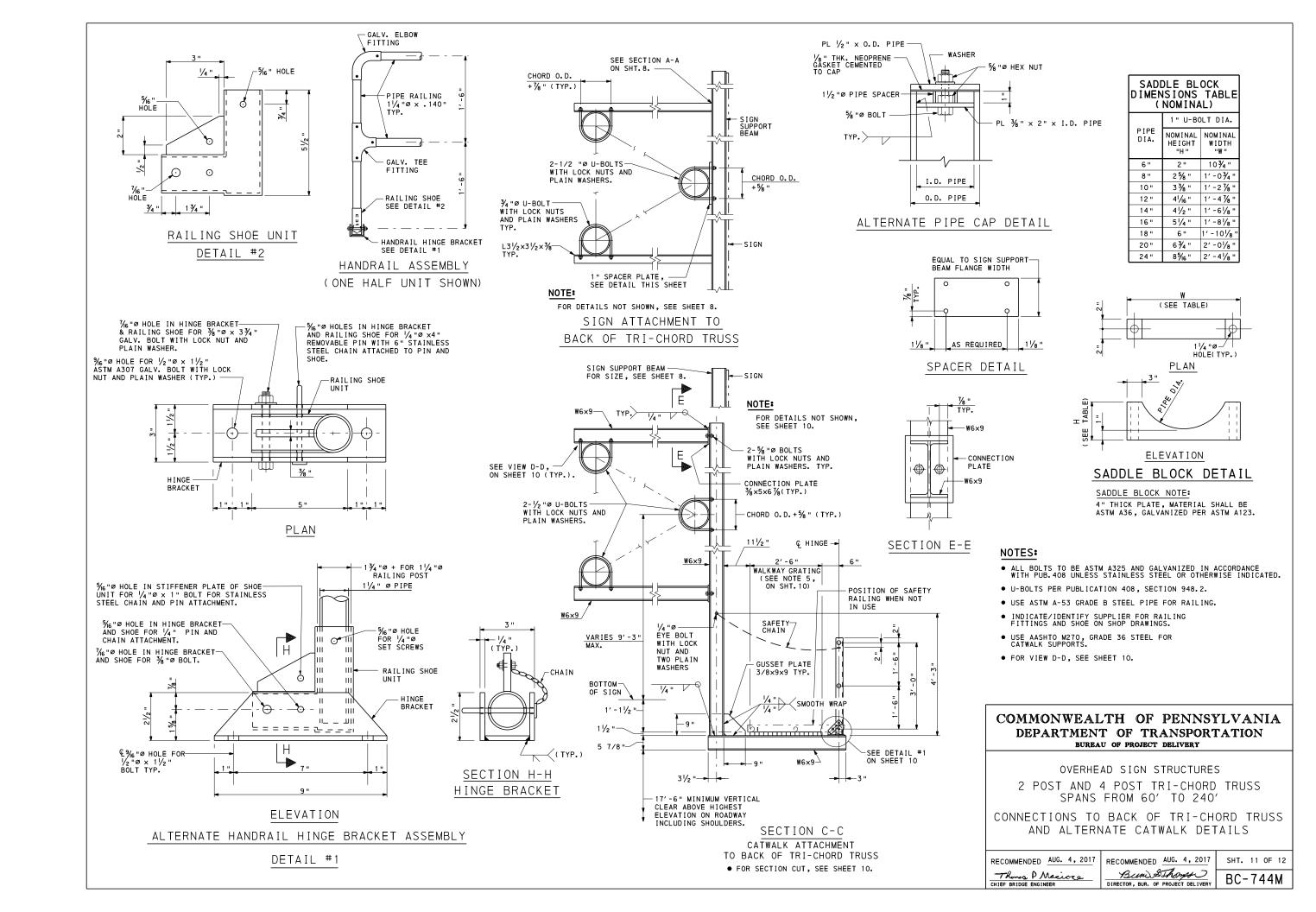
RECOMMENDED AUG. 4, 2017 Thomas P Macioca

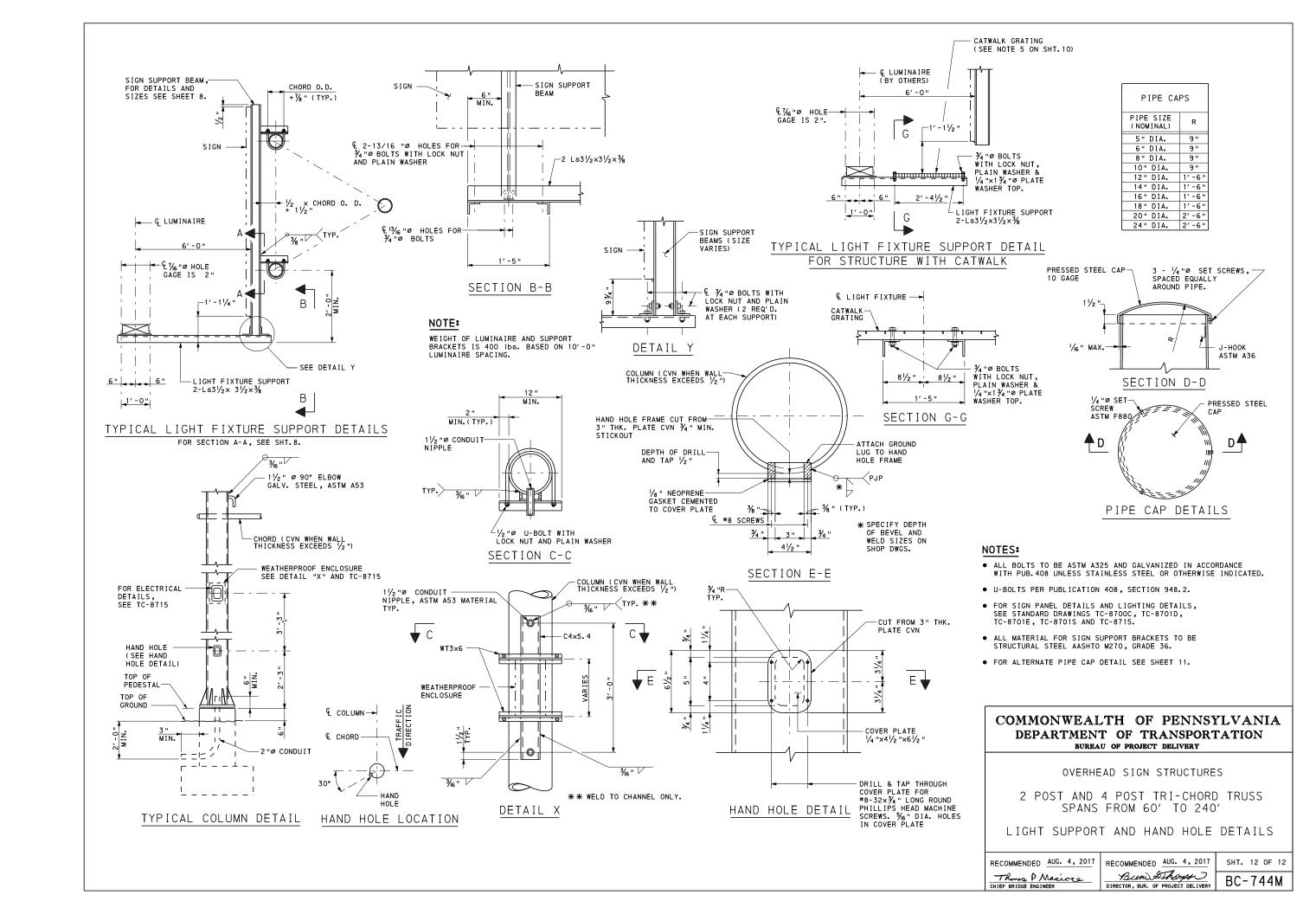
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SHT. 8 OF 12 BC-744M









INFORMATION CONTAINED IN THE BD-645M DESIGN TABLES

- DESIGN TABLES ON STANDARD DRAWING BD-645M WERE DEVELOPED USING A COMPUTER PROGRAM AND ARE BASED ON THE DESIGN CRITERIA SHOWN ON THIS SHEET.
- THE MEMBER SIZES INDICATED IN THE DESIGN TABLES MEET THE FATIGUE REQUIREMENTS FOR FATIGUE CATEGORY II. THE DESIGNER MUST CHECK THE ADEQUACY OF THE MEMBER SIZES INDICATED WHEN THE FATIGUE CATEGORY IS SPECIFIED TO BE I FOR THE PROJECT.
- THE SPAN RANGE INCLUDED ON STANDARD DRAWING BD-645M IS AS FOLLOWS: BD-645M: 4 POST 4 CHORD TRUSS SPANS FROM 100' TO 200'.
- THE DESIGN TABLES INCLUDE MEMBER SIZES FOR THE STRUCTURES FOR VARIOUS COMBINATIONS OF COLUMN HEIGHT, SPAN LENGTH, AND SIGN AREA. THEY ALSO INCLUDE SPREAD FOOTING DESIGNS. THE CORRESPONDING FABRICATION AND CONSTRUCTION DETAILS ARE CONTAINED IN THIS STANDARD.

GENERAL NOTES

- 1. PROVIDE 3-INCH CONCRETE COVER ON REINFORCEMENT BARS. EXCEPT AS NOTED.
- 2. USE CLASS A CEMENT CONCRETE f'c = 3000 PSI IN PEDESTALS, FOOTINGS AND CAISSONS.
- 3. PROVIDE GRADE 60 REINFORCING STEEL BARS THAT MEET THE REQUIREMENTS OF ASTM A615 FOR CONCRETE REINFORCEMENT. DO NOT WELD REINFORCING STEEL BARS.
- 4. RAKE-FINISH ALL HORIZONTAL CONSTRUCTION JOINTS, EXCEPT AS INDICATED.
- 5. VERIFY ALL DIMENSIONS AND GEOMETRY OF THE EXISTING STRUCTURES IN THE FIELD AS NECESSARY FOR PROPER FIT OF THE PROPOSED CONSTRUCTION.
- 6. CHAMFER EXPOSED CONCRETE EDGES 1 INCH BY 1 INCH.

CHANGE 1

- 7. ALL DIMENSIONS SHOWN ARE HORIZONTAL, EXCEPT AS NOTED.
- 8. DIMENSIONS ARE BASED ON A NORMAL TEMPERATURE OF 68 DEGREES F.
- SPREAD FOOTINGS MAY BE ORDERED BY THE ENGINEER TO BE AT ANY ELEVATION OR OF ANY DIMENSIONS NECESSARY TO PROVIDE A PROPER FOUNDATION.
- 10. GALVANIZE ALL STRUCTURAL STEEL, BOLTS, NUTS & WASHERS IN ACCORDANCE WITH PUB. 408 UNLESS STAINLESS STEEL OR OTHERWISE INDICATED.
- 11. PIPE DIAMETERS SHOWN UP TO AND INCLUDING 12 INCHES ARE NOMINAL DIAMETERS. PIPE DIAMETERS SHOWN FROM 14 INCHES AND UP ARE ACTUAL DIAMETERS.
- 12. USE STANDARD SIZE HOLE. THE STANDARD HOLE DIAMETER FOR BOLTS SMALLER THAN
 1" DIAMETER SHALL BE THE NOMINAL DIAMETER OF THE BOLT PLUS 1/6". FOR BOLTS
 1" DIAMETER AND LARGER, THE WIDTH OF EACH STANDARD HOLE SHALL BE THE NOMINAL DIAMETER OF THE BOLT PLUS 1/8".
- 13. CLEAR DISTANCE BETWEEN BOLT HOLES OR BETWEEN THE BOLT HOLE AND THE END OF THE MEMBER IN THE DIRECTION OF THE APPLIED BEARING FORCE SHALL BE CHECKED.
- 14. PROVIDE ANCHOR BOLT HOLES 1/4" LARGER THAN BOLT DIAMETER.
- 15. PROVIDE A MINIMUM ANCHOR BOLT EMBEDMENT LENGTH OF 20 ANCHOR BOLT DIAMETERS.
- 16. PROVIDE DOUBLE NUTS AND WASHER FOR EACH ANCHOR BOLT.
- 17. STEEL MEMBER COMPONENTS REQUIRING CHARPY V-NOTCH TESTING ARE DESIGNATED ON THE PLANS BY (CVN), PROVIDE STEEL CONFORMING TO THE CVN REQUIREMENTS FOR ZONE 2, NON FRACTURE CRITICAL AS GIVEN IN THE AASHTO MATERIAL SPECIFICATIONS.

DESIGN CRITERIA FOR PENNDOT SIGN STRUCTURES

TC-8701E OR TC-8701S BC-745M, SHT. 10 BC-745M, SHT. 8 CALCULATED INTERNALLY WITHIN PROGRAM SIGN PANELS LIGHT FIXTURES SIGN SUPPORT BEAM COLUMNS, CHORDS CATWALK BC-745M, SHT. 8 AND 9

PENNDOT STD. DWGS. (U.N.O.) *

 EXTERNAL LOADS AASHTO SIGN SPECS. 3.7
APPENDIX C, SECTION C.3,
EQ. C-1, WITH 80 MPH
WIND AND 30% GUST FACTOR ICE LOAD WIND LOAD CATWALK

• GROUP LOADS AASHTO SIGN SPECS. 3.4

 STEEL CRITERIA AASHTO SIGN SPECS.

SECTION PROPERTIES FOR TUBULAR SHAPES MAXIMUM STRESSES FOR TUBULAR SHAPES ALLOWABLE STRESSES FOR SIGN SUPPORTS ALLOWABLE STRESSES FOR SIGN SUPPORTS ALLOWABLE STRESSES FOR BASE PLATES APPENDIX B, TABLE B-1 APPENDIX B, TABLE B-2 ALLOWABLE STRESSES FOR COMBINED STEEL STRESS FATIGUE REQUIREMENTS (FATIGUE CATEGORY II) 5.12 SECTION 11 ALLOWABLE DEFLECTION
PERMANENT CAMBER
ALLOWABLE STRESSES FOR STRUCTURAL STEEL 10.4 10.5 SECTION 5

 BOLT CRITERIA AASHTO HIGHWAY BRIDGES (U.N.O.) ALLOWABLE BOLT STRESSES SLIP-CRITICAL BOLT ALLOWABLE BOLT PRYING ACTION COMBINED BOLT SHEAR AND TENSION BOLT DESIGN CRITERIA TABLE 10.32.3B 10.32.3.2.1 10.32.3.3.2 10.32.3.3.3 AASHTO SIGN SPECS. 5.16 AASHTO SIGN SPECS. 5.17

ALLOWABLE ANCHOR BOLT STRESSES CONCRETE CRITERIA AASHTO HIGHWAY BRIDGES (U.N.O.)

8. 15. 2. 1. 3 8. 15. 2. 2 8. 15. 5. 6. 1 8. 15. 5. 6. 2 8. 15. 5. 6. 4 8. 16. 5. 2 8. 17. 1 ALLOWABLE BEARING STRESS REINFORCEMENT TENSILE STRESS SHEAR CAPACITY OF FOOTINGS SHEAR STRESS IN FOOTINGS SHEAR STRESS IN FOOTINGS
ALLOWABLE SHEAR STRESS
SLENDERNESS OF COLUMNS
MINIMUM REINF. OF FLEXURAL MEMBERS
SPACING LIMITS FOR REINFORCEMENT
MINIMUM CONCRETE COVER
PRESSURES FOR ECCENTRICALLY LOADED FOOTINGS
DISTRIBUTION OF REINFORCEMENT
FOOTING STABILITY REQUIREMENTS
TORSION
COLUMN DESIGN (PEDESTALS) 8.21 DM4 D8.22.1* FIG. 4.4.7.1.1.1C 4.4.11.2.2 DM4 D5.5.5 ACI SECTION A.7.3* COLUMN DESIGN (PEDESTALS)

SPREAD FOOTINGS

DEAD LOADS

MAXIMUM DESIGN PRESSURE MINIMUM AREA IN BEARING UNIT WEIGHT OF SOIL 1.5 TONS PER SQUARE FOOT 100 POUNDS PER CUBIC FOOT

DRILLED SHAFTS (CAISSONS) DM4 SEC.4.6, PENNDOT COM624 COMPUTER PROGRAM

MAXIMUM DESIGN PRESSURE
MAXIMUM DESIGN LATERAL DISPLACEMENT
MODULUS OF SUBGRADE REACTION
UNIT WEIGHT OF SOIL
ANGLE OF INTERNAL FRICTION 1.5 TONS PER SQUARE FOOT 10.0 POUNDS PER CUBIC INCH 100 POUNDS PER CUBIC FOOT O KIPS PER SQUARE FOOT COHESTON

SEISMIC DESIGN CRITERIA

STRUCTURES ARE DESIGNED FOR A SEISMIC ACCELERATION COEFFICIENT = 0.15

CONSTRUCTION GENERAL NOTES

MATERIALS AND WORKMANSHIP

PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH THE CURRENT VERSIONS OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, AASHTO/AWS WELDING CODE D1.5, CONTRACT SPECIAL PROVISIONS, AND AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS". USE AASHTO/AWS D1.1 FOR WELDING NOT COVERED IN

• PROVIDE STRUCTURAL STEEL CONFORMING TO THE FOLLOWING:

COLUMNS, PIPE CHORDS & PIPE BRACING:

SEE PUBLICATION 408, SECTION 948.2.

AASHTO M270, GRADE 36 ASTM A709, GRADE 36 ANGLES, SHAPES, AND PLATES:

• ALTERNATE PRESS-BREAK MEMBERS

ALTERNATE PRESS-BREAK MEMBERS MUST HAVE THE EQUIVALENT STRENGTH OF THE MEMBER THEY ARE REPLACING. EQUIVALENT RADIUS FOR PRESS-BREAK MEMBERS IS MEASURED FROM THE CENTER OF THE MEMBER TO THE MID-POINT OF ANY CHORD OF THE MEMBER. MINIMUM THICKNESS OF PRESS-BREAK MEMBERS TO BE %"." PENNDOT SIGN STRUCTURE COMPUTER PROGRAM OR AN APPROVED FINITE ELEMENT ANALYSIS COMPUTER PROGRAM MUST BE RUN TO VERIFY THE ADEQUACY OF PRESS-BREAK MEMBERS FOR STRENGTH AND FATIGUE. ALTERNATE PRESS-BREAK MEMBERS ARE ONLY PERMITTED FOR COLUMNS. PRESS-BREAK MEMBERS APE NOT PERMITTED FOR COLUMNS. PRESS-BREAK MEMBERS ARE NOT PERMITTED FOR CHORDS.

• PROVIDE BOLTS CONFORMING TO THE FOLLOWING:

ANCHOR BOLTS: ASTM, F1554 GRADE 55 PER PUBLICATION 408 SECTION 1105.02(c) 3.

AASHTO M164 (ASTM A325) H.S. BOLTS EXCEPT AS NOTED BOLTS:

DESIGN SPECIFICATIONS:

AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS", 2001 WITH CURRENT INTERIMS (UNLESS NOTED OTHERWISE); AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 1996 WITH INTERIMS THROUGH AND INCLUDING 2000; PENNDOT DESIGN MANUAL - PART 4, AUGUST 1993 EDITION (INCLUDING AUGUST 1995 REVISIONS)

• ALL FILLET WELDS SHOWN ARE MINIMUM SIZE UNLESS NOTED OTHERWISE.

NOTES TO FABRICATOR

- 4-POST 4-CHORD STRUCTURE TYPES AS PRESENTED IN THESE STANDARDS ARE RECOMMENDED TO BE USED TO SUPPORT DYNAMIC/VARIABLE MESSAGE SIGNS (DMS/VMS).
- DESIGN COMPUTATIONS ARE REQUIRED FOR ANY PORTION OF A STRUCTURE FOR WHICH THE INFORMATION IS NOT TAKEN DIRECTLY FROM THE CONTRACT DRAWINGS OR THE DETAILS CONTAINED IN THIS STANDARD. DO NOT VIOLATE CRITERIA USED FOR THE DEVELOPMENT OF THE DESIGN TABLES ON STANDARD DRAWING BD-645M AND THE DETAILS IN THIS STANDARD.

* LEGEND:

- AASHTO SIGN SPEC: AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS"
- AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" • AASHTO HIGHWAY BRIDGES:
- DM4: PENNSYLVANIA DEPARTMENT OF TRANSPORTATION, DESIGN MANUAL PART 4,
- UNLESS NOTED OTHERWISE • U. N. O. :
- AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE WITH COMMENTARY (ACI 318-99).
- CVN: CHARPY V-NOTCH

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

4 POST 4 CHORD TRUSS SPANS FROM 100' TO 200'

NOTES AND DESIGN CRITERIA

RECOMMENDED AUG. 4, 2017

SHT. 1 OF 10 BC-745M

TC-8700C | SPACING CHARTS/DIRECT APPLIED LETTERS, NUMERALS, & ARROWS TC-8701D SIGN DETAILS/FREEWAY AND EXPRESSWAY GUIDE SIGNS TC-8701E EXTRUDED ALUMINUM CHANNEL SIGN TC-8701S FLAT SHEET ALUMINUM SIGNS WITH EXTRUDED ALUMINUM STIFFENERS TC-8715 BC-736M REINFORCEMENT BAR FABRICATION DETAILS RC-11M CLASSIFICATION OF EARTHWORK FOR STRUCTURES RC-51M TYPE 31 STRONG POST GUIDE RAIL RC-53M TYPE 2 WEAK POST GUIDE RAIL RC-54M BARRIER PLACEMENT AT OBSTRUCTIONS RC-58M SINGLE FACE CONCRETE BARRIER PLACEMENT AT MEDIAN PIERS

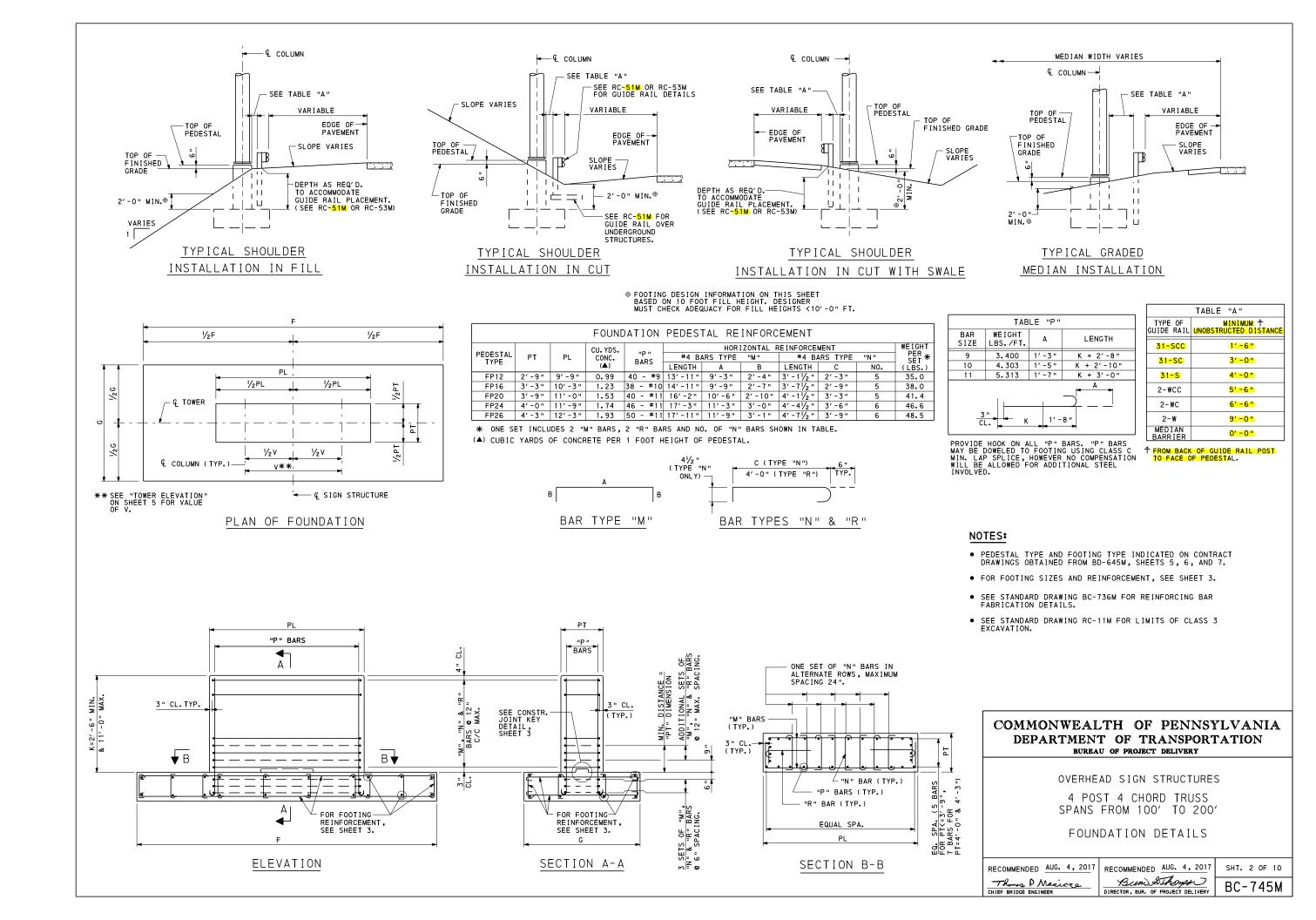
RECOMMENDED AUG. 4, 2017

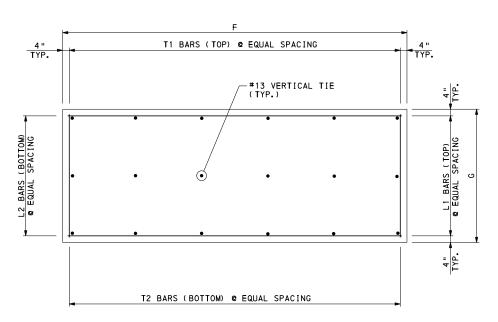
Bun SThomps

REFERENCE DRAWINGS

Thomas P Macioca

DIRECTOR, BUR, OF PROJECT DELIVERY

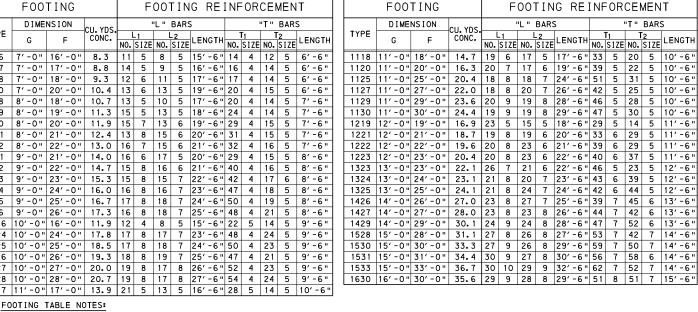




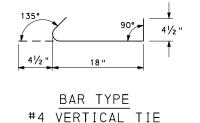
PLAN VIEW -	FOOTING	REINFORCEMENT
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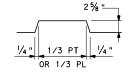
	F00	TING			F	00	TIN	G RE	INF	OR	CEI	MEN	Т
	DIME	NSION	CU. YDS.			'L "	BARS	5			"T"	BARS	S
TYPE	G	F	CONC.	ı	-1 SIZE		-2 SIZE	LENGTH		T ₁ SIZE		2 SIZE	LENGTH
716	7'-0"	16'-0"	8.3	11	5	8	5	15'-6"	14	4	12	5	6'-6"
717	7'-0"	17'-0"	8.8	14	5	9	5	16'-6"	16	4	14	5	6' -6"
718	7'-0"	18'-0"	9.3	12	6	11	5	17'-6"	17	4	14	5	6' -6"
720	7'-0"	20' -0"	10.4	13	6	13	5	19'-6"	20	4	15	5	6'-6"
818	8'-0"	18'-0"	10.7	13	5	10	5	17'-6"	20	4	14	5	7′-6"
819	8'-0"	19'-0"	11.3	15	5	13	5	18'-6"	24	4	14	5	7′-6"
820	8'-0"	20'-0"	11.9	15	7	13	6	19'-6"	29	4	15	5	7′-6"
821	8'-0"	21'-0"	12.4	13	8	15	6	20' -6"	31	4	15	5	7′-6"
822	8'-0"	22'-0"	13.0	16	7	15	6	21'-6"	32	4	16	5	7'-6"
921	9'-0"	21'-0"	14.0	16	6	17	5	20' -6"	29	4	15	5	8'-6"
922	9'-0"	22'-0"	14.7	15	8	16	6	21'-6"	40	4	16	5	8'-6"
923	9'-0"	23'-0"	15.3	15	8	15	7	22' -6"	42	4	17	6	8'-6"
924	9'-0"	24'-0"	16.0	16	8	16	7	23′-6"	47	4	18	5	8'-6"
925	9'-0"	25'-0"	16.7	17	8	18	7	24'-6"	50	4	19	5	8'-6"
926	9'-0"	26'-0"	17.3	16	8	18	7	25'-6"	48	4	21	5	8'-6"
1016	10'-0"	16'-0"	11.9	12	4	8	5	15'-6"	22	5	14	5	9'-6"
1024	10'-0"	24'-0"	17.8	17	8	17	7	23′-6"	48	4	24	5	9'-6"
1025	10'-0"	25'-0"	18.5	17	8	18	7	24'-6"	50	4	23	5	9'-6"
1026	10'-0"	26'-0"	19.3	18	8	19	7	25'-6"	47	4	21	5	9'-6"
1027	10'-0"	27'-0"	20.0	19	8	17	8	26'-6"	52	4	23	5	9'-6"
1028	10'-0"	28'-0"	20.7	19	8	17	8	27′-6"	54	4	24	5	9'-6"
1117	11'-0"	17'-0"	13.9	21	5	13	5	16' -6"	28	5	14	5	10'-6"

- PROVIDE 90° OR 180° HOOKS ON ALL "L" AND "T" BARS.
- LENGTH FOR "L" AND "T" BARS DOES NOT INCLUDE 90° OR 180° HOOK LENGTHS.
- FOR ADDITIONAL FOUNDATION NOTES, SEE SHEET 2.

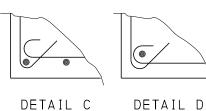


Α FOR PEDESTAL REINFORCEMENT, SEE SHEET 2. FOR PEDESTAL SEE CONSTR. JOINT KEY DETAIL REINFORCEMENT, SEE SHEET 2. "T1" BARS CONSTR. JOINT KEY--"L1" BARS "T1" BARS -**♦** В В₩ 3 SETS OF "M", "N" & "R" BARS @ 6" SPACING. SEE SHEET 2 ─ "T2" BARS ∠"L2" BARS SEE DETAIL D -SEE DETAIL C 4" CL. TYP. "T2" BARS "L1" BARS TOP "L2" BARS BOTT. #4 VERTICAL TIES @ 48" (MAX.) -GRID PATTERN W/135° HOOKS. ALTERNATE 135° HOOK UP AND DOWN BETWEEN THE TOP AND BOTTOM MATS OF REINFORCING. ELEVATION SECTION A-A FOR SECTION B-B, SEE SHEET 2.









NOTES:

- PEDESTAL TYPE AND FOOTING TYPE INDICATED ON CONTRACT DRAWINGS OBTAINED FROM BD-645M, SHEETS 5, 6, AND 7.
- FOR INSTALLATION DETAILS, SEE SHEET 2.
- FOOTING DESIGN INFORMATION ON THIS SHEET BASED ON 10 FOOT FILL HEIGHT. DESIGNER MUST CHECK ADEQUACY FOR FILL HEIGHTS < 10 FT.
- SEE STANDARD DRAWING BC-736M FOR REINFORCING BAR FABRICATION DETAILS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

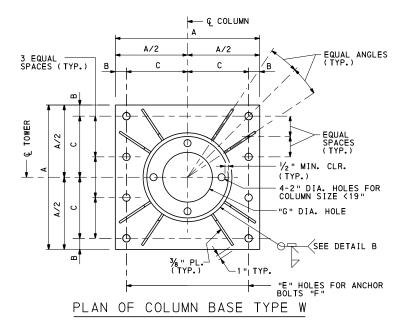
4 POST 4 CHORD TRUSS SPANS FROM 100' TO 200'

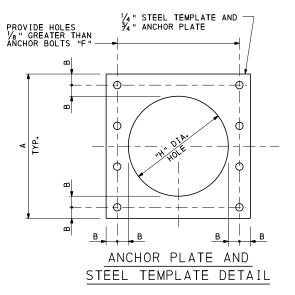
FOUNDATION DETAILS

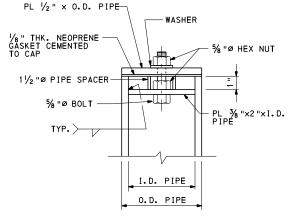
RECOMMENDED AUG. 4, 2017 Thomas P Macioca CHIEF BRIDGE ENGINEER

RECOMMENDED AUG. 4, 2017 Bun SThomps

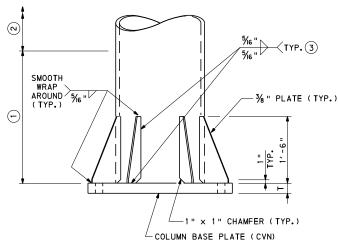
SHT. 3 OF 10 BC-745M DIRECTOR, BUR. OF PROJECT DELIVERY





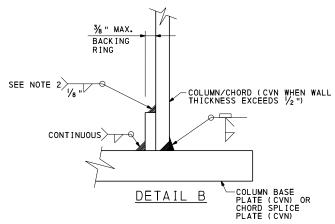


ALTERNATE PIPE CAP DETAIL



ELEVATION - TYPE W

- 1) FOR PRESS BREAK COLUMN, 2'-6" LENGTH OF SEAM WELD TO BE COMPLETE PENETRATION GROOVE WELD.
- (2) SEAM WELD TO HAVE 60% MIN. PENETRATION.
- (3) TERMINATE WELDS 1/4" SHORT OF STIFFENER CHAMFER.



DETAIL B NOTES:

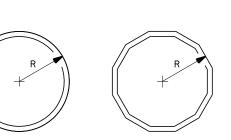
- BACKING RING MUST BE FITTED/SIZED TO THE PIPE COLUMN AND CONTINUOUSLY FILLET WELDED TO THE BASE PLATE BEFORE THE FULL PENETRATION GROOVE WELD IS MADE. BACKING RING MUST BE FABRICATED AS A CONTINUOUS RING.
- 2. FOR COLUMNS AND CHORDS LESS THAN 19", THIS FILLET WELD IS NOT REQUIRED BUT SHOP IS TO APPLY SILICON CAULKING TO THIS LOCATION AFTER POLE ASSEMBLY IS GALVANIZED.

					С	OLUMN	BASES								
COLUMN NOMINAL SIZE X WALL THK.*	NOMINAL BASE A B C E F G H T WASHER PRO- EMBED SIZE X ALL THK.* TYPE A B C E F G H T SIZE JECTION MENT														
10 "x. 365 "	W	1′-8"	21/2"	71/2"	1½ "D	1 1/4 "D	31/4"	10"	2 "	3½ "D×¾ "	7 3/4 "	2′ -1 "			
12 "x. 375 "	W	1′-10"	21/2"	8 1/2 "	1 ¾ "D	1 ½ "D	51/4"	1'-0"	2 "	3½ "D×¾ "	8 1/2 "	2′-6"			
14 "x. 375 "	W	2′-0"	21/2"	91/2"	1 ¾ "D	1 ½ "D	6½"	1′-2"	2 "	3½ "D×¾ "	8 1/2 "	2′-6"			
16 "x. 375 "	W	2′ -2"	21/2"	101/2"	2 "D	1 ¾ "D	8 "	1'-4"	2 "	4 "D×¾ "	91/4"	2'-11"			
18 "x. 375 "	W	2' -4"	2 1/2 "	111/2"	2 "D	1 ¾ "D	91/4"	1′-6"	2 "	4 "D×3/8 "	91/4"	2' -11"			
20 "x• 375 "	W	2′-9"	3 "	1′-01/2"	2 1/4 "D	2 "D	1′-5"	1'-7"	3 "	5 "D×3% "	11"	3′ -4"			
24 "x. 375 "	W	2′-11"	3 "	1'-21/2"	2 1/4 "D	2 "D	1′-6"	1'-11"	3 "	5 "D×3% "	11"	3′ - 4 "			
24"x.500"	W	3′-0"	31/2"	1'-21/2"	2½ "D	2 1/4 "D	1′-6"	1′-10"	3 "	5 "D×3/8"	113/4"	3′-9"			

NOTE: D DENOTES DIAMETER * CVN REQUIRED FOR WALL THICKNESSES EXCEEDING 1/2 " (.500").

NOTES:

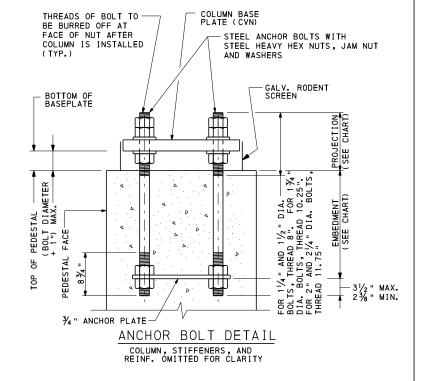
- ANCHOR BOLTS SHALL BE PROVIDED WITH FOUR HEAVY HEX NUTS, ONE JAM NUT AND TWO WASHERS AS SHOWN ON THE ANCHOR BOLT DETAIL.
- ANCHOR BOLTS SHALL BE GALVANIZED AFTER THREADING.
- USE STEEL TEMPLATE TO SET ANCHOR BOLTS IN ACCORDANCE WITH PUBLICATION 408, SECTION 948.3(b).
- STEEL TEMPLATE AND ANCHOR PLATE TO BE PROVIDED BY SIGN FABRICATOR.
- TEMPLATE PLATE WITH NUTS ON BOTH SIDES SHALL BE USED TO MAINTAIN THE SPACING AND ALIGNMENT OF ANCHOR BOLTS.
- FOR PIPE CAP DETAILS SEE SHEET 5.
- SEAL BASE PLATE TO FOUNDATION GAP WITH GALVANIZED STEEL SCREEN, 1/2" BY 1/2" MESH AND 0.063" DIAMETER WIRES. SCREEN IS TO PREVENT ENTRY OF RODENTS WHILE PERMITTING DRAINAGE. SCREEN IS TO BE REMOVABLE AND ATTACHED TO BASEPLATE WITH STAINLESS STEEL HARDWARE.



LLUSTRATION OF DIMENSION "R" FOR CIRCULAR MEMBERS AND EQUIVALENT "PRESS-BREAK" MEMBERS

"PRESS-BREAK" NOTE:

ALTERNATE "PRESS-BREAK" MEMBERS ARE PERMITTED FOR COLUMNS. "PRESS-BREAK" MEMBERS MUST HAVE THE EQUIVALENT STRENGTH AND FATIGUE RESISTANCE OF THE CIRCULAR MEMBER BEING REPLACED. A MINIMUM NUMBER OF 12 BREAKS IS REQUIRED. A CHANGE IN STEEL MATERIAL OR WALL THICKNESS REQUIRES A SPECIAL DESIGN TO BE SUBMITTED FOR REVIEW. CONTRACTOR MUST SUBMIT DESIGN CALCULATIONS AND DESIGN DRAWINGS FOR REVIEW AND ACCEPTANCE FOR LONGITUDINAL SEAM WELDS INDICATING TYPE OF WELD, WELD PENETRATION, EFFECTIVE DEPTH AND LENGTH OF EACH WELD TYPE. LONGITUDINAL SEAM WELDS SHALL HAVE 60 PERCENT MINIMUM PENETRATION, EXCEPT LONGITUDINAL SEAM WELDS WITHIN 6" OF THE ENDS OF THE PRESS BREAK MEMBER OR LENGTH SHOWN ON DETAILS SHALL BE COMPLETE PENETRATION WELDS. COMPLETE PENETRATION LONGITUDINAL SEAM WELDS MUST BE 100% RADIOGRAPHICALLY INSPECTED. FOR THE COLUMN CONNECTION TO BASE PLATE, AND AT COLUMN CONNECTION SPLICE PLATE LOCATIONS, WELD SHALL START AND STOP IN THE MIDDLE THIRD REGION OF FLAT SECTIONS BETWEEN BREAK POINTS.



COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

4 POST 4 CHORD TRUSS SPANS FROM 100' TO 200'

COLUMN BASE DETAILS

RECOMMENDED AUG. 4, 2017

Thoma P Macioca

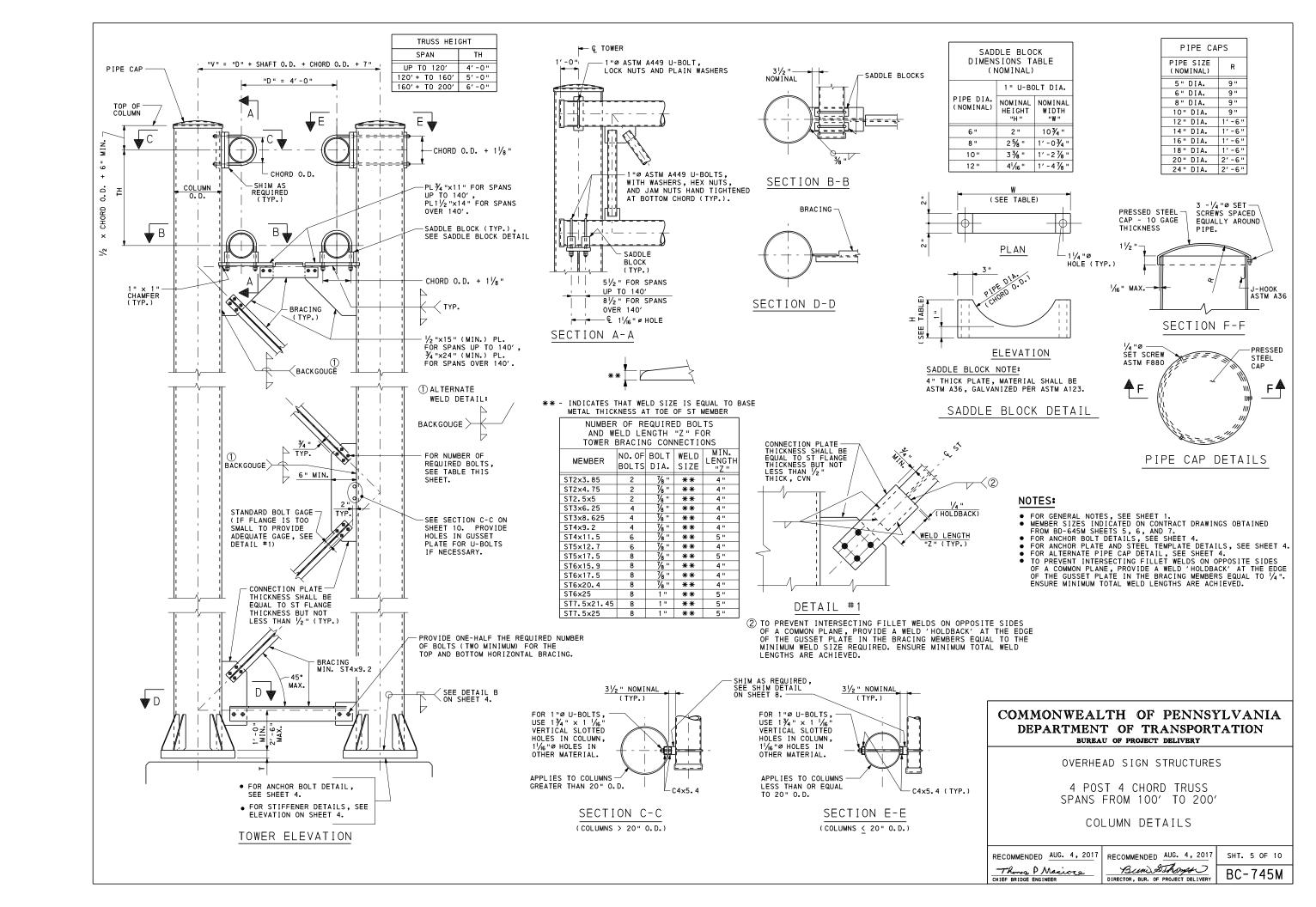
CHIEF BRIDGE ENGINEER

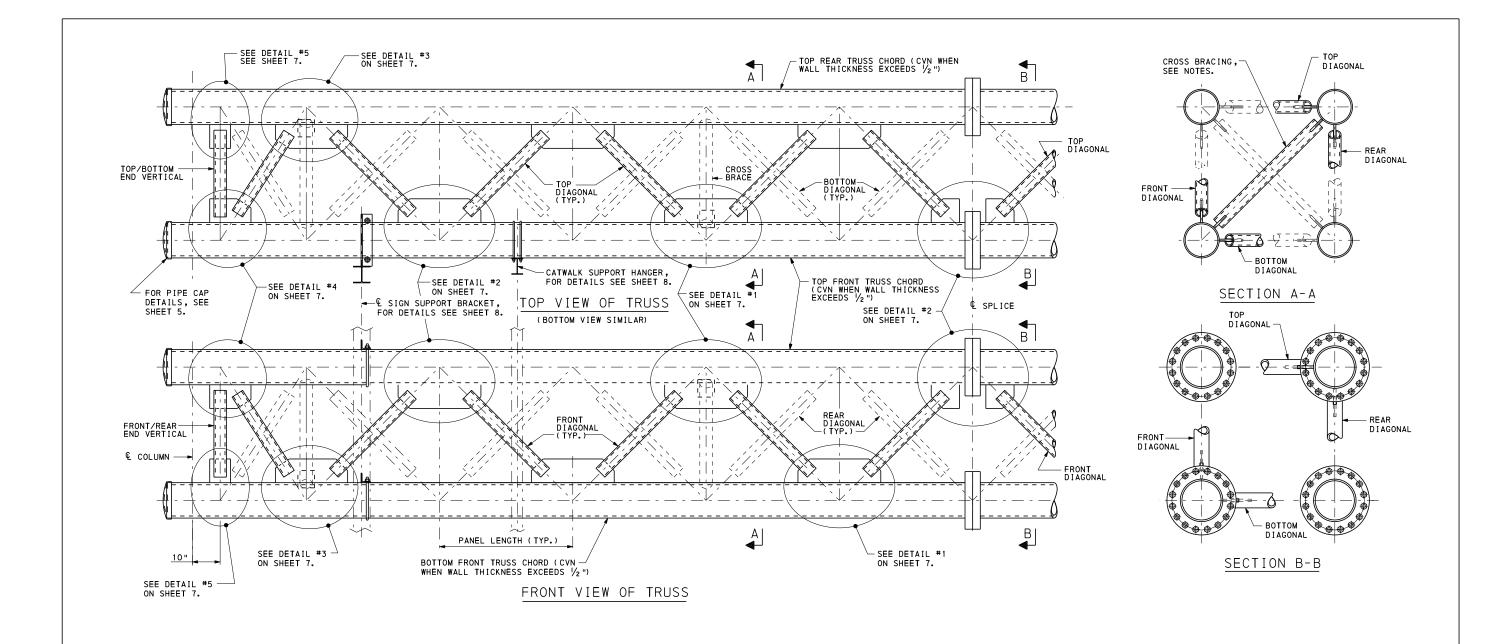
RECOMMENDED AUG. 4, 2017

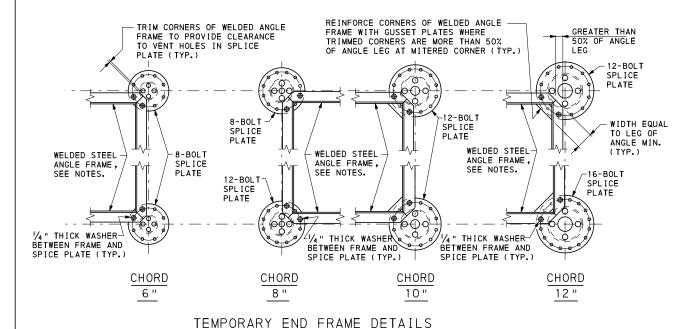
Bund Sthongs

DIRECTOR, BUR. OF PROJECT DELIVERY

BC-745M







NOTES:

- FOR GENERAL NOTES, SEE SHEET 1.
- MEMBER SIZES INDICATED ON CONTRACT DRAWINGS OBTAINED FROM BD-645M SHEETS 5-7.
- ONE OR MORE SPLICES IN THE TRUSS MAY BE ADDED OR ELIMINATED AT THE OPTION OF THE FABRICATOR. IN CASE OF THE ADDITION OR ELIMINATION OF SPLICES, THE HEAVIER CHORD MATERIAL MUST BE EXTENDED TOWARD THE LIGHTER CHORD MATERIAL TO THE DESIRED SPLICE LOCATION.
- TEMPORARY END FRAME TO BE USED TO PROVIDE ADDITIONAL SUPPORT TO ENDS OF TRUSS CHORDS DURING FABRICATION AND GALVANIZING PROCESSES. REMOVE AND REPAIR GALVANIZING AT POINTS OF CONTACT PRIOR TO TRUSS ASSEMBLY AND ERECTION. TEMPORARY FRAME IS NOT PART OF THE STRUCTURE AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR.
- TRUSSES SHALL BE FABRICATED WITH CAMBER AT THE CENTER OF THE SPAN EQUAL TO THE VALUE GIVEN BY THE CAMBER DIAGRAM ON THE CONTRACT DRAWING. ALL TRUSSES SHALL BE ASSEMBLED IN THE SHOP IN A NO LOAD CONDITION TO ENSURE FIT AT SPLICES AND TO CHECK CAMBER.
- CROSS BRACING ALTERNATING IN DIRECTION AT MAXIMUM SPACING OF 3 PANEL LENGTHS, SHALL NOT BE PLACED AT END VERTICALS NOR AT SPLICE POINTS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

4 POST 4 CHORD TRUSS SPANS FROM 100' TO 200'

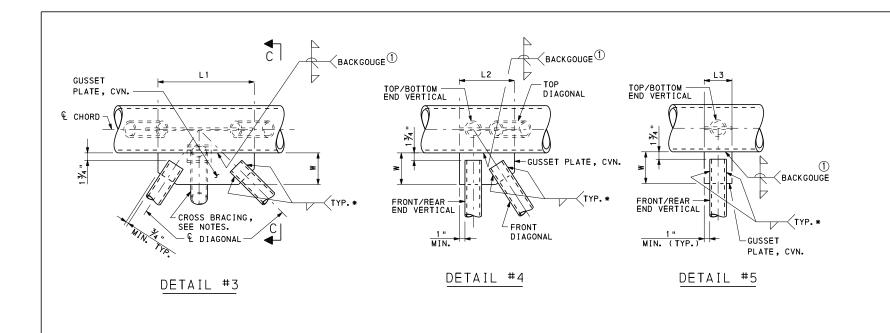
TRUSS DETAILS

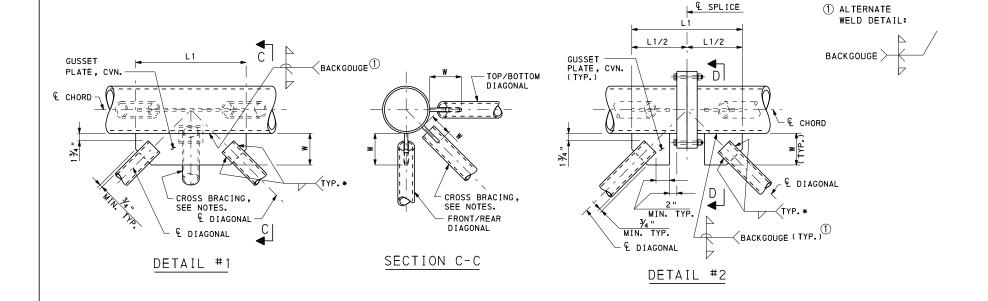
RECOMMENDED AUG. 4, 2017 Thomas P Macioca

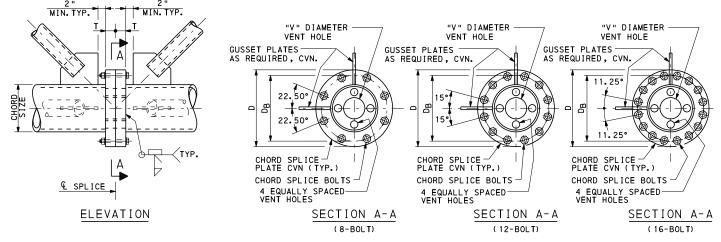
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Bun SThomps BC-745M DIRECTOR, BUR. OF PROJECT DELIVERY

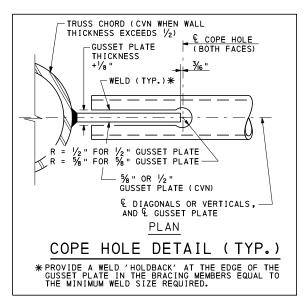
SHT. 6 OF 10











	CHO	RD SPLI	CE		
CHORD NOMINAL SIZE X WALL THK. A	D	D _B	BOLTS	Т	٧
6 "x. 280"	1′-25/8"	11%"	8- 1/8 "Ø	2 "	0
8 "x. 322 "	1′-45/8"	1′-1%"	8- 1/8 "Ø	21/4"	2 "
8 "x. 500 "	1′-45/8"	1′-15/8"	12-	23/4"	2 "
10 "x• 365 "	1'-63/4"	1′-3¾"	12- ¾ "ø	2 3/8 "	31/4"
12 "x• 375 "	1'-83/4"	1'-5¾"	16- ¾ "ø	21/2"	51/4"
12 "x. 500 "	1'-9¾"	1′-5¾"	12-11/8 "Ø	23/4"	51/4"
12 "x. 562 "	1'-101/4"	1'-5¾"	12-11/4 "Ø	3"	51/4"
12 "x. 688 "	1'-10¾"	1'-53/4"	12-13% "Ø	31/4"	51/4"
12 "x. 844"	1'-111/4"	1'-53/4"	12-1½"ø	31/2"	51/4"

WHERE LARGER CHORD SIZE SPLICES TO SMALLER CHORD SIZE, USE SPLICE AS SHOWN FOR SMALLER CHORD.

▲ CVN REQUIRED FOR WALL THICKNESSES EXCEEDING 1/2" (0.500").

	TRUSS GUSSET PLATE TABLE														
CHORD	PLATE	BRAC	ING	MIN. WELD	MIN.	MIN. GUSSET	MAX. GL	JSSET LEN	GTH						
SIZE (NOMINAL)	THICK. ▲	SIZE	WALL SIZE	SIZE "Z"	WELD LENGTH	WIDIL	L1	L2	L3						
6"	1/2 "	21/2"	. 203 "	1/4	21/2"	63%"	1'-71/4"	1'-01/8"	4 1/8 "						
8 "	1/2 "	21/2"	. 203 "	1/4	21/2"	63/8"	1'-11"	1'-2"	4 % "						
8 "	1/2 "	3 "	.216"	1/4	31/4"	7 3/8 "	1'-111/4"	1'-23/8"	51/2"						
8 "	1/2 "	3 "	.300"	1/4	41/2"	81/4"	2'-1"	1'-31/4"	51/2"						
10"	1/2 "	21/2"	. 203 "	1/4	21/2"	61/2 "	2'-63/8"	1′-5 1/8 "	4 1/8 "						
10"	1/2 "	3 "	.216"	1/4	31/4"	71/4"	2'-01/8"	1'-31/4"	5½"						
12"	5/8 "	3 "	.216"	1/4	31/4"	7 3/8 "	2′-85%"	1'-71/8"	51/2"						
12"	5% "	31/2"	.226"	1/4	4 "	81/4"	3'-11/2"	1'-93/4"	6"						
12"	5/8 "	4"	.237"	1/4	4 3/4 "	91/8"	3' -01/2 "	1'-91/4"	61/2"						
12 "	5/8 "	4 "	. 337"	5/16	51/4"	91/2"	3'-43/8"	1'-111/2"	61/2"						
12"	5/8 "	5 "	. 258 "	5/16	5 "	10"	3′ -5¾ "	2' -03/4"	7 5/8 "						

▲ CVN REQUIRED FOR WALL THICKNESSES EXCEEDING 1/2 " (0.500").

NOTES:

- \bullet CHORD SPLICE BOLTS SHALL BE ASTM A325 HIGH STRENGTH STEEL BOLTS. HOLES IN SPLICE PLATE SHALL BE $1/\!\!/_6$ " LARGER THAN BOLT DIAMETER.
- ASTM A325 SPLICE BOLTS SHALL BE HEAVY HEXAGON TYPE AND SHALL BE FURNISHED WITH HEAVY HEXAGON NUTS AND WASHERS.
- THE THREADED PORTION OF THE SPLICE BOLTS SHALL BE EXCLUDED FROM THE SHEAR PLANE OF THE SPLICE.
- 4 EQUALLY SPACED VENT HOLES 2" DIAMETER HOLES, TYPICAL, EXCEPT 1½" DIAMETER HOLES FOR 8" CHORD SIZE AND 1¼" DIAMETER HOLES FOR CHORDS LESS THAN 8".
- GUSSET PLATE SIZES PROVIDED AS A GUIDE. FABRICATOR MUST PROVIDE PLATES OF ADEQUATE SIZE TO ACHIEVE MIN. WELD SIZE AND LENGTH REQUIRED.
- CROSS BRACING ALTERNATING IN DIRECTION AT MAXIMUM SPACING OF 3 PANEL LENGTHS, SHALL NOT BE PLACED AT END VERTICALS NOR AT SPLICE POINTS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

OVERHEAD SIGN STRUCTURES

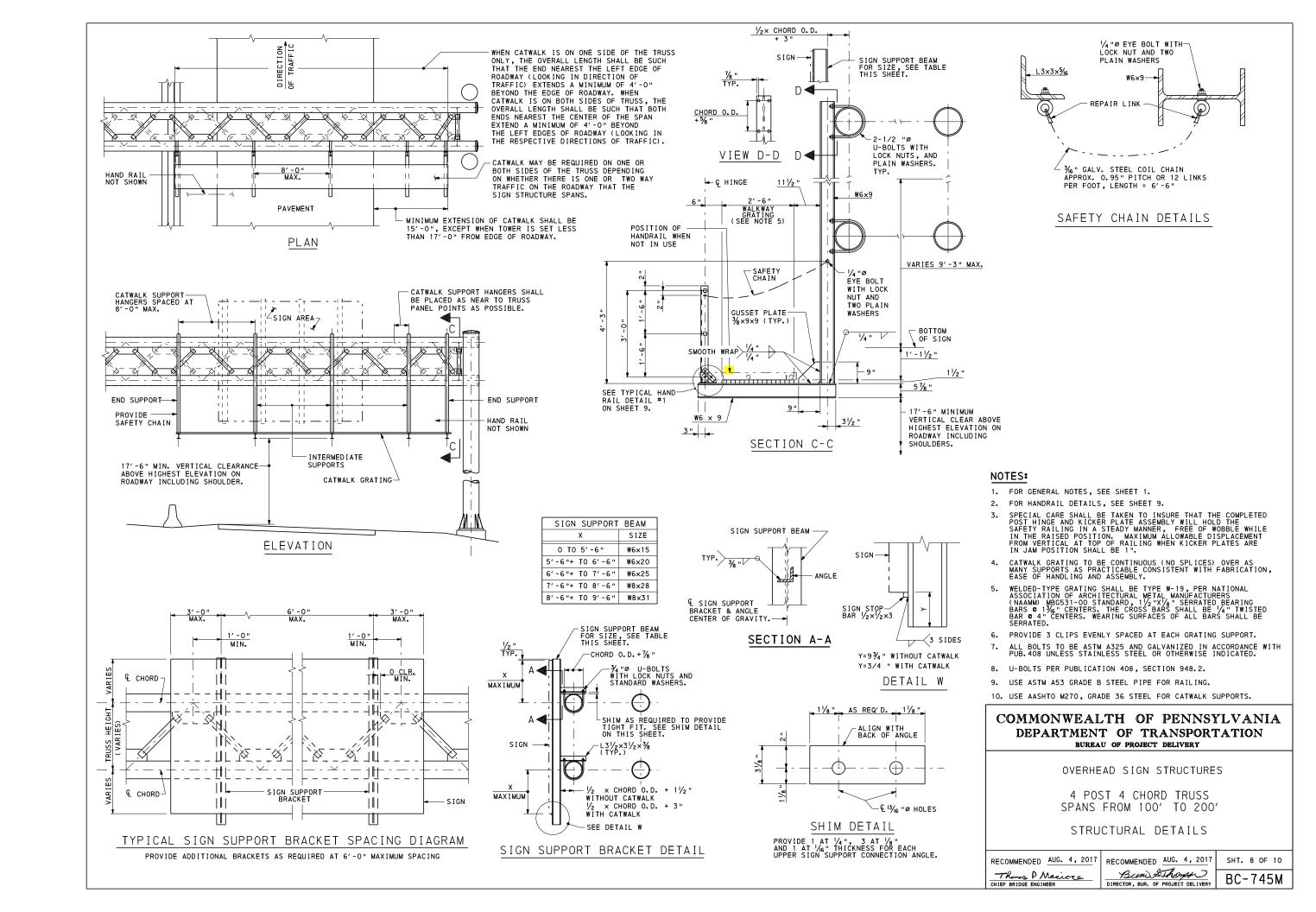
4 POST 4 CHORD TRUSS SPANS FROM 100' TO 200'

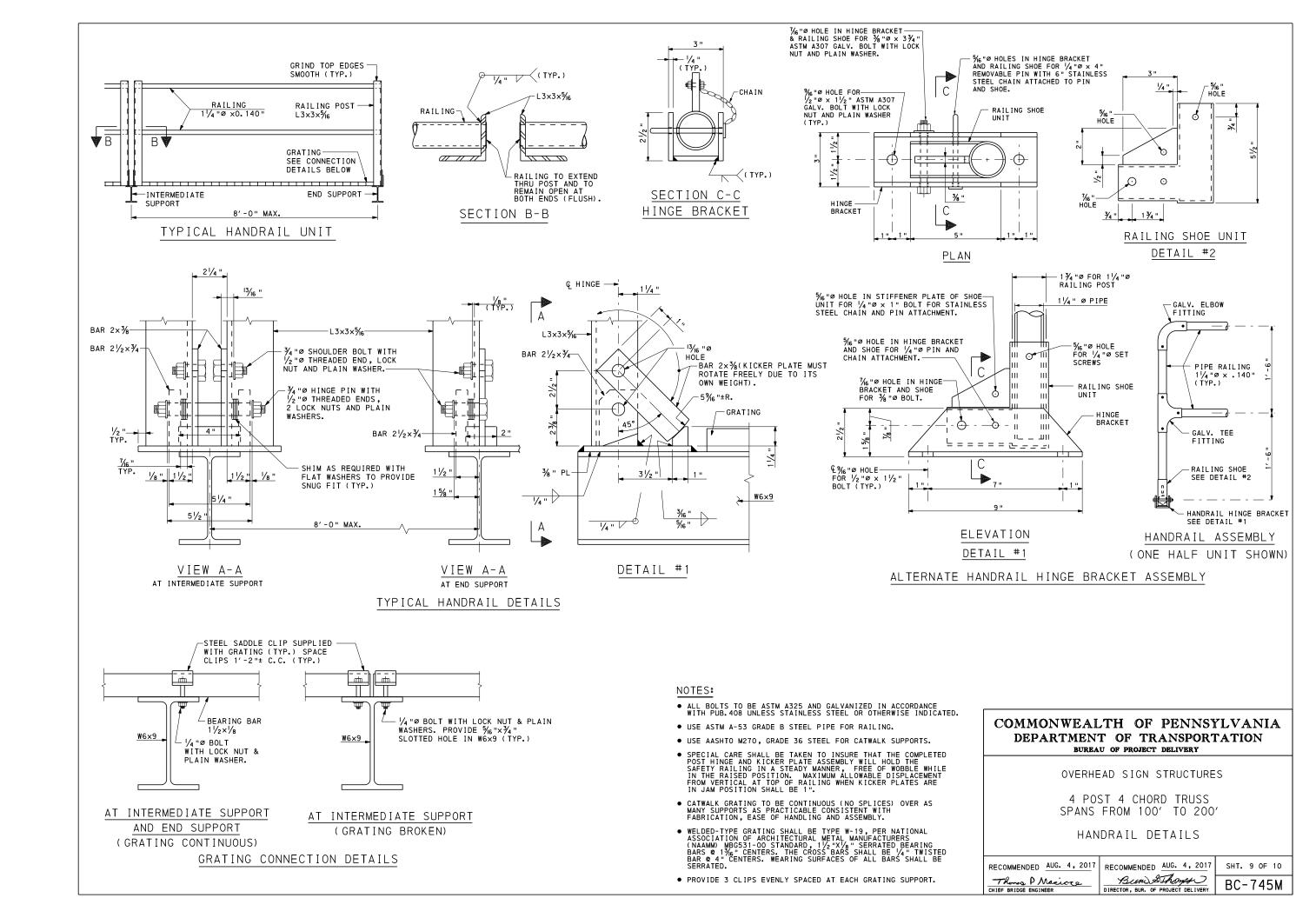
TRUSS DETAILS

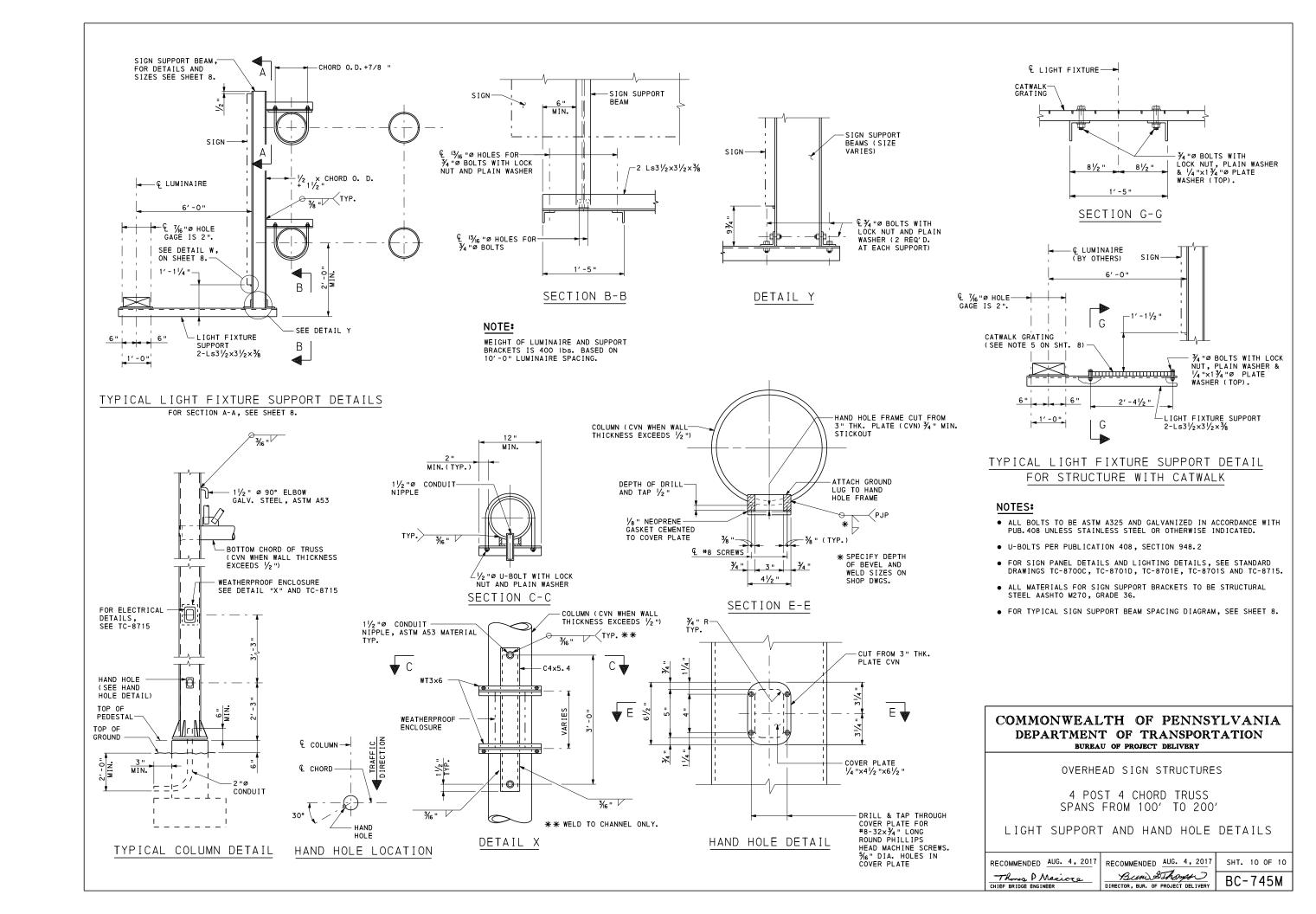
RECOMMENDED AUG. 4, 2017 RECOMMENDED AUG. 4, 2017 Bun SThomps Thoma P Macioca

BC-745M DIRECTOR, BUR. OF PROJECT DELIVERY

SHT. 7 OF 10







INFORMATION CONTAINED IN THE BD-647M DESIGN TABLES

- THE MEMBER SIZES INDICATED IN THE DESIGN TABLES MEET THE EATLGUE REQUIREMENTS
- THE SPAN RANGE INCLUDED ON STANDARD DRAWING BD-647M IS AS FOLLOWS:

CANTILEVER STRUT LENGTHS UP TO 38' AND FRAME LENGTHS UP TO 200'.

THE DESIGN TABLES INCLUDE MEMBER SIZES FOR THE STRUCTURES FOR VARIOUS COMBINATIONS OF SPAN LENGTH AND SIGN AREA. THE CORRESPONDING FABRICATION AND CONSTRUCTION DETAILS ARE CONTAINED IN THIS STANDARD.

GENERAL NOTES

- 1. PROVIDE 3-INCH CONCRETE COVER ON REINFORCEMENT BARS, EXCEPT AS NOTED.
- USE CLASS A CEMENT CONCRETE f'c = 3000 PSI IN PEDESTALS, FOOTINGS AND CAISSONS.
- PROVIDE GRADE 60 REINFORCING STEEL BARS THAT MEET THE REQUIREMENTS OF ASTM A615 FOR CONCRETE REINFORCEMENT. DO NOT WELD REINFORCING STEEL BARS.
- RAKE-FINISH ALL HORIZONTAL CONSTRUCTION JOINTS, EXCEPT AS INDICATED.
- VERIFY ALL DIMENSIONS AND GEOMETRY OF THE EXISTING STRUCTURES IN THE FIELD AS NECESSARY FOR PROPER FIT OF THE PROPOSED CONSTRUCTION.
- CHAMFER EXPOSED CONCRETE EDGES 1 INCH BY 1 INCH.
- 7. ALL DIMENSIONS SHOWN ARE HORIZONTAL. EXCEPT AS NOTED.
- 8. DIMENSIONS ARE BASED ON A NORMAL TEMPERATURE OF 68 DEGREES F.
- PIPE DIAMETERS SHOWN IN THE DESIGN TABLES ARE OUTSIDE DIAMETERS.
- USE STANDARD SIZE HOLE. THE STANDARD HOLE DIAMETER FOR BOLTS SMALLER THAN 1" DIAMETER SHALL BE THE NOMINAL DIAMETER OF THE BOLT PLUS 1/16". FOR BOLTS 1" DIAMETER AND LARGER, THE WIDTH OF EACH STANDARD HOLE SHALL BE THE NOMINAL DIAMETER OF THE BOLT PLUS 1/26".
- PROVIDE ANCHOR BOLT HOLES 1/4" LARGER THAN BOLT DIAMETER.
- PROVIDE DOUBLE NUTS AND WASHER FOR EACH ANCHOR BOLT.

CHANGE 1

STEEL MEMBER COMPONENTS REQUIRING CHARPY V-NOTCH TESTING ARE DESIGNATED ON THE PLANS BY (CVN), PROVIDE STEEL CONFORMING TO THE CVN REQUIREMENTS FOR ZONE 2, NON FRACTURE CRITICAL AS GIVEN IN THE AASHTO MATERIAL SPECIFICATIONS.

DESIGN

- SPECIFICATIONS: "AASHTO 4TH EDITION STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS" (2001 WITH INTERIMS THROUGH 2006) AND "AASHTO 17TH EDITION STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" (2004).
- 2. WIND LOADS ARE BASED ON 90 MPH (3 SECOND GUST) BASIC WIND SPEED.
- CAISSON FOUNDATIONS ARE BASED ON A MAXIMUM 1/2" LATERAL DEFLECTION AT TOP OF CAISSON AND ON THE FOLLOWING SOIL PARAMETERS:

 A) LOOSE GRANULAR SOIL WITH 100 PCF UNIT WEIGHT, 28 DEGREE INTERNAL FRICTION ANGLE, 0 PSF COHESION, AND 25 PCI MODULUS OF SUBGRADE REACTION.

 B) SOFT COHESIVE SOIL WITH 100 PCF UNIT WEIGHT, 0 DEGREE INTERNAL FRICTION ANGLE, 800 PSF COHESION, 200 PCI MODULUS OF SUBGRADE REACTION, AND 0.02 F50 STRAIN.
- DESIGN TABLES MEMBER SIZES ARE ADEQUATE FOR FATIGUE CATEGORY I, THEREFORE, PENNDOT MINIMUM REQUIREMENT OF FATIGUE CATEGORY II IS MET.

MATER I AL

- PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH THE CURRENT VERSIONS OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, ANSI/AWS WELDING CODE D1.5, CONTRACT SPECIAL PROVISIONS, AND AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS". USE ANSI/AWS D1.1 FOR WELDING NOT COVERED IN ANSI/AASHTO/AWS D1.5.
- PROVIDE STRUCTURAL STEEL CONFORMING TO AASHTO M270, GRADE 36 (ASTM A709, GRADE 36) DESIGNATION EXCEPT WHEN NOTED OTHERWISE.
- PROVIDE WELDED OR SEAMLESS STEEL PIPE CONFORMING TO PUBLICATION 408, SECTION
- PROVIDE HIGH-STRENGTH STEEL BOLTS CONFORMING TO AASHTO M164 (ASTM A325) MECHANICALLY GALVANIZE ALL BOLTS (EXCEPT ANCHOR BOLTS), NUTS AND WASHERS.
 EITHER MECHANICALLY GALVANIZE ALL ANCHOR BOLTS OR HOT-DIP GALVANIZE ALL ANCHOR
 BOLTS IN ACCORDANCE WITH FABRICATION NOTE 6 ON THIS SHEET. PROVIDE U-BOLTS CONFORMING TO ASTM A449. PROVIDE ANCHOR BOLTS CONFORMING TO ASTM F1554, GRADE

FABRICATION

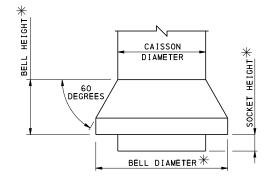
- CONSTRUCT SIGN STRUCTURES TRUE TO DIMENSION, FREE FROM KINKS, TWISTS OR BENDS, AND UNIFORM IN APPEARANCE. ASSEMBLE COMPLETED SECTIONS IN THE SHOP AND CHECK FOR STRAIGHTNESS, ALIGNMENT, DIMENSION, AND THE FIRM SEATING OF THE SPLICE PLATES. CORRECT ANY VARIATIONS TO THE SATISFACTION OF THE ENGINEER.
- FORM MASTS FOR SIGN STRUCTURES TO THE RADII SHOWN ON THE PLANS IN ACCORDANCE WITH THE TUBE AND PIPE ASSOCIATION INTERNATIONAL RECOMMENDED STANDARDS FOR INDUCTION BENDING OF PIPE AND TUBE (TPA-IBS-98).
- AFFIX CLIPS, EYES, OR REMOVABLE BRACKETS TO ALL MASTS AND MAST ARMS, AS NECESSARY, TO SECURE THE SIGN STRUCTURE DURING SHIPPING AND FOR LIFTING AND MOVING DURING ERECTION. THIS IS TO PREVENT DAMAGE TO THE FINISHED GALVANIZED OR PAINTED SURFACES. REMOVE BRACKETS ON SIGN STRUCTURES AFTER ERECTION. INCLUDE DETAILS OF SUCH DEVICES ON THE SHOP DRAWINGS.
- 4. FABRICATE ALL SIGN STRUCTURES INTO THE LARGEST PRACTICAL SECTIONS PRIOR TO GALVANIZING. SUBMIT SPLICE LOCATIONS TO THE ENGINEER FOR APPROVAL. DO NOT COMMENCE FABRICATION UNTIL SUCH SPLICE LOCATIONS ARE APPROVED.
- GRIND ALL AREAS TO BE WELDED TO BRIGHT METAL. BUTT WELD SPLICES ARE NOT PERMITTED, UNLESS SHOWN ON THE PLANS. COMPLETE ALL WELDING AND REQUIRED TESTING BEFORE ANY MATERIAL IS GALVANIZED. NON-DESTRUCTIVELY TEST ALL CIRCUMFERENTIAL AND STIFFENER WELDS USING THE METHODS AND PROCEDURES IN ACCORDANCE WITH SECTION 948. THE ACCEPTABLE CRITERIA ARE STATED IN TABLE 6.1 OF ANSI/AWS D1.1/D1.1M. PROVIDE FULL PENETRATION GROOVE WELDS FOR ALL LONGITUDINAL WELDS WITHIN 6" OF A FULL PENETRATION CIRCUMFERENTIAL GROOVE WELD AND INSPECT AS SPECIFIED ABOVE.
- HOT-DIP GALVANIZE ALL COMPONENTS (EXCEPT REINFORCEMENT BARS, ALUMINUM, AND NON-FERROUS INCIDENTALS) AFTER FABRICATION PER ASTM A123 OR ASTM A153, AS

CONSTRUCTION

- 1. USE TEMPLATES TO ACCURATELY SET BASE PLATE ANCHOR BOLTS TO CORRECT ELEVATION AND ALIGNMENT. SECURELY BRACE ANCHOR BOLTS AGAINST DISPLACEMENT BEFORE CAISSON CONCRETE IS PLACED AND DURING CONCRETE CURING.
- ERECT SIGN STRUCTURE ONLY AFTER CAISSON CONCRETE MEETS 7 DAY STRENGTH
- TEMPORARILY SUPPORT MAST ARMS TO RELIEVE LOAD FROM THE SPLICES WHILE HIGH-STRENGTH BOLTS ARE BEING TIGHTENED IN ORDER TO FIRMLY SEAT THE CONNECTION PLATES.
- 4. PRIOR TO ERECTION, DEMONSTRATE TO THE ENGINEER BY PREASSEMBLY OR OTHER APPROVED METHOD THAT FRAME STRUCTURE LENGTH IN A NO-LOAD CONDITION MATCHES FIELD MEASURED CAISSON SPACING WITHIN 1/2".
- 5. ADEQUATELY SUSPEND FRAME STRUCTURES TO AVOID DISTORTIONS OR CHANGES IN SPAN LENGTH IF ERECTED ONTO FOUNDATIONS AS ONE UNIT.

NOTES TO FABRICATOR

- DYNAMIC/VARIABLE MESSAGE SIGNS (DMS/VMS) ARE PROHIBITED ON MONOPIPE STRUCTURES.
- DESIGN COMPUTATIONS ARE REQUIRED FOR ANY PORTION OF A STRUCTURE FOR WHICH THE INFORMATION IS NOT TAKEN DIRECTLY FROM THE CONTRACT DRAWINGS OR THE DETAILS CONTAINED IN THIS STANDARD, DO NOT VIOLATE CRITERIA USED FOR THE DEVELOPMENT OF THE DESIGN TABLES ON STANDARD DRAWING BD-647M AND THE



CAISSON BELL DETAIL

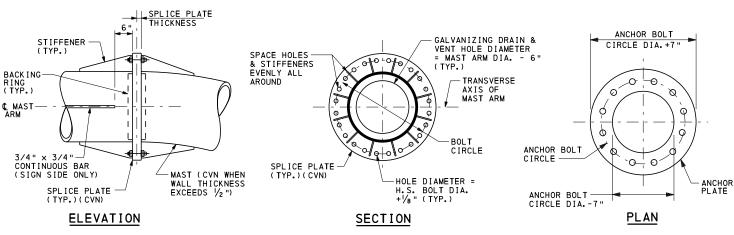
* SUBMIT THE PROPOSED DRILLING EQUIPMENT TO THE REPRESENTATIVE FOR ACCEPTANCE AND INCLUDE THE SOCKET AND BELL DIMENSIONS.

l FÖR	SOFT	ELL DIA COHESIVE STRUCTUF	SOIL
SPAN (FEET)	PANEL AREA (S.F.)	CAISSON DIAMETER (INCHES)	BELL DIAMETER (INCHES)
100	1,000	48	54
120	800	54	60
140	420	48	54

COMMONWEALTH OF PENNSYLVANIA TC-8700C SPACING CHARTS/DIRECT APPLIED LETTERS, NUMERALS, & ARROWS DEPARTMENT OF TRANSPORTATION TC-8701D | SIGN DETAILS/FREEWAY AND EXPRESSWAY GUIDE SIGNS BUREAU OF PROJECT DELIVERY TC-8701E EXTRUDED ALUMINUM CHANNEL SIGN MONOPIPE SIGN STRUCTURES TC-8701S FLAT SHEET ALUMINUM SIGNS WITH EXTRUDED ALUMINUM STIFFENERS TC-8715 SIGN LIGHTING/MERCURY VAPOR LAMPS FRAME STRUCTURE SPANS UP TO 160' ERECTION DETAILS/EXTRUDED ALUMINUM CHANNEL SIGNS TC-8716 AND CANTILEVER MONOPIPE STRUCTURE FLAT SHEET ALUMINUM WITH STIFFENERS/OVERHEAD STRUCTURES STRUT LENGTHS UP TO 27' BC-736M REINFORCEMENT BAR FABRICATION DETAILS RC-11M CLASSIFICATION OF EARTHWORK FOR STRUCTURES GENERAL NOTES RC-51M TYPE 31 STRONG POST GUIDE RAIL TYPE 2 WEAK POST GUIDE RAIL RC-53M RC-54M BARRIER PLACEMENT AT OBSTRUCTIONS RECOMMENDED AUG. 4, 2017 RECOMMENDED AUG. 4, 2017 SHEET 1 OF 5 RC-58M SINGLE FACE CONCRETE BARRIER PLACEMENT AT MEDIAN PIERS Bun SThomps Thoma P Macioca CHIEF BRIDGE ENGINEER DIRECTOR, BUR. OF PROJECT DELIVERY BC-747M REFERENCE DRAWINGS

MAS	T ARM	& END	CONNEC	TIC	N COMP	ONENT	SELECT	ION TA	BLE	(CANT	ILEVER	STRUC	TURES)
CDAN	PANEL	MAST	ARM		H.S. BO	LTS	SPLIC	E PLATE			STIFFEN	IERS	
SPAN (FEET)	AREA	DIAMETER (INCHES)	THICKNESS (INCHES)	NO.	DIAMETER (INCHES)	CIRCLE (INCHES)	DIAMETER (INCHES)	THICKNESS (INCHES)	NO.	THICKNESS (INCHES)	WIDTH (INCHES)	HEIGHT (INCHES)	WELD (INCHES)
27	350	24	0.375 (SCH. 20)	20	1	27 1/2	31	2					
	250	24	0.375 (SCH. 20)	18	1	27 1/2	31	2		(NONE REQU	JIKED)	

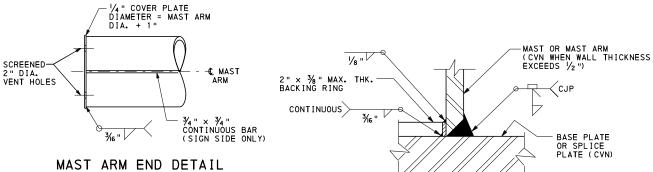
	MA	ST &	BASE C	ONNECT:	ION	COMPO	NENT SI	ELECTIO	ON TABL	Ε.	CANTI	EVER	STRUCTU	JRES)			
Γ	SPAN	PANEL	MA	ST		ANCHOR BOLTS BASE PLATE							STIFFENERS				
	(FEET)	AREA (S.F.)	DIAMETER (INCHES)	THICKNESS (INCHES)	NO.	DIAMETER (INCHES)	CIRCLE (INCHES)	DIAMETER (INCHES)	THICKNESS (INCHES)	NO.	THICKNESS (INCHES)	WIDTH (INCHES)	HEIGHT (INCHES)	WELD (INCHES)			
	27	350	24	0.562 (SCH. 30)	16	1 3/4	31	37	2 1/4	8	3/8	5 1/2	15 1/2	5/16			
		250	24	0.500 (WT.XS)	18	1 1/2	30 1/2	35 1/2	2	9	3/8	4 3/4	13 1/2	5/16			



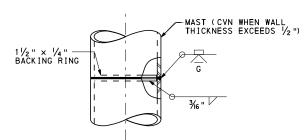
END CONNECTION DETAILS

(MAST ARM SPLICE CONNECTION SIMILAR)
(24 BOLT CONFIGURATION SHOWN)

ANCHOR PLATE DETAIL (12 BOLT CONFIGURATION SHOWN)

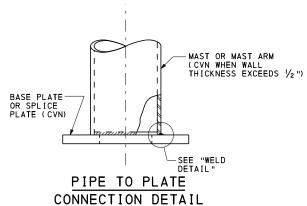


(CANTILEVER STRUCTURES)



OPTIONAL SHOP CONNECTION DETAIL

STIFFENER DETAILS



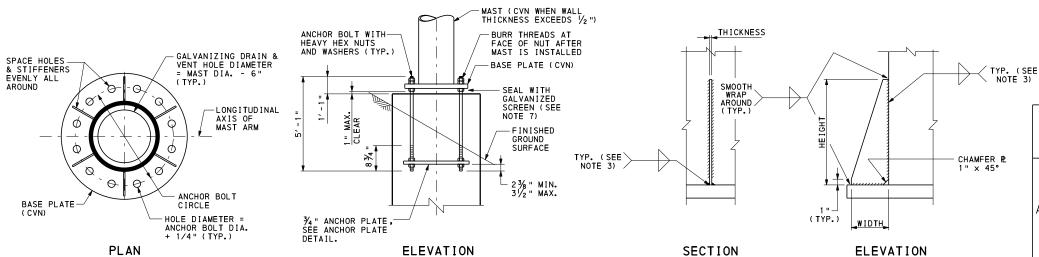
WELD DETAIL

WELD DETAIL NOTE:

BACKING RING MUST BE FITTED/SIZED TO THE PIPE COLUMN AND CONTINUOUSLY FILLET WELDED TO THE BASE PLATE BEFORE THE FULL PENETRATION GROOVE WELD IS MADE. BACKING RING MUST BE FABRICATED AS A CONTINUOUS RING.

NOTES:

- 1. FOR GENERAL NOTES, SEE SHEET 1.
- PROVIDE MAXIMUM 2'-0" SPACE BETWEEN ADJACENT SIGN PANELS WHEN PRESENT.
- 3. TERMINATE WELDS 1/4" SHORT OF STIFFENER CHAMFER.
- 4. PROVIDE STIFFENERS AS INDICATED IN CONNECTION COMPONENT SELECTION TABLES.
- 5. FOR CAISSON INFORMATION, SEE SHEET 6.
- 6. FOR SIGN PANEL SUPPORT BEAM DETAILS, SEE SHEET 5.
- 7. SEAL BASE PLATE TO FOUNDATION GAP WITH GALVANIZED STEEL SCREEN, ½" BY ½" MESH AND 0.063" DIAMETER WIRES. SCREEN IS TO PREVENT ENTRY OF RODENTS WHILE PERMITTING DRAINAGE. SCREEN IS TO BE REMOVABLE AND ATTACHED TO BASEPLATE WITH STAINLESS STEEL HARDWARE.
- 8. FOR FRAME STRUCTURE COMPONENT SELECTION TABLE, SEE SHEETS 3 AND 4.



NOTE: STIFFENERS NOT SHOWN FOR CLARITY

BASE CONNECTION DETAILS

(12 BOLT CONFIGURATION SHOWN)

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

MONOPIPE SIGN STRUCTURES FRAME STRUCTURE SPANS UP TO 160' AND CANTILEVER MONOPIPE STRUCTURE STRUT LENGTHS UP TO 27'

MAST AND MAST ARM DETAILS - 1

RECOMMENDED AUG. 4, 2017

RECOMMENDED AUG. 4, 2017 Bun SThomps

SHEET 2 OF 5

Thoma P Macioca

DIRECTOR, BUR. OF PROJECT DELIVERY BC-747M

										MA	ST ARM	& SPL	ICE CON	INECT I	ON COMP	ONENT S	ELECTION	TAB	BLE										
				Į	USIN	IG MAXI	MUM LE	NGTH 0	F MAST	ARM	SEGMEN	NTS						US I	NG MINI	MUM LE	NGTH 0	F MAST	ARM	SEGMEN	ITS				
CDAN	PANEL	MAST	ARM	SEGMENT		H.S. BOL	_TS	SPL I C	E PLATE			STIFFEN	ERS			T ARM	SEGMENT		H.S. BOL	_TS		PLATE			STIFFEN	ERS		PANEL	CDAN
SPAN (FEET)	(S.F.)	DIAMETER (INCHES)	THICKNESS (INCHES)	ARRANGEMENT	NO.	DIAMETER (INCHES)	CIRCLE (INCHES)	DIAMETER (INCHES)		N0.	THICKNESS (INCHES)	WIDTH (INCHES)	HEIGHT (INCHES)	WELD (INCHES)	DIAMETER (INCHES)	THICKNESS (INCHES)	ARRANGEMENT	NO.	DIAMETER (INCHES)	CIRCLE (INCHES)	DIAMETER (INCHES)	THICKNESS (INCHES)	NO.	THICKNESS (INCHES)	WIDTH (INCHES)	HEIGHT (INCHES)	WELD (INCHES)	AREA (S.F.)	SPAN (FEET)
60	1,040	24	0.375 (SCH. 20)	A	-	-	-	-	-	1	-	-	-	-	24	0.375 (SCH. 20)	В	22	1	28	31 1/2	2	11	3/8	2 3/4	8	5/16	1,040	60
	760	24	0.375 (SCH. 20)	A	-	-	-	-	-	-	-	-	-	-	24	0.375 (SCH. 20)	В	22	1	28	31 1/2	2	11	3/8	2 3/4	8	5/16	760	
	440	20	0.375 (SCH. 20)	А	-	-	-	-	-	-	-	-	-	-	20	0.375 (SCH. 20)	В	20	1	23 1/2	27	2	10	3/8	2 1/2	7	5/16	440	
80	1,000	24	0.500 (WT. XS)	В	22	1 1/4	30 1/2	35	2	11	3/8	4 1/2	12 1/2	5/16	24	0.500 (WT. XS)	С	20	1 1/4	31	35 1/2	2	10	3/8	4 3/4	13 1/2	5/16	1,000	80
	880	24	0.500 (WT. XS)	В	22	1 1/4	29	33 1/2	2	11	3/8	3 3/4	10 1/2	5/16	24	0.500 (WT. XS)	С	20	1 1/4	29	33 1/2	2	10	3/8	3 3/4	10 1/2	5/16	880	
	600	24	0.375 (SCH. 20)	В	22	1 1/8	29 1/2	33 1/2	2	11	3/8	3 3/4	10 1/2	5/16	24	0.375 (SCH. 20)	С	20	1 1/8	29	33	2	10	3/8	3 1/2	10	5/16	600	
	360	20	0.375 (SCH. 20)	В	20	1 1/8	24	29	2	10	3/8	3 1/2	10	5/16	20	0.375 (SCH. 20)	С	18	1 1/8	24	28	2	9	3/8	3	8 1/2	5/16	360	
100	520	24	0.500 (WT. XS)	В	24	1 1/8	32 1/2	36 1/2	2	12	3/8	5 1/4	14 1/2	5/16	24	0.375 (SCH. 20)	С	22	1 1/8	29	33	2	11	3/8	3 1/2	10	5/16	520	100
	280	20	0.500 (SCH. 30)	В	22	1 1/8	27	31	2	11	3/8	4 1/2	12 1/2	5/16	20	0.375 (SCH. 20)	С	20	1 1/8	24	28	2	10	3/8	3	8 1/2	5/16	280	
120	520	24	0.500 (WT. XS)	С	22	1 1/4	32	36 1/2	2	11	3/8	5 1/4	14 1/2	5/16	24	0.688 (SCH. 40)	D	24	1 1/4	34 1/2	39	2	12	3/8	6 1/2	18	5/16	520	120
	360	24	0.375 (SCH. 20)	С	24	1 1/8	29	33	2	12	3/8	3 1/2	10	5/16	24	0.500 (WT. XS)	D	24	1 1/8	31 1/2	35 1/2	2	12	3/8	4 3/4	13 1/2	5/16	360	
140	420	24	0.500 (WT. XS)	С	22	1 1/4	31	35 1/2	2	11	3/8	4 3/4	13 1/2	5/16	24	0.688 (SCH. 40)	D	24	1 1/4	34 1/2	39	2	12	3/8	6 1/2	18	5/16	420	140
	300	24	0.375 (SCH. 20)	С	22	1 1/8	29	33	2	11	3/8	3 1/2	10	5/16	24	0.500 (WT. XS)	D	24	1 1/8	31 1/2	36	2	12	3/8	5	14	5/16	300	
160	300	24	0.500 (WT. XS)	D	22	1 1/4	30 1/2	35	2	11	3/8	4 1/2	12 1/2	5/16	24	0.500 (WT. XS)	E	20	1 1/4	32	36 1/2	2	10	3/8	5 1/4	14 1/2	5/16	300	160

				BA	SE CONN	ECTION			CTI	ON TABLE	<u> </u>		
SPAN	PANEL	M.A	ST		ANCHOR BO	DLTS		PLATE			STIFFEN	ERS	
(FEET)	AREA (S.F.)	DIAMETER (INCHES)	THICKNESS (INCHES)	NO.	DIAMETER (INCHES)	CIRCLE (INCHES)	DIAMETER (INCHES)	THICKNESS (INCHES)	NO.	THICKNESS (INCHES)	WIDTH (INCHES)	HEIGHT (INCHES)	WELD (INCHES)
60	1,040	24	0.500 (WT. XS)	10	2 1/4	31 1/2	39	2 1/2	10	3/8	6 1/2	18	5/16
	760	24	0.375 (SCH. 20)	10	2	32	39 1/2	2 1/4	10	3/8	6 3/4	19	5/16
	440	20	0.375 (SCH. 20)	8	2	27 1/2	35	2 1/2	8	3/8	6 1/2	18	5/16
80	1,000	24	0.500 (WT. XS)	12	2 1/4	31 1/2	40 1/2	2	12	3/8	7 1/4	20	5/16
	880	24	0.500 (WT. XS)	10	2 1/4	31 1/2	41	2 1/4	10	3/8	7 1/2	21	5/16
	600	24	0.375 (SCH. 20)	10	2	31 1/2	38 1/2	2 1/2	10	3/8	6 1/4	17 1/2	5/16
	360	20	0.375 (SCH. 20)	8	2	27	34	2 1/2	8	3/8	6	16 1/2	5/16
100	520	24	0.500 (WT. XS)	10	2	31 1/2	39	2 1/4	10	3/8	6 1/2	18	5/16
	280	20	0.375 (SCH. 20)	8	2	26 1/2	34	2 1/4	8	3/8	6	16 1/2	5/16
120	520	24	0.500 (WT. XS)	10	2 1/4	31 1/2	39	2 1/2	10	3/8	6 1/2	18	5/16
	360	24	0.375 (SCH. 20)	10	2	30 1/2	38	2	10	3/8	6	16 1/2	5/16
140	420	24	0.500 (WT. XS)	10	2	31 1/2	38	2 1/2	-	-	1	-	-
	300	24	0.375 (SCH. 20)	10	2	30 1/2	37 1/2	2	-	-	-	-	_
160	300	24	0.500 (WT. XS)	10	2	30 1/2	37	2 1/4	-	-	-	-	-

END CONNECTION COMPONENT SELECTION TABLE												
	PANEL		H.S. BOL			PLATE	STIFFENERS					
SPAN (FEET)	AREA (S.F.)	NO.	DIAMETER (INCHES)	CIRCLE (INCHES)	DIAMETER (INCHES)	THICKNESS (INCHES)	NO.	THICKNESS (INCHES)	WIDTH (INCHES)	HEIGHT (INCHES)	WELD (INCHES)	
60	1,040	12	1	27 1/2	31	2	-	-	-	-	-	
	760	12	1	27 1/2	31	2	-	-	-	-	-	
	440	11	1	23 1/2	27	2	-	-	-	-	-	
80	1,000	18	1 1/8	28	32	2	-	-	-	-	-	
	880	18	1 1/8	28	32	2	-	-	-	-	-	
	600	14	1 1/8	28	32	2	-	-	-	-	-	
	360	11	1 1/8	24	28	2	-	-	-	-	-	
100	520	20	1 1/8	28 1/2	32 1/2	2	10	3/8	3 1/4	9	5/16	
	280	12	1 1/8	24	28	2	-	-	-	-	-	
120	520	20	1 1/4	29	33 1/2	2	10	3/8	3 3/4	10 1/2	5/16	
	360	18	1 1/8	28	32	2	9	3/8	3	8 1/2	5/16	
140	420	20	1 1/4	30	34 1/2	2	10	3/8	4 1/4	12	5/16	
	300	20	1 1/8	28	32	2	10	3/8	3	8 1/2	5/16	
160	300	22	1 1/8	29	33	2	11	3/8	3 1/2	10	5/16	

MAST A	RM SEGMENT ARRANGEMENT TABLE
ARRANGEMENT	SEGMENT LENGTH / MAST ARM LENGTH
А	1.00
В	0.50 0.50
С	0.33 0.33 0.33
D	0.25 0.25 0.25 0.25
E	0.20 0.20 0.20 0.20 0.20
F	0.17 0.17 0.16 0.16 0.17 0.17

NOTES:

- 1. FOR ADDITIONAL NOTES, SEE SHEET 2.
- 2. FABRICATOR HAS THE OPTION TO ADD OR ELIMINATE SPLICES ALONG MAST ARM.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

MONOPIPE SIGN STRUCTURES

FRAME STRUCTURE SPANS UP TO 160'

MAST AND MAST ARM DETAILS - 2

RECOMMENDED AUG. 4, 2017

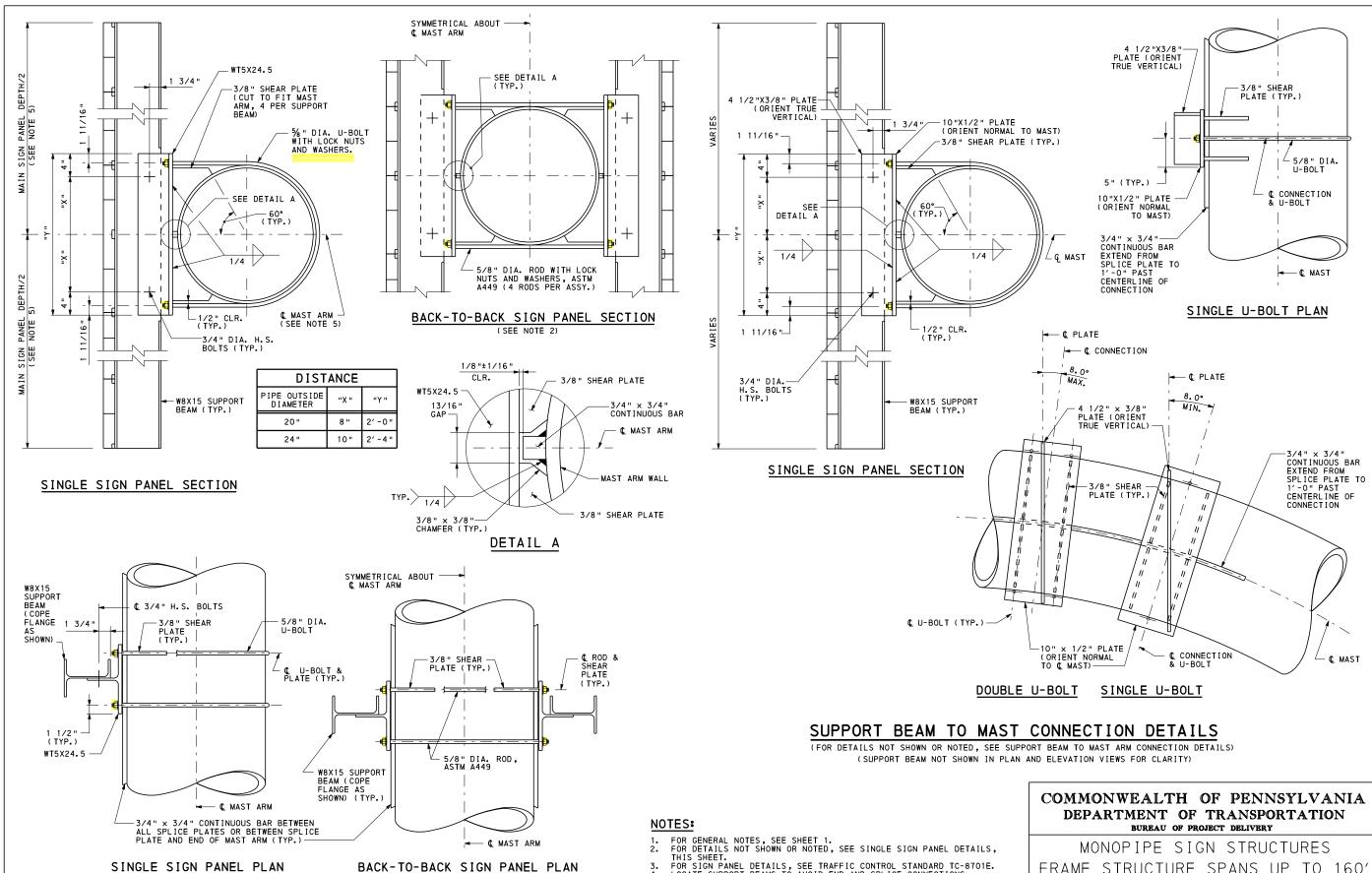
Thomas P Macioca
CHIEF BRIDGE ENGINEER

RECOMMENDED AUG. 4, 2017

SHEET 3 OF 5

BLOW STANGED
DIRECTOR, BUR. OF PROJECT DELIVERY

BC-747M



SUPPORT BEAM TO MAST ARM CONNECTION DETAILS

- FOR SIGN PANEL DETAILS, SEE TRAFFIC CONTROL STANDARD TC-8701E.
 LOCATE SUPPORT BEAMS TO AVOID END AND SPLICE CONNECTIONS.
 MAXIMUM SPACING = 5'-0". MAXIMUM DISTANCE TO PANEL EDGE =
- 2'-5".
 SIGN PANEL SUPPORT BEAM DETAILS GIVEN ON THIS SHEET ARE ONLY VALID FOR SIGNS WHERE THE HORIZONTAL CENTERLINE OF THE SIGN PANEL IS AT THE SAME LOCATION AS THE CENTERLINE OF MAST ARM. PROVIDE SIGN PANEL SUPPORT BEAM DETAILS ON SHOP DRAWINGS WHEN THE HORIZONTAL CENTERLINE OF THE SIGN PANEL IS NOT AT THE SAME LOCATION AS THE CENTERLINE OF THE MAST ARM.

COMMONWEALTH OF PENNSYLVANIA

FRAME STRUCTURE SPANS UP TO 160' AND CANTILEVER MONOPIPE STRUCTURE STRUT LENGTHS UP TO 27'

SIGN PANEL SUPPORT BEAM DETAILS

RECOMMENDED	AUG. 4, 2017	RECOMMENDED	AUG. 4, 2017	SHEET 4 OF 5
Thoma P Macioca CHIEF BRIDGE ENGINEER			STANSON DELIVERY	BC-747M

CAISSON COMPONENT SELECTION TABLE FRAME STRUCTURES											
SPAN PANEL		CAISSON	CA	ISSON	EMBED	EET)	VERT.	REINF.			
(FEET)	(S.F.)	DIAMETER	SOIL	MAX. GROUND SLOPE 8:1 4:1 2:1 1.5:1				NO.	SIZE		
60	1,040	54	С	23.0	24.0	25.0	25.5	18	#8		
"	. , ,	•	G	19.5	20.0	21.5					
	760	48	G	22.0 18.5	23.0 19.0	24.0	24.5	16	#8		
			C	18.5	19.0	19.5	20.0				
	440	48	G	16.0	16.5	18.0	-	13	#8		
80	1,000	60	С	24.5	25.5	26.5	27.5	20	#8		
l °°	1,000	60	G	21.0	22.0	22.5	-	20			
	880	54	С	24.5	25.5	27.5	29.0	19	#8		
	880		G	20.5	21.0	22.5	-		0		
	600	48	С	22.0	22.5	23.5	24.5	16	#8		
		,,,	G	18.5	19.0	21.0	-	'			
	360	48	С	18.0	18.5	19.5	20.0	12	#8		
			G	16.0	16.5	18.0					
100	520	48	G	22.0	23.0	24.0	24.5	16	#8		
			C	18.5	19.0	21.0	19.0				
	280	48	G	15.5	16.0	17.0	19.0	12	#8		
	-				C	23.5	24.5	26.5	28.5		
120	520	48	G	19.5	20.0	22.0	-	17	#8		
			c	20.0	20.5	21.5	22.0				
	360	48	G	17.5	18.0	19.5	-	14	#8		
140	400	48*	С	23.5	24.0	26.0	28.0	1,0	#8		
140	420		G	19.5	20.0	22.0	-	16	"8		
	300	48	С	20.0	20.5	21.5	22.0	14	#8		
	300	70	G	17.0	17.5	19.5	-				
160	300	48	С	21.5	22.0	23.5	24.0	15	#8		
		'0	G	18.0	18.5	20.5	-				

CAISSON COMPONENT SELECTION TABLE CANTILEVER STRUCTURES

SPAN (FEET)

27

350

250

48

48

CAISSON EMBEDMENT (FEET) VERT. REINF.

NO.

15

12

SIZE

#8

#8

MAX. GROUND SLOPE

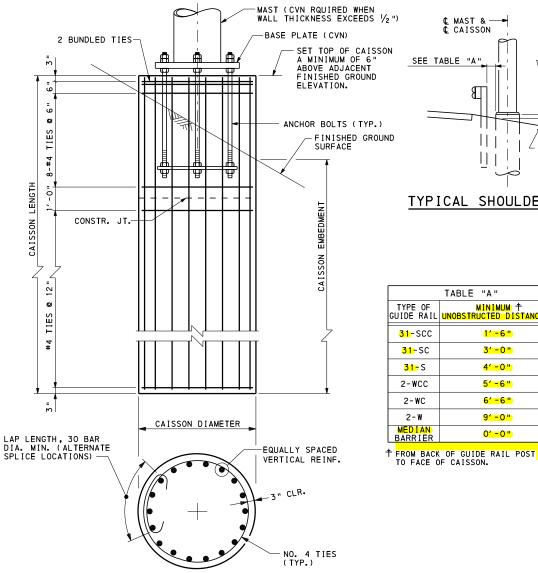
8:1 4:1 2:1 1.5:1

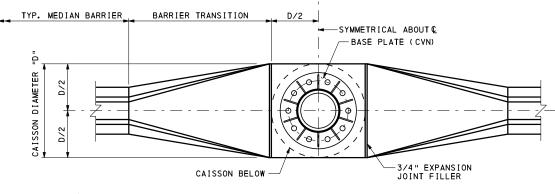
20.5 21.0 21.5 22.0

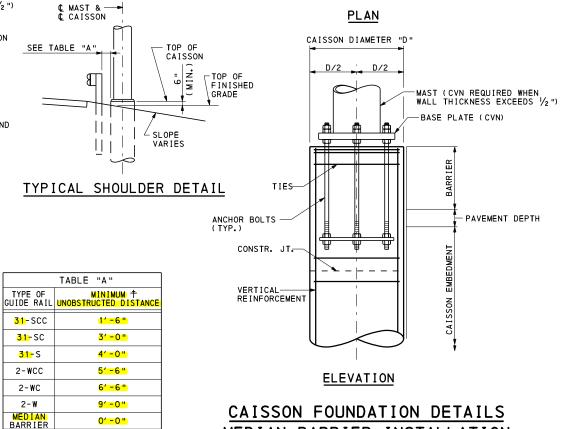
17.5 18.0 18.5 19.0 19.0 19.0 19.0 -

G 21.0 21.0 21.0

OPTIONAL CAISSON COMPONENT SELECTION TABLE FRAME STRUCTURES - MEDIAN BARRIER INSTALLATION										
SPAN	PANEL CAISSON CAISSON EMBEDMENT (FEET)							VERT.	REINF.	
(FEET)	AREA (S.F.)	DIAMETER (INCHES)	SOIL	MAX. GROUND SLOPE				NO.	SIZE	
	(3.6.)	(INCHES)	3011	8: 1	4: 1	2:1	1.5:1	NO.	SIZE	
60	1.040	48	С	24.0	25.0	27.0	28.5	18	#8	
80	1,040	70	G	20.0	20.5	22.5	-			
80	1.000	48	С	27.0	28.0	31.0	33.0	20	#8	
**	1,000	40	G	21.5	22.0	24.5	-			
	880	48	С	25.5	27.0	29.5	31.0	19	#8	
	880	40	G	21.0	21.5	23.5	-	19		
100	1 000	40	С	29.5	31.5	35.0	37.0	00	#8	
100	1,000	48*	G	23.0	23.5	27.0	-	22		
	740	48	С	25.5	27.0	29.5	31.5	19	#8	
	140	40	G	21.0	21.5	23.5	-	19	_ ~6	







CAISSON FOUNDATION DETAILS ROADSIDE INSTALLATION

CAISSON SELECTION NOTES:

- 1. SOIL TYPE "C" IS SOFT COHESIVE SOIL AND SOIL TYPE "G" IS LOOSE GRANULAR SOIL, SEE DESIGN NOTE 3 ON SHEET 1 OF 6.
- 2. *REPRESENTS THAT A BELLED CAISSON IS REQUIRED FOR THE SOFT COHESIVE SOIL, SEE SHEET 1 OF 6.

CAISSON DRILLING AND INSTALLATION NOTES:

- CONTACT THE STRUCTURE CONTROL ENGINEER IF ANY OF THE FOLLOWING SOIL CONDITIONS ARE ENCOUNTERED DURING DRILLING: THE SOIL HAS A HIGH ORGANIC CONTENT OR CONSISTS OF SATURATED SILT AND CLAY.
- THE SITE WILL NOT SUPPORT THE WEIGHT OF THE DRILLING RIG. FIRM BEDROCK IS ENCOUNTERED.
- 2. CONSTRUCT DRILLED CAISSONS PER PUB. 408, SECTION 1006.3
 3. SUBMIT AN AS-BUILT SURVEY OF EACH CAISSON FOUNDATION TO THE REPRESENTATIVE THAT IDENTIFIES ANCHOR BOLT LOCATION, ANCHOR BOLT ORIENTATION, DISTANCE BETWEEN ANCHOR BOLT GROUPS (FOR FRAME STRUCTURES), TOP OF ANCHOR BOLT ELEVATIONS, TOP OF CAISSON ELEVATIONS, AND ADJACENT FINISHED GROUND ELEVATIONS. INCLUDE A COPY OF THE SURVEY NOTES. RECONCILE ANY
 DIFFERENCES BETWEEN SURVEY INFORMATION AND DATA ON THE APPROVED SHOP DRAWINGS. SUBMIT ALL PROPOSED ADJUSTMENTS OR MODIFICATIONS TO THE REPRESENTATIVE FOR ACCEPTANCE.

NOTES:

MEDIAN BARRIER INSTALLATION

(SEE ROADSIDE INSTALLATION, THIS SHEET, FOR ADDITIONAL INFORMATION)
(10 BOLT CONFIGURATION SHOWN)

FOR GENERAL NOTES, SEE SHEET 1.
FOR ANCHOR BOLT INFORMATION, SEE SHEET 2
(CANTILEVER STRUCTURES) OR SHEET 4 (FRAME

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

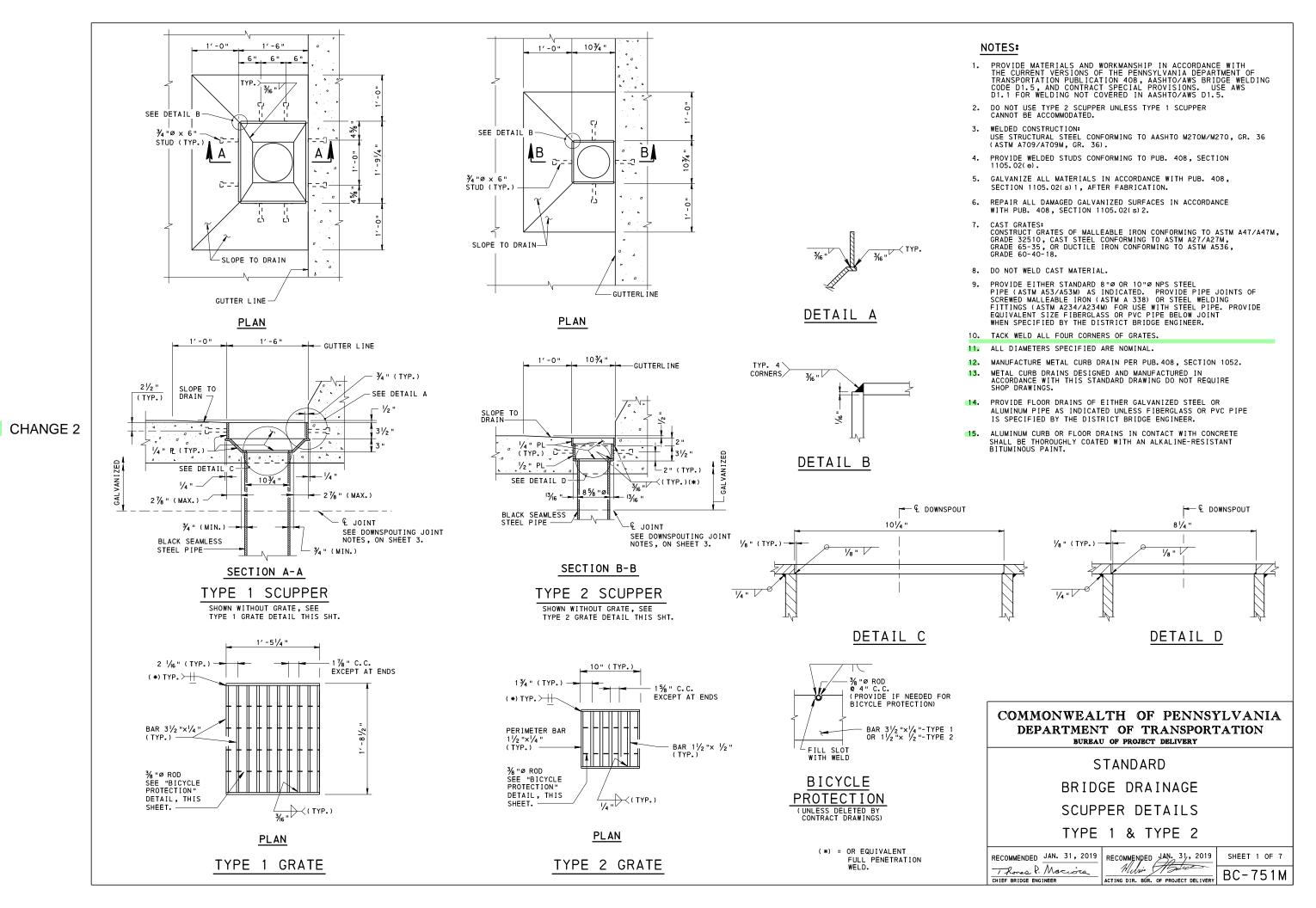
MONOPIPE SIGN STRUCTURES FRAME STRUCTURE SPANS UP TO 160' AND CANTILEVER MONOPIPE STRUCTURE STRUT LENGTHS UP TO 27'

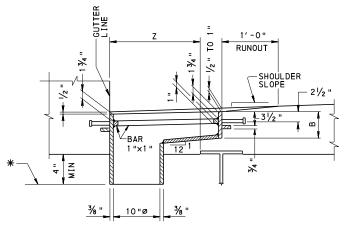
FOUNDATION DETAILS

RECOMMENDED AUG. 4, 2017 Thoma P Macioca

RECOMMENDED AUG. 4, 2017 Bun SThomps DIRECTOR, BUR. OF PROJECT DELIVERY BC-747M

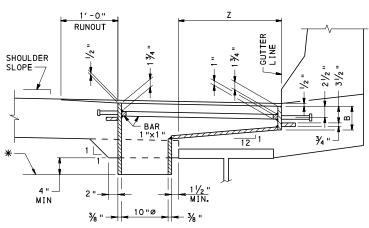
SHEET 5 OF 5





- 1. USE CONTINUOUS FILLET WELD FOR INSIDE AND OUTSIDE. 1/4 " MIN. SIZE.

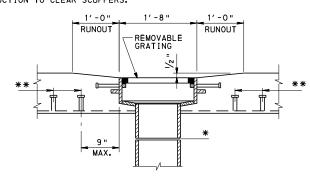
SECTION C-C (TYPE A OR B SCUPPER)



- 1. USE CONTINUOUS FILLET WELD FOR INSIDE AND OUTSIDE. 1/4" MIN. SIZE.
 2. GRATING NOT SHOWN

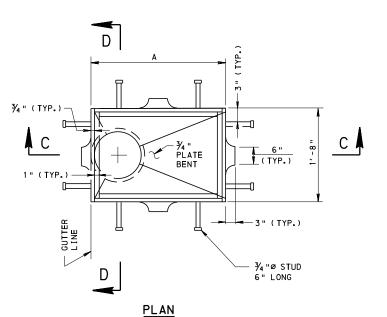
SECTION C-C (TYPE C OR D SCUPPER)

- * SEE DOWNSPOUTING JOINT NOTES ON SHEET 3.
- ** RESPACE SHEAR CONNECTORS ON COMPOSITE CONSTRUCTION TO CLEAR SCUPPERS.



SECTION D-D

WELDED SCUPPER DETAILS



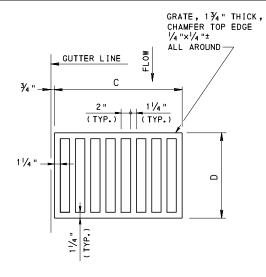
NOTE: MILL OR CUT OPENING IN SOLID PLATE AS INDICATED. GUTTER LINE GRATE, 13/4" THICK, CHAMFER TOP EDGE 1/4 "×1/4 "± ALL AROUND (TYP.) BICYCLE

PROTECTION ROD,

3/8 "Ø, SEE DETAIL ON SHEET 1

ALTERNATE STRUCTURAL STEEL SCUPPER GRATE

PLATE



CAST GRATING PLAN

PROVIDE ALTERNATE STRUCTURAL
STEEL GRATE WHEN BICYCLE PROTECTION
IS REQUIRED, SEE DETAIL THIS SHEET.

THE SCUPPERS DETAILED ON THIS SHEET ARE FOR GUIDANCE ONLY IN REHABILITATION PROJECTS. DO NOT USE IN NEW CONSTRUCTION UNLESS SPECIFICALLY APPROVED BY THE DISTRICT BRIDGE ENGINEER.

TABLE I

SPECIFY TYPE A SCUPPER FOR Z UP TO 1'-6"

SPECIFY TYPE B SCUPPER FOR Z OVER 1'-6" TO 3'-0"

SPECIFY TYPE C SCUPPER FOR Z UP TO 1'-101/2"

SPECIFY TYPE D SCUPPER FOR Z OVER 1'-101/2" TO 2'-81/4"

TABLE II - U.S. CUSTOMARY UNITS										
		APPROXIMA	MATE WEIGHT							
	Α	В	С	D	SCUPPER	W/ GRATE				
TYPE A SCUPPER	1' -101/2 "	6"	1′-8¾"	1′-61/4"	295 LB	395 LB				
TYPE B SCUPPER	2'-81/4"	5 "	2'-61/2"	1'-61/4"	380 LB	525 LB				
TYPE C SCUPPER	2' -11 1/2 "	5 "	2'-9¾"	1′-61/4"	400 LB	545 LB				
TYPE D SCUPPER	3′ -91/4 "	5 "	3' -71/2 "	1'-61/4"	465 LB	655 LB				

SCUPPER WEIGHTS LISTED ARE FOR A SCUPPER ASSEMBLY DEPTH OF 1'-3".

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD BRIDGE DRAINAGE SCUPPER DETAILS TYPE A, B, C & D

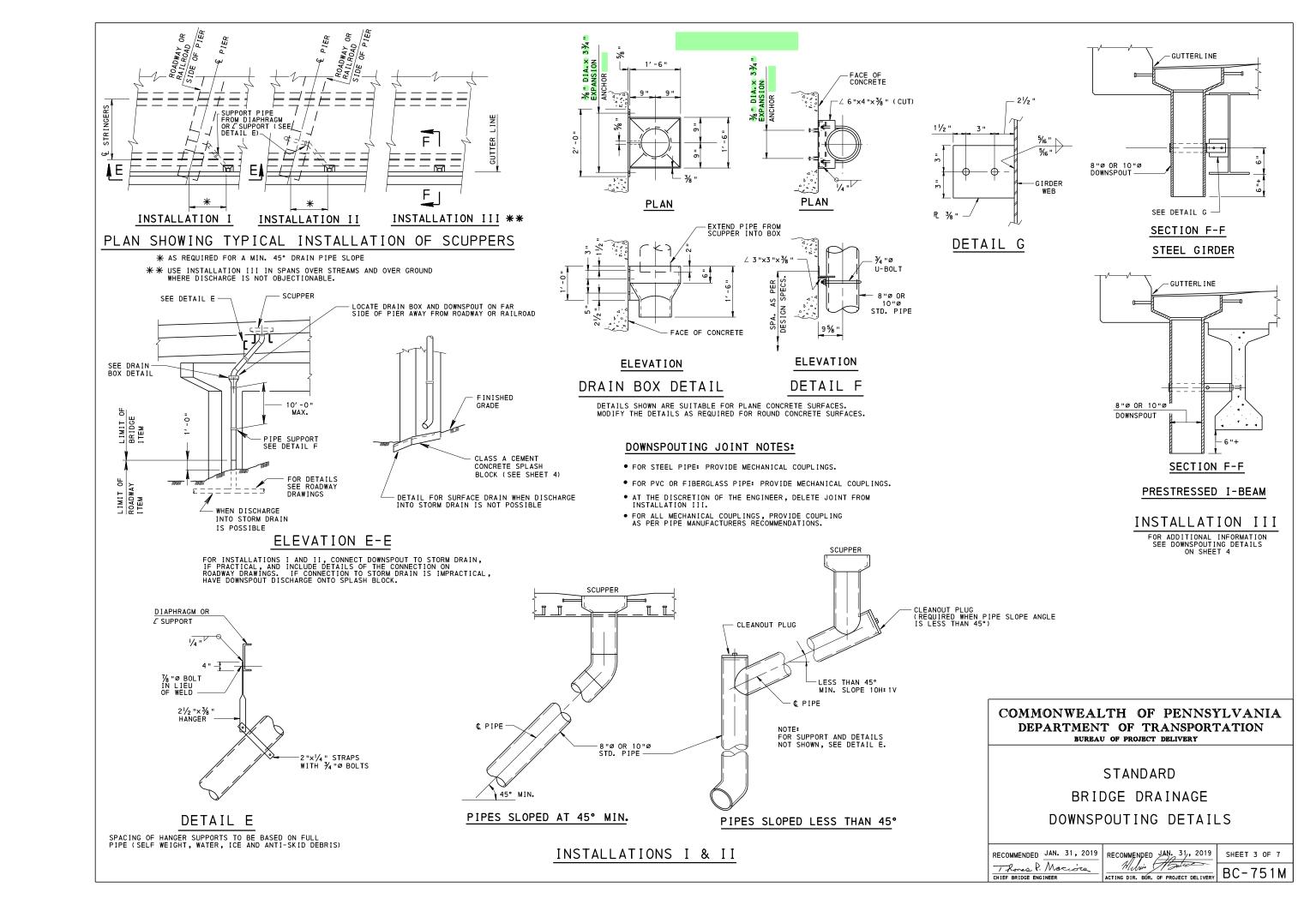
CHIEF BRIDGE ENGINEER

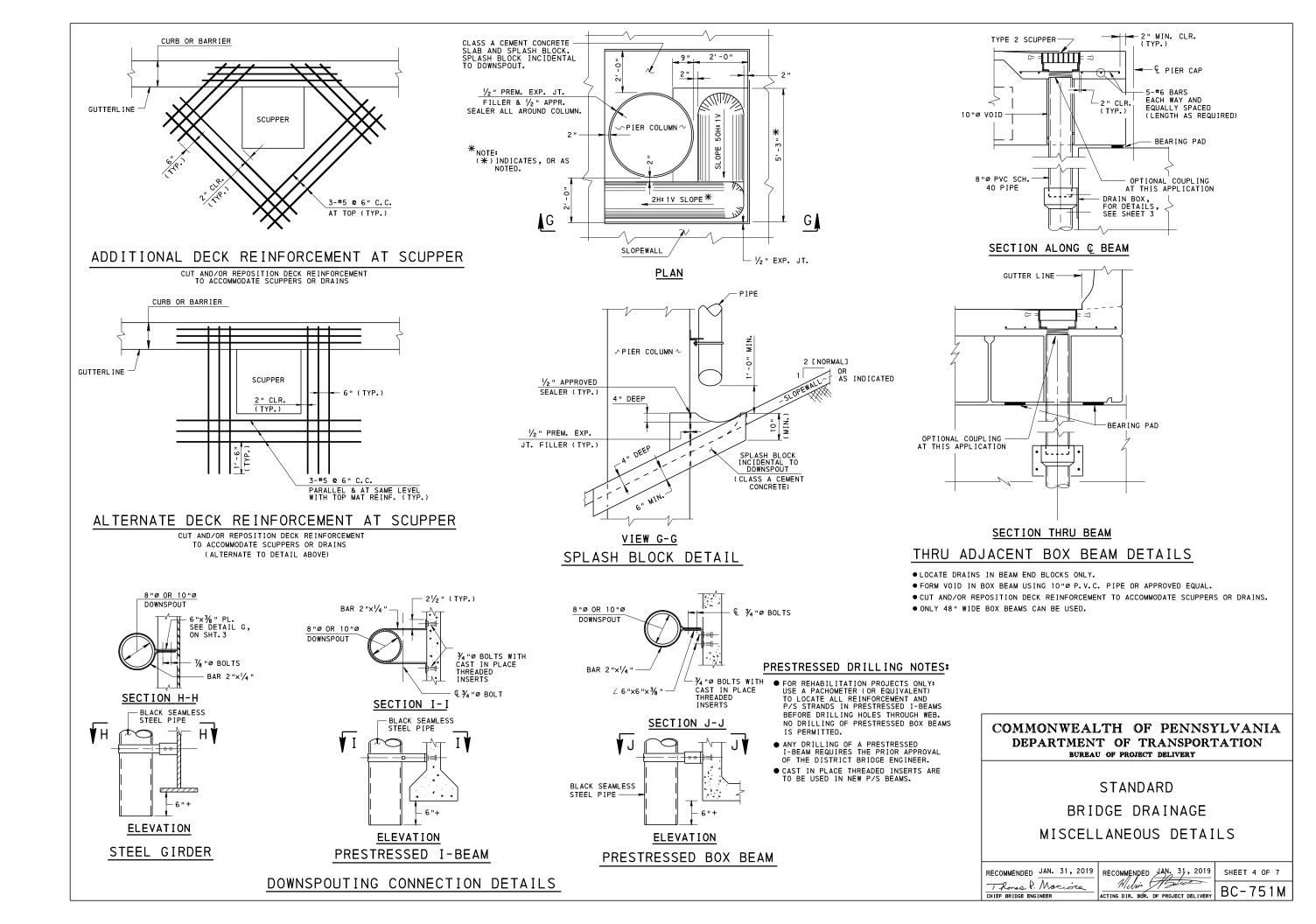
RECOMMENDED JAN. 31, 2019

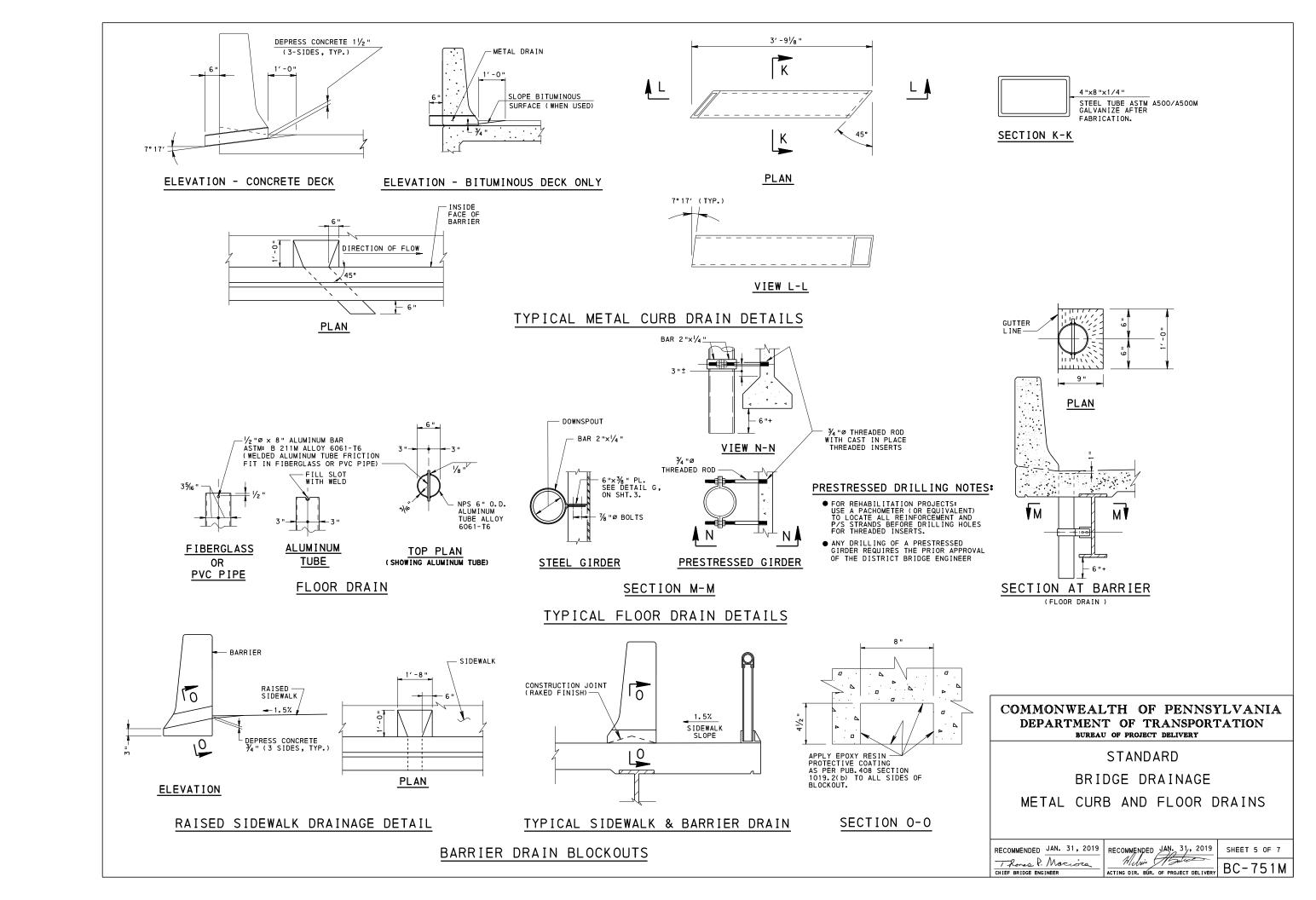
RECOMMENDED JAN. 31, 2019

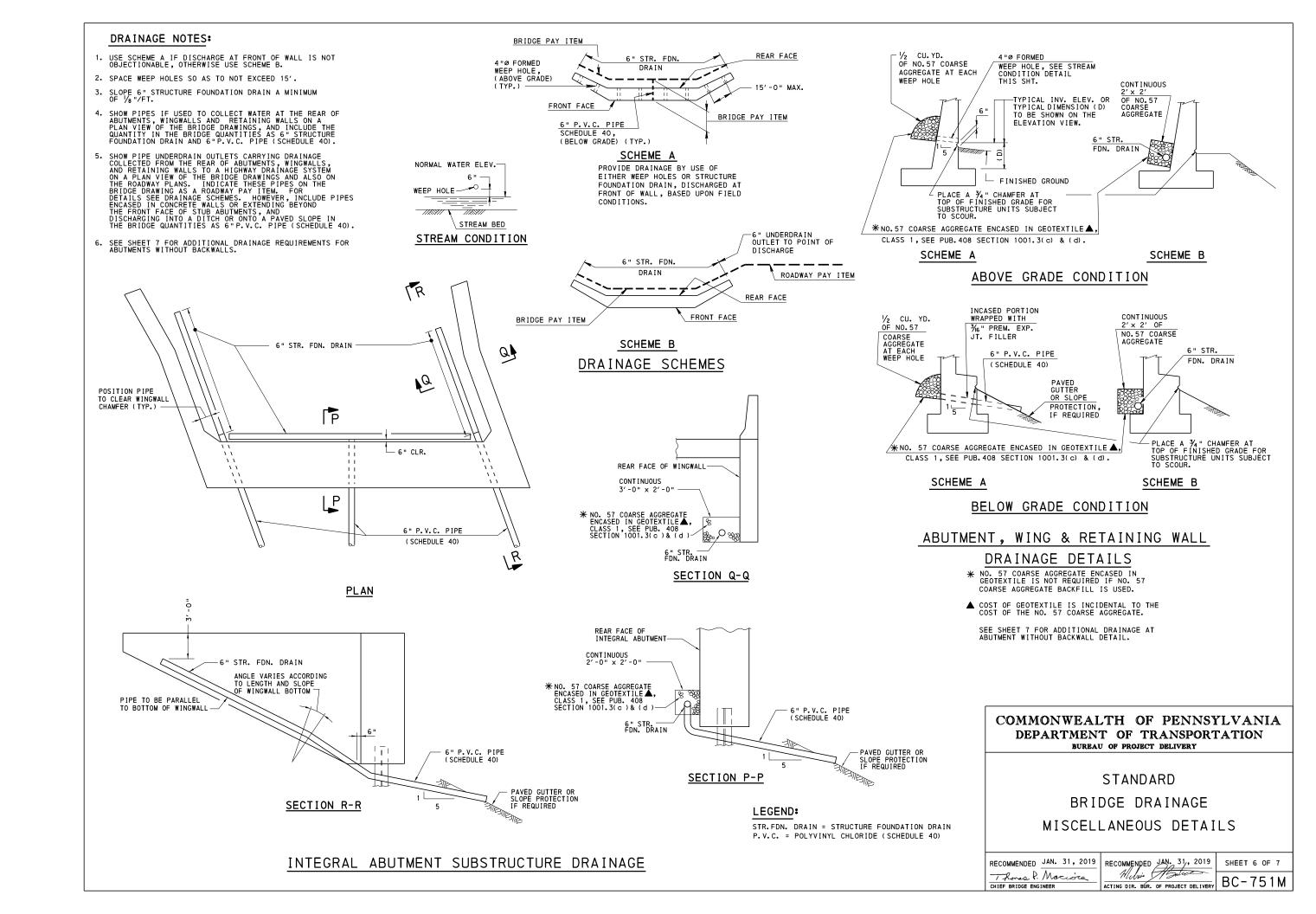
RECOMMENDED JAN. 31, 2019

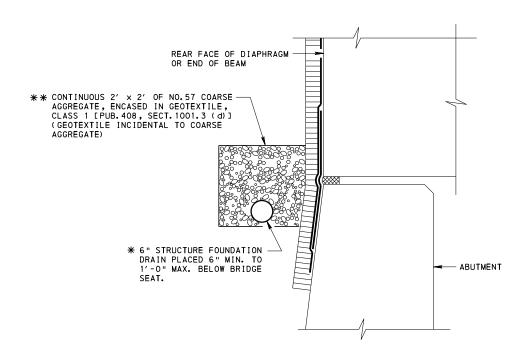
Millie Haria ACTING DIR. BUR. OF PROJECT DELIVERY BC-751M











ADDITIONAL DRAINAGE DETAIL AT ABUTMENT WITHOUT BACKWALL

- * SLOPE FOUNDATION DRAIN A MINIMUM OF 1/4" PER FOOT. CONNECT TO LOWER STRUCTURE FOUNDATION DRAIN OR OUTLET TO GRADE SIMILAR TO SECTION P-P ON SHEET 6.
- ** NO. 57 COARSE AGGREGATE ENCASED IN GEOTEXTILE IS NOT REQUIRED IF NO. 57 COARSE AGGREGATE BACKFILL IS USED.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD BRIDGE DRAINAGE MISCELLANEOUS DETAILS

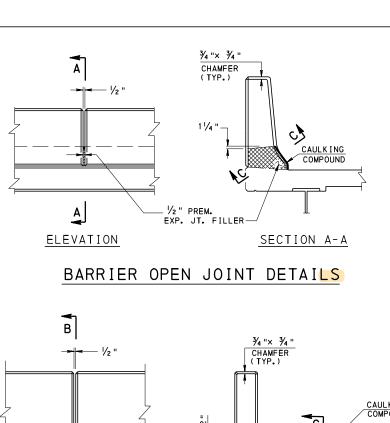
RECOMMENDED JAN. 31, 2019

Thoras P. Marcira
CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019

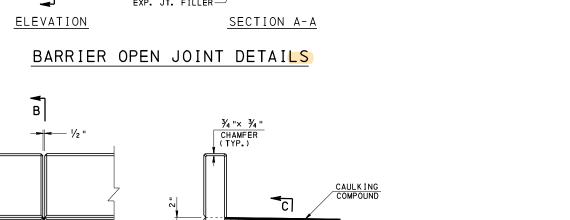
Millian Francisca
ACTING DIR. BUR. OF PROJECT DELIVERY

BC-751M

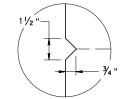


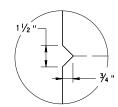


1/2 " PREM. EXP. JT. FILLER —



SECTION B-B





V-NOTCH DETAIL

NOTES:

OPEN JOINT DETAILS APPLY TO ALL TYPE OF BARRIERS. MODIFIED DEFLECTION JOINTS APPLY TO THE FOLLOWING TYPES OF BARRIERS EXCEPT WHEN SUPPORTING A SOUND BARRIER: 32", 42" AND 45" F-SHAPE CONCRETE BARRIERS, 32" AND 50" SPLIT CONCRETE MEDIAN BARRIERS, 32" AND 50" CONCRETE MEDIAN BARRIERS, AND 32" AND 42" VERTICAL WALL CONCRETE BARRIERS.

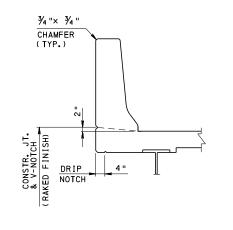
PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH PUBLICATION 408.

ALL REINFORCEMENT STEEL BARS SHOWN MEET THE REQUIREMENTS OF ASTM A 615, A 996, OR A 706.

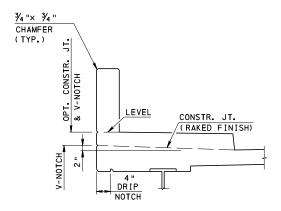
FOR LOCATION OF CONSTRUCTION JOINTS AND OPEN JOINTS, REFER TO DESIGN DRAWINGS.

REMOVE FORMS AND BULKHEAD MATERIALS AT CONSTRUCTION JOINTS IN ACCORDANCE WITH PUBLICATION 408 SECTION 1001.3(q)1. APPLY EPOXY BONDING COMPOUND CONFORMING TO PUBLICATION 408 SECTION 1001.3(m) BEFORE PLACING FRESH CONCRETE ADJACENT TO PREVIOUSLY PLACED CONCRETE.

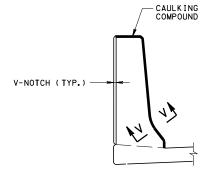
- 6. PROVIDE PREMOLDED EXPANSION JOINT FILLER CONFORMING TO PUBLICATION 408, SECTION 705.1.
- PROVIDE JOINT BACKING MATERIAL CONFORMING TO PUBLICATION 408, SECTION 705.8.
- 8. PROVIDE CAULKING COMPOUND CONFORMING TO PUBLICATION 408, SECTION 705.7 (b).
- 9. FOR DRIP NOTCH DETAILS, SEE BC-775M.
- 10. PROVIDE HIGH MOLECULAR WEIGHT METHACRYLATE (HMWM)
 CONSTRUCTION JOINT FILLER AND SEALER CONFORMING
 TO PUBLICATION 408, SECTION 1090.2(e).



SLOPED BARRIER DETAIL



ALTERNATE SIDEWALK DETAIL

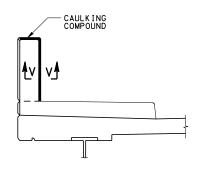


ELEVATION

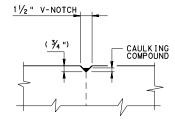
CHANGE 3

CHANGE 4

SLOPED BARRIER



ALTERNATE SIDEWALK



TOOLED EDGES

½ "PREM. EXP.
JOINT FILLER

SECTION C-C

JOINT

BACKING MATERIAL

CAULKING COMPOUND

SECTION V-V

NOTE: LONGITUDINAL REINFORCEMENT IS CONTINUOUS THROUGH THE JOINT

MODIFIED DEFLECTION JOINT DETAILS

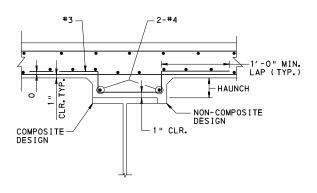
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD CONCRETE DECK SLAB DETAILS

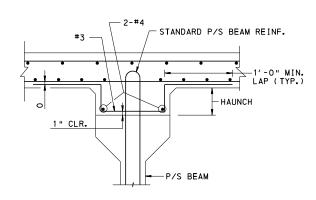
BC-775M MISCELLANEOUS PRESTRESS DETAILS BC-788M TYPICAL WATERPROOFING AND EXPANSION DETAILS REFERENCE DRAWINGS

RECOMMENDED NOV. 23, 2022 Havin E. Hray
CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 1 OF 3 BC-752M



2-#4 — -STANDARD P/S BEAM REINF. - HAUNCH -P/S BEAM



STEEL BEAMS

PROVIDE WHEN HAUNCH THICKNESS IS 3" OR GREATER ANYWHERE ACROSS WIDTH OF HAUNCH

AASHTO TYPE P/S CONC. I-BEAM (P/S SPREAD BOX BEAM SIMILAR)

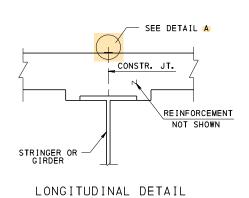
PROVIDE WHEN HAUNCH THICKNESS IS 5" OR GREATER ANYWHERE ACROSS WIDTH OF HAUNCH

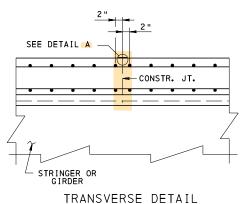
P/S CONC. PA I-BEAM & PA BULB-TEAM BEAM

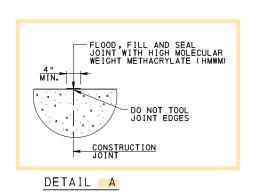
PROVIDE WHEN SIP FORMS ARE PRESENT AND HAUNCH THICKNESS IS 5" OR GREATER ANYWHERE ACROSS WIDTH OF HAUNCH OR WHEN SIP FORMS ARE NOT PRESENT AND THE HAUNCH THICKNESS IS 3" OR GREATER ANYWHERE ACROSS WIDTH OF HAUNCH.

HAUNCH REINFORCEMENT DETAILS

- EPOXY COAT ALL REINFORCEMENT IN DECK SLAB (INCLUDES HAUNCH REINFORCEMENT AND REINFORCEMENT PROJECTING FROM P/S CONC. BEAMS)
- 2. IN NEGATIVE MOMENT REGIONS, DO NOT SPLICE LONGITUDINAL REINFORCEMENT OVER PIERS.
- 3. FOR DECK TOP REINFORCEMENT MAT: TRANSVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.







CONSTRUCTION JOINT DETAILS

- FOR STAGED CONSTRUCTION, FLOOD, FILL AND SEAL LONGITUDINAL AND TRANSVERSE CONSTRUCTION JOINTS IN BRIDGE DECKS, APPROACH SLABS, AND LATEX MODIFIED CONCRETE (LMC) OVERLAYS WITH HIGH MOLECULAR WEIGHT METHACRYLATE (HMWM) AS SPECIFIED IN PUBLICATION 408, SECTION 1090.3(e).
- 2. PROVIDE CONSTRUCTION JOINTS ONLY WHERE INDICATED. DO NOT ADD LONGITUDINAL OR TRANSVERSE CONSTRUCTION JOINTS.

- LONGITIUDINAL SLAB REINFORCEMENT (TYP.) ALTERNATE TRANSVERSE CONSTRUCTION AND CRACK CONTROL JOINT

SEE DETAIL A

-RIBBED WATER STOP

SMOOTH DOWEL

BAR (TYP.)

- 1. FOR CONTINUOUS BRIDGES USING ALTERNATE PLACEMENT SEQUENCE, SEE BD-660M.
- FOR DECK TOP REINFORCEMENT MAT: TRANSVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.

TRANS. CONSTR. JOINT -

TOP OF DECK SLAB

APPLY BOND

BREAKER (TYP.)

3. DOWELS ARE SAME NOMINAL SIZE AS LAPPED BAR AND 3 FT. LONG.

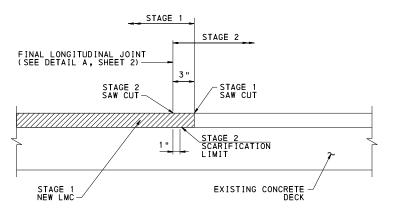
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD CONCRETE DECK SLAB DETAILS

RECOMMENDED NOV. 23, 2022 Havin E. Hray

HIEF ENGINEER, HIGHWAY ADMIN

SHEET 2 OF 3 BC-752M



STAGE 1

1. SAWCUT 3 INCHES BEYOND (TOWARDS STAGE 2) FINAL LOCATION OF LONGITUDINAL CONSTRUCTION JOINT.

2. PERFORM SCARIFICATION.

3. PERFORM HYDRODEMOLITION WHEN INDICATED.

4. PLACE LMC WEARING SURFACE TO STAGE 1 SAWCUT LOCATION.

- STAGE 2

 1. SAWCUT STAGE 1 LMC WEARING SURFACE AT FINAL LOCATION OF LONGITUDINAL CONSTRUCTION JOINT.

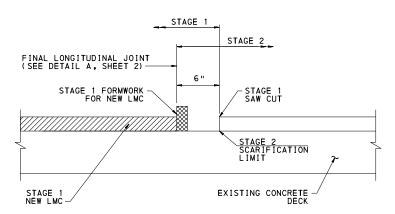
 2. PERFORM SCARIFICATION TO 1 INCH FROM FINAL LOCATION OF LONGITUDINAL CONSTRUCTION JOINT. USE PNEUMATIC HAMMERS NOT EXCEEDING 30 FT-LBS TO REMOVE REMAINING 1 INCH OF STAGE 1 LMC WEARING SURFACE.

 3. PERFORM HYDRODEMOLITION WHEN INDICATED.

 4. WATER BLAST VERTICAL FACE OF JOINT. PROVIDE WATER BLASTING EQUIPMENT WITH A MINIMUM RATED CAPACITY OF 5,000 PSI.

 5. PLACE LMC WEARING SURFACE FLUSH TO STAGE 1. DO NOT TOOL JOINT EDGE.

OPTION 1



- STAGE 1

 1. SAWCUT 6 INCHES BEYOND (TOWARDS STAGE 2) FINAL LOCATION OF LONGITUDINAL CONSTRUCTION JOINT.

 2. PERFORM SCARIFICATION

 3. PERFORM HYDRODEMOLITION WHEN INDICATED.

 4. INSTALL FORMWORK ALONG FINAL LOCATION OF LONGITUDINAL CONSTRUCTION JOINT AND PLACE LMC WEARING SURFACE.

- STAGE 2

 1. PERFORM SCARIFICATION.

 2. PERFORM HYDRODEMOLITION WHEN INDICATED.

 3. REMOVE STAGE 1 FORMWORK AND WATER BLAST VERTICAL FACE OF JOINT. PROVIDE WATER BLASTING EQUIPMENT WITH A MINIMUM RATED CAPACITY OF 5,000 PSI.

 3. PLACE LMC WEARING SURFACE FLUSH TO STAGE 1. DO NOT TOOL JOINT EDGE.

OPTION 2

LATEX MODIFIED CONCRETE (LMC) WEARING SURFACE CONSTRUCTION JOINT PREPARATION

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

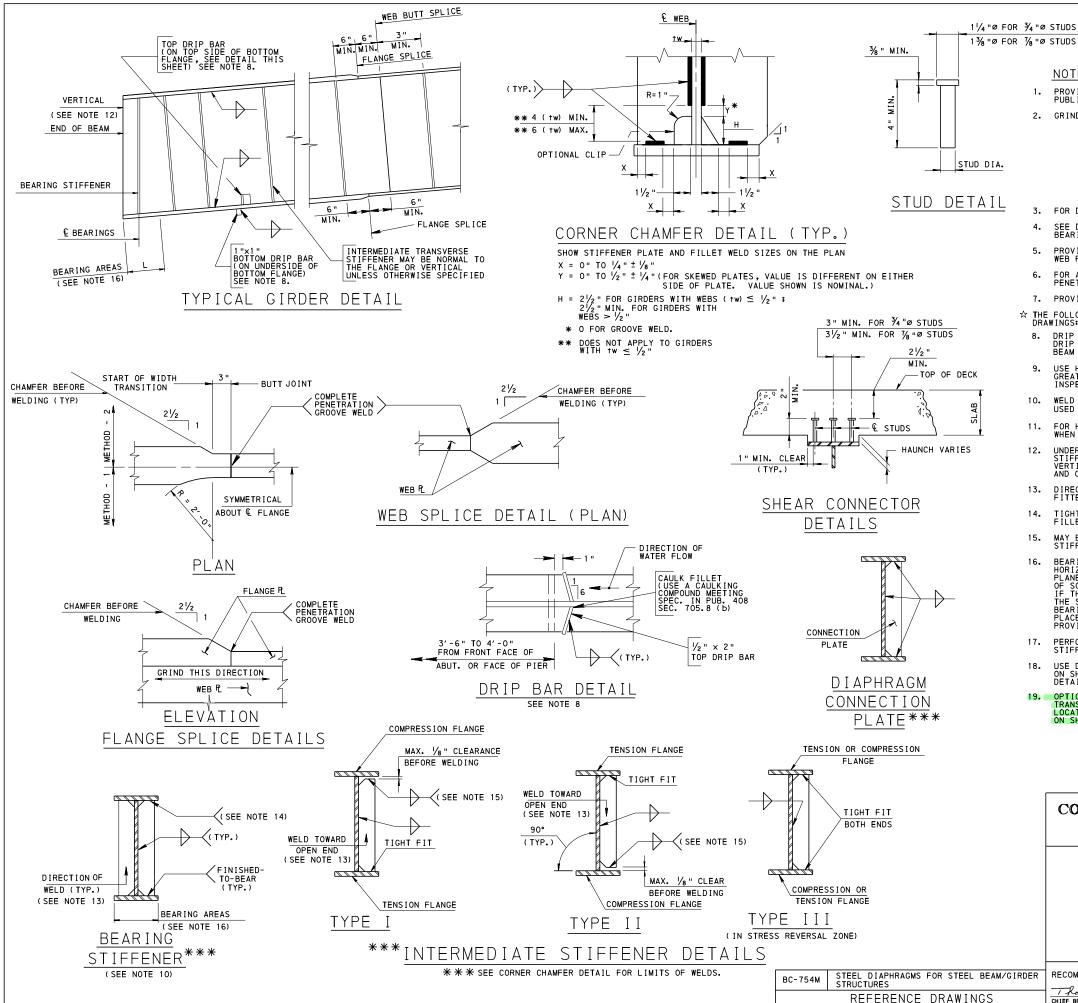
STANDARD CONCRETE DECK SLAB DETAILS

RECOMMENDED NOV. 23, 2022

LAWN E. HAWY

CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 3 OF 3 BC-752M



CHANGE 2

NOTES:

- PROVIDE MATERIALS AND WORKMANSHIP CONFORMING TO PUBLICATION 408, AND AASHTO/AWS D1.5 WELDING CODE.
- 2. GRIND GROOVE WELDED SPLICES AS FOLLOWS:
- GRIND FLUSH ALL WELDS SUBJECTED TO NON-DESTRUCTIVE TESTING.
 - GRIND FLUSH ALL EXPOSED WELDS OF FASCIA WEBS AND FASCIA BOTTOM FLANGES.
 - GRIND SMOOTH ALL OTHER WELDS THAT EXCEED 1/8 "
 REINFORCEMENT AND WHERE REQUIRED TO PRODUCE A GRADUAL
 TRANSITION TO THE PLANE OF THE BASE METAL ALONG WITH
 REMOVING ANY DISCONTINUITIES.
- 3. FOR DIAPHRAGM CONNECTION PLATE DETAILS, SEE BC-754M.
- SEE DESIGN DRAWINGS FOR LOCATION OF LONGITUDINAL, BEARING, AND INTERMEDIATE TRANSVERSE STIFFENERS.
- PROVIDE ALL INTERMEDIATE STIFFENERS ON OPPOSITE SIDE OF WEB FROM LONGITUDINAL STIFFENER.
- FOR ALL GROOVE WELDS USE PREQUALIFIED COMPLETE PENETRATION WELDS.
- 7. PROVIDE CONTROLLED PEENING IF INDICATED OR SPECIFIED.
- $\stackrel{\mbox{\tiny Δ}}{\sim}$ The following notes are to be used when referenced on the drawings:
- DRIP BARS TO BE PROVIDED FOR WEATHERING STEEL GIRDERS ONLY. DRIP BARS SHOULD NOT BE PROVIDED AT THE ABUTMENT WHEN THE BEAM SLOPE IS AWAY FROM THE ABUTMENT.
- USE HANDRAILS AS SHOWN ON GIRDERS 6'-0" DEEP OR GREATER. REFER TO DESIGN MANUAL PART 4 FOR ADDITIONAL INSPECTION DEVICES.
- WELD AT TOP AND BOTTOM WHEN THE BEARING STIFFNER IS USED AS CONNECTION PLATE.
- FOR HANDRAIL, USE CLIP ANGLES OR PLATES BETWEEN STIFFENERS WHEN STIFFENERS ARE SPACED AT 8^{\prime} -6" OR GREATER.
- UNDER FULL DEAD LOAD BEAM ENDS AND ALL BEARING STIFFENERS, INCLUDING BEARING STIFFENERS AT PIERS, ARE VERTICAL TO WITHIN APPLICABLE AASHTO/AWS FABRICATION AND CONSTRUCTION TOLERANCES.
- DIRECTION OF WELDS IS NOT APPLICABLE IF STIFFENERS ARE FITTED WITH TACK WELDS.
- TIGHT FIT AT TENSION FLANGES AT INTERIOR SUPPORTS. FILLET WELDS OVER END SUPPORTS
- MAY BE TIGHT FITTED AT THE COMPRESSION FLANGE WHEN STIFFENERS ARE PLACED IN PAIRS, ONE ON EACH SIDE OF THE WEB.
- BEARING AREAS: PROVIDE BOTTOM FLANGE IN A TRUE HORIZONTAL PLANE IN TRANSVERSE DIRECTION AND IN A TRUE PLANE LONGITUDINALLY OVER DIMENSION "L", WHERE L = WIDTH OF SOLE PLATE + 6" AHEAD AND BACK, WHERE APPLICABLE. IF THE SOLE PLATE IS WELDED TO THE BOTTOM FLANGE PROVIDE THE SOLE PLATE MEETING THE SAME FLATNESS REQUIREMENTS. EACH BEARING MUST BE STRESSED UNIFORMLY AFTER ALL DEAD LOAD IS PLACED. MAKE NECESSARY SHOP AND/OR FIELD ADJUSTMENTS TO PROVIDE UNIFORM BEARING STRESS UNDER ALL DEAD LOADS.
- 17. PERFORM NON-DESTRUCTIVE TESTING (NDT) ON LONGITUDINAL STIFFENER BUTT WELDS PRIOR TO ATTACHMENT TO GIRDER WEB.
- USE DETAILS A & B ON SHEET 2 IN COMPRESSION ZONE. USE DETAIL C ON SHEET 2 IN REVERSAL ZONE. NO FATIGUE CATEGORY D, E, OR E' DETAILS ALLOWED IN TENSION ZONE.
- 19. OPTIONAL METHOD FOR STUD LOCATION IS TO STAGGER STUD ROWS TRANSVERSELY ACROSS DECK, BY NOT PLACING STUDS AT SAME LOCATION ON ALL THE BEAMS. SEE STAGGERED STUD ROW PLAN ON SHEET 3.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

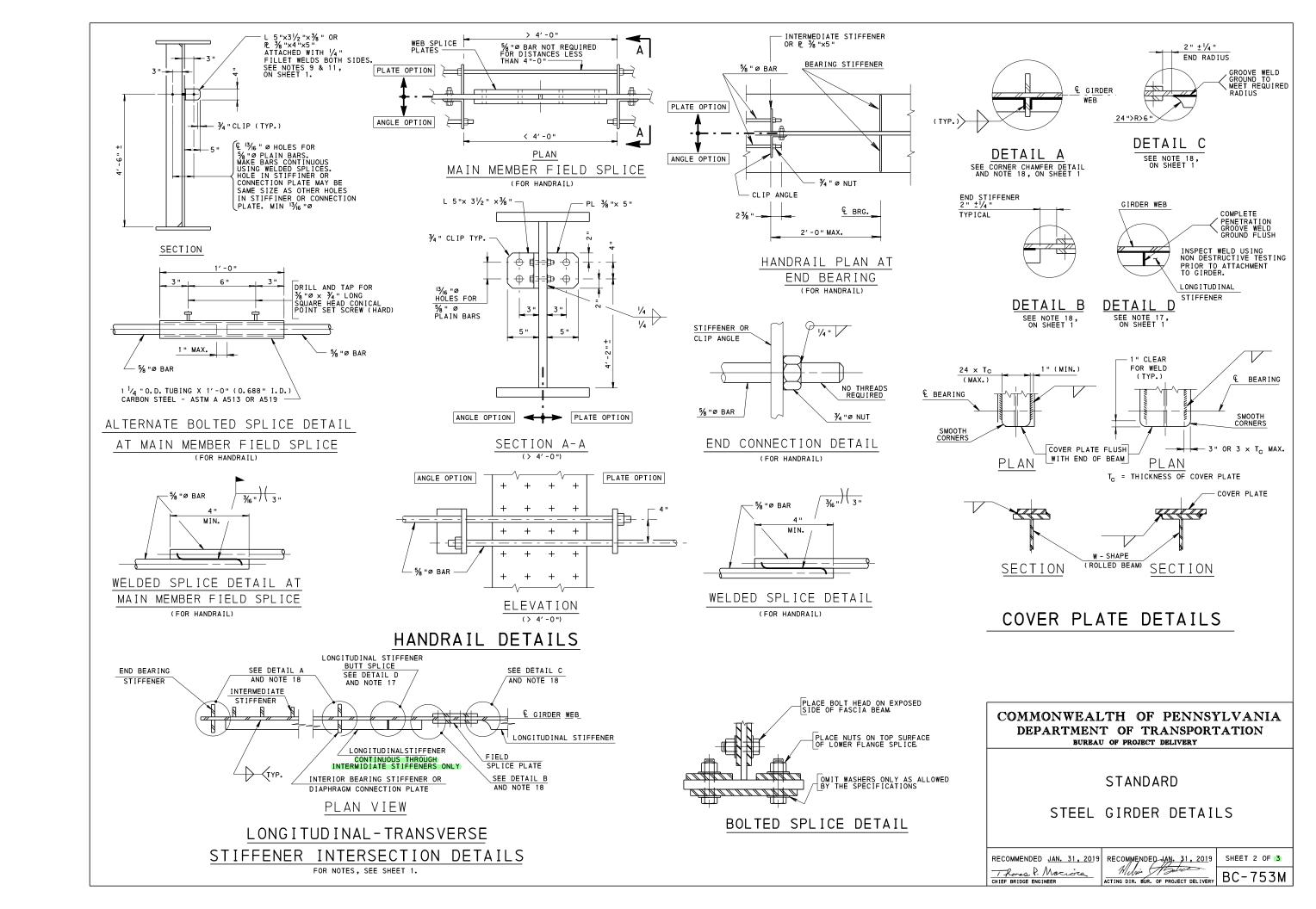
STANDARD

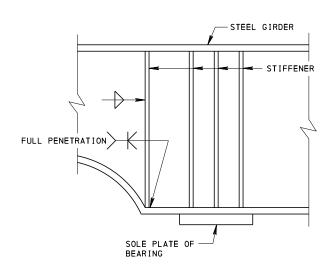
STEEL GIRDER DETAILS

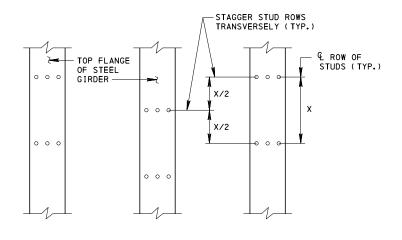
RECOMMENDED JAN. 31, 2019 Thomas P. Macrioca

RECOMMENDED JAN. 31, 2019 Allin Hodice ACTING DIR. BUR. OF PROJECT DELIVERY BC-753M

SHEET 1 OF 3







GIRDER HAUNCH STIFFENER DETAIL

(PARABOLIC WEB DEPTH VARIATION SHOWN; STRAIGHT LINE WEB DEPTH VARIATION SIMILAR)

STAGGERED STUD ROW PLAN

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

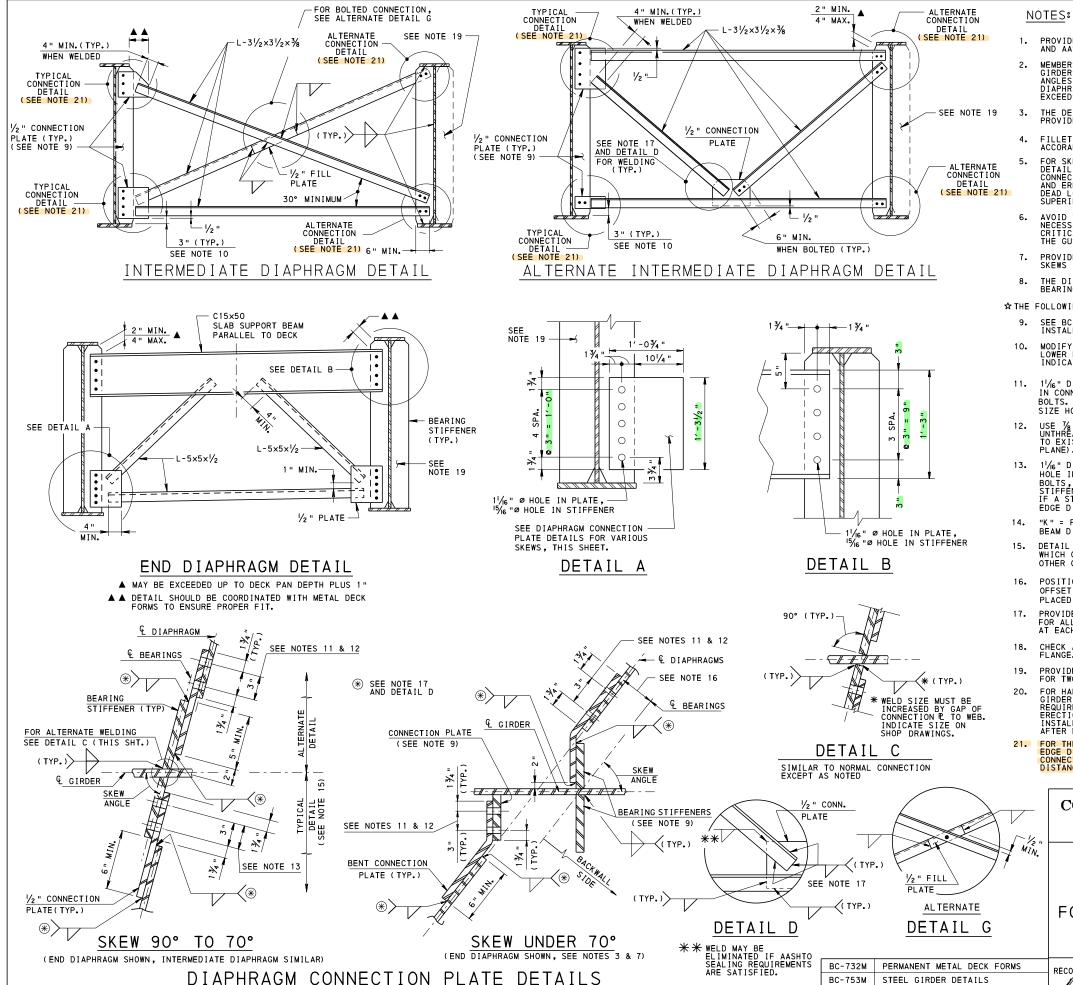
STEEL GIRDER DETAILS

RECOMMENDED JAN. 31, 2019 RECOMMENDED JAN. 31, 2019 SHEET 3 OF 3

Thomas P. Marine
CHIEF BRIDGE ENGINEER

ACTING DIR. BUR. OF PROJECT DELIVERY

BC-753M



CHANGE 2

CHANGE 4

- PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH PUB. 408 AND AASHTO/AWS D1.5 SPECIFICATIONS.
- MEMBERS, WELDS AND PLATE SIZES SHOWN ARE VALID FOR STRAIGHT GIRDERS WITH MAXIMUM GIRDER SPACING OF 8'-0" AND FOR SKEW ANGLES BETWEEN 70° AND 90°. PROVIDE SPECIAL DESIGNS FOR ALL THE DIAPHRAM MEMBERS, WELDS AND PLATE SIZES WHEN THE GIRDER SPACING EXCEEDS 8'-0"AND/OR THE SKEW ANGLE IS LESS THAN 70°.
- THE DETAILS SHOWN ARE VALID FOR SKEW ANGLES 25°TO 70°. PROVIDE SPECIAL DETAILS FOR SKEW ANGLES LESS THAN 25°.
- FILLET WELD SIZES ARE GOVERNED BY MATERIAL THICKNESS IN ACCORANCE WITH AASHTO/AWS EXCEPT AS NOTED.
- FOR SKEW ANGLES BETWEEN 90° AND 70°, DEVELOP SHOP DRAWINGS WHICH DETAIL ALL WEBS VERTICAL WHEN GIRDERS ARE ERECTED AND DIAPHRAGMS CONNECTED. FOR SKEW ANGLES LESS THAN 70°, DEVELOP SHOP DRAWINGS AND ERECTION PROCEDURES WHICH DETAIL ALL WEBS VERTICAL AFTER ALL DEAD LOADS ARE APPLIED (WEIGHT OF STEEL, WEIGHT OF DECK SLAB AND SUPERIMPOSED DEAD LOAD NOT INCLUDING THE FUTURE WEARING SURFACE).
- AVOID WELDING GUSSET PLATE TO WEB PLATE. HOWEVER, IF ABSOLUTELY NECESSARY THE WEB BENDING STRESS MUST BE LESS THAN 0.75 X CRITICAL BUCKLING STRESS AND THE FATIGUE CATEGORY ASSOCIATED WITH THE GUSSET PLATE MUST BE CONSIDERED AS CATEGORY C.
- PROVIDE INTERMEDIATE DIAPHRAGMS NORMAL TO THE MAIN MEMBERS FOR SKEWS <70°.
- THE DIAPHRAGMS SHOWN DO NOT INCLUDE WIND LOAD TRANSFERRED TO THE BEARINGS THROUGH CONNECTIONS.
- ☆ THE FOLLOWING NOTES ARE TO BE USED WHEN REFERENCED ON THE DRAWINGS:
- SEE BC-753M FOR THE BEARING STIFFENER AND CONNECTION PLATE INSTALLATION DETAILS.
- MODIFY THE DISTANCE BETWEEN THE GIRDER BOTTOM FLANGE AND THE LOWER DIAPHRAGM COMPONENT WHEN LOWER LATERAL BRACING IS USED. INDICATE MODIFICATIONS ON THE DESIGN DRAWINGS.
- 11/16" DIAMETER HOLE IN CONNECTION PLATE; 15/16" DIAMETER HOLE IN CONNECTION MEMBER, FOR 7/8" DIAMETER ASTM F3125 GRADE A325 BOLTS. OVERSIZE HOLE IN CONNECTION PLATE IS OPTIONAL. STANDARD SIZE HOLF IS PERMITTED.
- USE $\frac{7}{8}$ " DIAMETER ASTM F3125 GRADE A325 BOLTS HAVING AN UNTHREADED SHANK OF SUFFICIENT LENGTH TO NOT ALLOW ANY THREADS TO EXIST IN THE PLANE BETWEEN THE TWO CONNECTED PARTS (SHEAR
- 11/6" DIAMETER HOLE IN BEARING STIFFENERS; 15/6" DIAMETER HOLE IN CONNECTION PLATE FOR 1/8" DIAMETER ASTM F3125 GRADE A325 BOLTS, NOTE 12 DOES NOT APPLY. OVERSIZE HOLE IN BEARING STIFFENERS IS OPTIONAL. STANDARD SIZE HOLE IS PERMITTED. IF A STANDARD HOLE IS PROVIDED IN THE CONNECTION PLATE, THE EDGE DISTANCE MAY BE REDUCED TO 11/2 ".
- "K" = FLANGE THICKNESS + FILLET, AS INDICATED IN AISC TABLES OF BEAM DIMENSIONS.
- DETAIL SHOWN FOR CONNECTION PLATES (SKEW 90° TO 70°), INDICATING WHICH COMPONENTS ARE WELDED OR BOLTED, MAY BE APPLIED TO ALL OTHER CASES WHERE APPLICABLE.
- POSITION DIAPHRAGM CONNECTION COMPONENTS SO AS TO CREATE MINIMUM OFFSET FROM $^{\mathbb{Q}}$ BEARINGS. DIAPHRAGM BENT CONNECTION PLATE MAY BE PLACED BEHIND THE BEARING STIFFENER TO MINIMIZE OFFSET.
- PROVIDE WELDING AS SHOWN IN "DETAIL D". THIS DETAIL IS TYPICAL FOR ALL WELDED CONNECTIONS. TERMINATE WELDS 1/2" SHORT OF EDGE AT EACH END OF EACH WELD.
- CHECK ANCHOR BOLT CLEARANCES WHEN STIFFENERS ARE WIDER THAN FLANGE.
- PROVIDE CONNECTION PLATES ON THE OUTSIDE FACE OF FASCIA GIRDERS FOR TWO AND THREE GIRDER SYSTEMS.
- FOR HALF-WIDTH OR PHASED CONSTRUCTION, THE DEFLECTION OF EACH GIRDER IS CALCULATED TO A PRECISION THAT IS MUCH LESS THAN THAT REQUIRED FOR BOLT HOLES. AVOID DIAPHRAGM FABRICATION AND ERECTION PLANS THAT REQUIRE HOLES OF SHOP FABRICATED DIAPHRAGMS INSTALLED BEFORE DECK PLACEMENT TO ALIGN WITH HOLES IN STIFFENERS AFTER DEAD LOAD IS APPLIED.
- FOR THE TYPICAL AND ALTERNATE CONNECTION DETAILS BOLT SPACING, EDGE DISTANCES, AND CLEARANCES, SEE DETAIL A. FOR THE ALTERNATE CONNECTION DETAIL, USE K + 1" (MIN.) FOR THE VERTICAL EDGE DISTANCE MEASURED AS SHOWN IN DETAIL F.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

STEEL DIAPHRAGMS

FOR STEEL BEAM/GIRDER STRUCTURES (STRAIGHT PLATE GIRDERS ONLY)

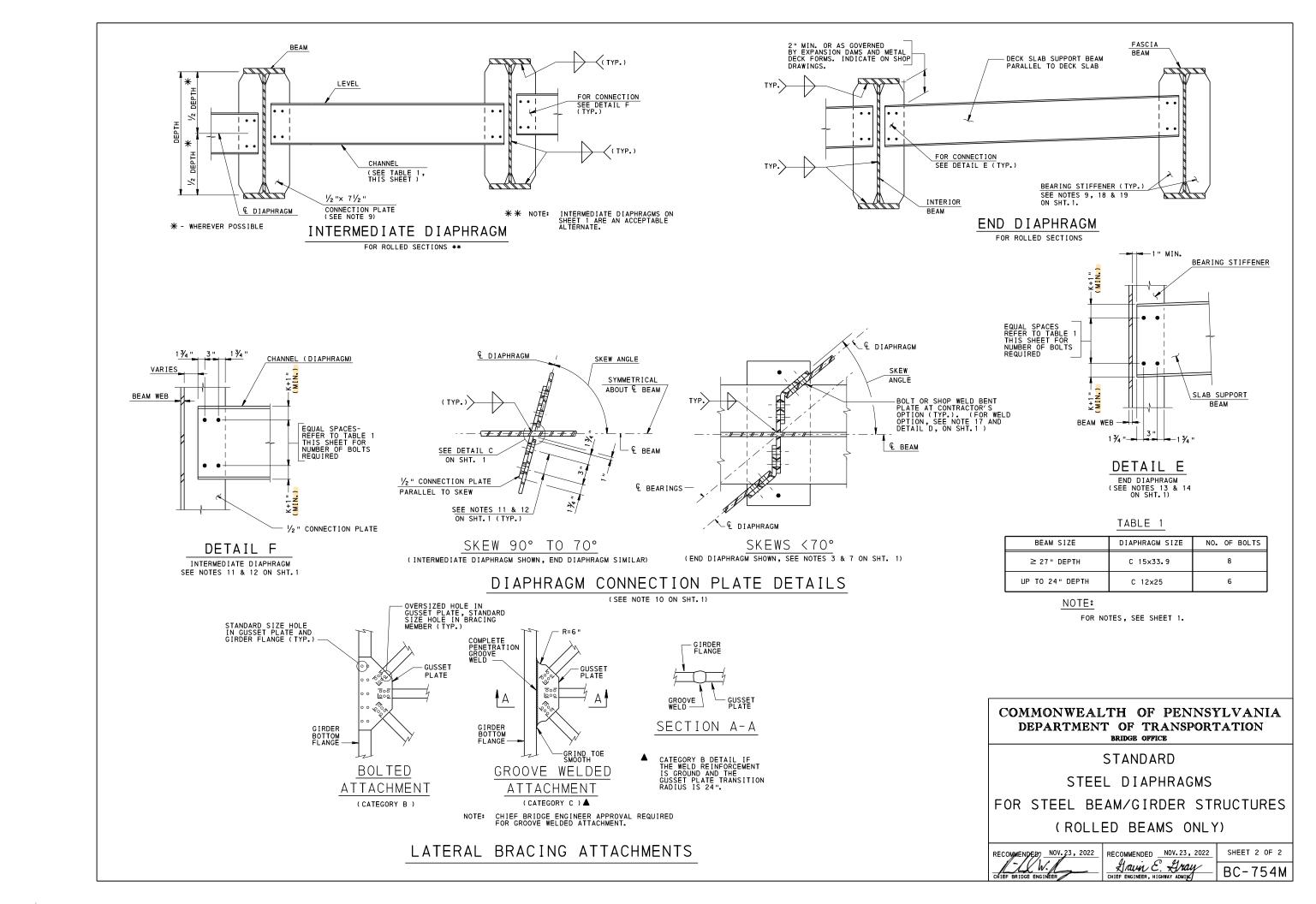
RECOMMENDED NOV. 23, 2022

BC-753M | STEEL GIRDER DETAILS

REFERENCE DRAWINGS

RECOMMENDED NOV. 23, 2022 SHEET 1 OF 2 Havin E. Hray
ITEF ENGINEER, HIGHWAY ADMIN

BC-754M



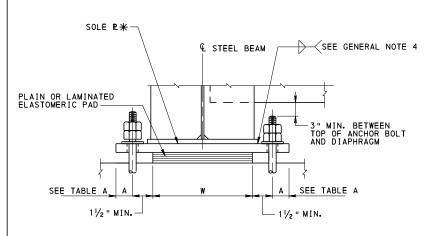
* PROVIDE FLATNESS TOLERANCE IN ACCORDANCE WITH PUB.408, SECTION 1105.03(q).
PROVIDE SOLE PLATE IN ACCORDANCE WITH DESIGN MANUAL PART 4, SECTION D14.7.6.3.9P

END OF STEEL BEAM TOP OF PEDESTAL FACE OF ABUTMENT OR PIER 1" MIN. PLAIN OR LAMINATED ELASTOMERIC PAD EXPANSION BEARING & SWEDGE ANCHOR BOLT

ELEVATION - EXPANSION BEARING

CHANGE 2

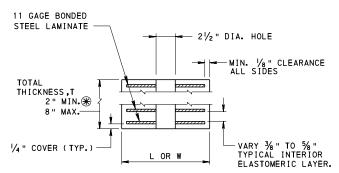
PT = SOLE PLATE THICKNESS AT & BEARING 1" MIN.



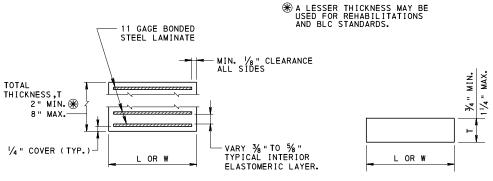
BEARING SECTION A-A

ELASTOMERIC BEARING PADS NOTES:

- 1. ELASTOMERIC BEARINGS DESIGNED AND MANUFACTURED IN ACCORDANCE WITH THIS STANDARD DRAWING DO NOT REQUIRE SHOP DRAWINGS.
- 2. MANUFACTURE ALL BEARINGS IN ACCORDANCE WITH THE COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PLANS AND SPECIFICATIONS (PUB. 408) SECTION 1113.
- 3. ALL BEARING PADS ARE TO BE MOLDED TO DESIGN DIMENSIONS. CUTTING TO SIZE AFTER FABRICATION IS PROHIBITED UNLESS INDICATED ON THE DESIGN DRAWINGS.
- 4. HOLES ARE NOT PERMITTED IN ELASTOMERIC BEARINGS UNLESS INDICATED ON THE DESIGN DRAWINGS.
- 5. PROVIDE NEOPRENE 50 ±5 DUROMETER.
- 6. VULCANIZE PATCH PIN GROOVES.
- 7. PROVIDE MINIMUM LOW-TEMPERATURE NEOPRENE GRADE 3.

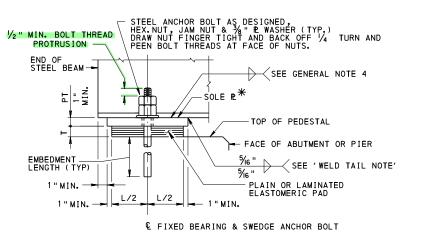


LAMINATED ELASTOMERIC PAD WITH HOLE



LAMINATED ELASTOMERIC PAD

PLAIN PAD



ELEVATION - FIXED BEARING

GENERAL NOTES:

- PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH SPECIFICATIONS, PUBLICATION 408, AASHTO/AWS BRIDGE WELDING CODE AND CONTRACT SPECIAL PROVISIONS.
- PROVIDE MECHANICALLY GALVANIZED OR HOT DIPPED GALVANIZED SWEDGE OR APPROVED TYPE OF ANCHOR BOLTS. SET IN THE MASONRY AS SHOWN ON THE DESIGN DRAWINGS.
- 3. PROVIDE STRUCTURAL STEEL CONFORMING TO AASHTO M270, GRADE 36 (ASTM A709, GRADE 36) UNLESS OTHERWISE NOTED.
- 4. PROVIDE MINIMUM SIZE WELD IN ACCORDANCE WITH AASHTO/AWS CODE UNLESS LARGER WELD IS REQUIRED BY DESIGN.
- 5. PROVIDE PLATE WASHERS OF SUFFICIENT SIZE TO COVER THE ROUND HOLE OR SLOT FOR EXTREMES OF MOVEMENT OF THE BEARINGS. WASHERS MAY BE CLIPPED IF REQUIRED.
- 6. DRILLING OF ANCHOR BOLT HOLES IS PERMITTED AT ABUTMENTS ONLY. RINSE CLEAN WITH WATER AND DRY HOLE BEFORE FILLING WITH NON-SHRINK GROUT.
- 7. MARK THICKER END OF BEVELED SOLE PLATES TO IDENTIFY THICKER END IN THE FIELD.
- 8. PAINT ALL STEEL SURFACES IN ACCORDANCE WITH PUB. 408, SECTION 1060. APPLY ALL COATS IN FABRICATION SHOP.
- 9. PREPARE BEARING AREAS IN ACCORDANCE WITH PUB. 408, SECTION 1001.3 (k) 9. DO NOT APPLY PROTECTIVE COATINGS TO THE BEARING SURFACES WITHIN 2" OF THE BEARING PAD, MASONRY PLATE, OR NEOPRENE SPONGE.
- 10. OBTAIN THE FOLLOWING INFORMATION FROM THE DESIGN DRAWINGS:

 (a) SOLE PLATE DIMENSIONS, ORIENTATION AND CONNECTION TO BEAM
 (b) ANCHOR BOLT SIZES (DIAMETER, EMBEDMENT LENGTH AND PROJECTION)
 (c) BEARING PAD SIZES (LENGTH, WIDTH, THICKNESS, AND SHIMS)
- 11. FOR SKEW LESS THAN 45° AND LARGER SIZE BEARINGS, CIRCULAR BEARINGS MAY BE DESIGNED AND DETAILED.
- 12. WHERE SOLE PLATES ARE WELDED TO BEAMS THAT ARE SUBSEQUENTLY GALVANIZED, PROVIDE AN ALL AROUND $\frac{5}{6}$ " FILLET WELD TO SEAL JOINT AND PREVENT ACID INTRUSION DURING PICKLING.
- 13. FOR LEGEND, SEE SHEET 2.

WELD TAIL NOTES:

- FOR BEAMS THAT ARE HOT DIP GALVANIZED SUBSEQUENT TO WELDING, CALL OUT "SEAL". SEE GENERAL NOTE 12.
- FOR BEAMS THAT ARE PAINTED SUBSEQUENT TO WELDING CALL OUT " 4" HOLD BACK".

TABLE A ANCHOR BOLT CLEARANCE INCHES DIM. DIA. 1 ¹³/₁₆ " 11/8" 21/16" 11/4" 21/16" 1 3% " 23/6" 25/16" 1 1/2 " 1 3/4 " 2 3/4 " 2 " 31/4"

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BURBAU OF PROJECT DELIVERY

STANDARD
BEARINGS
ELASTOMERIC BEARING PADS
FOR STEEL BEAM BRIDGES
AND GENERAL

BC-753M STEEL GIRDER DETAILS
BC-788M TYPICAL WATERPROOFING AND EXPANSION DETAILS

REFERENCE DRAWINGS

CHIEF BRIDGE ENGINEER

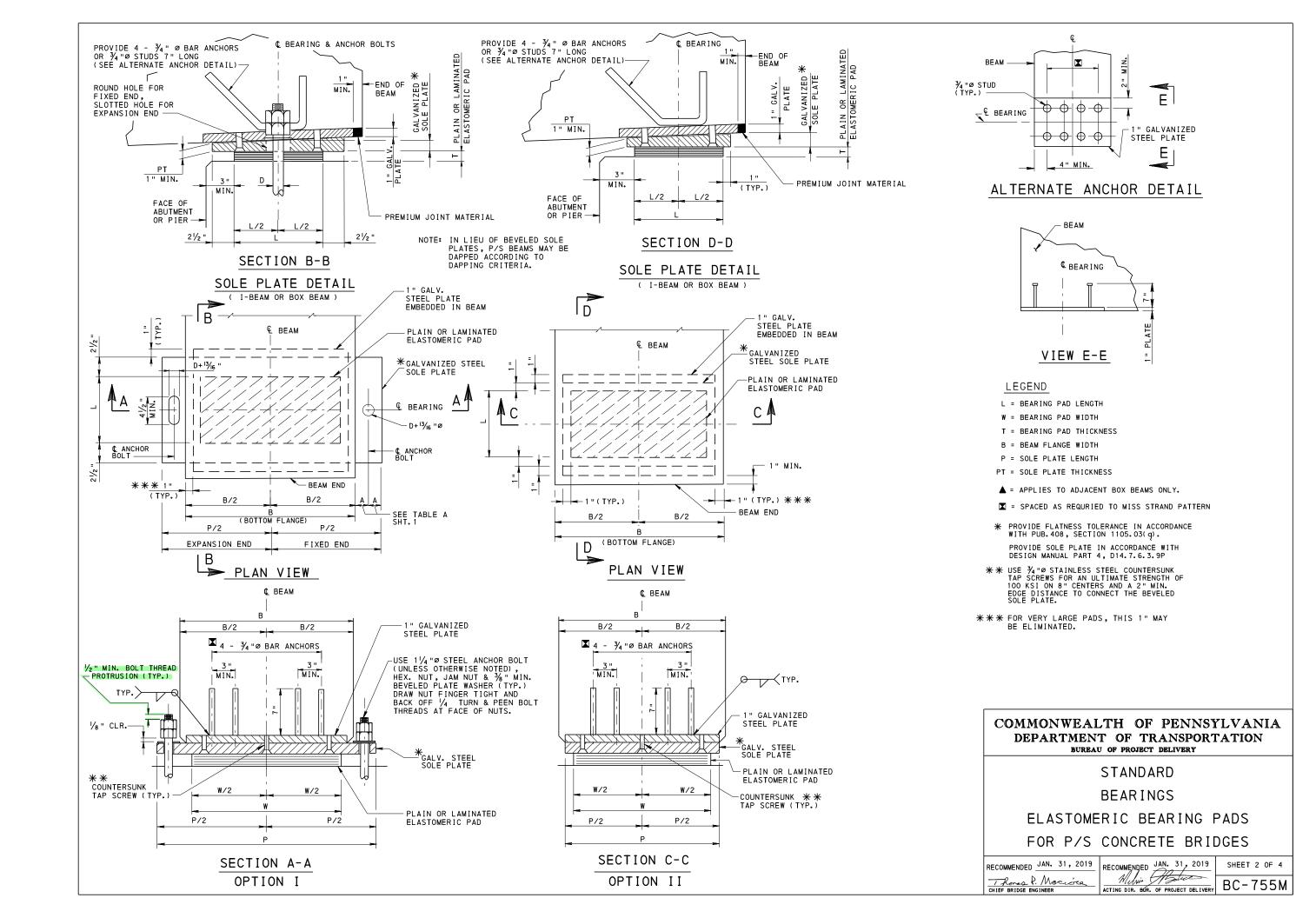
RECOMMENDED JAN. 31, 2019

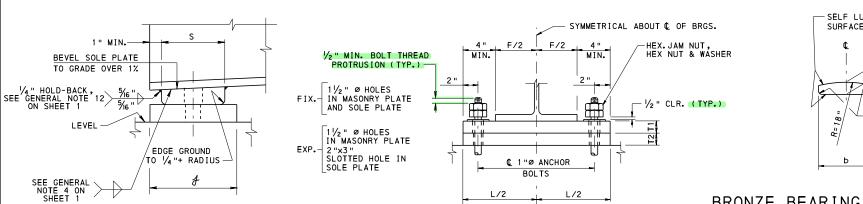
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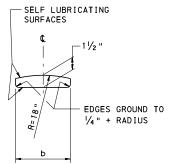
ACTING DIR. BUR. OF PROJECT DELIVERY

BC-755M

SHEET 1 OF 4





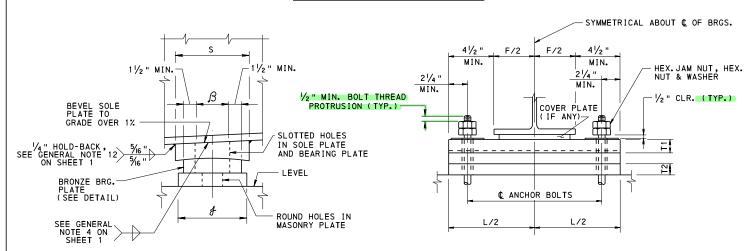


BRONZE BEARING PLATE DETAIL

FIXED BEARINGS IF & EXPANSION BEARINGS IE

FOR SPANS UP TO 50 FT. (EXPANSION BEARING SHOWN) -SYMMETRICAL ABOUT & OF BRGS. __ F/2 ___ F/2 - HEX.NUT & WASHER. NUT TO BE DRAWN UP FINGER TIGHT, THEN BACKED OFF ¼ TURN, AND THRADS OF BOLT BURRED OFF AT FACE OF NUT (TYP.). 1/2 " MIN. BOLT THREAD PROTRUSION (TYP.) BEVEL SOLE PLATE TO GRADE OVER 3% C HOLES IN SOLE PLATE 1% "Ø HOLES FOR 1"Ø ANCHOR BOLTS 1 1/4 "Ø HOLES FOR 11/4 "Ø ANCHOR BOLTS SEE GENERAL NOTE 12 5/6" ON SHEET 1 21/8 "Ø HOLES FOR 11/2 "Ø ANCHOR BOLTS C ANCHOR BOLTS LEVEL-C HOLES IN MASONRY PLATE 1½" Ø HOLES FOR 1 "Ø ANCHOR BOLTS 1¾ "Ø HOLES FOR 1¼ "Ø ANCHOR BOLTS L/2 2"Ø HOLES FOR 11/2 "Ø ANCHOR BOLTS NOTE 4 ON SHEET 1

FIXED BEARINGS IIF



HOLES IN MASONRY PLATE:

2"Ø HOLES FOR 1"Ø ANCHOR BOLTS 2 1/4 "Ø HOLES FOR 1 1/4 "Ø ANCHOR BOLTS 21/2 "Ø HOLES FOR 11/2 "Ø ANCHOR BOLTS

HOLES IN SOLE PLATE & BEARING PLATE: 2 "Ø x 5 " SLOTTED HOLE FOR 1 "Ø ANCHOR BOLTS $2\frac{1}{4}$ "Ø x $5\frac{1}{4}$ " SLOTTED HOLE FOR $1\frac{1}{4}$ "Ø ANCHOR BOLTS $2\frac{1}{2}$ "Ø x $5\frac{1}{2}$ " SLOTTED HOLE FOR $1\frac{1}{2}$ "Ø ANCHOR BOLTS

EXPANSION BEARINGS IIIE

USE THE FOLLOWING INFORMATION AS A GUIDE WHEN DESIGNING EXPANSION BEARINGS IIIE:

MINIMUM β = DIA. OF HOLE IN MASONRY PLATE + $\Delta \ell$ MINIMUM $b = (\beta + \Delta l)$ BUT NOT LESS THAN $(\beta + 3)$ MINIMUM $f = (b + \Delta l)$ BUT NOT LESS THAN (b + 2") IN WHICH $\Delta \ell$ = TOTAL LONGITUDINAL MOVEMENT MINIMUM I = F + 9"

MINIMUM S = b + 11/2"

METAL BEARING NOTES - FOR SKEWS > 75°

- 1. THICKNESS SHOWN IS AT & BEARINGS.
- FACING OF BEARING SURFACES SPECIFIED IN PUB. 408
 APPLIES TO FLAT SURFACES OR CIRCULAR SURFACES IN
 ACCORDANCE WITH DIMENSIONS AND RADII GIVEN IN THIS
 STANDARD DRAWING.
- THE DESIGNER IS REQUIRED TO COMPLETE THE APPROPRIATE BEARING DATA TABLE/TABLES BASED ON THE DESIGN CALCULATIONS.
- 4. USE SERVICE LOADS (UNFACTORED) IN THE TABLES.
- DO NOT USE THESE BEARINGS FOR SKEW LESS THAN 75°. SEE SHEET 4 FOR APPROPRIATE BEARINGS FOR SKEW LESS THAN 75°.
- 6. PROVIDE 1/8 " THICK TYPE II BEDDING MATERIAL FOR BRIDGE SHOES. CONFORMING TO THE REQUIREMENTS OF PUB. 408, SECTION 1113.03(h).

FIXED BEARINGS IF & EXPANSION BEARINGS IE									
DEAD LOAD	LIVE	IVE TOTAL	MARK	DIMENSIONS					WEIGHT
LUAD	LOAD	LOAD	WAIN	S	<i>[}</i>	L	T1	T2	

MINIMUM L = F + 8" FOR BEARINGS IF OR IE

	FIXED BEARINGS IIF								
DEAD LOAD	LIVE	TOTAL LOAD	MARK	DIMENSIONS S & L T1 T2 WEIGHT				WEIGHT	

MINIMUM L = F + 8"

	EXPANSION BEARINGS IIIE								
DEAD LOAD	LIVE	TOTAL LOAD	MARK	DIMENSIONS			WEIGHT		

DESIGN BEARINGS TO PROVIDE FOR A TOTAL LONGITUDINAL MOVEMENT OF 3". FOR LARGER MOVEMENT, SPECIAL DESIGN IS REQUIRED.

LEGEND

F = FLANGE WIDTH

T = PLATE THICKNESS

S = SOLE PLATE WIDTH

B = SOLE PLATE LENGTH

b = BEARING PLATE WIDTH L = LENGTH OF PLATE

R = RADIUS OF BEVEL SOLE PLATE

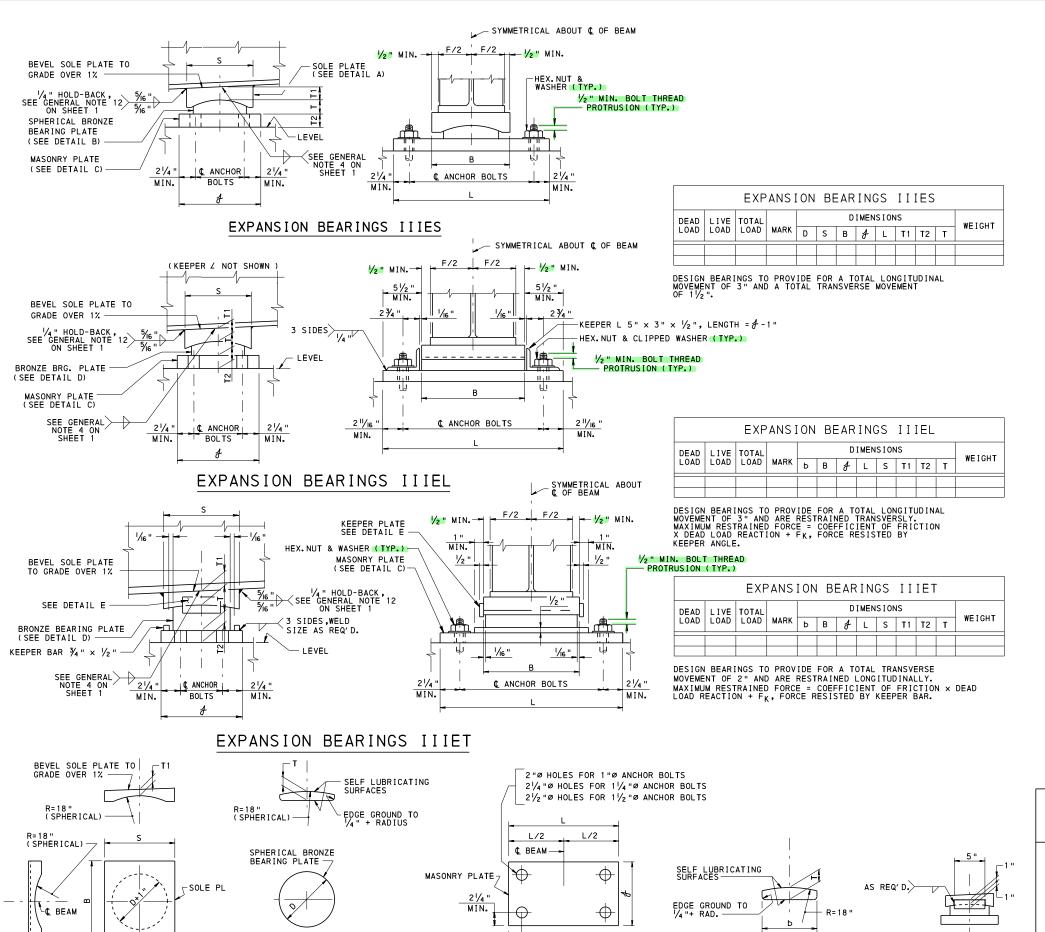
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD **BEARINGS** METAL BEARINGS FOR STEEL BEAM BRIDGES

RECOMMENDED JAN. 31, 2019 Thoras P. Maccica CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019 ACTING DIR. BUR. OF PROJECT DELIVERY BC-755M

SHEET 3 OF 4



MIN. FOR IIIES & IIIET BEARINGS
MIN. & VARIES FOR IIIEL BEARINGS

DETAIL C

DETAIL B

DETAIL A

METAL BEARING NOTES - FOR SKEW ≤ 75° OR CURVED STEEL BEAM BRIDGES

- 1. THICKNESS SHOWN IS AT @ BEARING.
- FACING OF BEARING SURFACES SPECIFIED IN PUB. 408 APPLIES TO FLAT SURFACES OR CIRCULAR SURFACES IN ACCORDANCE WITH DIMENSIONS AND RADII GIVEN IN THIS STANDARD DRAWING. 2.
- THE DESIGNER IS REQUIRED TO COMPLETE THE APPROPRIATE BEARING DATA TABLE/TABLES BASED ON THE DESIGN CALCULATIONS.
- 4. USE SERVICE LOADS (UNFACTORED) IN THE TABLES.
- USE THESE BEARINGS FOR SKEW LESS THAN OR EQUAL TO 75° AND CURVED STEEL BRIDGES.
- PROVIDE $\frac{1}{8}$ " THICK TYPE II BEDDING MATERIAL CONFORMING TO THE REQUIREMENTS OF PUB. 408, SECTION 1113.03 (h).
- 7. USE THE FOLLOWING AS A GUIDE:

MINIMUM VALUES	TYPES OF BEARINGS (in)							
OF OF	III ES	III EL	III ET					
В	=F +1" BUT.≮ S	=F+1"	=F +1 "					
ð	=D+Δl+ 1/2 " MIN.	=b+Δ및 + ½ " MIN.	=b+ 2 " MIN.					
L	=B+Δt+ 8"	=B+ 11"MIN.	=B+∆t+ 9 "					
S	=D+ 2 "	=b+1½"	=b+1½"					

SPECIFY TYPE OF STEEL ON THE DESIGN DRAWINGS. Δ ℓ = TOTAL LONGITUDINAL MOVEMENT

 Δt = TOTAL TRANSVERSE MOVEMENT

8. SEE LEGEND ON SHEET 3.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD BEARINGS METAL BEARINGS FOR SKEW ≤ 75° OR CURVED STEEL BEAM BRIDGES

RECOMMENDED JAN. 31, 2019 Thoras P. Marcioca CHIEF BRIDGE ENGINEER

DETAIL E

DETAIL D

RECOMMENDED JAN. 31, 2019 Millia Holice ACTING DIR. BUR. OF PROJECT DELIVERY BC-755M

A. GENERAL NOTES:

- 1. PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH SPECIFICATIONS, PUBLICATION 408, ANSI/AASHTO/AWS/D1.5 BRIDGE WELDING CODE AND CONTRACT SPECIAL PROVISIONS.
- 2. SANDBLAST IN ACCORDANCE WITH SSPC-SP10 TO REMOVE MILL SCALE FROM BEARINGS.
- GRIND SMOOTH ALL STEEL SURFACES AND EDGES AND REMOVE ANY SHARP PROTRUSIONS. FABRICATION TOLERANCES AND THE LIMITATIONS ON SURFACE FINISH WILL BE N ACCORDANCE WITH PUBLICATION 408,
- 4. PAINT ALL STEEL SURFACES IN ACCORDANCE
 WITH PUBLICATION 408, SECTION 1060.
 APPLY ALL COATS IN THE FABRICATION SHOP
 ONLY, DO NOT PAINT PITE, STAINLESS
 STEEL OR THE INSIDE OF THE POT.
 ONLY PIME COAT TO THE CONTACT AREA
 BETWEEN BEAM BOTTOM FLANGE AND SOLE
 PLATE AND TO THE BOTTOM SIDE OF
 THE MASCARPY PLATE THE MASONRY PLATE.
- 5. ROUND ALL PTFE CORNERS TO ACCOMMODATE THE MACHINED RECESS IN STEEL GUIDE
- 6. ETCH PTFE ON ONE SIDE FOR BONDING INTO THE MACHINED RECESS.
- 7. PTFE ON THE SIDE OF GUIDE PLATE MUST BE
- 8. PRIOR TO THE APPLICATION OF ADHESIVE, CLEAN ALL MATING STEEL AND PTFE SURFACES BY CRIT BLASTING AND DEGREASING. APPLY ADHESIVE AS PER THE MANUFACTURER'S RECOMMENDATION.
- 9. LUBRICATE ALL SURFACES OF NEOPRENE DISC WITH SILICONE GREASE IN ACCORDANCE WITH MILITARY SPECIFICATION SAE-AS8660.
- 10. CUT FLAT BRASS SEALING RING ENDS AT 45° ANGLE WITH A MAXIMUM GAP OF 0.05". STAGGER THE OPENINGS IN THE BRASS RINGS 120° APART.
- 11. MARK THE THICKER EDGE OF THE SOLE PLATE
 AS SUCH FOR THE PURPOSE OF FIELD
 IDENTIFICATION. PLACE MARK ON THE
 EDGE OF SOLE PLATE SO THAT IT WILL BE
 VISIBLE AFTER BEARING INSTALLATION. IN
 THE CASE OF A SOLE PLATE WITH A
 COMPOUND BEVEL PLACE THE MARK ON EITHER
 EDGE OF THE THICKEST SOLE PLATE CORNER.
- 12. MARK CENTERLINE OF GUIDED AND
 NON-GUIDED POT BEARINGS ON THE SIDES OF
 MASONRY PLATE AND SOLE PLATE. THE
 CENTERLINE IDENTIFICATION MARKS WILL BE
 USEFUL TO LOCATE OFFSET DISTANCES IN
 THE FIELD. USE INDELIBLE INK TO PLACE
- 13. MARK EACH BEARING WITH THE NAME OF THE MANUFACTURER AND TYPE OR MODEL NUMBER. PLACE THE IDENTIFICATION MARK IN A PERMANENT MANNER AND LOCATION SO THAT IT IS VISIBLE AFTER ERECTION.
- 14. WHEN THE POT IS RECESSED INTO THE MASONRY PLATE SEAL AROUND THE POT PERIMETER WITH AN APPROVED CAULKING COMPOUND IN THE SHOP AFTER PAINT COATING
- 15. ENSURE ALL BEARING SURFACES INCLUDING THE BEARING SEAT ARE LEVEL PRIOR TO INSTALLATION OF POT BEARINGS IN ACCORDANCE WITH PUBLICATION 408.
- 16. TEST ONE BEARING PER TYPE OR PER LOT SIZE OF 25 FOR A HORIZONTAL FORCE CAPACITY PRIOR TO SHIPMENT.

B. MATERIALS:

- 1. STRUCTURAL STEEL:
 - MATERIAL 4" THICK OR LESS AASHTO M270 (ASTM A709/ A A709M) GRADE 50
 - MATERIAL GREATER THAN 4" THICK ASTM A572
- 2. ANCHOR BOLTS: ASTM F1554, GRADE 55
- 3. NUTS: ASTM A563, GRADE DH
- 4. WASHERS: ASTM F436. TYPE 1
- GALVANIZING OF ANCHOR BOLTS, NUTS AND WASHERS: PUBLICATION 408, SECTION 1105.02(S).
- STAINLESS STEEL: ASTM A240, GRADE 30, TYPE 304 WITH AN ANSI 0.02 mil SURFACE FINISH OR LESS.
- FLAT BRASS SEALING RINGS: ASTM B36 (HALF HARD) SPECIFICATION.
- ELASTOMERIC DISC: VIRGIN PLAIN NEOPRENE OR NATURAL RUBBER WITH HARDNESS OF 50 DUROMETER (+/- 10) PER AASHTO M251.
- 9. PTFE SHEET: MADE FROM VIRGIN TFE RESIN PER ASTM D4894.
 - MAIN SLIDING SURFACE PTFE UNFILLED, DIMPLED AND LUBRICATED. DIMPLES MUST HAVE A MINIMUM EDGE DISTANCE OF 0.5" AND CONFORM TO 1998 AASHTO
 - · GUIDE BAR SURFACE PTFE PIGMENTED, FILLED OR UNFILLED.
- 10. CAULK FOR SEALING AROUND THE POT PERIMETER: SIKAFLEX IA OR APPROVED EQUAL.
- 11. BEDDING MATERIAL: PUBLICATION 408, SECTION 1113.03 (h), TYPE II.

C. MATERIAL DESIGN PARAMETERS:

- 1. ALLOWABLE PRESSURE IN ELASTOMER AND PTFE: MAXIMUM = 3500 psi ELASTOMER & PTFE MINIMUM = 700 psi ELASTOMER
- COEFFICIENT OF FRICTION BETWEEN PTFE AND STAINLESS STEEL: 0.04
- 3. CONCRETE BEARING STRENGTH: f'c = 3000 psi

D. ANCHOR BOLT INSTALLATION:

PEEN THREADS AFTER — HEX NUT INSTALLATION

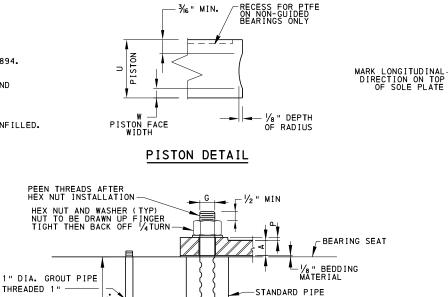
ANCHOR BOLT

AS PER DESIGN

- IF ANCHOR BOLTS ARE INSTALLED BEFORE THE MASONRY PLATE INSTALLATION, USE ANCHOR BOLT DETAIL 1. THE USE OF A BLOCKOUT FORM IS OPTIONAL.
- IF ANCHOR BOLTS ARE INSTALLED AFTER THE BEARINGS ARE INSTALLED, USE ANCHOR BOLT DETAIL 2.
- IF BLOCKOUTS ARE USED, REMOVE BLOCKOUT FORM AND DEBRIS FROM HOLE PRIOR TO GROUTING. INSTALL NON-SHRINK GROUT IN ACCORDANCE WITH PUBLICATION , SECTION 1001. DO NOT GROUT UNTIL ALL GIRDER UNITS PROPERLY ALIGNED.
- 4. PREVENT WATER FROM ACCUMULATING IN THE PREFORMED ANCHOR BOLT HOLES OR STANDARD PIPE AND ENSURE THE HOLES ARE COMPLETELY FILLED WITH GROUT.

ALTERNATE POT PLATE ATTACHMENT

NOTE: CAN BE USED IN LIEU OF RECESSING POT PLATE.



-SWEDGED ANCHOR BOLT

HEX NUT AND WASHER (TYP) NUT TO BE DRAWN UP FINGER TIGHT THEN BACK OFF 1/4 TURN

-BEARING SEAT

BEDDING MATERIAL

FILL WITH GROUT

- CLOSURE

PLATE

PEEN THREADS AFTER HEX NUT INSTALLATION

ANCHOR BOLT DETAIL 2

ANCHOR BOLT DETAIL 1

PREFERRED

ALTERNATE

D (TYP.) ___ C/2 (TYP.)

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HOLE DIAMETER G + 1/2 " (TYP)

THREADED 1"

HEX NUT AND WASHER (TYP) NUT TO BE DRAWN UP FINGER

1/8 " BEDDING

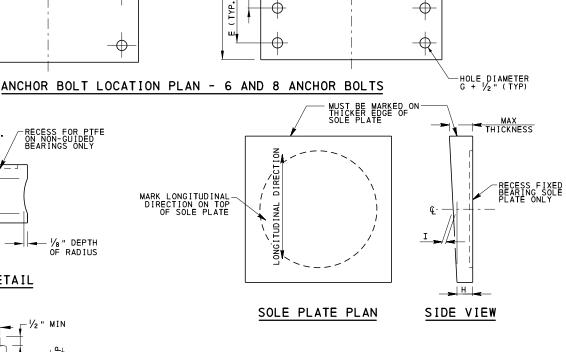
MATERIAL

TIGHT THEN BACK OFF 1/4 TURN

-BEARING SEAT

BLOCKOUT, 2" LARGER THAN ANCHOR BOLT, FILL WITH NONSHRINK GROUT. —

SWEDGED ANCHOR BOLT

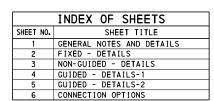


D (TYP.) C/2 (TYP.)

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NOTE:

THE INDICATED BEARING COMPONENT DIMENSION VARIABLES TO BE TAKEN FROM CONTRACT DRAWINGS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

HIGH LOAD MULTI-ROTATIONAL

POT BEARINGS

GENERAL NOTES AND DETAILS

RECOMMENDED NOV. 23, 2022

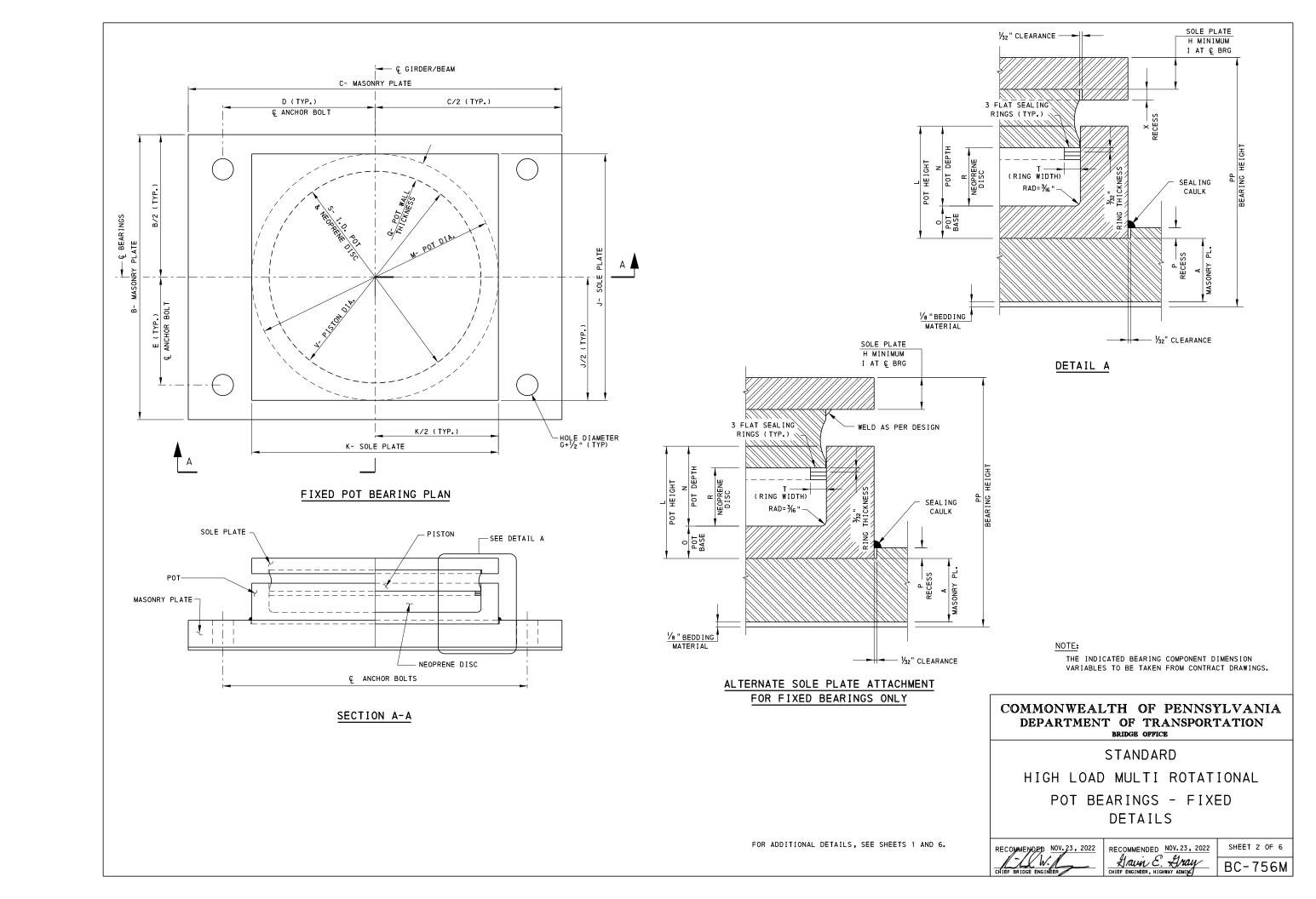
RECOMMENDED NOV. 23, 2022

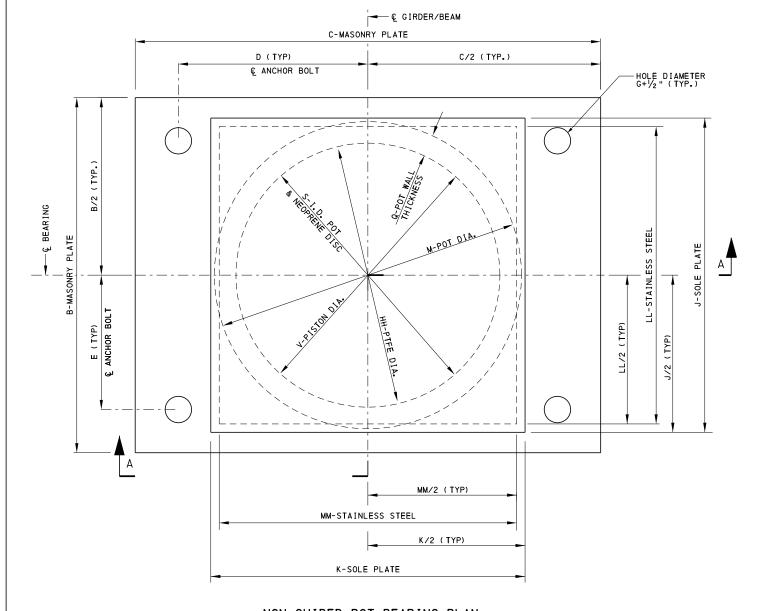
SHEET 1 OF 6

BC-756M

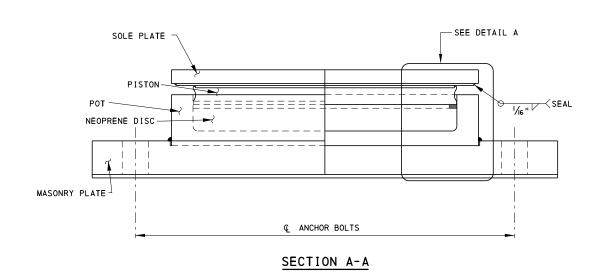
Havin E. Hray

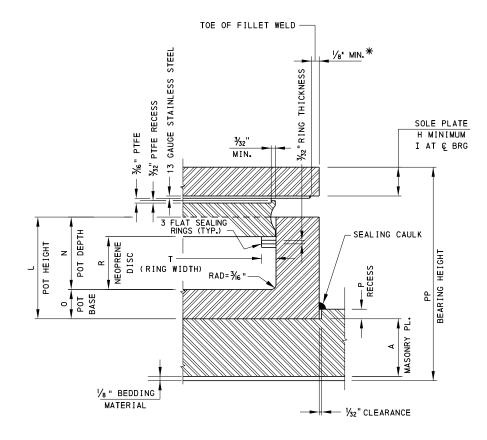
CHANGE 4





NON-GUIDED POT BEARING PLAN





DETAIL A

* 1/8" MIN. MAY BE REDUCED TO ZERO IN ORDER TO ELIMINATE BLASTING AND PAINTING OF SMALL EDGE AREA BENEATH SOLE PLATE AS LONG AS THE QUALITY OF WELD IS NOT COMPROMISED.

NOTE:

THE INDICATED BEARING COMPONENT DIMENSION VARIABLES TO BE TAKEN FROM CONTRACT DRAWINGS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

HIGH LOAD MULTI ROTATIONAL POT BEARINGS - NON-GUIDED DETAILS

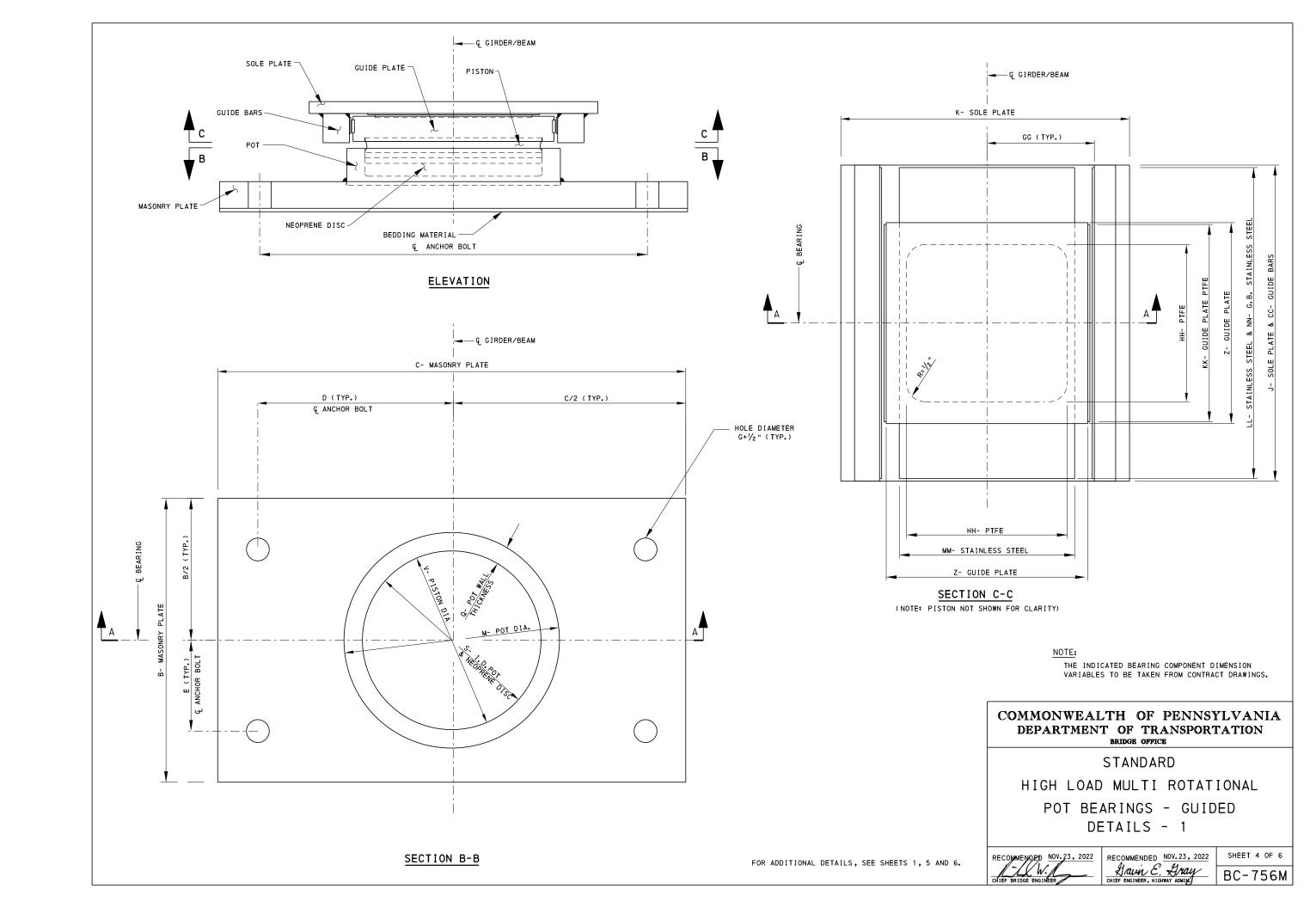
RECOMMENDED NOV. 23, 2022

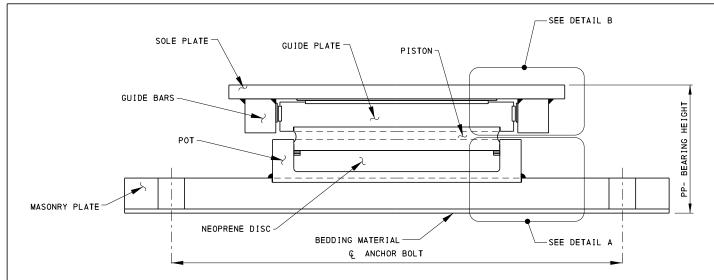
Havin E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN

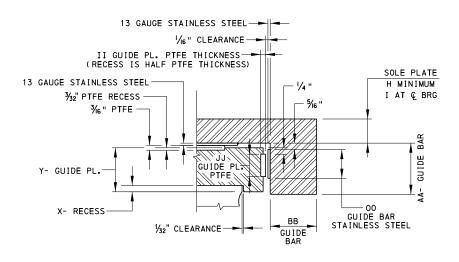
SHEET 3 OF 6 BC-756M

FOR ADDITIONAL DETAILS, SEE SHEETS 1 AND 6.



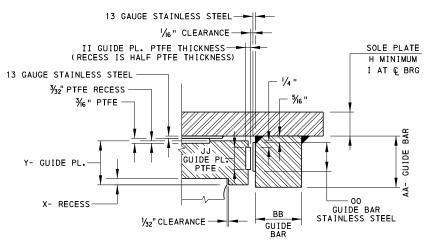


SECTION A-A

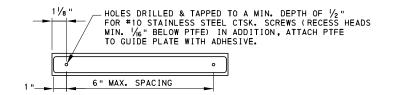


ALTERNATE GUIDE BAR FABRICATION DETAIL

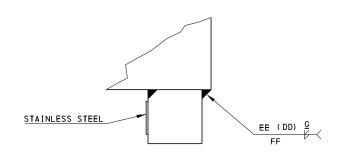
(GUIDE BAR FABRICATED FROM SINGLE PLATE)



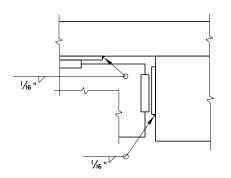
DETAIL B



GUIDE PLATE PTFE DETAIL



GUIDE BAR WELD DETAIL



STAINLESS STEEL WELD DETAIL

NOTE:

THE INDICATED BEARING COMPONENT DIMENSION VARIABLES TO BE TAKEN FROM CONTRACT DRAWINGS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

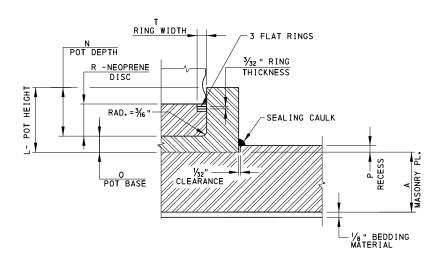
STANDARD

HIGH LOAD MULTI ROTATIONAL
POT BEARINGS

GUIDED DETAILS - 2

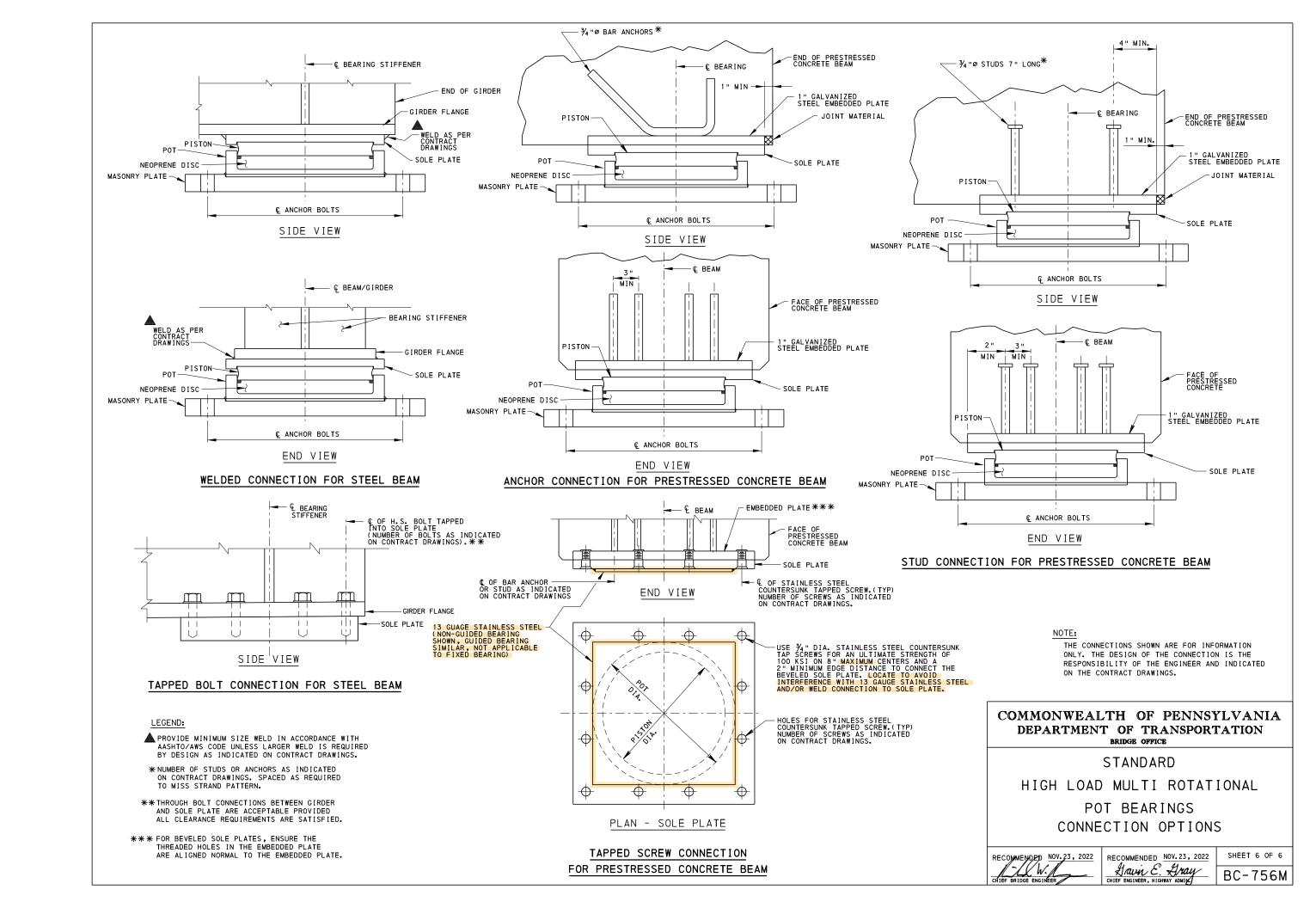
RECOMMENDED NOV. 23, 2022 RECOMMENDED NOV. 23, 2022 SHEET 5 OF 6

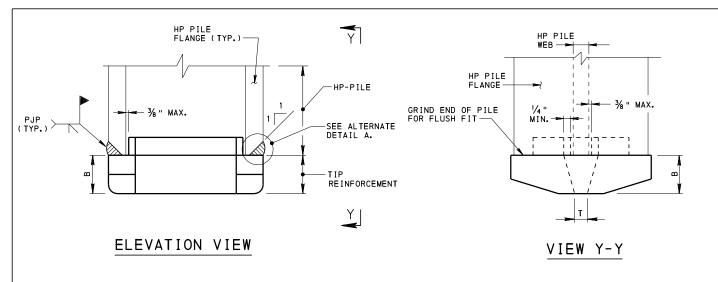
Law E. Law BC-756M

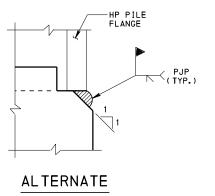


DETAIL A

FOR ADDITIONAL DETAILS, SEE SHEETS 1, 4 AND 6.







DETAIL A

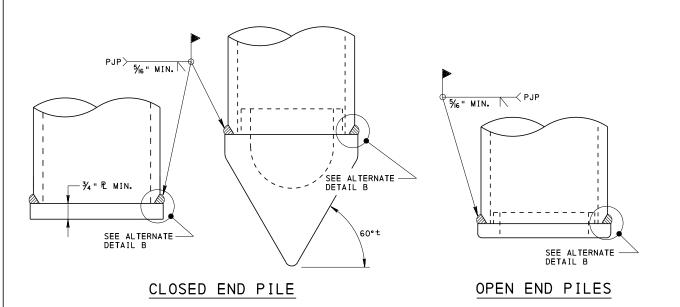
P PILE (ABOVE)

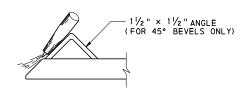
PLAN TYPICAL HP-PILE TIP

HP-	WELD SIZE	CAST TIPS						
PILE TYPE	A	A (MIN.)	B (MIN.)	H (MIN.)	T (MIN.)			
14 X 117	3/16							
14 X 102	3/ ₁₆ 3/ ₁₆	*	3	**	,			
14 X 89	3/16		3	**	'			
14 X 73	3/16							
12 X 84	3/16							
12 X 74	3/16	*	21/2	**	3/4			
12 X 63	3/16				/4			
12 X 53	3/16							
10 X 57	3/16	*	21/8	**	11/16			

- ▲ = REFER TO NOTES 6 AND 7, THIS SHT.
- * = FLANGE THICKNESS + 1/4" MIN. + FIT-UP TOLERANCE 3/8" MAX.
- ** = WEB THICKNESS + 2 × 1/4 " MIN. + 2 × FIT-UP TOLERANCE 3/8" MAX.

HP-PILE TIP REINFORCEMENT DETAILS





SCARFING END OF PILE (H OR PIPE PILE)

ALTERNATE DETAIL B

PIPE PILE TIP REINFORCEMENT

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD STEEL PILE TIP REINFORCEMENTS & SPLICES

RECOMMENDED SEPT. 30, 2016 RECOMMENDED SEPT. 30, 2016 Thomas P Macioca

Bun SThomps DIRECTOR, BUR. OF PROJECT DELIVERY

BC-757M

SHEET 1 OF 3

GENERAL NOTES:

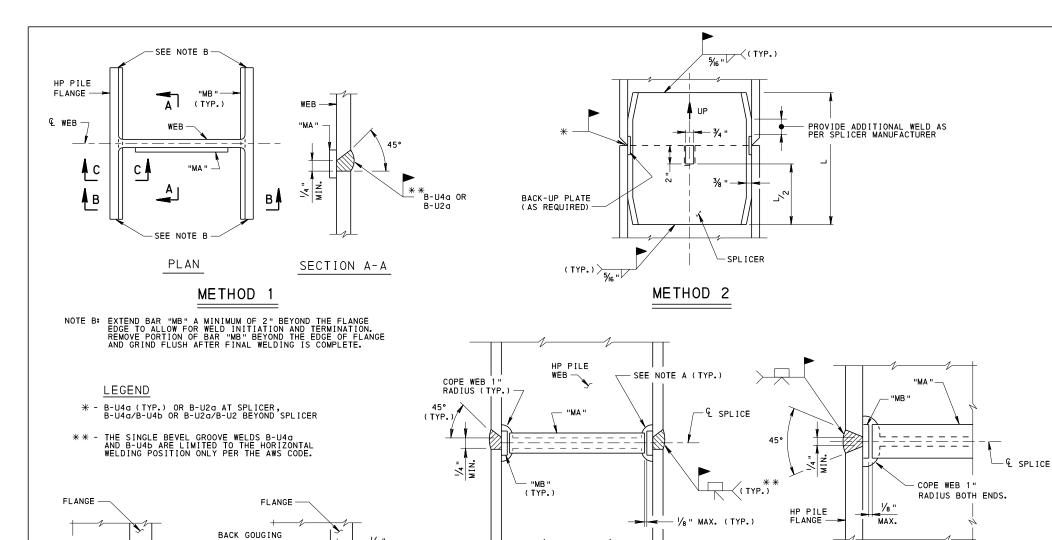
- PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH THE CURRENT VERSIONS OF PUBLICATION 408 AND AASHTO/AWS WELDING CODE DI.5. USE AWS DI.1 FOR WELDING NOT COVERED IN AASHTO/AWS DI.5. THE NDT REQUIREMENTS IN DI.5 MAY BE WAIVED BY THE STRUCTURE CONTROL ENGINEER.
- THIS STANDARD COVERS ONLY "NORMAL DUTY" PILE TIP REINFORCEMENT. DESIGNER MAY SPECIFY "HEAVY-DUTY" PILE TIP REINFORCEMENT FOR HARD DRIVING CONDITIONS. THE CONNECTION FOR "HEAVY-DUTY" PILE TIP REINFORCEMENT TO BE IN ACCORDANCE WITH NOTE 6 ON THIS SHEET.
- FURNISH AND INSTALL TIP REINFORCEMENT SUPPLIED BY BULLETIN 15 APPROVED SUPPLIERS.
- FURNISH A CERTIFIED STATEMENT THAT THE TIP REINFORCEMENT STEEL COMPLIES WITH THE SPECIFICATION REQUIREMENTS INCLUDING CERTIFIED REPORT SHOWING THE CHEMICAL AND PHYSICAL PROPERTIES, AND ROLLING DIRECTION FOR PLATES USED IN THE PREFABRICATED TIPS.
- 5. DO NOT USE FILLET WELD FOR ATTACHING CAST TIP REINFORCEMENT TO HP-PILES.
- 6. CONNECTION OF TIP REINFORCEMENT TO PILE:

JOIN HP PILE TO CAST TIPS USING GROOVE WELDS ONLY. WELD SIZE TO BE THE GREATER OF $\frac{3}{16}$ " OR MINIMUM GROOVE WELD SIZE RECOMMENDED BY THE TIP MANUFAC-TURER FOR THE PILE/TIP COMBINATION REQUIRED.

BEVEL OUTSIDE OF EACH FLANGE OF THE HP-PILE FOR GROOVE WELD, WHERE TIP REINFORCEMENTS ARE NOT PRE-BEVELED OR TO ACHIEVE THE MINIMUM GROOVE

ATTACH A PILE TIP REINFORCEMENT ON THE SQUARE CUT END OF THE PILE AND HOLD IT IN CLOSE CONTACT AGAINST THE PILE OR TO ACHIEVE THE MINIMUM GROOVE WELD SIZE.

- THE WELDS SHOWN ARE SUGGESTED ACCEPTABLE GROOVE WELDS. THE CONTRACTOR MAY USE ANY PREQUALIFIED GROOVE WELDS APPROVED BY THE ENGINEER.
- THE DEPARTMENT MAY REJECT AN APPROVED PILE TIP TYPE, IF FOUND UNSUITABLE FOR A JOB SITE BASED UPON DRIVING RECORDS.



BACK-UP

B-U4a**

PLATE

(RECOMMENDED)

B-U2a

PLATE

IS MANDATORY

OR

FLANGE

OR

JOINT DETAILS

WEB COPE NOT SHOWN, SEE SECTION B-B AND C-C FOR DETAILS. THE CONTRACTOR MAY ELECT

TO USE THE BACK GOUGED JOINT DETAILS B-U4b OR B-U2 IN LIEU OF DETAILS B-U4a OR B-U2a.

60°

FOR THIS JOINT

0" TO 1/8"

B-U4b**

B-U2

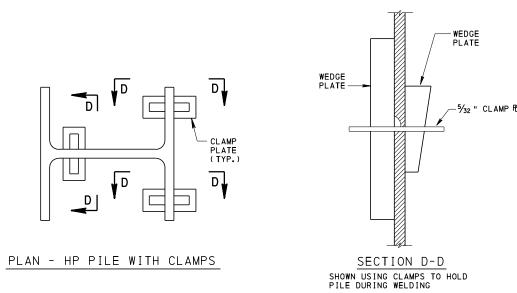
BACK GOUGING IS MANDATORY

__ O" TO

FOR THIS JOINT.

WEB COPE AND BACK-UP PLATE DETAIL

NOTE A: END OF WELD TO BE GROUND SMOOTH AND FLUSH WITH WEB COPE.



SECTION C-C

(B-U2a ALTERNATE FLANGE WELD SHOWN)

POSITIONING DETAILS

HP-PILE SPLICE DETAILS

SECTION B-B

(B-U4a FLANGE WELD SHOWN)

GENERAL NOTES:

- METHOD 1 SHOWS GROOVE WELDED FLANGE AND WEB SPLICES.
- METHOD 2 SHOWS SPLICE DETAILS USING SPLICER.
- BARS "MA" AND "MB" TO BE IN ACCORDANCE WITH AWS CODE FOR WELD BACKING. RECOMMENDED MINIMUM SIZE IS 1 1/4 " imes 3/6".

FIELD WELDING NOTES:

- SUBMIT A WELD PROCEDURE SPECIFICATION TO THE ENGINEER FOR APPROVAL BEFORE WELDING IS PERFORMED.
- USE THE MANUAL SHIELDED METAL ARC PROCESS WITH PROPERLY DRIED ELECTRODES CONFORMING TO AWS CLASSIFICATION E-7016, E-7018 OR E-7028.
- DRY THE ELECTRODES FOR AT LEAST TWO HOURS AT A TEMPERATURE BETWEEN 450° AND 500°F PRIOR TO USE. REDRY ELECTRODES IF NOT USED WITHIN FOUR HOURS. DO NOT REDRY ELECTRODES MORE THAN ONE TIME. DO NOT USE ELECTRODES WHICH HAVE DRIED OUT AND CRACKED, OR THOSE WHICH HAVE BEEN WET. STORE ALL LOW HYDROGEN ELECTRODES IN SUITABLE OVENS HELD AT A TEMPERATURE OF AT LEAST 250°F.
- DO NOT WELD WHEN SURFACES ARE WET OR EXPOSED TO RAIN, SNOW, WIND OR WHEN WELDERS ARE EXPOSED TO INCLEMENT CONDITIONS THAT WILL HAMPER GOOD
- REMOVE ANY MOISTURE FROM FOG, DEW, ETC. PRESENT BEFORE WELDING.
- PROVIDE WIND BREAKS TO PROTECT WORKING AREAS FROM DIRECT WIND.
- 7. DO NOT WELD WHEN THE AMBIENT TEMPERATURE IS BELOW O'F.
- PREHEAT METAL TO AT LEAST 70°F IN AN AREA AT LEAST 3" AWAY FROM THE WELD IN ALL DIRECTIONS AND MAINTAIN AT THIS MINIMUM TEMPERATURE DURING WELDING.
- 9. PROVIDE BACKING PLATES AND WELD TABS FOR FLANGE WELDS OF THE SAME MATERIAL AS THE PILE TO BE SPLICED. MAY LEAVE BACKING PLATES IN PLACE.
- 10. ONLY AWS CERTIFIED WELDERS ALLOWED TO PERFORM THE WELDING.
- 11. FOR SCARFING DETAILS, SEE SHEET 1.

SPLICE NOTES:

- DO NOT ALLOW PILE SPLICING ON ANY PORTION OF PILE THAT IS TO REMAIN EXPOSED ABOVE FINISHED GROUND LINE IN COMPLETED STRUCTURE.
- PROVIDE SPLICED SLEEVE MATERIAL SAME AS PILE MATERIAL.
- USE EITHER THE "SPLICER SLEEVE" OR "ALL WELDED ALTERNATES".
- LET WELDS COOL TO AIR TEMPERATURE BEFORE DRIVING PILES.
- SPLICE MUST DEVELOP THE YIELD STRENGTH OF THE PILE IN BEARING AND BENDING.
- REFER TO SEC. 1005.2(c) OF PUB. 408 FOR SPLICE LOCATION REQUIREMENTS.
- GRIND WELD SMOOTH WITH EDGE OF FLANGE IF PILE IS UNSUPPORTED IN WELD AREA SUCH AS: IN AIR, WATER OR SOFT MUD (INCLUDING SCOUR ZONES OR OTHER VOID AREAS).

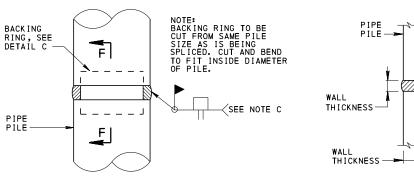
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD STEEL PILE TIP REINFORCEMENTS & SPLICES

RECOMMENDED SEPT. 30, 2016 RECOMMENDED SEPT. 30, 2016 Thmo P Macioca

Buni & Thomps DIRECTOR, BUR. OF PROJECT DELIVERY BC-757M

SHEET 2 OF 3

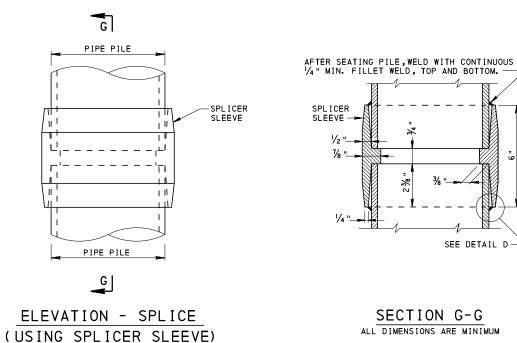


ELEVATION - SPLICE USING ALL WELDED ALTERNATE

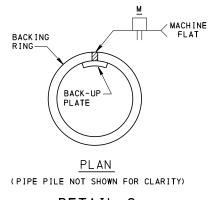
_ 2" MIN. (TYP.) PIPE PILE RING

SECTION F-F

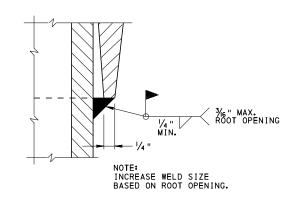
NOTE C: IF PIPE WALL THICKNESS EXCEEDS 1/4", USE WELD DETAIL B-U2g OR B-U4g SHOWN ON SHEET 2 OF 3.







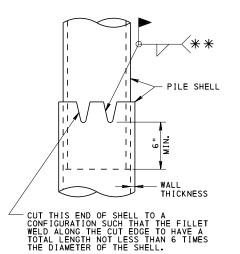
DETAIL C



DETAIL D

SPLICE NOTES:

- DO NOT ALLOW PILE SPLICING ON ANY PORTION OF PILE THAT IS TO REMAIN EXPOSED ABOVE FINISHED GROUND LINE IN COMPLETED STRUCTURE.
- 2. PROVIDE SPLICED SLEEVE MATERIAL SAME AS PILE MATERIAL.
- 3. USE EITHER THE "SPLICER SLEEVE" OR "ALL WELDED ALTERNATES.
- 4. LET WELDS COOL TO AIR TEMPERATURE BEFORE DRIVING PILES.
- REFER TO SEC. 1005.2(b) OF PUB. 408 FOR SPLICE LOCATION REQUIREMENTS.



FLUTED TUBE SPLICE DETAIL

* * WELD SIZE DEPENDS ON PIPE WALL THICKNESS.

PIPE PILE SPLICE DETAILS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

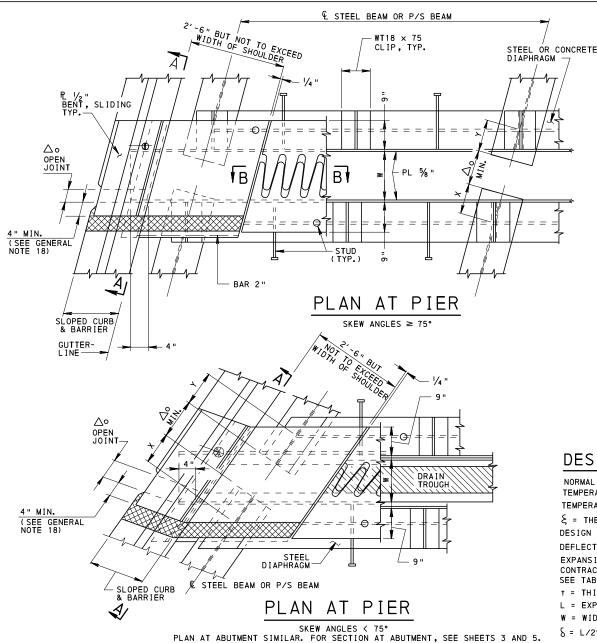
STEEL PILE TIP REINFORCEMENTS & SPLICES

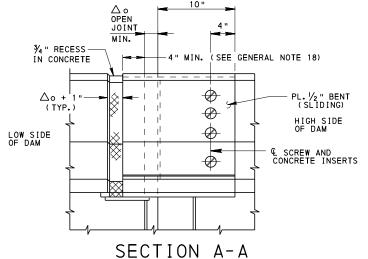
RECOMMENDED SEPT. 30, 2016 RECOMMENDED SEPT. 30, 2016 Thoma P Macioca CHIEF BRIDGE ENGINEER

Bun & Thomps

DIRECTOR, BUR. OF PROJECT DELIVERY BC-757M

SHEET 3 OF 3





FOR STEEL BEAMS * 500 550 600 650 5 3/4 6 6 3/8 6 3/4 41/4 43/4 51/4 53/4 61/4 65/8 \triangle (IN.) 2 $\frac{3}{8}$ 2 $\frac{7}{8}$

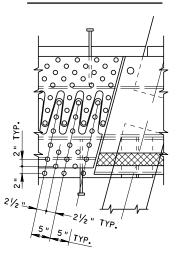
SECTION A-A NOTE:

CHANGE 2

FORM CONCRETE RECESS AREA IN BARRIER AND GRIND TO PROVIDE SMOOTH SURFACE. APPLY ONE COAT OF ASPHALT CEMENT PAINT WA-1 OR PERFORMANCE GRADED ASPHALT CEMENT PG 64-22 TO ALLOW BENT SLIDING PLATE TO MOVE FREELY WITHOUT FRICTION.

TOOTH PL SLIDING PL

SECTION B-B



BALL STUD DETAIL

(SEE GENERAL NOTE 6)

DESIGN INFORMATION

NORMAL TEMPERATURE = 68°F.

TEMPERATURE RANGE = -10°F TO 110°F FOR STEEL & 10°F TO 100°F FOR P/S.

TEMPERATURE CHANGE = 42°F RISE, 78°F FALL FOR STEEL & 32°F RISE, 58°F FALL FOR P/S. ξ = THERMAL COEFFICIENT = 0.0000065 PER °F FOR STEEL & 0.0000060 PER °F FOR P/S.

DESIGN LIVE LOAD = 100 PSI + 60% IMPACT = 160 PSI

DEFLECTION OF TOOTH SHALL NOT EXCEED t/300 WHERE t=0 CANTILEVER LENGTH OF TOOTH.

EXPANSION: MIN. \triangle o = \S + \S Tc L (L IN IN.) = 0.00672 L @ 68 °F (L IN FT.)

CONTRACTION: MIN. \triangle = \S + \S To L (L IN IN.) = 0.00953 L @ 68 °F (L IN FT.) SEE TABLE BELOW FOR VALUES @ 68° F

t = THICKNESS OF STEEL PLATE OR THICKNESS OF TOOTH..

L = EXPANDED LENGTH.

W = WIDTH OF TOOTH EXPANSION DAM.

AWS D1.5 UNLESS OTHERWISE NOTED.

 $\S = L/290$, BUT NOT LESS THAN 1" (L IN FT.).

THE VALUE OF \triangle o(T) FOR TEMPERATURE OF TIME OF DAM ERECTION OTHER THAN 68° F: $\triangle \circ (T) = \triangle \circ (68^{\circ} F) - (T-68^{\circ} F) L$

FILLET WELD SIZE SHALL BE THE 'MINIMUM FILLET WELD SIZE' AS SPECIFIED IN

 \triangle o(68°F) = \triangle o FOR T 68°F NORMAL TEMPERATURE

AS SHOWN ON PLAN.

GENERAL NOTES:

- 1. DO NOT WELD GRADE 60 STEEL REINFORCEMENT BARS UNLESS SPECIFIED.
- 2. PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408 AND AASHTO/AWS WELDING SPECIFICATIONS.
- 3. GALYANIZE STEEL IN ACCORDANCE WITH SECTION 1105.02 (s) OF PUBLICATION 408. IF SPECIFIED, PAINT ALL GALYANIZED STEEL SURFACES IN THE SHOP IN ACCORDANCE WITH PUBLICATION 408,
- 4. PROVIDE AASHTO M 270, GRADE 36 (ASTM A 709, GRADE 36), GALVANIZED, UNLESS OTHERWISE SPECIFIED ON DESIGN DRAWINGS. ANCHOR STUDS TO BE IN ACCORDANCE WITH SECTION 1105.02 (e) OF PUB. 408. STUDSMAY BE PIGGY BACKED TO ACHIEVE REQUIRED
- 5. USE FLATHEAD STAINLESS STEEL ASTM F 738 OR F 593 (TYPE 304) FOR COUNTERSUNK SCREWS WITH INSERTS. ALL CONCRETE INSERTS AND COUNTERSUNK MACHINE SCREWS ARE 3/4" DIAMETER UNLESS
- 6. BALL TYPE OR MILD STEEL KNOCK-OFF STUDS SHOULD BE PROVIDED UNLESS OTHERWISE SPECIFIED. BALL STUDS ARE TO BE % "DIAMETER BY ½" HEIGHT. KNOCK-OFF STUDS WILL BE ANTI-SKID TYPE. KNOCK-OFF STUDS ARE TO BE % NOMINAL DIAMETER BY ½" HEIGHT. ALTERNATE PATTERNS OTHER THAN SHOWN ON BALL OR KNOCK-OFF STUD DETAIL MUST BE APPROVED BY THE
- 7. ALL BOLTS TO CONFORM TO ASTM A 325.
- 8. USE THIS DRAWING AS A GUIDE IN THE PREPARATION OF SHOP DRAWINGS.
- 9. CONSTRUCT EXPANSION DAM TO MATCH ROADWAY GRADE AND CROSS
- 10. PLACE CONCRETE UNDER THE DAM AND VIBRATE UNTIL THE CONCRETE IS FORCED THROUGH THE ½ "DIAMETER AIR HOLES. STRIKE OFF EXCESS CONCRETE. AFTER CONCRETE HAS CURED, INSPECT THE HOLES AND REMOVE UNSOUND CONCRETE. CLEAN THE HOLES WITH AN AIR JET AND FILL WITH APPROVED SEALER.
- 11. CONTROL THE MAXIMUM DEPTH OF THE TROUGH SUCH THAT IT DOES NOT COME INTO CONTACT WITH THE SUBSTRUCTURE OF THE BRIDGE.
- 12. SET DAM AFTER ADJACENT DECKS HAVE BEEN PLACED. DO NOT PLACE CONCRETE IN TOP OF ABUTMENT BACKWALLS UNTIL THE BEAMS, DAMS AND DECK SLAB HAVE BEEN PLACED.
- 13. FABRICATOR TO PROVIDE A CHART SHOWING JOINT OPENING FOR TEMPERATURES BETWEEN -10°F TO 110°F FOR STEEL STRUCTURES AND 10°F TO 100°F FOR P/S CONCRETE STRUCTURES, IN 10°F INTERVALS ON SHOP DRAWINGS.
- 14. PERFORM NON-DESTRUCTIVE TESTING OF WELDS AS REQUIRED IN ACCORDANCE WITH AASHTO/AWS SPECIFICATIONS.
- 15. BEFORE PLACING BLOCKOUT CONCRETE APPLY APPROVED EPOXY BONDING AGENT TO TRANSVERSE DECK CONSTRUCTION JOINTS.
- 16. FABRICATOR TO SHOW DETAIL OF ALL SHIPPING AND ERECTION TEMPORARY ATTACHMENTS ON SHOP DRAWINGS. AFTER ERECTION, AND AFTER OPENING IS ADJUSTED FOR ERECTION TEMPERATURE, TEMPORARY ATTACHMENTS ARE TO BE REMOVED BY CHIPPING CONNECTION WELDS AND GRINDING SURFACE SMOOTH.
- 17. PLACE CLASS AAAP CEMENT CONCRETE IN THE BLOCKOUT AREA EXCEPT AS SPECIFIED OR INDICATED. THIS WORK IS INCIDENTAL TO DECK CONCRETE EXCEPT AS SPECIFIED OR INDICATED.
- 18. MAINTAIN 4" MIN. BETWEEN EDGE OF STEEL TO THE EDGE OF CONCRETE AT TEMPERATURE OF -10°F FOR STEEL AND 10°F FOR P/S CONCRETE.
 GRIND ALL EDGES EXPOSED TO TRAFFIC OR PEDESTRIANS TO %" MIN. RADIUS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

TOOTH EXPANSION DAM FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES

RECOMMENDED JAN. 31, 2019 Thomas P. Macioca

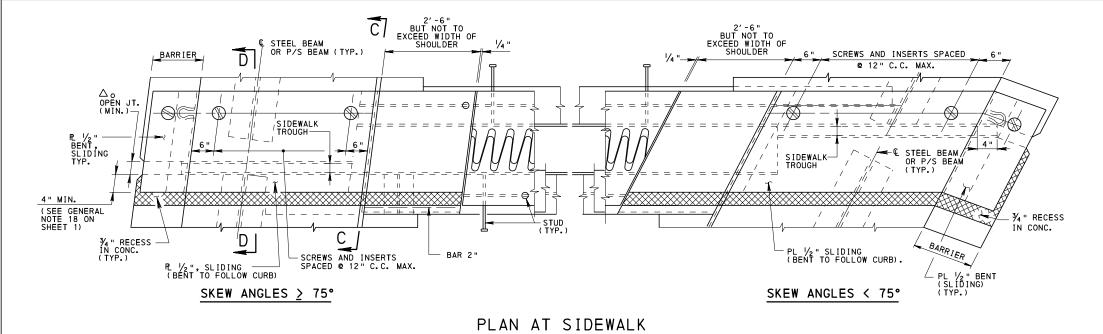
RECOMMENDED JAN. 31, 2019 Alchie Holice ACTING DIR. BUR. OF PROJECT DELIVERY BC-762M

SHEET 1 OF 7

* FOR P/S BRIDGES, USE $\frac{3}{4}$ OF \triangle 0 & \triangle VALUES IN THIS TABLE.

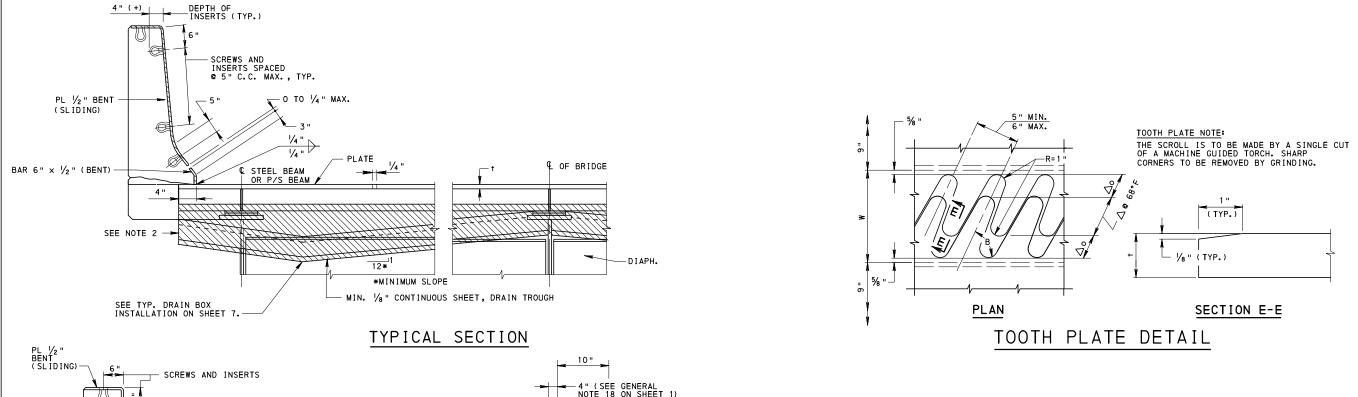
BC-734M STANDARD ANCHOR SYSTEMS WALL CONSTR. & EXPANSION JT. DETAILS BC-735M BRIDGE DRAINAGE TYPICAL WATERPROOFING AND EXPANSION

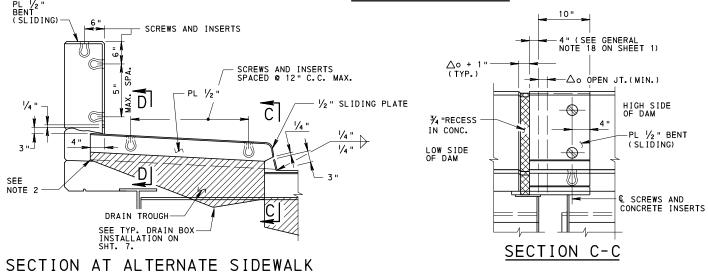
REFERENCE DRAWINGS

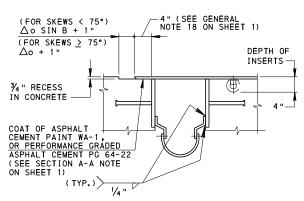


NOTES:

- 1. TO INSURE THAT INSERTS & SCREWS
 ARE ALIGNED PROPERLY, PLACE CURB
 & SIDEWALKS WITH 1/2" SIDING
 PLATES IN PLACE. APPLY BOND BREAKER
 TO SLIDING PLATES PRIOR TO
 INSTALLATION
- 2. ENDS OF DRAIN TROUGH TO BE CLOSED AND MADE WATER TIGHT IN A MANNER ACCEPTABLE TO THE DEPARTMENT.
- 3. TYPE OF DRAINAGE DISCHARGE IS DEPENDENT UPON THE LOCATION OF THE STRUCTURE.
- 4. CONTRACT DRAWINGS TO SHOW DETAILS OF TIE-IN TO EXISTING DRAINAGE SYSTEM.
- 5. TROUGH SYSTEM AS SHOWN MAY BE SUBSTITUTED BY APPROVED EQUAL.
- 6. ALL DETAILS ARE SHOWN WITH A SEPARATE SIDEWALK TROUGH. DETAILS MAY BE MODIFIED TO SHOW A SINGLE TROUGH TO REDUCE DECK DRAINS. SEE SHEET 7 FOR EXAMPLE. ALL DRAIN LOCATIONS MUST BE SHOWN ON THE DESIGN DRAWINGS.







SECTION D-D

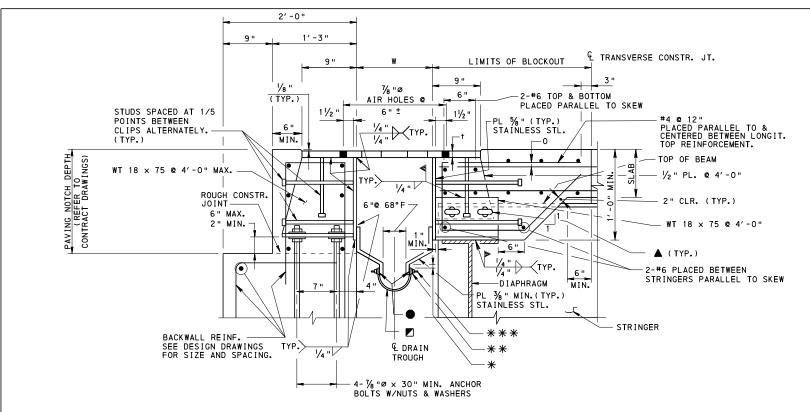
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD TOOTH EXPANSION DAM FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES

RECOMMENDED JAN. 31, 2019 Thomas P. Macioca

RECOMMENDED JAN. 31, 2019 Aldrin Abstract ACTING DIR. BUR. OF PROJECT DELIVERY BC-762M

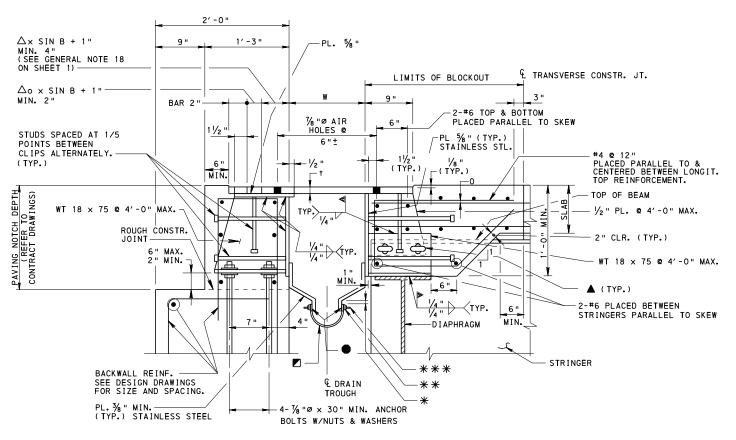
SHEET 2 OF 7



SECTION AT ABUTMENT

FOR STEEL BEAMS

FOR DECK TOP REINFORCEMENT MAT: TRANSVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.



← 3/8 " MIN.

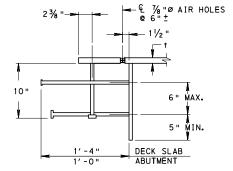
TYPICAL STUD

LEGEND:

- * VARY TO PROVIDE MINIMUM 1V: 12H. SLOPE TO DRAIN. SEE DESIGN DWG. FOR ACTUAL DESIGN SLOPE.
- ** 1½" x ¼" PL. STAINLESS STEEL (TYPE 304), FULL LENGTH OF DRAIN TROUGH.
- *** ¾ № "Ø STAINLESS STEEL STUDS WITH SELF LOCKING NUT & WASHER @ 12" C.C..
 - \blacktriangle 1" \times 1¾" SLOTTED HOLES FOR % "Ø H.S. BOLTS.
 - APPLY 1/4" BEAD OF AN EXTERIOR RATED SILICONE CAULK SEALANT PRIOR TO ASSEMBLY.
 - SEE PUBLICATION 408 SECTION 1020.3 FOR MATERIAL SPECIFICATION.

SECTION NOTES:

- 1. ALL VERTICAL STUDS ARE 3/4 "Ø
 × 10" LONG.
- 2. HORIZONTAL STUDS IN ABUTMENT ARE 3/4 "Ø x 12" LONG.
- 3. HORIZONTAL STUDS IN SLAB ARE $\frac{3}{4}$ "Ø × 16" LONG.
- 4. MINIMUM DEPTH OF CONCRETE OVER DIAPHRAGMS IS 12".
- 5. BEFORE PLACING BLOCKOUT APPLY APPROVED EPOXY BONDING AGENT TO TRANSVERSE CONSTRUCTION JOINTS.



INDIVIDUAL STUDS MAY BE BENT OR SHORTER STUDS MAY BE USED (WHERE CLEARANCE IS LIMITED), IF PERMITTED BY THE STRUCTURE CONTROL ENGINEER OR DISTRICT BRIDGE ENGINEER.

STUD DETAIL

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

TOOTH EXPANSION DAM FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES

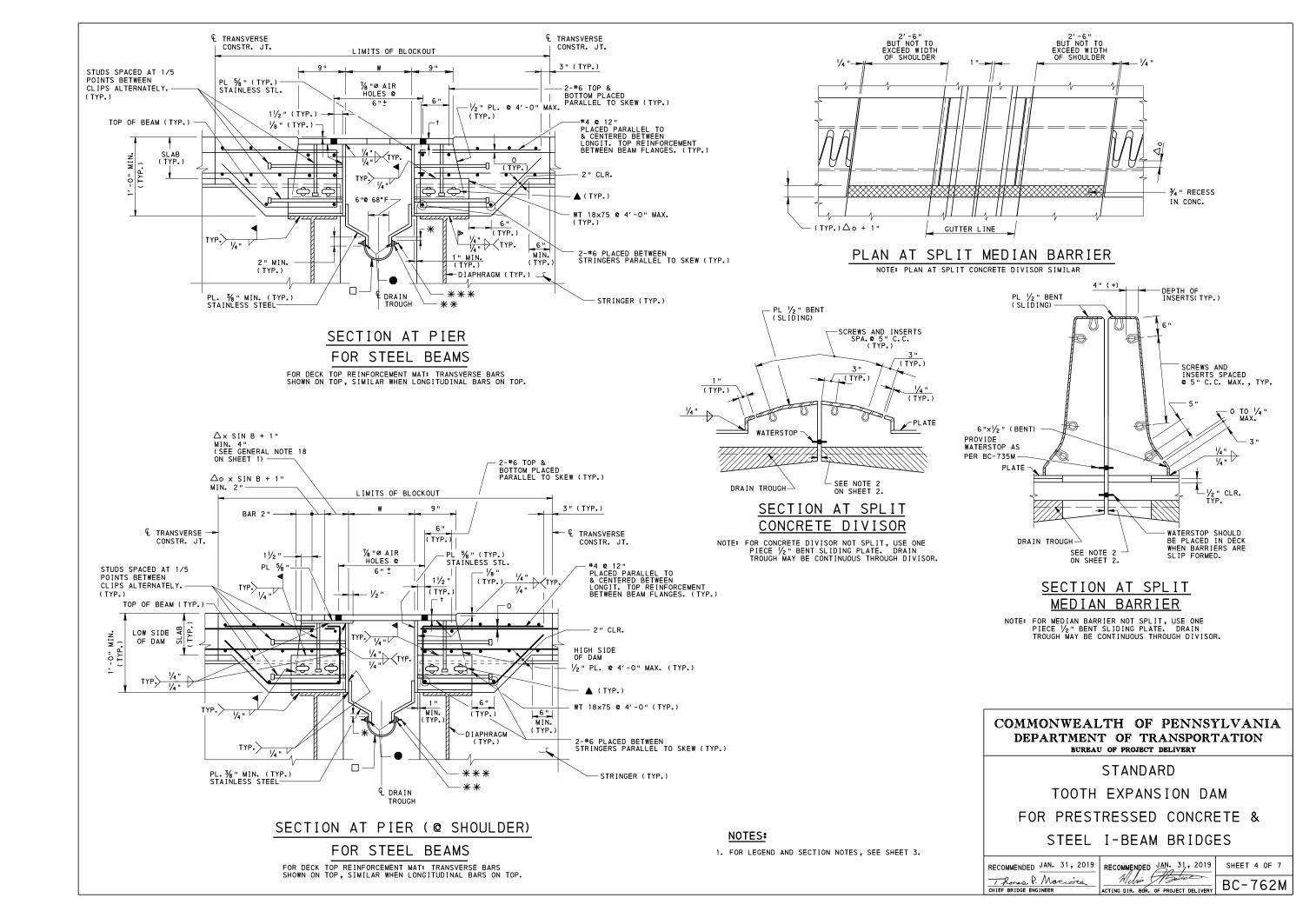
RECOMMENDED JAN. 31, 2019 Thomas P. Macióne

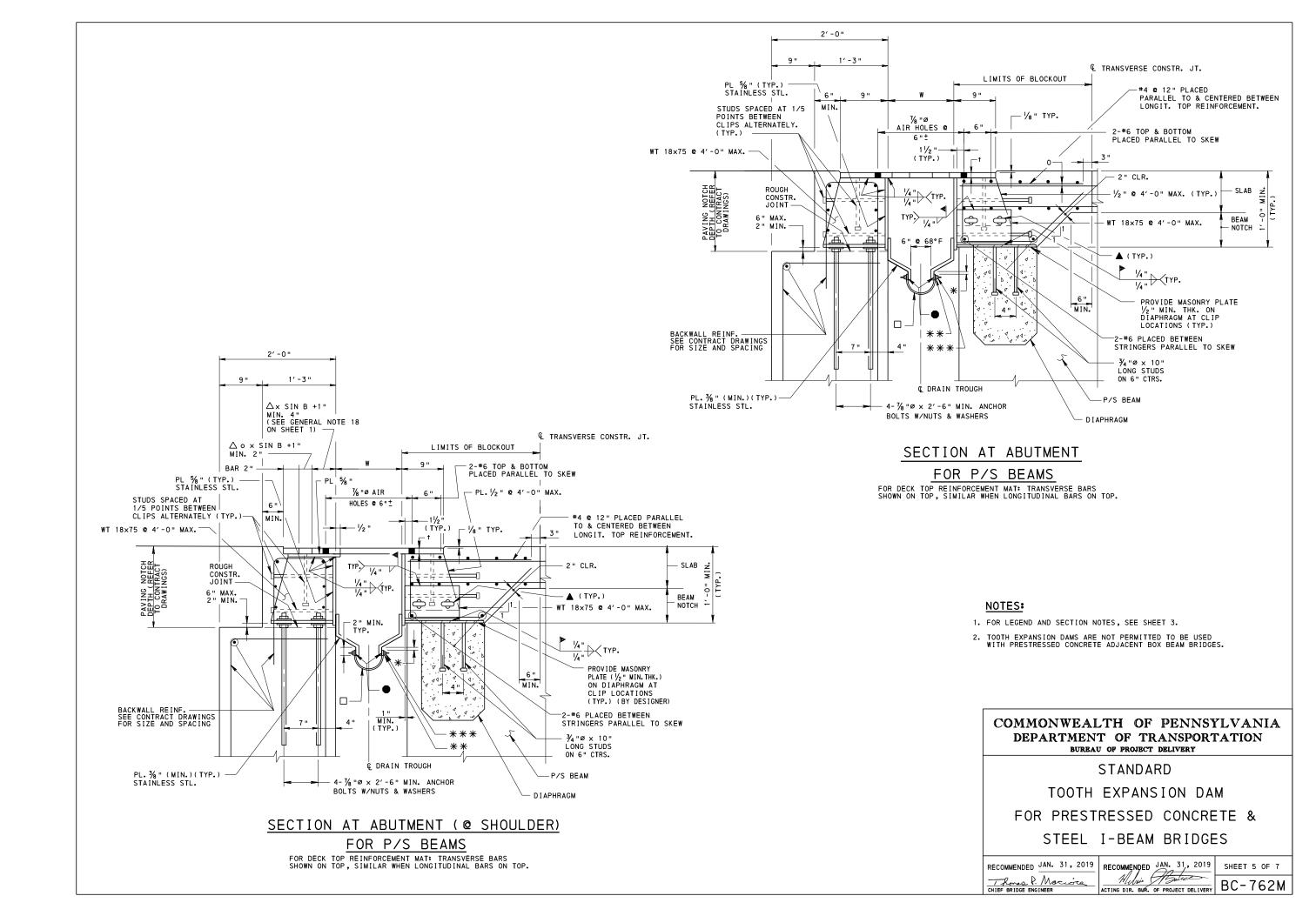
RECOMMENDED JAN. 31, 2019 SHEET 3 OF 7 Allin Astron ACTING DIR. BUR. OF PROJECT DELIVERY BC-762M

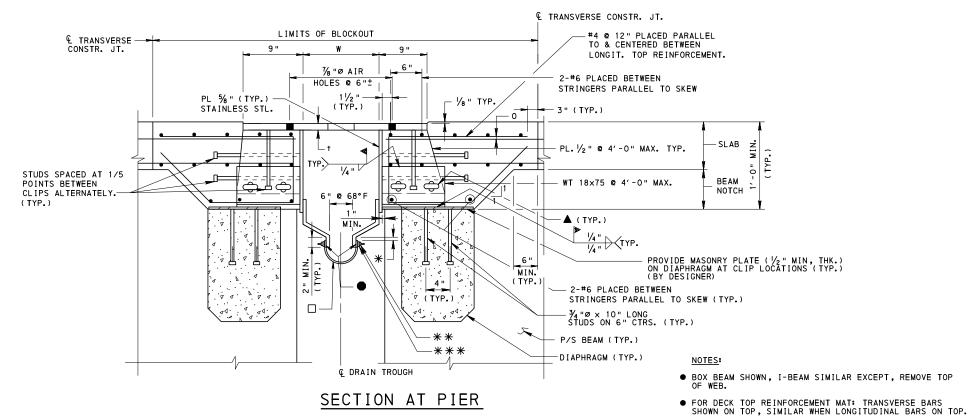
SECTION AT ABUTMENT (@ SHOULDER)

FOR STEEL BEAMS

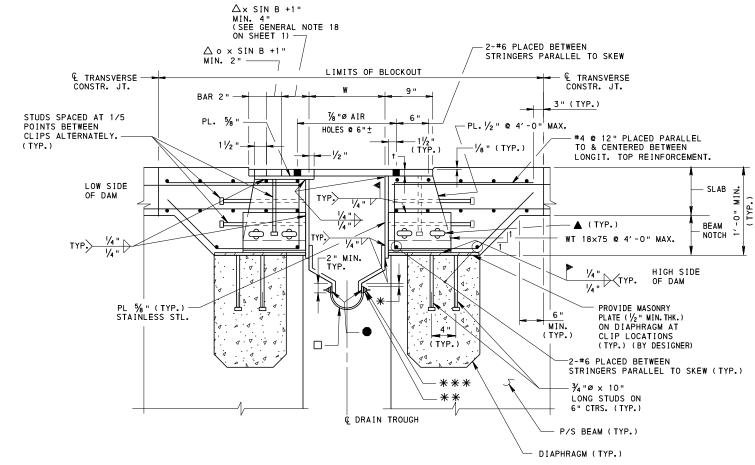
FOR DECK TOP REINFORCEMENT MAT: TRANSVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.







FOR P/S BEAMS



SECTION AT PIER (@ SHOULDER)

FOR P/S BEAMS

FOR DECK TOP REINFORCEMENT MAT: TRANSVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.

NOTES:

- 1. FOR LEGEND AND SECTION NOTES, SEE SHEET 3.
- 2. TOOTH EXPANSION DAMS ARE NOT PERMITTED TO BE USED WITH PRESTRESSED CONCRETE ADJACENT BOX BEAM BRIDGES.

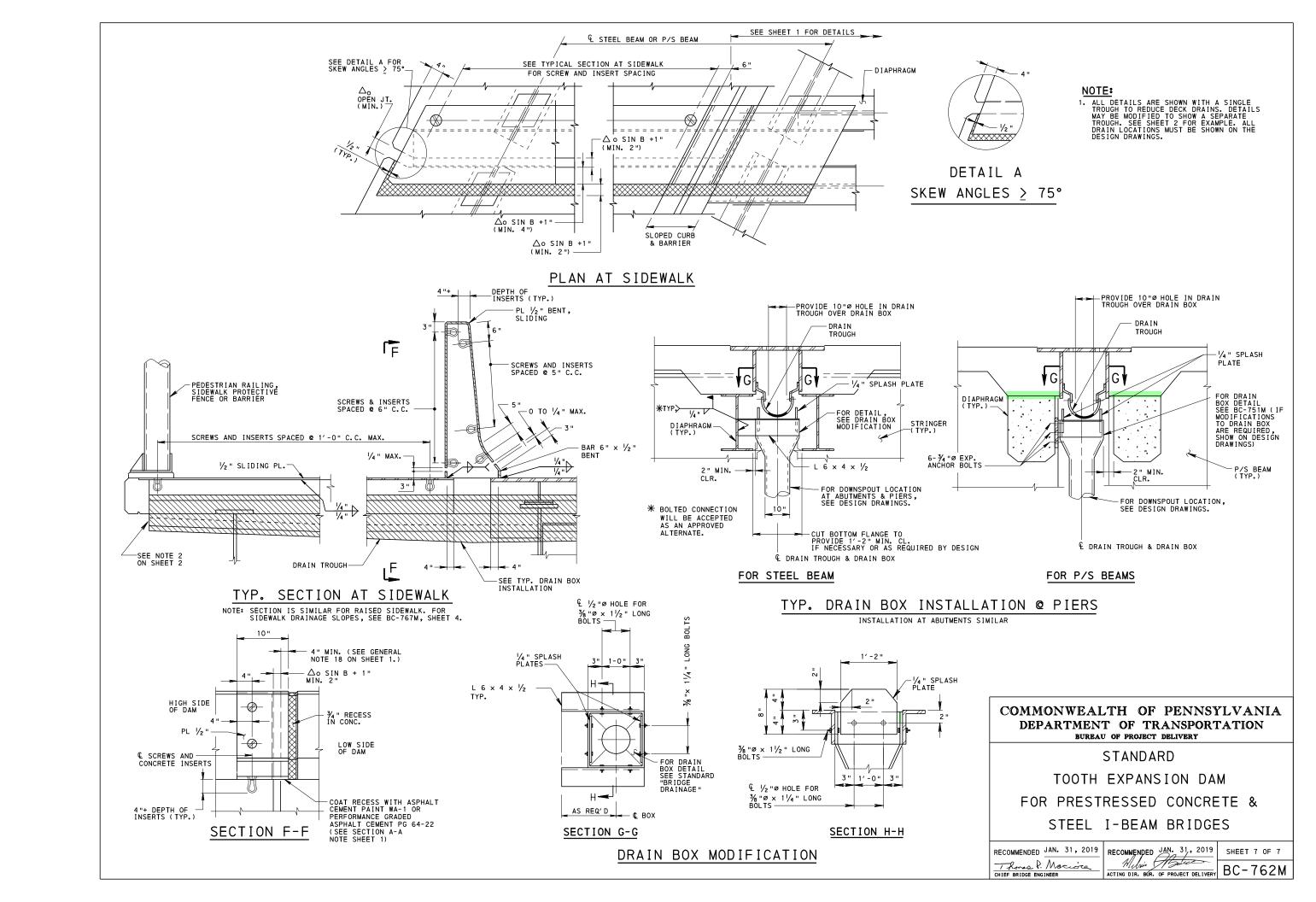
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

TOOTH EXPANSION DAM FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES

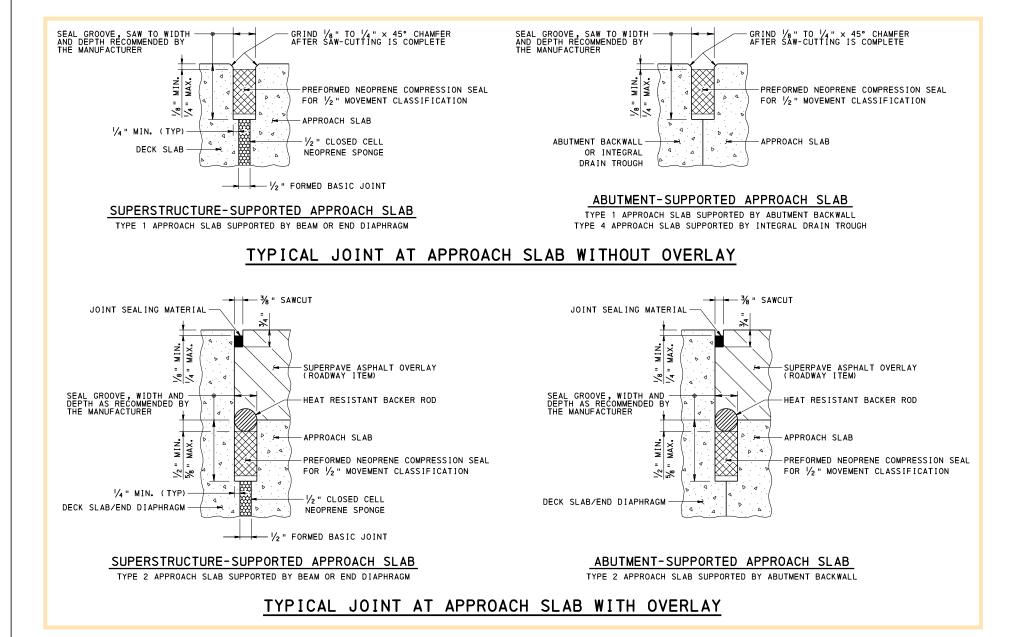
RECOMMENDED JAN. 31, 2019 Thomas P. Mariore

RECOMMENDED JAN. 31, 2019 SHEET 6 OF 7 Allin Astron ACTING DIR. BUR. OF PROJECT DELIVERY BC-762M



PLAN

APPROACH SLAB WITHOUT OVERLAY SHOWN; APPROACH SLAB WITH OVERLAY SIMILAR



CHANGE 2

CHANGE 4

GENERAL NOTES:

- PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408.
- ALL STEEL SHALL CONFORM TO AASHTO M 270, GRADE 36 (ASTM A 709 GRADE 36) UNLESS OTHERWISE SPECIFIED ON THE DESIGN DRAWINGS.
- 3. GALVANIZE STEEL IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02 (s). IF SPECIFIED, PAINT ALL GALVANIZED STEEL SURFACES IN THE SHOP IN ACCORDANCE WITH PUBLICATION 408, SECTION 1060.2 (b).
- 4. USE FLATHEAD STAINLESS STEEL ASTM F 738M (TYPE 304) FOR COUNTERSUNK SCREWS WITH INSERTS. ALL CONCRETE INSERTS AND M20 COUNTERSUNK MACHINE SCREWS ARE 3/4" DIAMETER.
- 5. USE THIS STANDARD AS A GUIDE IN THE PREPARATION OF SHOP DRAWINGS.
- 6. PROVIDE PREFORMED NEOPRENE COMPRESSION SEAL WITH 1/2" MOVEMENT CLASSIFICATION.
- THE SEALED JOINT IS CONSTRUCTED AT VARIOUS STAGES DURING DECK CONSTRUCTION; THEREFORE, PRECISELY CONTROL AND EXECUTE ALL OPERATIONS AS SPECIFIED IN SECTION 1008.3 OF PUBLICATION 408.
- 8. THE SEAL SHALL BE CONTINUOUS THROUGH THE DECK (BARRIER TO BARRIER). FIELD SPLICING OF SEAL IS NOT PERMITTED. TEMPORARY SEAL MAY BE REQUIRED DEPENDING ON THE OF CONSTRUCTION.
- 9. ASCERTAIN THAT THE TOP OF THE INSTALLED SEAL IS
 1/8" MINIMUM TO 1/4" MAXIMUM BELOW THE DECK SURFACE
 AND THAT THE INTERSECTION OF THE VERTICAL AND THE
 HORIZONTAL SEALS AT THE GUTTER LINE IS WATERPROOF.
- 10. FOR JOINTS AT SHARP SKEWS, MODIFICATIONS TO BE MADE AS REQUIRED, BY THE DESIGNER, AND SHOWN ON THE DESIGN DRAWINGS.
- 11. USE DOUBLE BLADE FOR SAW-CUTTING THE SEAL GROOVE.
- 12. TO INSURE THAT INSERTS AND SCREWS ARE ALIGNED PROPERLY, PLACE CURBS AND SIDEWALKS WITH 1/2" SLDIDING PLATES IN PLACE. APPLY BOND BREAKER TO SLIDING PLATES PRIOR TO INSTALLATION.
- 13. DETAILS SHOWN ON SHEET 2 ARE APPLICABLE FOR A
 TYPE 1 APPROACH SLAB (WITHOUT OVERLAY) SUPPORTED BY
 THE SUPERSTRUCTURE. DETAILS FOR OTHER APPROACH SLAB
 TYPES AND/OR ABUTMENT-SUPPORTED SLABS ARE SIMILAR.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD
PREFORMED NEOPRENE COMPRESSION
SEAL JOINT FOR APPROACH SLABS

RECOMMENDED NOV. 23, 2022

RECOMMENDED NOV. 23, 2022

Haw E. Hray

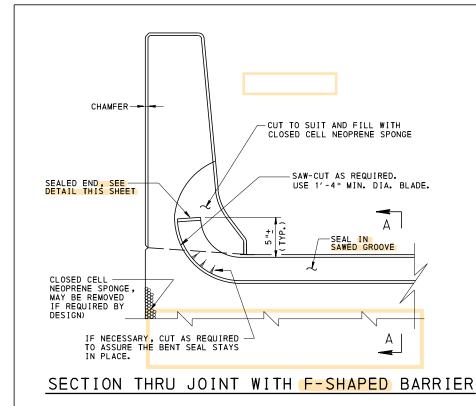
CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 1 OF 2
BC-766M

BC-735M WALL CONSTRUCTION & EXPANSION JOINT DETAILS

REFERENCE DRAWINGS

BC-735M WALL



PROVIDE WATERSTOP AS PER BC-735M (TYP.) OPTIONAL WATERSTOP - SEALED END (TYP.), SEE SHOULD BE PLACED IN DECK WHEN BARRIERS DETAIL THIS SHEET. ARE SLIPFORMED

SEALED END, SEE DETAIL THIS SHEET ** SLOPE SIDEWALK TO DRAIN SECTION THRU JOINT AT TYPICAL SIDEWALK NOTE: FOR ADDITIONAL INFORMATION, SEE SECTION THRU JOINT WITH F-SHAPED BARREIR. THIS SHEET.

PEDESTRIAN RAILING, SIDEWALK PROTECTIVE FENCE OR BARRIER

SEAL IN SAWED GROOVE

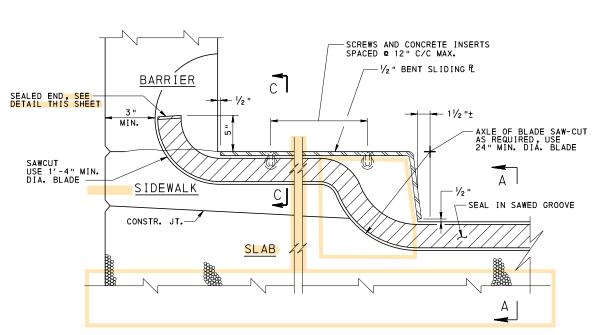
SCREWS AND CONCRETE INSERTS SPACED @ 12" C/C MAX.

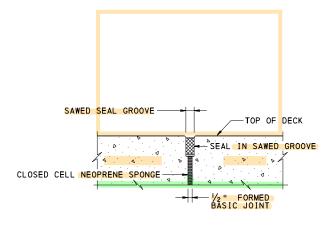
· 1/2 " SLIDING ₽

SEALED DETAIL

SECTION AT SPLIT MEDIAN BARRIERS

NOTE: FOR ADDITIONAL INFORMATION, SEE SECTION THRU JOINT WITH F-SHAPED BARREIR. THIS SHEET.





SECTION A-A NOTE: FOR ADDITIONAL INFORMATION, SEE JOINT DETAILS ON SHEET 1.

SEAL GROOVE WIDTH
(SEE DETAILS ON SHEET 1) 1/2 " SLIDING ₽ APPROVED SEALER, TOP SURFACE TO BE SMOOTH INSERT RECESS IN CONCRETE 1/4"× 45° CHAMFER

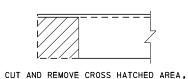
SECTION C-C

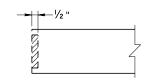
NOTE: FOR ADDITIONAL INFORMATION, SEE JOINT DETAILS ON SHEET 1.

FORM CONCRETE RECESS AREA IN SIDEWALK AND GRIND TO PROVIDE SMOOTH SURFACE. APPLY ONE COAT OF ASPHALT CEMENT PAINT WA-1 OR PERFORMANCE GRADED ASPHALT PG 64-22 TO ALLOW BENT SLIDING PLATE TO MOVE FREELY WITHOUT FRICTION.

SECTION THRU JOINT WITH ALTERNATE SIDEWALK

NOTE: FOR ADDITIONAL INFORMATION, SEE SECTION THRU JOINT WITH F-SHAPED BARREIR. THIS SHEET.





THEN BEND TOP OVER TO SEAL END.

CUT OUT AND REMOVE SEALS' WEBS AND REPLACE WITH CLOSED CELL

ALTERNATE "A"

ALTERNATE "B"

ALTERNATES FOR SEALED ENDS

SEAL CONTACT AREA WITH APPROVED ADHESIVE.

NOTES:

1. DETAILS SHOWN ON THIS SHEET ARE APPLICABLE FOR A TYPE 1 APPROACH SLAB (WITHOUT OVERLAY) SUPPORTED BY THE SUPERSTRUCTURE. DETAILS FOR OTHER APPROACH SLAB TYPES AND/OR ABUTMENT-SUPPORTED SLABS ARE SIMILAR.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD PREFORMED NEOPRENE COMPRESSION SEAL JOINT FOR APPROACH SLABS

RECOMMENDED NOV. 23, 2022

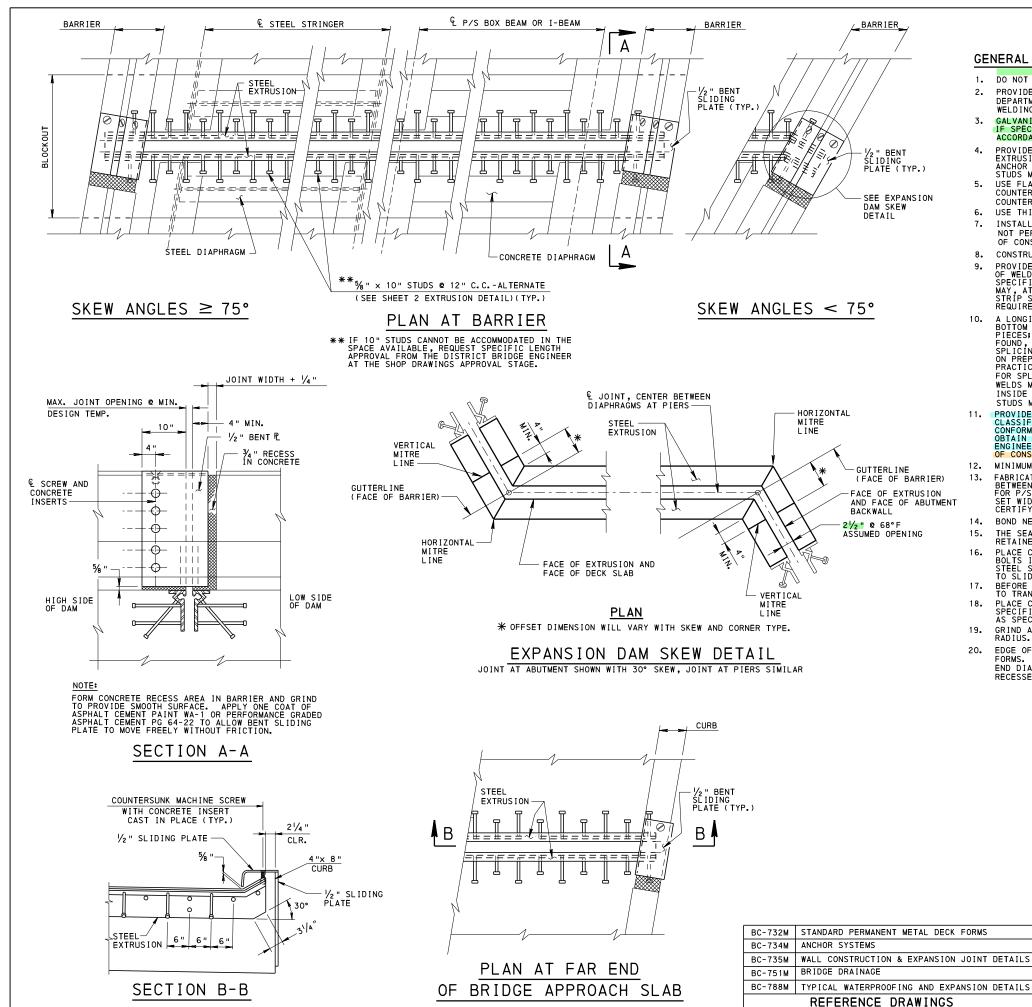
RECOMMENDED NOV. 23, 2022 SHEET 2 OF 2 Havin E. Hray

HIEF ENGINEER, HIGHWAY ADMIN

BC-766M

-SEAL IN SAWED GROOVE

CONTINUOUS SEAL FOR SKEWS 75° TO 90° ONLY



CHANGE 2

CHANGE 3

CHANGE 4

GENERAL NOTES:

- DO NOT WELD GRADE 60 STEEL REINFORCEMENT BARS UNLESS SPECIFIED.
- PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408 AND AASHTO/AWS WELDING SPECIFICATIONS.
- GALVANIZE STEEL IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02 (s). IF SPECIFIED, PAINT ALL GALVANIZED STEEL SURFACES IN THE SHOP IN ACCORDANCE WITH PUBLICATION 408, SECTION 1060.2 (b).
- PROVIDE AASHTO M270 GRADE 36 (ASTM AT09 GRADE 36), GALVANIZED, FOR STEEL EXTRUSION UNLESS OTHERWISE SPECIFIED ON THE DESIGN DRAWINGS. ANCHOR STUDS TO BE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(e). STUDS MAY BE PIGGY BACKED TO ACHIEVE REQUIRED LENGTH.
- USE FLATHEAD STAINLESS STEEL ASTM F738M OR F593 (TYPE 304) FOR COUNTERSUNK SCREWS WITH INSERTS. ALL CONCRETE INSERTS AND M 20 COUNTERSUNK MACHINE SCREWS ARE 3/4 " DIAMETER UNLESS OTHERWISE NOTED.
- USE THIS STANDARD DRAWING AS A GUIDE IN THE PREPARATION OF SHOP DRAWINGS.
- INSTALL CONTINUOUS NEOPRENE STRIP SEAL IN THE FIELD. SPLICING OF SEAL IS NOT PERMITTED. TEMPORARY SEAL MAY BE REQUIRED DEPENDING ON STAGES OF CONSTRUCTION.
- CONSTRUCT EXPANSION DAM TO MATCH ROADWAY GRADE AND CROSS SLOPE.
- PROVIDE ALL WELDING IN THE SHOP. PROVIDE NON-DESTRUCTIVE TESTING OF WELDS IF REQUIRED BY THE ENGINEER, IN ACCORDANCE WITH AASHTO/AWS SPECIFICATIONS. EXPANSION DAMS GREATER THAN 40' IN LENGTH, MAY, AT THE DISCRETION OF THE FABRICATOR, HAVE THE RETAINERS FOR THE STRIP SEALS WELDED IN THE FIELD. REFER TO NOTE 10 FOR SPECIAL REQUIREMENTS AND DETAILS FOR TRANSVERSE BUTT WELDS.
- A LONGITUDINAL FULL PENETRATION WELD IS REQUIRED TO JOIN THE TOP AND BOTTOM (PLATE) OF THE RETAINER TOGETHER WHEN IT IS FURNISHED IN TWO PIECES; 25% OF THE WELD TO BE TESTED ULTRASONICALLY, AND IF DEFECTS ARE FOUND, 100% OF THE WELD TO BE TESTED. TRANSVERSE BUTT WELDS FOR SPLICING THE RETAINER TO BE PARTIAL PENETRATION DOUBLE V-GROOVE WELDS ON PREPARED BEVELED EDGES, EXTENDING ALL AROUND THE JOINT AS FAR AS PRACTICAL TO ACHIEVE A WATERTIGHT SEAL. FULL PENETRATION GROOVE WELD FOR SPLICING THE RETAINER IN LIEU OF PARTIAL PENETRATION DOUBLE V-GROOVE WELDS MAY BE USED IF SHOWN ON APPROVED SHOP DRAWINGS. DO NOT WELD INSIDE SEAL CAVITY. WHEREVER A TRANSVERSE JOINT OCCURS, TWO ADDITIONAL STUDS MIST BE ADDED ON FACH SIDE OF THE JOINT. STUDS MUST BE ADDED ON EACH SIDE OF THE JOINT.
- PROVIDE SEALS WITH MOVEMENT CLASSIFICATION NOT LESS THAN THE CLASSIFICATION SPECIFIED ON THE DESIGN DRAWINGS. ALL SEALS SHALL CONFORM TO THE REQUIREMENTS OF PUBLICATION 408, SECTION 705. OBTAIN APPROVAL FOR USE OF THE SEAL FROM THE CHIEF MATERIALS ENGINEER, LABORATORY TESTING SECTION, MATERIALS DIVISION, BUREAU OF CONSTRUCTION AND MATERIALS.
- MINIMUM MOVEMENT CLASSIFICATION IS 3".
- FABRICATOR TO PROVIDE A CHART SHOWING JOINT OPENING FOR TEMPERATURES BETWEEN -10°F TO 110°F FOR STEEL STRUCTURES AND 10°F TO 100°F FOR P/S CONCRETE STRUCTURES, IN 10°F INTERVALS ON SHOP DRAWINGS. SET WIDTH © 68°F. ALSO FURNISH MOVEMENT OF SEAL ON SKEW AND CERTIFY THAT SEAL IS CORRECT FOR DESIGN.
- BOND NEOPRENE STRIP SEAL TO EXTRUSION WITH APPROVED ADHESIVE.
- THE SEALS FURNISHED WITH THE RETAINERS MUST BE COMPATIBLE WITH THE RETAINER AND PROVIDE A WATERTIGHT JOINT.
- PLACE CURBS AND SIDEWALKS WITH STEEL SLIDING PLATES, WITH INSERTS AND BOLTS IN PLACE TO INSURE PROPER ALIGNMENT OF INSERTS AND HOLES IN THE STEEL SLIDING PLATE. REMOVE PLATES TO INSTALL SEAL. APPLY BONDBREAKER TO SLIDING PLATES PRIOR TO INSTALLATION.
- BEFORE PLACING BLOCKOUT CONCRETE, APPLY APPROVED EPOXY BONDING AGENT TO TRANSVERSE CONSTRUCTION JOINTS.
- PLACE CLASS AAAP CEMENT CONCRETE IN THE BLOCKOUT AREA EXCEPT AS SPECIFIED OR INDICATED. THIS WORK IS INCIDENTAL TO CONCRETE DECK EXCEPT AS SPECIFIED OR INDICATED.
- GRIND ALL STEEL EDGES EXPOSED TO TRAFFIC OR PEDESTRIANS TO 36" MIN.

NOV.23, 2022

EDGE OF DECK SURFACES SHOWN ARE OBTAINED WITH USE OF REMOVABLE WOOD FORMS. WHEN PERMANENT METAL FORMS ARE USED, PLACE FORM ON TOP OF STEEL END DIAPHRAGMS AND BEHIND EXPANSION DAM EXTRUSIONS WHICH WILL RESULT IN A RECESSED FRONT FACE BELOW EXTRUSIONS.

COMMONWEALTH OF PENNSYLVANIA

DEPARTMENT OF TRANSPORTATION

NEOPRENE STRIP SEAL DAM

FOR PRESTRESSED CONCRETE

& STEEL I-BEAM BRIDGES

RECOMMENDED NOV. 23, 2022

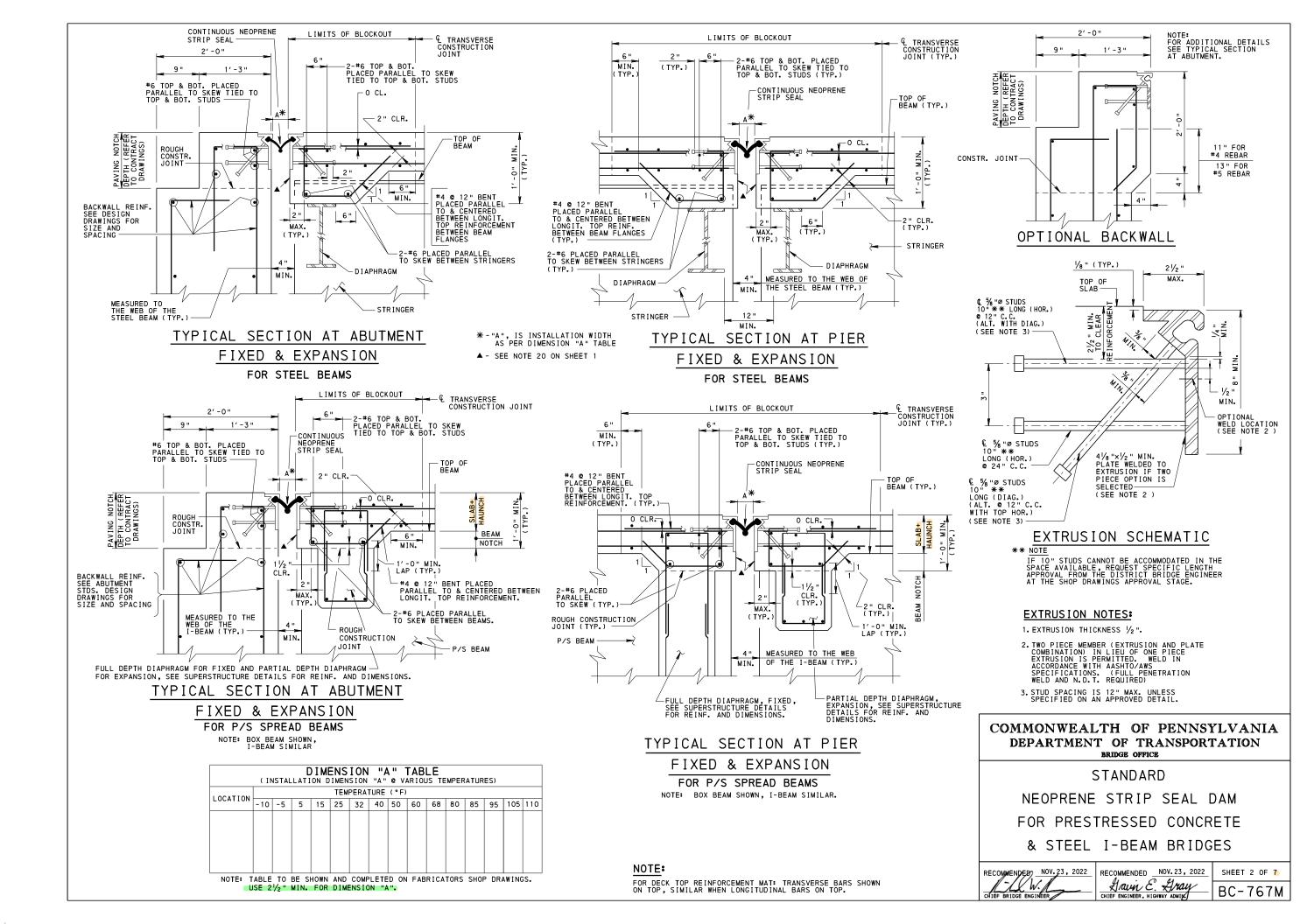
Havin E. Hray

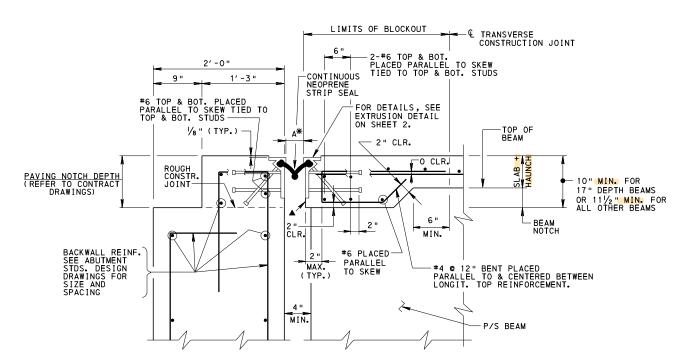
HIEF ENGINEER, HIGHWAY ADMIN

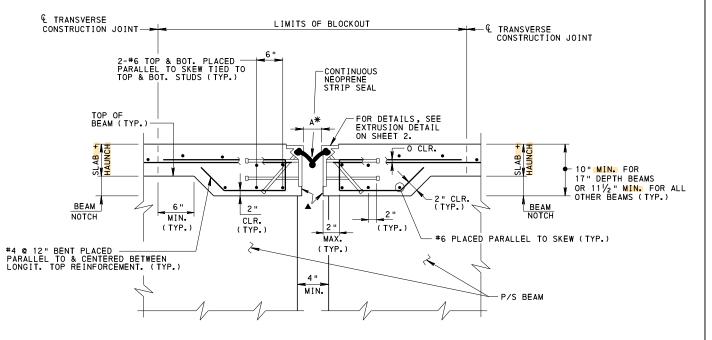
SHEET 1 OF 7

BC-767M

BRIDGE OFFICE STANDARD







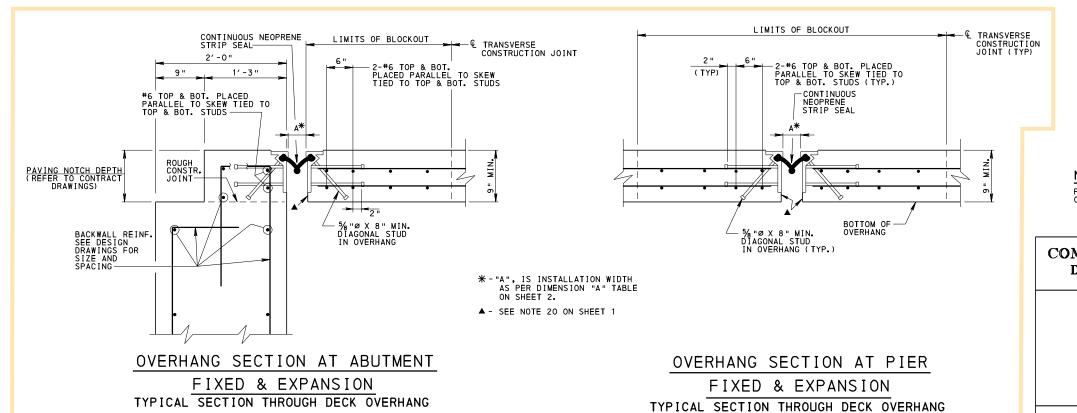
TYPICAL SECTION AT ABUTMENT
FIXED & EXPANSION
FOR P/S ADJACENT BEAMS

*-"A", IS INSTALLATION WIDTH AS PER DIMENSION "A" TABLE ON SHEET 2.

▲ - SEE NOTE 20 ON SHEET 1

TYPICAL SECTION AT PIER

FIXED & EXPANSION FOR P/S ADJACENT BEAMS



NOTE:

FOR DECK TOP REINFORCEMENT MAT: TRANSVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

NEOPRENE STRIP SEAL DAM FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES

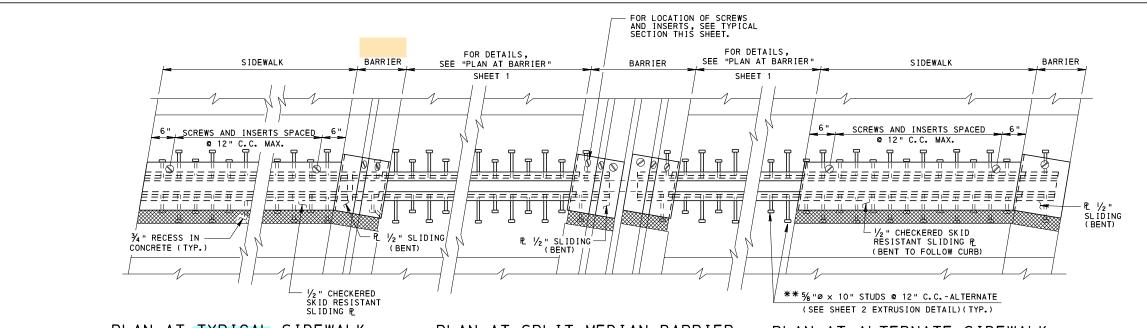
RECOMMENDED NOV. 23, 2022

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Law E. Gray

CHIEF ENGINEER, HIGHWAY ADMIN

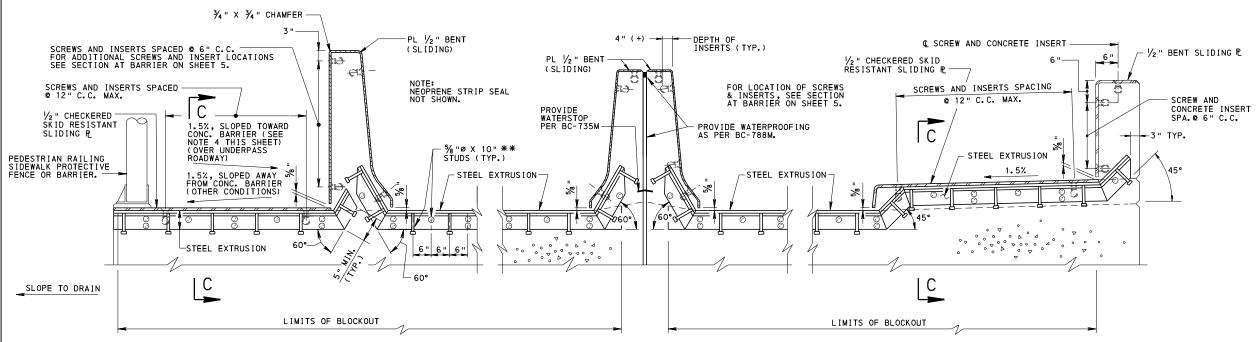
SHEET 3 OF 7 BC-767M



PLAN AT TYPICAL SIDEWALK

PLAN AT SPLIT MEDIAN BARRIER

PLAN AT ALTERNATE SIDEWALK



SECTION AT TYPICAL SIDEWALK

NOTE: SECTION IS SIMILAR FOR RAISED SIDEWALK.

SECTION AT_ SPLIT MEDIAN BARRIER

NOTE: FOR MEDIAN BARRIER NOT SPLIT, USE ONE PIECE V_2 " BENT SLIDING PLATE. STEEL EXTRUSION IS SHOWN FOR 90° SKEW. DETAIL STEEL EXTRUSION AS REQUIRED FOR SKEWS LESS THAN 90°.

** NOTE:

TF 10" STUDS CANNOT BE ACCOMMODATED IN THE SPACE AVAILABLE, REQUEST SPECIFIC LENGTH APPROVAL FROM THE DISTRICT BRIDGE ENGINEER AT THE SHOP DRAWINGS APPROVAL STAGE.

NOTES:

1. MAXIMUM DISTANCE FROM EDGE OF EXTENSION OR BEND TO FIRST STUD IS 3 ".

SECTION AT ALTERNATE SIDEWALK

- 2. FOR STEEL EXTRUSION DETAIL, SEE SHEET 2.
- 3. FOR SECTION C-C, SEE SHEET 5.
- 4. DRAIN RUNOFF WITH CURB DRAINS THROUGH CONCRETE BARRIER OR WITH TYPE 2 SCUPPERS IN SIDEWALK SLAB. WHERE CURB DRAINS ARE USED, SET SIDEWALK ELEVATION AT REAR FACE OF BARRIER 1" ABOVE GUTTERLINE ELEVATION. BEVEL DRAINS AS PER BC-751M.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

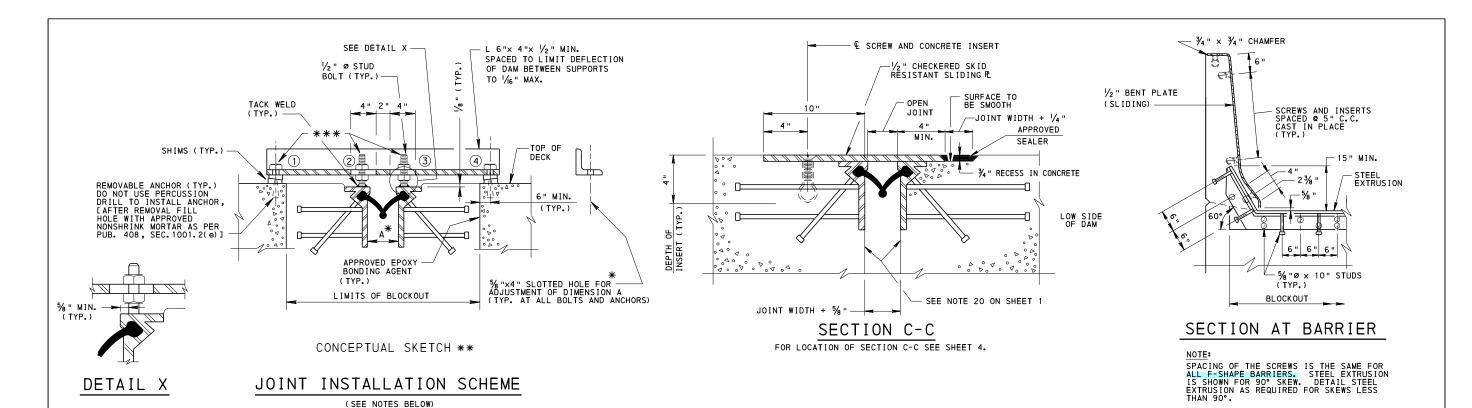
NEOPRENE STRIP SEAL DAM FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES

RECOMMENDED) NOV.23, 2022

RECOMMENDED NOV. 23, 2022 SHEET 4 OF 7

Law E. Law BC-767M

CHIEF ENGINEER, HIGHWAY ADMIN.



JOINT INSTALLATION NOTES:

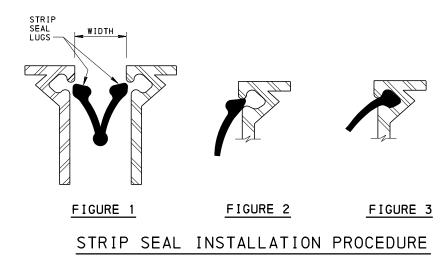
- * TO BE ADJUSTED FOR INSTALLATION TEMPERATURE FOR SPECIFIC MOVEMENT CLASSIFICATION.
- ** CONTRACTOR MAY USE ALTERNATE SCHEMES ONLY WITH THE APPROVAL OF THE DISTRICT BRIDGE ENGINEER OR DISTRICT STRUCTURE CONTROL ENGINEER.
- ***

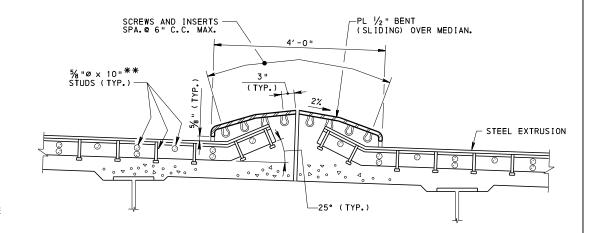
 DURING ASSEMBLY INSTALLATION TIGHTEN
 BOLT ① AND BOTH NUTS ON STUD BOLTS
 ② AND ③ MAKING SURE DIMENSION A*
 REFLECTS THE INSTALLATION TEMPERATURE.
 - DO NOT TIGHTEN BOLT 4
 - IMMEDIATELY AFTER BLOCKOUT IS CAST, LOOSEN BOTTOM NUT ON STUD BOLT ③ TO PERMIT TEMPERATURE MOVEMENT IN THE ASSEMBLY ON ONE SIDE OF JOINT.
 - PROVIDE LOW FRICTION INTERFACE BETWEEN BOLT HEADS AND ANGLE AND TOP NUTS ON STUD BOLTS AND ANGLE.

 - 1. THE SURFACE OF THE BLOCKOUT MUST BE COMPLETELY CLEAN WHEN THE JOINT IS INSTALLED.
 - 2. THE EXPANSION JOINT DEVICE MUST BE SUSPENDED IN THE BLOCKOUT TO THE PROPER LINE AND GRADE, WITH THE DISTANCE BETWEEN EXTRUSIONS SET WITH RESPECT TO THE TEMPERATURE AS SHOWN ON THE PLANS.
 - 3. AFTER THE CONCRETE OF THE BLOCKOUT ACHIEVES
 PRESCRIBED STRENGTH IN ACCORDANCE WITH PUBLICATION 408
 SECTION 1001.3 (q) 1, REMOVE THE TEMPORARY SUPPORT
 ASSEMBLY AND GRIND OFF TACK WELD UNTIL SMOOTH.
 - 4. APPLY TOUCH-UP PAINT.

STRIP SEAL INSTALLATION NOTES

- 1. THE FRAME RAILS SHALL BE CLEANED THOROUGHLY AND SEAL CHANNELS SHALL BE INSPECTED TO ASCERTAIN THE ABSENCE OF CONCRETE AND DEBRIS. THE SEAL CHANNEL SHALL ALSO BE INSPECTED AT ALL FIELD SPLICES, AND ALL WELD SPLATTER AND/OR SHARP EDGES SHALL BE REMOVED.
- 2. LIBERALLY COAT THE STRIP SEAL LUGS WITH LUBRICANT ADHESIVE. COAT ONLY 3'-0" TO 4'-0" PRECEDING THE INSTALLATION.
- 3. COLLAPSE THE STRIP SEAL INTO THE THE JOINT OPENING UNTIL THE LUG IS ALIGNED WITH THE FRAME RAIL CHANNEL. (SEE FIGURE 1)
- 4. PUSH THE LUG INTO THE CHANNEL AND THEN USE A BENT BAR TO FORCE THE LUG INTO THE CHANNEL (MAKE SURE THAT THE BAR IS DULL TO PREVENT PUNCTURING OF THE SEAL) (SEE FIGURE 2)
- 5. AFTER THE SEAL LOCKS INTO PLACE, PUSH THE TOP OF THE LUG AGAINST THE FRAME RAIL TO INSURE PROPER SEATING. (SEE FIGURE 3)
- 6. AS THE WORK PROGRESSES DOWN THE LENGTH OF THE JOINT, WORK BOTH SIDES OF THE STRIP SEAL INTO THE RAIL CHANNEL.





SECTION AT SPLIT CONCRETE MOUNTABLE DIVISOR

NOTE: FOR CONCRETE DIVISOR NOT SPLIT, USE ONE PIECE 1/2" BENT SLIDING PLATE.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

NEOPRENE STRIP SEAL DAM FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES

RECOMMENDED NOV. 23, 2022

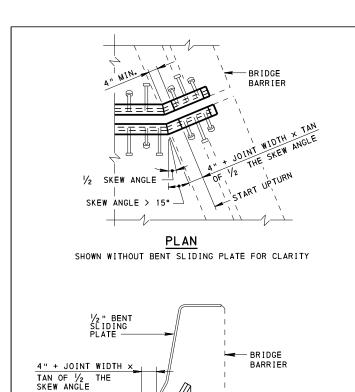
RECOMMENDED NOV. 23, 2022

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CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 5 OF 7

BC-767M



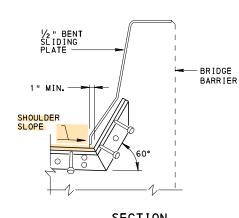
SECTION

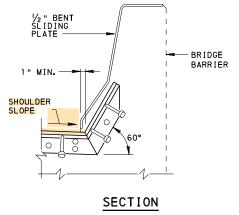
EXPANSION DAM JOINT

MITERED AT BARRIER FACE

SKEW ANGLES > 15° PERPENDICULAR

SHOULDER SLOPE



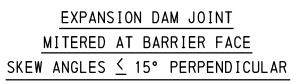


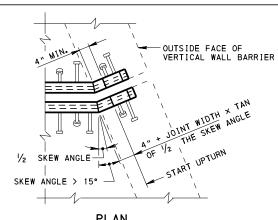
PLAN

SHOWN WITHOUT BENT SLIDING PLATE FOR CLARITY

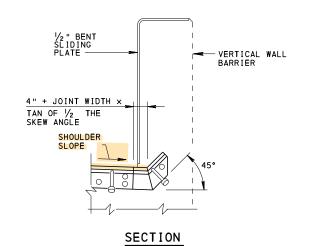
START UPTURN

BRIDGE BARRIER

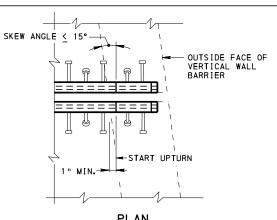




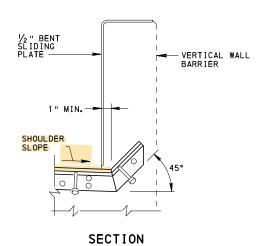
SHOWN WITHOUT BENT SLIDING PLATE FOR CLARITY



EXPANSION DAM JOINT MITERED AT VERTICAL WALL BARRIER FACE SKEW ANGLES > 15° PERPENDICULAR



SHOWN WITHOUT BENT SLIDING PLATE FOR CLARITY



EXPANSION DAM JOINT MITERED AT VERTICAL WALL BARRIER FACE SKEW ANGLES ≤ 15° PERPENDICULAR

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

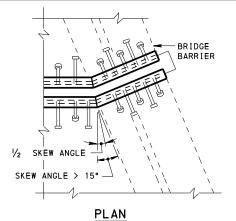
STANDARD NEOPRENE STRIP SEAL DAM MISCELLANEOUS DETAILS FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES

RECOMMENDED NOV. 23, 2022

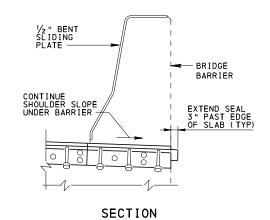
Haw E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN

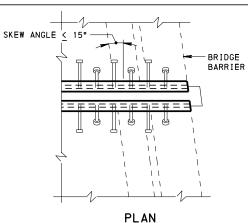
SHEET 6 OF 7 BC-767M



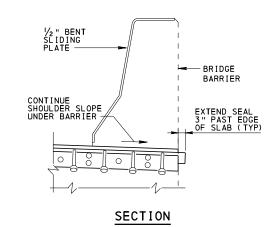




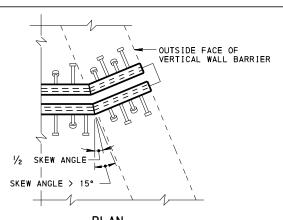
EXPANSION DAM JOINT MITERED AT BARRIER FACE SKEW ANGLES > 15° PERPENDICULAR



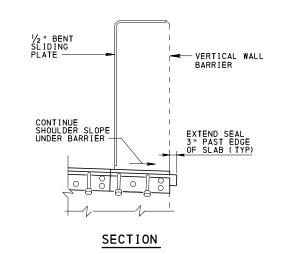
SHOWN WITHOUT BENT SLIDING PLATE FOR CLARITY



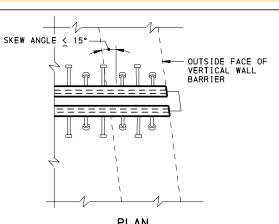
EXPANSION DAM JOINT WITHOUT MITER UNDER BARRIER SKEW ANGLES ≤ 15° PERPENDICULAR



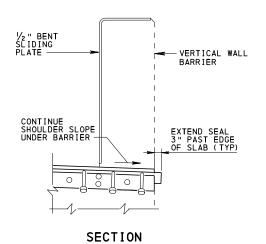
SHOWN WITHOUT BENT SLIDING PLATE FOR CLARITY



EXPANSION DAM JOINT MITERED AT VERTICAL WALL BARRIER FACE SKEW ANGLES > 15° PERPENDICULAR



SHOWN WITHOUT BENT SLIDING PLATE FOR CLARITY



EXPANSION DAM JOINT WITHOUT MITER UNDER VERTICAL WALL BARRIER SKEW ANGLES ≤ 15° PERPENDICULAR

NOTE:

ALTERNATE DETAILS MAY ONLY BE USED IF SHOWN ON THE CONTRACT PLANS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

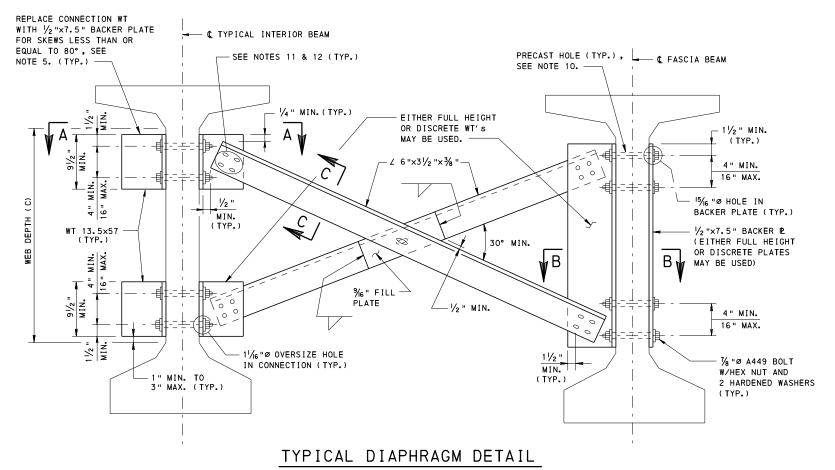
STANDARD ALTERNATE NEOPRENE STRIP SEAL DAM MISCELLANEOUS DETAILS FOR PRESTRESSED CONCRETE & STEEL I-BEAM BRIDGES

RECOMMENDED NOV. 23, 2022

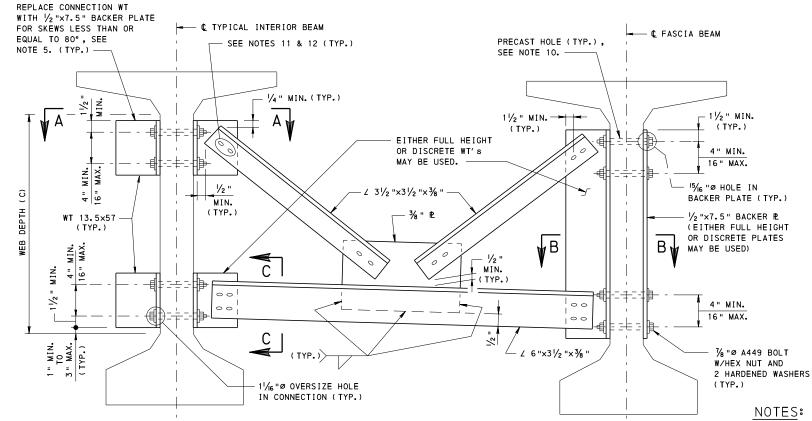
Haw E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN SHEET 7 OF 7

BC-767M



CHANGE 2



ALTERNATE DIAPHRAGM DETAIL

GENERAL NOTES:

- 1. PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH PUB. 408 AND AASHTO/AWS D1.5 SPECIFICATIONS. ALL STRUCTURAL STEEL INCLUDING BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED AS PER PUB. 408, SECTION 1105.02(S).
- 2. FABRICATED STRUCTURAL STEEL SHALL CONFORM TO AASHTO M270, GRADE 50 (ASTM A709, GRADE 50) UNLESS OTHERWISE NOTED.
- 3. ALL FASTENERS ARE 1/8" DIAMETER ASTM F3125 GRADE A325 BOLTS, EXCEPT AS NOTED.
- 4. MEMBERS, WELDS AND PLATE SIZES SHOWN ARE VALID FOR STRAIGHT GIRDERS WITH SPAN LENGTH OF 160'-0" OR LESS, GIRDER SPACING BETWEEN 6'-0" AND 10'-9" AND FOR SKEW ANGLES BETWEEN 45° AND 90°. PROVIDE SPECIAL DESIGNS FOR ALL THE DIAPHRAGM MEMBERS AND CONNECTIONS WHEN THE GIRDER SPACING EXCEEDS 10'-9" AND/OR THE SKEW ANGLE IS
- 5. PROVIDE DIAPHRAGMS NORMAL TO THE MAIN MEMBERS FOR ALL SKEWS. FOR SKEWS LESS THAN OR EQUAL TO 80°, STAGGER DIAPHRAGMS AND USE BACKER PLATES ON INTERIOR BEAM CONNECTIONS.
- 6. DIAPHRAGMS SHALL BE SLOPED ACCORDING TO THE ELEVATION OF THE BEAMS. DIAPHRAGMS IN EXTERNAL BAYS FOR SPANS WITH VERTICAL CLEARANCE LESS THAN 16'-0" OVER VEHICULAR TRAFFIC MAY BE LEVEL.
- 7. USE THIS STANDARD IN THE PREPARATION OF SHOP DRAWINGS.
- 8. INCLUDE PAYMENT FOR FURNISHING AND INSTALLING STEEL MID-SPAN DIAPHRAGMS IN CONTRACT UNIT PRICE FOR PRESTRESSED CONCRETE GIRDERS.
- 9. ALL MID-SPAN DIAPHRAGMS TO BE OF SAME MATERIAL TYPE; MIXING OF STEEL AND CONCRETE MID-SPAN DIAPHRAGMS IS NOT PERMITTED.
- 10. THE HOLES FOR THE $\frac{7}{8}$ " DIAMETER ASTM A449 BOLTS SHALL BE CAST INTO THE BEAM WEB USING $\frac{1}{4}$ " I.D. P.V.C. TUBING OR GALVANIZED STEEL SLEEVES (CHROMATED) AND AVOID PRESTRESSING STRANDS AND OTHER REINFORCEMENT. DRILLING IS NOT ALLOWED.
- 11. $1\frac{1}{6}$ " DIAMETER HOLE IN CONNECTION WT's OR DOUBLE ANGLES; $1\frac{5}{6}$ "x $2\frac{3}{6}$ " LONG SLOTTED HOLE IN DIAPHRAGM MEMBER, FOR $\frac{7}{8}$ " DIAMETER ASTM F3125 GRADE A325 BOLTS.
- 12. ALL SLOTTED HOLES TO BE COVERED BY A $\frac{5}{16}$ " PLATE WASHER WITH STANDARD HOLES. THE PLATE WASHER SHALL PROVIDE A MINIMUM 2 $\frac{1}{4}$ " EDGE DISTANCE IN THE DIRECTION OF THE SLOTS AND 1" EDGE DISTANCE IN THE DIRECTION PERPENDICULAR TO THE SLOTS.
- 13. THE $\frac{7}{6}$ " DIAMETER ASTM F3125 GRADE A325 BOLTS SHALL HAVE AN UNTHREADED SHANK OF SUFFICIENT LENGTH TO NOT ALLOW ANY THREADS TO EXIST IN THE PLANE BETWEEN THE TWO CONNECTED PARTS (SHEAR PLANE).
- 14. BOLTS IN DIAPHRAGMS LOCATED DIRECTLY UNDER LONGITUDINAL CONSTRUCTION JOINTS SHOULD NOT BE TIGHTENED UNTIL BOTH STAGES OF BRIDGE DECK HAVE BEEN PLACED. OTHERWISE, TIGHTEN ALL BOLTS PRIOR TO PLACING BRIDGE DECK CONCRETE.
- 15. FILLET WELD SIZES ARE GOVERNED BY MATERIAL THICKNESS IN ACCORDANCE WITH AASHTO/AWS D1.5 EXCEPT AS NOTED.
- 16. TERMINATE WELDS 1/2" SHORT OF EDGE AT EACH END OF EACH WELD.
- 17. "K" = FLANGE THICKNESS + FILLET, AS INDICATED IN AISC TABLES OF BEAM DIMENSIONS.
- 18. PROVIDE A 6" MINIMUM BEARING HEIGHT. ALTERNATIVELY, A 4" MINIMUM BEARING HEIGHT MAY BE USED WITH 1" THICK END PLATES.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD STEEL MID-SPAN DIAPHRAGMS FOR P/S CONCRETE AASHTO I-BEAM AND PA BULB-TEE BEAM BRIDGES WEB DEPTH ≥40"

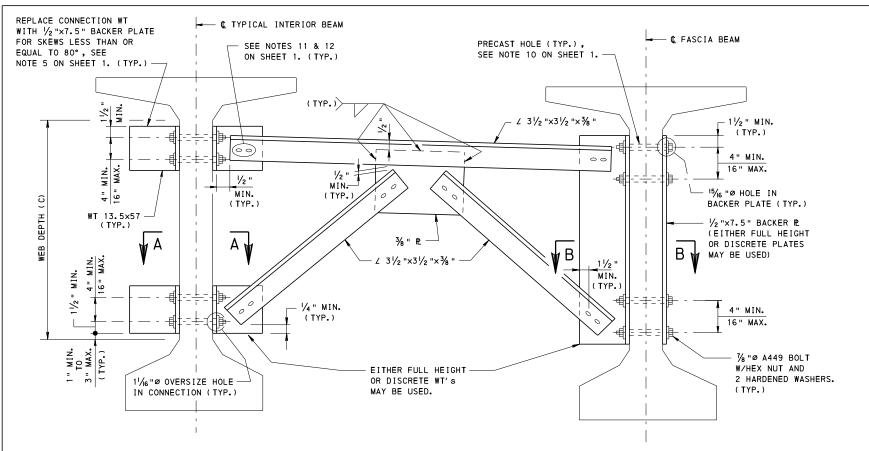
Thomas P. Macioca

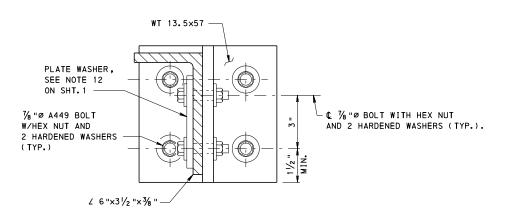
RECOMMENDED JAN. 31, 2019 RECOMMENDED JAN. 31, 2019 Allin Abolice ACTING DIR. BUR. OF PROJECT DELIVERY BC-770M

SHEET 1 OF 4

1. FOR SECTIONS A-A, B-B & C-C SEE SHEET 2.

2. USE DETAILS ON SHEET 3 FOR EXTERNAL BAYS WITH SPANS WITH VERTICAL CLEARANCE LESS THAN 16'-0" OVER VEHICULAR TRAFFIC.

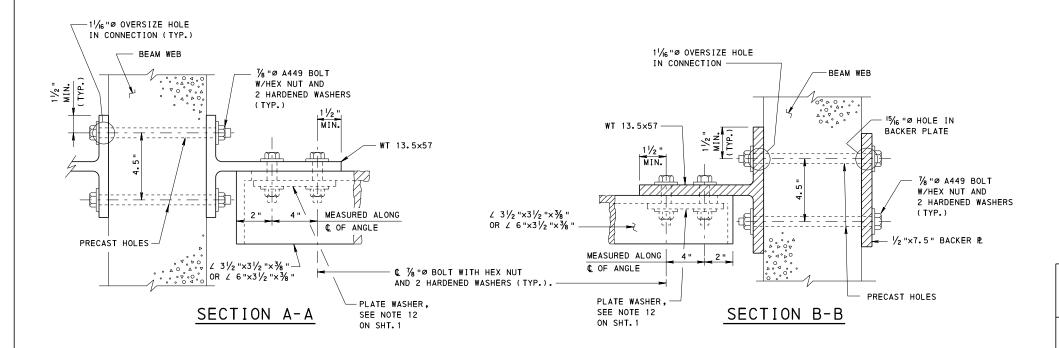




SECTION C-C FOR SECTION C-C CUT LINES SEE SHEET 1.

ALTERNATE DIAPHRAGM DETAIL FOR UTILITY ACCESS

THIS DETAIL MAY BE USED IN BAYS WITH UTILITIES



NOTE:

1. FOR GENERAL NOTES SEE SHEET 1.

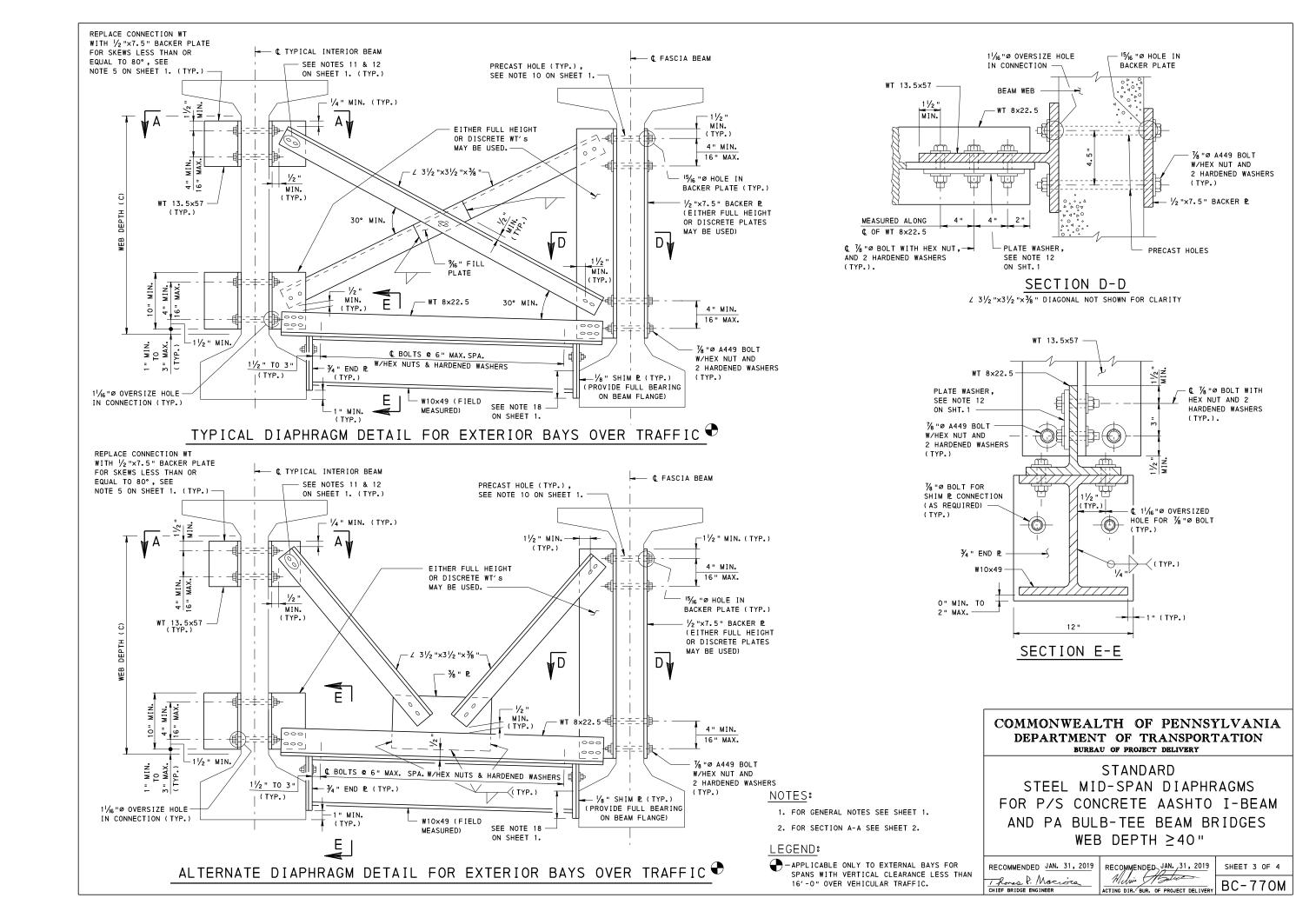
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

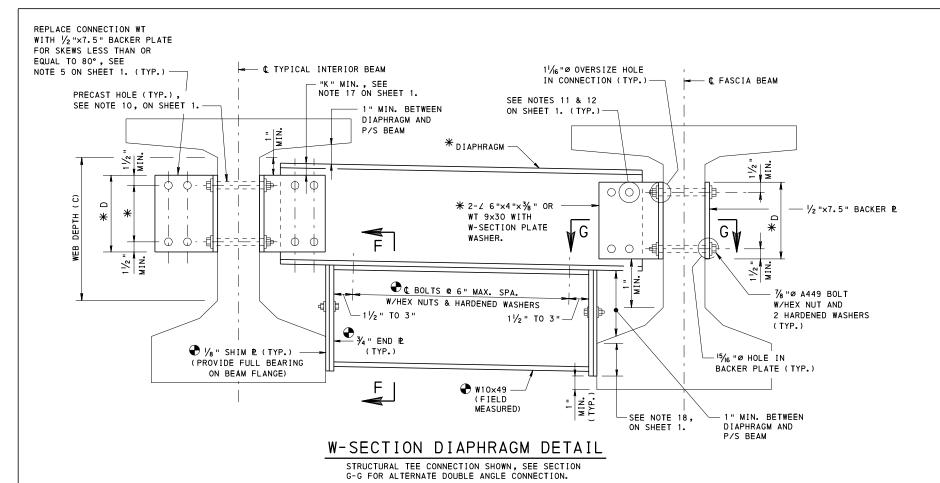
STANDARD STEEL MID-SPAN DIAPHRAGMS FOR P/S CONCRETE AASHTO I-BEAM AND PA BULB-TEE BEAM BRIDGES WEB DEPTH ≥40"

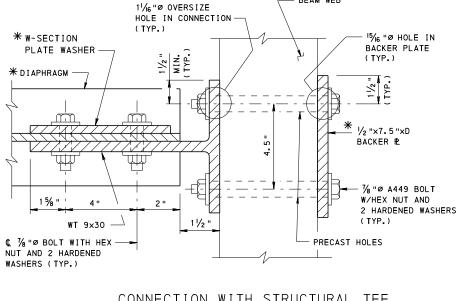
RECOMMENDED JAN. 31, 2019
Thoras P. Macière CHIEF BRIDGE ENGINEER

19 RECOMMENDED JAN. 31, 2019 ACTING DIR. BUR. OF PROJECT DELIVERY BC-770M

SHEET 2 OF 4

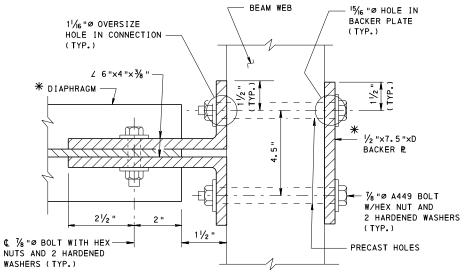






BEAM WEB

CONNECTION WITH STRUCTURAL TEE



CONNECTION WITH DOUBLE ANGLES

SECTION G-G

NOTE: W8x35 DIAPHRAGMS REQUIRE WT 13.5x42 CONNECTIONS WITH TWO ROWS OF 3 BOLTS AT EACH END. DOUBLE ANGLE CONNECTION MAY NOT BE USED WITH W8x35 DIAPHRAGMS.

NOTE:

1. FOR GENERAL NOTES SEE SHEET 1.

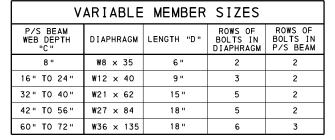
LEGEND

- * SEE "VARIABLE MEMBER SIZES TABLE"
- APPLICABLE ONLY TO EXTERNAL BAYS FOR SPANS WITH VERTICAL CLEARANCE LESS THAN 16'-0" OVER VEHICULAR TRAFFIC.

⅓"Ø BOLT WITH HEX NUTS AND 2 HARDENED WASHERS. ____*DIAPHRAGM CONNECTION % "Ø A449 BOLT W/HEX NUT AND 2 HARDENED WASHERS (TYP.) 1/8 "Ø BOLT FOR 1 1/2 " SHIM & CONNECTION ¢ 11/16 "Ø OVERSIZED (AS REQUIRED) HOLE FOR 1/8 " BOLT (TYP.) (TYP.) — ¾ " ₽L W10×49 M N X (TYP.)

SECTION F-F

DOUBLE ANGLE CONNECTION SHOWN . STRUCTURAL TEE CONNECTION SIMILAR.



4 "

PLATE WASHER

FOR W-SECTION

- 3% " PLATE

- ^{|5}/₁₆ "Ø HOLE (TYP.)

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD STEEL MID-SPAN DIAPHRAGMS FOR P/S CONCRETE AASHTO I-BEAM AND PA BULB-TEE BEAM BRIDGES ALL BEAM DEPTHS

RECOMMENDED JAN. 31, 2019
Thomas P. Macroca

RECOMMENDED JAN. 31, 2019 SHEET 4 OF 4 Allin Hodie ACTING DIR. BUR. OF PROJECT DELIVERY BC-770M

TEMPORARY LATERAL STABILITY BRACING (TLSB) DESIGN CRITERIA FOR

PRESTRESSED CONCRETE I-BEAM TYPE GIRDERS

GENERAL

- 1. PURPOSE OF TLSB IS TO PROVIDE A STANDARD METHOD TO MAINTAIN P/S I-BEAMS IN AN UPRIGHT POSITION DURING THE ERECTION PHASE. THE BRACING DETAILS CONTAINED IN THIS STANDARD ARE CONCEPTUAL; THE CONTRACTOR MAY ELECT TO PROVIDE ALTERNATE DETAILS AS APPROPRIATE.
- 2. PRIMARY BRACING SHALL BE DESIGNED TO PERMIT INSTALLATION, WHEN REQUIRED, PRIOR TO
- 3. BRACING IS TO REMAIN IN PLACE UNTIL SUFFICIENT SUPPORT IS PROVIDED BY MID SPAN DIAPHRAGMS AND POSITIVE MOMENT REGION DECK SLABS.
- 4. BOX BEAM TYPE GIRDERS ARE NOT INCLUDED IN THIS STANDARD.
- 5. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF THE BRACING SYSTEM.
- 6. BRACING CALCULATIONS AND ERECTION DRAWINGS SHALL BE SEALED BY A PROFFESSIONAL ENGINEER REGISTERED IN THE STATE OF PENNSYLVANIA, AND SUBMITTED IN ACCORDANCE WITH SECTION 105 OF PUBLICATION 408. CALCULATIONS SHALL INCLUDE VERIFICATION OF THE BEAM'S ABILITY TO CARRY THE BRACING-INDUCED FORCES.
- 7. PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH PUBLICATION 408 AND ITS CHANGE NUMBER.
- 8. THIS STANDARD SHALL BE REFERENCED ON THE APPROPRIATE CONTRACT PLAN SHEETS, TO ALERT THE CONTRACTOR OF THE POTENTIAL NEED TO MODIFY THE BEAM'S REINFORCEMENT BASED ON THE INTENDED BRACING METHODS.

BRACING REQUIREMENT CRITERIA

- 1. BEAMS SHALL BE RESTRAINED BY PRIMARY BRACING WHEN ANY OF THE FOLLOWING CONDITIONS EXIST:
 - BEAMS ARE DETERMINED TO BE INHERENTLY UNSTABLE (SEE STABILITY CRITERIA BELOW)
 BEAMS BEAR ON HIGH LOAD MULTIROTATIONAL (HMLR) BEARINGS
 - BEAM DEPTH EXCEEDS 66 INCHES
- 2. SECONDARY BRACING SHALL BE USED FOR BEAMS WITH DEPTHS EXCEEDING 48 INCHES WHEN
- 3. OTHERWISE NO LATERAL BRACING IS REQUIRED

STABILITY CRITERIA

CHANGE 4

- G. BEAMS SHALL BE CONSIDERED INHERENTLY STABLE IF THE VERTICAL REACTION AT THE BOTTOM OF THE BEAM IS LOCATED WITHIN THE MIDDLE 2/3 OF THE BEARING PAD. THE VERTICAL REACTION SHALL BE DETERMINED BY APPLYING THE LOADS DESCRIBED BELOW, AND SETTING THE SUM OF THE MOMENTS AT THE BOTTOM OF THE BEAM EQUAL TO ZERO. STABILITY SHALL BE CHECKED FOR THE 2 LOAD CASES DESCRIBED
- b. LOAD CASE I: STABILITY TO RELEASE CRANES SHALL BE DETERMINED BASED ON THE FOLLOWING LOADS. (LOADS TO BE APPLIED TO PRODUCE THE MAXIMUM OVERTURNING MOMENT)
 - WWH, WIND PRESSURE = 30 PSF ACTING HORIZONTALLY ABOUT THE DEPTH OF THE BEAM.
 RESULTANT FORCE ACTING AT MID DEPTH OF THE BEAM.
 - $\mathbf{W}_{\mathbf{WV}}$, WIND PRESSURE = 30 PSF ACTING UPWARD ABOUT ONE HALF OF TOP FLANGE OF THE
 - BEAM. RESULTANT FORCE ACTING AT MID POINT OF LOAD.

 PH , BEAM TILT = HORIZONTAL LOAD RESULTING FROM BEAM TILT IN SAME DIRECTION AS WIND LOAD,

 EQUAL TO 2% OF THE BEAM WT. REACTION, P, AND ACTING AT THE MID-DEPTH OF
 - CLB, LATERAL BOW = ECCENTRICITY RESULTING FROM 2" LATERAL BOW WHICH ADDS TO WIND AND TILT LOADS, EQUAL TO THE 2/3 POINT OF THE 2" LATERAL BOW. TO BE USED FOR LOCATING THE BEAM WT. REACTION, P.
- c. LOAD CASE II: STABILITY DURING CONSTRUCTION, PRIOR TO MID SPAN DIAPHRAGM AND POSITIVE MOMENT REGION DECK SLAB COMPLETION, SHALL BE DETERMINED BASED ON THE FOLLOWING LOADS: (LOADS TO BE APPLIED TO PRODUCE THE MAX. OVERTURNING MOMENT)

 - W_{WH}, WIND PRESSURE * = 30 PSF ACTING HORIZONTALLY ABOUT THE DEPTH OF THE BEAM.
 RESULTANT FORCE ACTING AT MID DEPTH OF THE BEAM.
 W_{WV}, WIND PRESSURE = 30 PSF ACTING UPWARD ABOUT ONE HALF OF TOP FLANGE OF THE BEAM
 RESULTANT FORCE ACTING AT MID POINT OF LOAD.
 - elb, lateral bow = eccentricity resulting from 1/2" Lateral bow which adds to wind and construction loads, equal to the 2/3 point of the 1/2" Lateral bow. To be used for locating the beam wt. reaction, p.
 - W_C, CONSTRUCTION LOAD = 20 PSF ACTING ACROSS $\frac{1}{2}$ OF THE TOP FLANCE OF THE BEAM. RESULTANT FORCE ACTING ON THE EDGE OF THE BEAM. (LOAD INCLUDES OVERHANG SYSTEM AND/OR DECK PANS.)
 - * THE APPLIED HORIZONTAL LOAD, ACTING AT MID-DEPTH AT THE END OF THE BEAM, SHALL BE TAKEN AS THE HORIZONTAL WIND LOAD (WWH *C*L/2), BUT NOT LESS THAN 2% OF THE TOTAL APPLIED VERTICAL LOAD [0.02*(P+Wc*b/2*L/2)]

INSTALLATION INSTRUCTIONS

PRIMARY BRACING:

- a. PRIMARY BRACING SHALL BE DESIGNED TO RESIST THE LOADS DESCRIBED FOR STABILITY CRITERIA.
- b. PRIMARY BRACING MUST BE INSTALLED PRIOR TO THE BEAM BEING RELEASED FROM THE CRANE.
- c. THE BRACING SYSTEM SHALL BE DESIGNED TO WITHSTAND THE ABOVE FORCES USING WORKING STRESS METHOD, WITH CONSIDERATION OF THE DEFLECTION OF THE BRACING SYSTEM.
- d. DRILLED ANCHORS SHALL BE LOAD TESTED TO 120% OF THE DESIGN LOAD. DESIGN LOAD AND TEST LOAD SHALL BE SHOWN ON THE ERECTION DRAWING.
- e. THE QUANTITY OF DRILLED ANCHORS TO BE LOAD TESTED SHALL BE 2 ANCHORS PER SUBSTRUCTURE UNIT.

SECONDARY BRACING:

- G. SECONDARY BRACING SHALL BE DESIGNED TO RESIST THE DESIGN HORIZONTAL WIND LOADS. DESIGN FOR STABILITY CRITERIA USING LRFD METHOD UNLESS OTHERWISE NOTED. HORIZONTAL WIND PRESSURE, WWH, NEEDS TO BE APPLIED TO THE EXTERIOR AND INTERIOR BEAMS IN ACCORDANCE WITH "GUIDE SPECIFICATION FOR WIND LOADS ON BRIDGES DURING CONSTRUCTION", 1ST EDITION (2017)
 b. SECONDARY BRACING MAY BE INSTALLED AFTER CRANE IS RELEASED, BUT MUST BE INSTALLED
- PRIOR TO THE END OF A WORKDAY OR UPON COMPLETION OF A SPAN, WHICHEVER COMES FIRST.
- THE SECONDARY BRACING SYSTEM SHALL HAVE TWO FIXED CONNECTIONS TO EACH SUPPORT,
 PREFERABLY LOCATED AT THE FIRST AND LAST GIRDER ON EACH SUPPORT.
 DRILLED ANCHORS SHALL BE LOAD TESTED TO 120% OF THE DESIGN LOAD. DESIGN LOAD AND TEST LOAD
- SHALL BE SHOWN ON THE ERECTION DRAWING.

 e. THE QUANTITY OF DRILLED ANCHORS TO BE LOAD TESTED SHALL BE 2 ANCHORS PER SUBSTRUCTURE
- f. DESIGN CRITERIA: BEAM SLIDING RESISTANCE AGAINST LATERAL WIND LOAD, CABLE STRENGTH, TURNBUCKLE STRENGTH, DRILLED ANCHOR STRENGTH AND WOOD STRENGTH.

 G. CABLES AND TURNBUCKLES SHALL BE DESIGNED USING THE WORKING STRESS METHOD. FOR THIS
- DESIGN APPROACH, THE LFRD WIND LOADS SHALL BE REDUCED BY A FACTOR OF 1.4.

BEARINGS

- BEARING PAUS

 a.1. THE LATERAL LOAD TRANSFERRED TO THE BEARING SHALL BE LESS THAN OR EQUAL TO 20% OF THE VERTICAL REACTION (BEAM WEIGHT ONLY).

 a.2. IF THE ABOVE CRITERIA CAN NOT BE MET, BEAM MUST BE BRACED TO PREVENT SLIDING.

- G.2. IF THE ABOVE CRITERIA CAN NOT BE MET, BEAM MUST BE BRACED TO PREVENT SLIDING.
 GUIDED HLMR BEARINGS
 b.1. THE LATERAL LOAD TRANSFERRED TO THE BEARING SHALL BE CHECKED AGAINST THE ALLOWABLE
 HORIZONTAL LOAD OF THE HLMR BEARING. SEE BD-613M FOR HLMR BEARING DESIGN TABLE.
 b.2. IF THE ABOVE CRITERIA CAN NOT BE MET, BEAM MUST BE BRACED TO PREVENT SLIDING.
 b.3. GUIDED HLMR BEARINGS SHALL BE LOCKED TO RESIST LONGITUDINAL MOVEMENT USING A GUIDED HLMR
 BEARING LOCK. ONLY ONE END OF BEAM IS TO BE LOCKED IN A LONGITUDINAL POSITION.
 OPPOSITE BEAM END IS TO HAVE FREEDOM OF MOVEMENT LONGITUDINALLY.
- b.4. BEARING LOCK SHALL NOT BE REMOVED UNTIL FINAL RESTRAINTS ARE IN PLACE. (I.E. END DIAPHRAGM, OR SHEAR BLOCKS)
 c. NON-GUIDED HLMR BEARINGS
 - C. 1. THE LATERAL LOAD TRANSFERRED TO THE BEARING SHALL BE RESISTED BY A NON-GUIDED HLMR
 - BEARING LOCK.

 c. 2. BEARING LOCK SHALL BE INSTALLED TO RESIST LATERAL MOVEMENT WHILE ALLOWING
 - LONGITUDINAL MOVEMENT, IF REQUIRED.

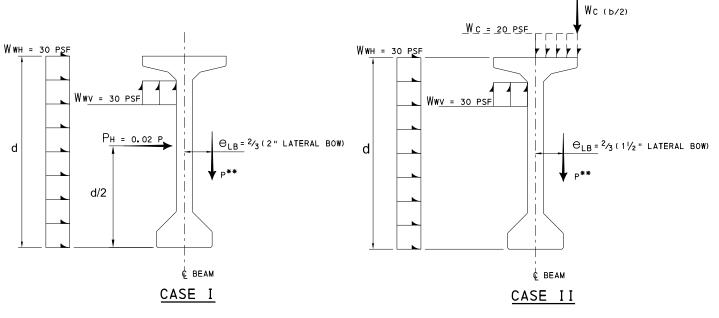
 c.3. BEARING LOCK SHALL NOT BE REMOVED UNTIL FINAL RESTRAINTS ARE IN PLACE. (I.E. END DIAPHRAGM, OR SHEAR BLOCKS)

DESIGN LOAD COMMENTARY

STABILITY CRITERIA:

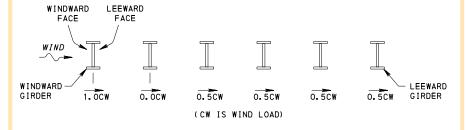
1. LOAD CASE I

- WIND PRESSURE OF 30 PSF INCLUDES 5 PSF FOR MEMBERS OVER OR ADJACENT TO TRAFFIC OPENINGS.
- LATERAL BOW IS THE RESULTANT OF 11/2" MAXIMUM ALLOWABLE LATERAL SWEEP AND 1/2 " SOLAR GAIN.
- 2. LOAD CASE II
 - WIND PRESSURE OF 30 PSF INCLUDES 5 PSF FOR MEMBERS OVER OR ADJACENT TO TRAFFIC OPENINGS.
 - LATERAL BOW IS THE RESULTANT OF 1" MAXIMUM ALLOWABLE LATERAL SWEEP AND 1/2" SOLAR GAIN.



** P = BEAM WEIGHT REACTION = BEAM UNIT WEIGHT/FT x SPAN LENGTH/2

WIND PRESSURE DISTRIBUTION TO GIRDERS



REFER TO AASHTO "GUIDE SPECIFICATION FOR WIND LOADS ON BRIDGES DURING CONSTRUCTION", 1ST EDITION (2017)

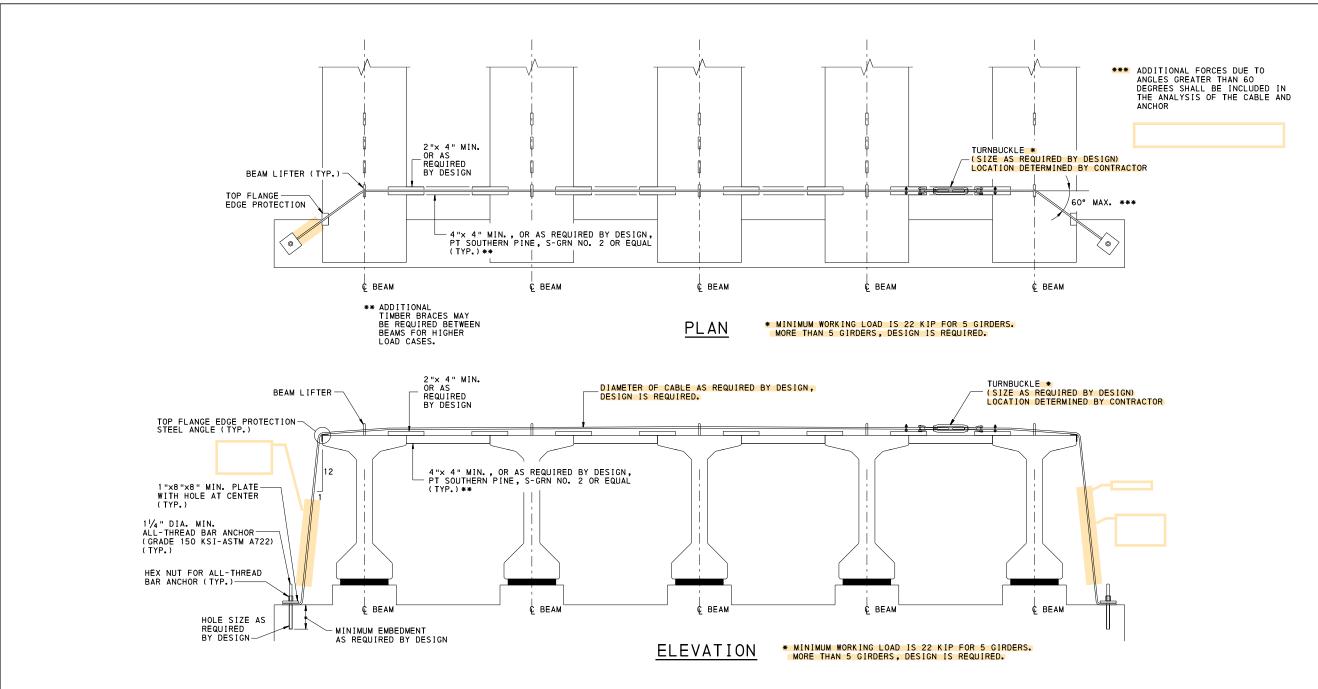
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD PRESTRESSED CONCRETE BEAM BRACING NOTES

RECOMMENDED NOV.23, 2022

RECOMMENDED NOV. 23, 2022 Davin E. Gray

SHEET 1 OF 5 BC-772M



CONCEPTUAL SECONDARY BRACING DETAILS

INSTALLATION

SECONDARY BRACING INSTALLATION:

- 1. INSTALL ALL-THREAD BAR ANCHORS ON EXTERIOR SIDE OF EACH END OF ALL FASCIA BEAMS AS PER PRIMARY BRACING SPECIFICATION.
- 2. CENTER BEAM ABOUT CENTER LINE OF BEAM AND BEARING AND LOWER BOTH ENDS TO TOUCH SURFACE OF BEARINGS. 2. d. CRANE TO MAINTAIN CONTROL OF BEAM.
- 3. SLOWLY RELEASE BEAM FROM CRANE WHILE CHECKING FOR PLUMBNESS AT WEB.
- 4. REPEAT FOR OPPOSITE BEAM END.
- 5. RE-CHECK BEAM FOR PLUMBNESS AT WEB.
- 6. RELEASE CRANES.
- 7. ATTACH AND TIE BACK FASCIA BEAM TO ABUTMENT/ PIER WITH CABLE AND TURNBUCKLE TAKING UP SLACK IN CABLE TO TAUGHT CONDITION.
- 8. ERECT FIRST INTERIOR BEAM.
- 9. INSTALL TIMBER BRACE.

10. REPEAT AS REQUIRED.
10.d. ALL BEAMS TO BE SECURED TOGETHER BY CABLE AND TIMBER TO POINT OF LAST BEAM ERECTED OR COMPLETION OF SPAN.
10.b. THE LAST BEAM PLACED DURING A WORK SHIFT WILL BE TIED BACK TO SUBSTRUCTURE AS AT FASCIA BEAM.

11. BRACING INSTALLATION COMPLETE

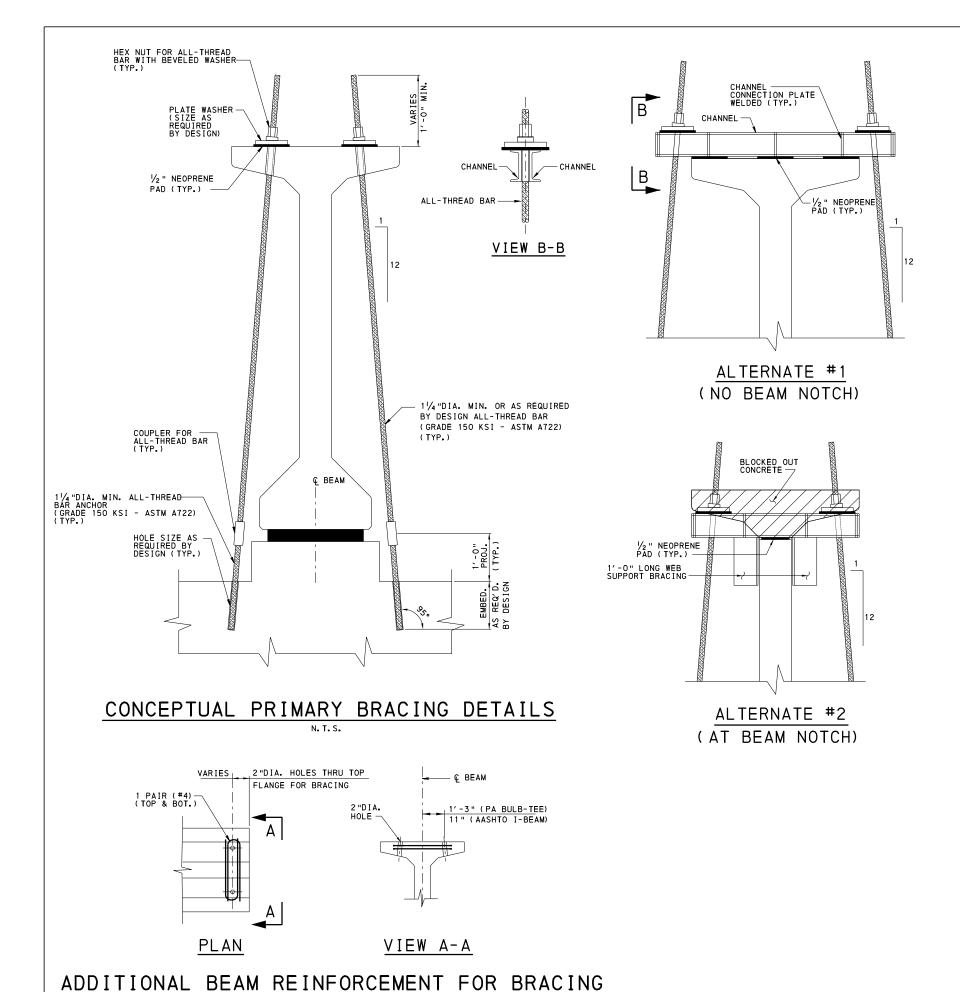
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD PRESTRESSED CONCRETE BEAM BRACING CONCEPTUAL SECONDARY BRACING

RECOMMENDED NOV. 23, 2022 Havin E. Hray

THEF ENGINEER, HIGHWAY ADMIN

SHEET 2 OF 5 BC-772M



INSTALLATION

ANCHOR INSTALLATION/

ANCHOR INSTALLATION/ REMOVAL:

1. HOLES IN ABUTS. & PIERS SHALL BE GENERALLY LOCATED ALONG CENTER LINE OF BEARING.

1. a. HOLES SHALL BE AIR DRILLED.

2. b. PACHOMETER SHALL BE USED TO LOCATE REINFORCEMENT PRIOR TO DRILLING.

3. c. IF STEEL IS CONTACTED DURING THE DRILLING PROCESS, THE HOLE SHALL BE ABANDONED AND FILLED WITH AN APPROVED NON-SHRINK GROUT. HOLE WILL BE RELOCATED / REDRILLED WITHIN BRACING DESIGN CRITERIA.

2. DRILLED HOLE SHALL BE PNEUMATICALLY CLEARED OF DEBRIS (ROCK DUST, WATER, ETC.)

3. ALL-THREAD BARS SHALL BE ANCHORED IN HOLES DRILLED IN SUBSTRUCTURE BY USING AN APPROVED HIGH STRENGTH POLYESTER RESIN ANCHORING MATERIAL AND INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

4. DRILLED ANCHORS SHALL BE LOAD TESTED AFTER MANUFACTURER'S SPECIFIED CURE TIME.

5. DRILLED ANCHORS SHALL BE REMOVED BY MECHANICAL MEANS TO A DEPTH OF 2" (+/-) BELOW THE SURFACE.

6. AFTER REMOVAL OF DRILLED ANCHORS, FILL HOLES WITH NON-SHRINK, NON-STAIN GROUT.

7. FILL HOLES IN BEAM TOP FLANGE WITH APPROVED NON-SHRINK GROUT.

PRIMARY BRACING INSTALLATION:

1. COUPLE ALL-THREAD BARS TO THE DRILLED ANCHORS EMBEDDED IN SUBSTRUCTURE.

1. a. MINIMUM LENGTH OF ALL-THREAD BAR SHALL BE DETAILED ON THE ERECTION DRAWINGS.

2. CENTER BEAM ABOUT CENTERLINE OF BEAM AND BEARING AND LOWER BOTH ENDS TO WITHIN 11/4" OF BEARING SURFACE WHILE INSERTING ALL-THREAD BAR THROUGH HOLE OR CLAMP ASSEMBLY LOCATED AT BEAM TOP FLANGE.

3. INSTALL NEOPPENE PADS, PLATE WASHERS, BEVELED WASHERS AND NUTS

ALL-THREAD BAR THROUGH HOLE OR CLAMP ASSEMBLY LOCATED AT BEAM TOP FLANGE.

3. INSTALL NEOPRENE PADS, PLATE WASHERS, BEVELED WASHERS AND NUTS AS REQUIRED TO LOOSE CONDITION.

4. MATE BEAM TO BEARINGS.
4. G. CRANE TO MAINTAIN CONTROL OF BEAM.

5. LAMINATED BEARING
5. G. PROCEED TO STEP (7)

6. HLMR BEARING
6. G. INSTALL BEARING LOCK AS SPECIFIED BY HLMR BEARING TYPE AND SNUG FASTENERS.
6. D. SHIM AS REQUIRED.

7. SLOWLY RELEASE BEAM FROM CRANE WHILE CHECKING FOR PLUMBNESS AT WEB.

7. SLOWLY RELEASE BEAM FROM CRANE WHILE CHECKING FOR PLUMBNESS AT WEB.

8. TIGHTEN BEARING LOCKS (IF REQUIRED) AND SNUG TIGHT BRACING AT TOP FLANGE.

9. RE-CHECK BEAM FOR PLUMBNESS AT WEB.

10. RELEASE CRANE.

11. BRACING INSTALLATION COMPLETE.

ALL-THREAD BAR ANCHORS MAY BE CAST INTO THE SUBSTRUCTURE AT THE CONTRACTOR'S OPTION.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD PRESTRESSED CONCRETE BEAM BRACING CONCEPTUAL PRIMARY BRACING

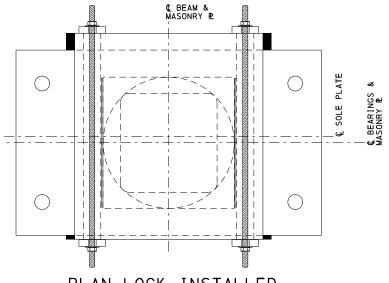
RECOMMENDED NOV. 23, 2022

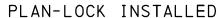
SHEET 3 OF 5

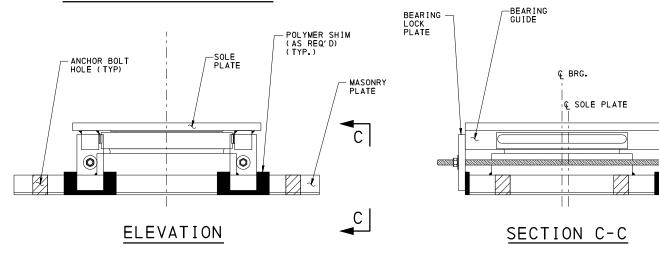
BC-772M

Havin E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN







CONCEPTUAL GUIDED HLMR BEARING LOCK

INSTALLATION

- GUIDED HLMR BEARING LOCK INSTALLATION:

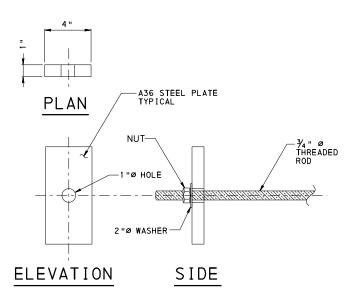
 1. MATE BEAM TO BEARING SOLE PLATE.

 1. G. CRANE TO MAINTAIN CONTROL OF BEAM.

 2. PLACE BEARING LOCK AND SNUG FASTENERS.

 3. SHIM CENTER LINE OF BEARING SOLE PLATE STATION AHEAD OR BEHIND AS DESIGNATED BY DESIGN TO AIR TEMPERATURE AT ERECTION, TO A LOCKED CONDITION.

 3. G. ONLY ONE END OF BEAM IS TO BE LOCKED IN A LONGITUDINAL POSITION. OPPOSITE BEAM END IS TO HAVE FREEDOM OF MOVEMENT LONGITUDINALLY.



GUIDED POT BEARING LOCK DETAILS

- PROVIDE 1/8" BEDDING MATERIAL AS BOND

NOTE: CONCEPTUAL DETAILS INDICATED ARE BASED ON "POT" HLMR BEARINGS. FOR OTHER HLMR BEARINGS, CONTRACTOR TO DEVELOP REQUIRED LOCK DETAILS AND SUBMIT WITH ERECTION DRAWINGS.

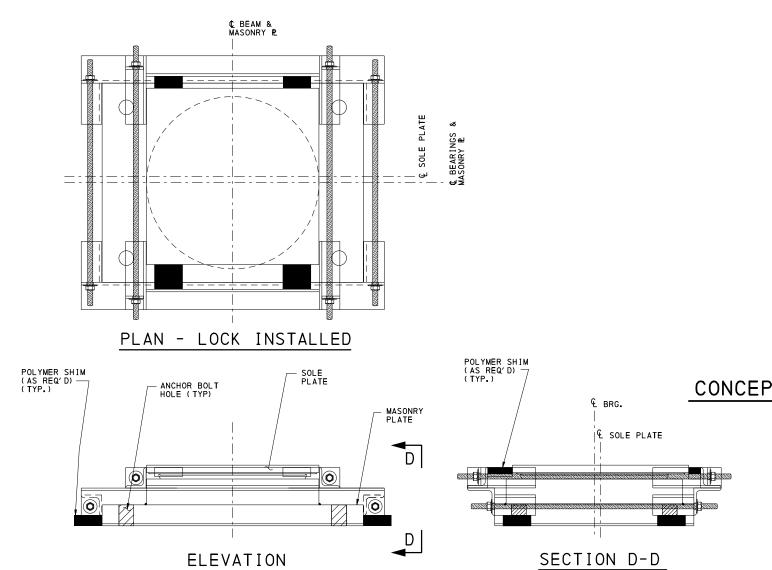
COMMONWEALTH OF PENNSYLVANIA

STANDARD PRESTRESSED CONCRETE BEAM BRACING CONCEPTUAL GUIDED HLMR BEARING LOCK

DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

RECOMMENDED NOV. 23, 2022 Havin E. Hray
CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 4 OF 5 BC-772M



CONCEPTUAL NON-GUIDED HLMR BEARING LOCK

INSTALLATION

NON-GUIDED POT BEARING LOCK INSTALLATION:

- NON-GUIDED POT BEARING LOCK INSTALLATION:

 1. MATE BEAM TO BEARING SOLE PLATE.

 1. GRANE TO MAINTAIN CONTROL OF BEAM.

 2. PLACE BEARING LOCK AND SNUG FASTENERS.

 3. SHIM CENTER LINE OF BEARING SOLE PLATE STATION AHEAD OR BEHIND AS DESIGNATED BY DESIGN TO AIR TEMPERATURE AT ERECTION , TO A LOCKED CONDITION.

 3. G. ONLY ONE END OF BEAM IS TO BE LOCKED IN A LONGITUDINAL POSITION. OPPOSITE BEAM END IS TO HAVE FREEDOM OF MOVEMENT LONGITUDINALLY.

-ANGLE, SIZE AS REQUIRED BY DESIGN -₱ W/ 1"Ø HOLE (TYP.) PLAN VARIES VARIES
BASED ON SOLE PL DIMENSION - ¾ "Ø THREADED ROD I"ٰØ HOLE (TYP•) -2"Ø WASHER BASED ON MASONRY & DIMENSION ELEVATION SIDE

CONCEPTUAL NON-GUIDED HLMR BEARING LOCK DETAILS

NOTE: CONCEPTUAL DETAILS INDICATED ARE BASED ON "POT" HLMR BEARINGS. FOR OTHER HLMR BEARINGS, CONTRACTOR TO DEVELOP REQUIRED LOCK DETAILS AND SUBMIT WITH ERECTION DRAWINGS.

> COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

> > BRIDGE OFFICE

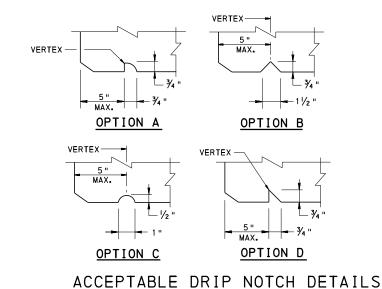
STANDARD PRESTRESSED CONCRETE BEAM BRACING CONCEPTUAL NON-GUIDED HLMR BEARING LOCK

RECOMMENDED NOV. 23, 2022 Havin E. Hray

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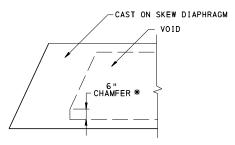
SHEET 5 OF 5 BC-772M

BEAM NOTCH DETAIL SPREAD BOX BEAM



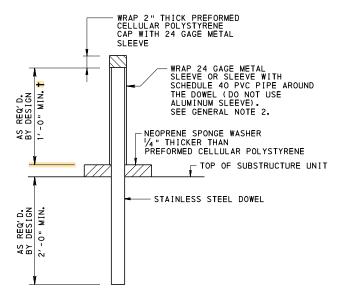
NOTE:

LOCATE THE VERTEX OF THE DRIP NOTCH AT THE MIDPOINT BETWEEN THE STRANDS



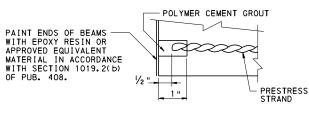
* NO CHAMFER REQUIRED FOR SKEWS 75° TO 90°

CHAMFER DETAIL FOR SKEWED END BLOCK



DOWEL DETAIL

† 6" MIN FOR 17" DEEP BEAM & 10" MIN FOR 21" DEEP BEAM



GENERAL NOTES:

-PRESTRESS STRAND

NOTES:

1. RECESS CAN BE MADE FOR A SINGLE STRAND OR A GROUP OF STRANDS.

INCLUDE APPLICABLE DETAILS SHOWN ON THESE SHEETS ON FABRICATOR'S SHOP DRAWINGS.

2. ASPHALT-SATURATED PAPER OR SCHEDULE 40 P.V.C. PIPE ARE PERMITTED

TO BE USED AS ALTERNATE BOND BREAKER MATERIALS IN LIEU OF THE METAL SLEEVE. OTHER BOND BREAKER MATERIALS MAY BE USED AROUND THE DOWEL ONLY WITH THE APPROVAL OF THE DISTRICT STRUCTURE CONTROL ENGINEER.

USE PREFORMED CELLULAR POLYSTYRENE CONFORMING TO ASTM C578, TYPE 1, EXCEPT LIMIT THE WATER ABSORPTION TO 2% BY VOLUME.

USE POLYMER CEMENT GROUT FROM A MANUFACTURER LISTED IN BULLETIN 15 UNDER MISCELLANEOUS POLYMER MODIFIED AND SPECIAL CEMENTS, MORTARS AND CONCRETES. APPLY IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

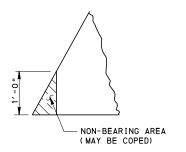
STRAND RECESS CAN BE OMITTED IF BEAM ENDS ARE TO BE INCORPORATED IN A CONTINUITY DIAPHRAGM (BRIDGES MADE CONTINUOUS FOR LIVE LOADS) OR INTEGRAL ABUTMENT (EMBEDDED IN 6" OR MORE OF CAST IN PLACE CONCRETE BEYOND THE END OF THE BEAM), SEE PUBLICATION 408 SECTION 1107.03(c)6.1.5. ONLY PAINT

BEAM ENDS WITH EPOXY RESIN OR APPROVED EQUIVALENT MATERIAL IN ACCORDANCE WITH PUBLICATION 408 SECTION 1019.2(b) IF SPECIFIED.

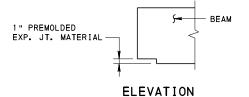
- 2. CLEAN THE STRANDS FROM ALL FOREIGN MATERIALS SUCH AS RUST, SLAG, ETC. PRIOR TO APPLICATION OF POLYMER CEMENT GROUT OR EPOXY RESÍN.
- 3. PAINT BEAM ENDS PRIOR TO SHIPMENT OR STORAGE.

GROUTED RECESS FOR STRANDS AT BEAM ENDS

(FOR DOWELS IN DIAPHRAGMS)



<u>PLAN</u>



TYPICAL CORNER BLOCKOUT-SKEWS < 85°

NOTES:

- (1) MODIFY IF REQUIRED TO ACCOMMODATE BEARING PAD ARRANGEMENT FOR SHARP SKEWS.
- (2) NOT PERMITTED IN CONJUNCTION

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

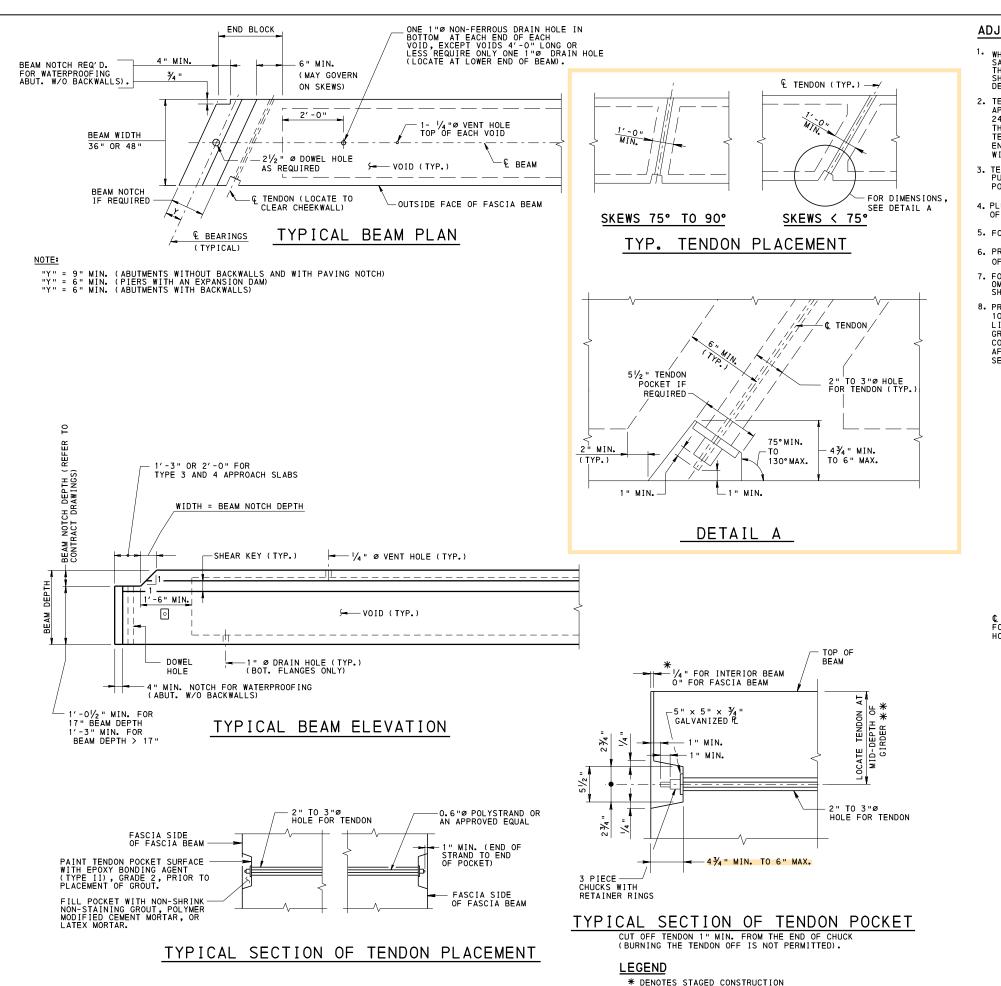
STANDARD MISCELLANEOUS PRESTRESS DETAILS

RECOMMENDED NOV. 23, 2022 Havin E. Hray
CHIEF ENGINEER, HIGHWAY ADMIN

BC-775M

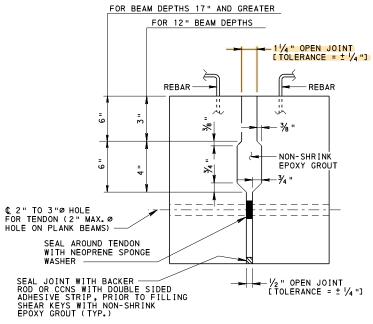
SHEET 1 OF 3

CHANGE 4



ADJACENT BOX BEAM PRESTRESS DETAIL NOTES:

- 1. WHEN THE COMMON EDGES OF TWO ADJOINING BEAMS ARE NOT AT THE SAME ELEVATION, ADJUST THE LOCATION OF THE SHEAR KEY SO THAT THE MAXIMUM ELEVATION DIFFERENCE BETWEEN THE BOTTOM OF THE SHEAR KEYS IS 1 " FOR 12" BEAM DEPTHS AND 2" FOR OTHER BEAM DEPTHS. APPLIES TO BRIDGES IN SUPERELEVATION TRANSITION ONLY.
- 2. TENDONS TO BE 0.6" Ø STRANDS, 270 ksi POLYSTRAND OR AN APPROVED EQUAL AND TO BE TENSIONED TO A FORCE OF 40,000 lbs 24 hours after the placement of the Shear Keys but not before THE GROUT HAS OBTAINED ITS MINIMUM STRENGTH OF 2500 psi. TIGHTEN TENDON AT CENTERLINE OF SPAN FIRST AND THEN PROGRESS TOWARD ENDS OF BEAM. ALTERNATE LEFT AND RIGHT OF CENTERLINE FOR BEAMS WITH 5 TENDONS.
- 3. TENSION THE TENDONS IN ACCORDANCE WITH SECTION 1108.03(e) OF PUB. 408. SECTION 1108.01 OF PUB. 408 DOES NOT APPLY TO POST-TENSIONING OF ADJACENT BOX BEAMS.
- 4. PLUG 1/4 "Ø VENT HOLE WITH NON-SHRINK GROUT AFTER REMOVAL OF BEAMS FROM THE FORMS.
- 5. FORM TENDON HOLES WITH NONMETALLIC PIPE.
- 6. PROVIDE SILICONE SEALANT MATERIAL IN ACCORDANCE WITH SECTION 705.4(a) OF PUB. 408.
- 7. FOR SHEAR KEY CLEANING AND GROUTING, SEE SECTION 1080.3(d) OF PUB. 408. OMIT SOAKING JOINTS WITH WATER, SPADING GROUT AND OVERFILLING THE SHEAR KEYS DUE TO THE USE OF NON-SHRINK EPOXY GROUT.
- 8. PROVIDE NON-SHRINK EPOXY GROUT IN ACCORDANCE WITH SECTION 910.2(b) AND 1080.2(g) OF PUB. 408 FOR SHEAR KEYS. USE EPOXY GROUT WITHIN THE SHELF LIFE AND TEMPERATURE LIMITATIONS SET BY THE MANUFACTURER. CURE THE EPOXY GROUT IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT PERMIT CONSTRUCTION ACTIVITY OR OTHER LOADINGS ON BRIDGE FOR AT LEAST 24 HOURS AFTER SHEAR KEY EPOXY GROUT HAS BEEN PLACED. FOR VEHICULAR LOADING, SECTION 1080.3(d) 5 OF PUB.408 APPLIES.



SHEAR KEY DETAIL OMIT SHEAR KEYS ON OUTSIDE FACE

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

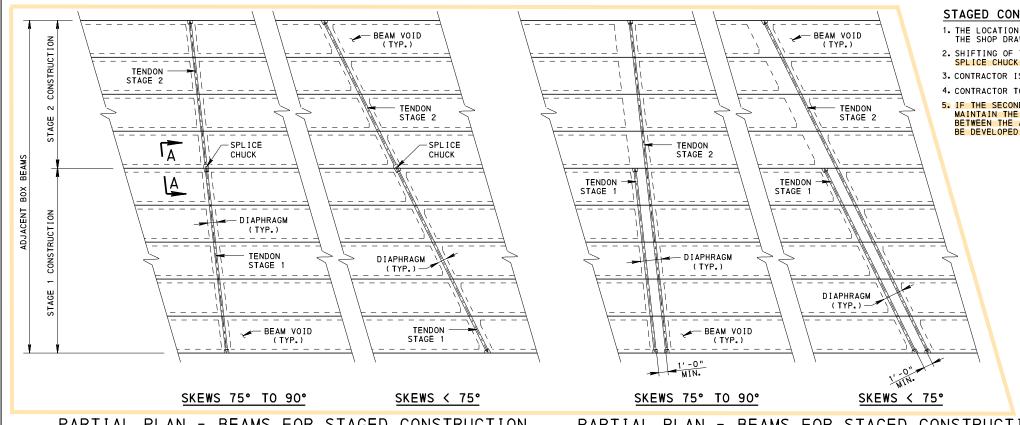
STANDARD MISCELLANEOUS PRESTRESS DETAILS ADJACENT BOX BEAM

RECOMMENDED NOV. 23, 2022

RECOMMENDED NOV. 23, 2022 Havin E. Hray
HIEF ENGINEER, HIGHWAY ADMIN

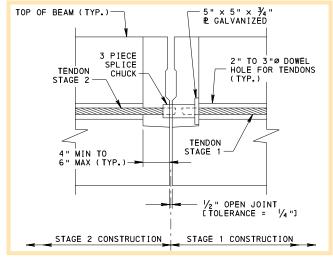
SHEET 2 OF 3 BC-775M

- ** SHIFTING OF TENDON BY UP TO 11/2" IS
 PERMITTED TO AVOID LOSS OF STRAND POSITIONS.



STAGED CONSTRUCTION NOTE:

- 1. THE LOCATION OF THE TRANSVERSE TENDONS FOR STAGED CONSTRUCTION MUST BE SHOWN ON THE SHOP DRAWINGS TO ACCOUNT FOR DIFFERENCES IN CAMBER OF THE BEAMS IN EACH STAGE.
- 2. SHIFTING OF TENDON BY UP TO 11/2" PERMITTED TO AVOID LOSS OF STRAND POSITIONS. SPLICE CHUCK POCKET PROHIBITS ALL STRAND LOCATIONS ABOVE TENDON.
- 3. CONTRACTOR IS RESPONSIBLE TO VERIFY POST-TENSIONING TUBE ALIGNMENT BETWEEN STAGES.
- 4. CONTRACTOR TO PROTECT SPLICE CHUCK FOR TIME LAPSE BETWEEN PHASES.
- 5. IF THE SECONDARY POST-TENSIONING DUCT IS OMITTED, A TEMPORARY MECHANISM TO MAINTAIN THE OPEN JOINT BETWEEN BEAMS, WHICH ALLOWS FOR THE VERTICAL DEFLECTION BETWEEN THE ACTIVE CONSTRUCTION STAGE AND THE COMPLETED CONSTRUCTION STAGE, MUST BE DEVELOPED TO PREVENT CLOSURE OF THE SHEAR KEY JOINT DURING POST-TENSIONING.



SECTION A-A

PARTIAL PLAN - BEAMS FOR STAGED CONSTRUCTION

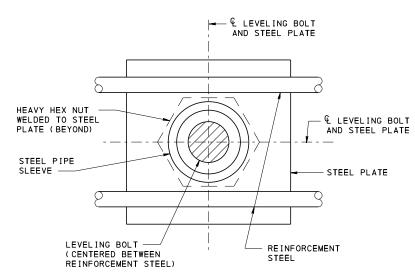
SPLICE CHUCK ALTERNATE

PARTIAL PLAN - BEAMS FOR STAGED CONSTRUCTION

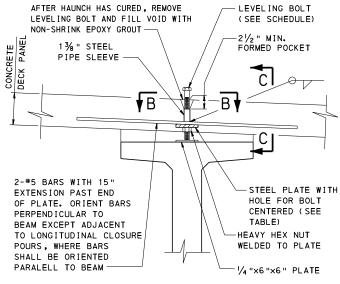
DOUBLE DUCT ALTERNATE

VERTICAL ADJUSTMENT DEVICE NOTES:

- 1. VERTICAL ADJUSTMENT DEVICES SHALL BE DESIGNED TO RESIST TWO TIMES THE ANTICIPATED PANEL DEAD LOAD POINT SUPPORT FORCE.
- ALTERNATE LEVELING DEVICES MAY BE SUBSTITUTED BY THE CONTRACTOR WITH THE APPROVAL FROM THE ENGINEER.
- 3. IF BOLT IS OILED OR GREASED TO FACILITATE LEVELING AND REMOVAL, ADEQUATELY CLEAN AND REMOVE DEBRIS PRIOR TO FILLING VOIDS WITH GROUT.



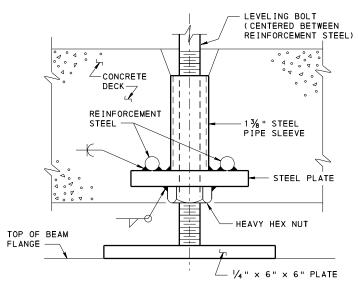
SECTION B-B



VERTICAL ADJUSTMENT DEVICE

(VERTICAL ADJUSTMENT ON STEEL BEAM/GIRDER SIMILAR)
(HAUNCH DETAILS NOT SHOWN FOR CLARITY)
(MIN. OF 2 LOCATIONS PER BEAM PER PANEL)

VERTICAL	_ ADJUST	MENT SCHEDULE
SERVICE LOAD	BOLT DIA.	STEEL PLATE WITH HOLE FOR BOLT CENTERED
10 KIPS	1 "	4 "×4 "× 5⁄8 "
20 KIPS	1 1/4 "	4 "×4 "× 7⁄8 "



SECTION C-C

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

MISCELLANEOUS PRESTRESS DETAILS

ADJACENT BOX BEAM AND

VERTICAL ADJUSTMENT DEVICE DETAILS

RECOMMENDED NOV. 23, 2022	l
1-1(/1.//	l
Mark W. Mary	ı
CHIEF BRIDGE ENGINEER	ı

RECOMMENDED NOV. 23, 2022 SHEET 3 OF 3

Law E. Law BC-775M

CHIEF ENGINEER, HIGHBAY ADMIN.

GENERAL NOTES

- 1. DESIGN SPECIFICATIONS:

 PENNDOT DESIGN MANUAL PART 4, STRUCTURES, APRIL 2015 EDITION.
 1989 AASHTO "GUIDE SPECIFICATIONS FOR STRUCTURAL DESIGN OF SOUND BARRIERS", INCLUDING THE 1992 AND 2002 INTERIMS.
 1992 AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES", 15TH EDITION, INCLUDING THE 1993 AND 1994 INTERIMS.
 2001 AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINARIES AND TRAFFIC SIGNALS", 4TH EDITION, INCLUDING THE INTERIMS THROUGH 2006.
 DESIGN IS IN ACCORDANCE WITH THE WORKING STRESS DESIGN METHOD. (NO INCREASE IN ALLOWABLE UNIT STRESSES ARE PERMITTED EXCEPT FOR GROUP III LOADINGS WHICH PERMITS A 33% OVERSTRESS.)
- 2. CONSTRUCTION SPECIFICATIONS AND WORKMANSHIP:

 PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH THE CURRENT VERSION OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, AASHTO/AWS/D1.5M/D1.5 2008. BRIDGE WELDING CODE AND THE CONTRACT SPECIAL PROVISIONS. (USE AWS/D1.1/D1.1M 2008. FOR WELDING NOT COVERED IN AASHTO/AWS/D1.5M/D1.5 2008.)
- 3. WALL HEIGHTS MUST EQUAL OR EXCEED THE ACOUSTICAL PROFILE.
- 4. PANEL HEIGHTS:

 2'-0" MINIMUM TO 9'-0" MAXIMUM

 PROVIDE STACKED PANELS WHEN THE WALL HEIGHT EXCEEDS 9'-0"
- 5. HORIZONTAL PANEL JOINTS:

 MINIMIZE THE NUMBER OF HORIZONTAL PANEL JOINTS.

 PROVIDE UNIFORM STEPS.

 IF STEPS ARE REQUIRED, THE ELEVATION DIFFERENCE BETWEEN ADJACENT PANELS IS NOT PERMITTED TO BE LESS THAN 6" OR GREATER THAN 2'-O".
- PROVIDE STEEL CABLES IN THE PRECAST CONCRETE PANELS AS INDICATED ON THE CONTRACT DRAWINGS. (REFER TO BC-779M FOR DETAILS)
- 7. INSTALL PANELS TRULY VERTICAL.
- 8. PROVIDE CONCRETE COVER IN ACCORDANCE WITH THIS STANDARD AND DESIGN
- 9. FILL ALL LIFTING INSERTS WITH NON-SHRINK GROUT. GROUT TO MATCH PANEL.
- 10. SEAL ALL OPEN JOINTS WITH CAULKING COMPOUND AND/OR JOINT SEALING MATERIAL. (COLOR TO MATCH PANEL).
- 11. REFER TO PUBLICATION 408, SECTION 1086.3(f) FOR FABRICATION AND ERECTIONS
- 12. CHAMFER EXPOSED CONCRETE EDGES ON PANELS $\frac{1}{2}$ " x $\frac{1}{2}$ ", EXCEPT AS NOTED.
- 13. ALL DIMENSIONS SHOWN ARE HORIZONTAL, EXCEPT AS NOTED.
- 14. DIMENSIONS SHOWN ARE FOR A NORMAL TEMPERATURE OF 68 DEGREES F.
- 15. REINFORCEMENT IN SOME SECTIONS IS NOT SHOWN FOR CLARITY.
- 16. COORDINATE, LOCATE, AND CONDUCT ALL WORK RELATED TO PUBLIC AND PRIVATE UTILITIES IN ACCORDANCE WITH PUBLICATION 408, SECTION 105.06 AND 107.12, AND THE CONTRACT SPECIAL PROVISIONS.
- 17. IF NEEDED DETAIL IS NOT FOUND IN THE SOUND BARRIER STANDARDS OR ON THE CONTRACT DRAWINGS A SPECIAL SUBMISSION REQUESTING APPROVAL FOR SPECIFIC DETAILS MUST BE MADE TO THE CHIEF BRIDGE ENGINEER.

ARCHITECTURAL SURFACE TREATMENTS

- 1. THE AVERAGE ARCHITECTURAL SURFACE TREATMENT THICKNESS, PER SIDE OF PANEL, IS PERMITTED TO VARY FROM O TO 1½ INCH, BUT THE TOTAL AVERAGE ARCHITECTURAL SURFACE TREATMENT, ON BOTH SIDES OF THE PANEL, MUST NOT BE GREATER THAN 1½ INCH UNLESS OTHERWISE INDICATED ON THE CONTRACT DRAWINGS.
- 2. IF A SMOOTH ARCHITECTURAL SURFACE TREATMENT IS PROVIDED, THE TREATMENT MAY EXTEND TO THE EDGES OF PANELS AS LONG AS THE PANEL FITS BETWEEN THE FLANGES OF THE POST.
- 3. STAMPED FINISHES MAY BE PERMITTED IF ACCEPTED BY THE DISTRICT BRIDGE ENGINEER.
- REFER TO PUBLICATION 408, SECTION 1086.3 AND/OR THE CONTRACT DOCUMENTS FOR ARCHITECTURAL SURFACE TREATMENT TOLERANCES.
- 5. REFER TO CONTRACT DOCUMENTS FOR ADDITIONAL INFORMATION.

BC-736M

BC-779M

PRECAST CONCRETE POSTS

	INDEX OF SHEETS
SHT. NO.	SHEET TITLE
1	GENERAL NOTES - 1
2	GENERAL NOTES - 2
3	GEOMETRY AND LAYOUT
4	PRECAST CONCRETE PANEL DETAILS - 1
5	PRECAST CONCRETE PANEL DETAILS - 2
6	PRECAST CONCRETE PANEL DETAILS - 3
7	PRECAST CONCRETE PANEL DETAILS - 4

CHANGE 2

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE PANELS

REINFORCEMENT BAR FABRICATION DETAILS GENERAL NOTES - 1 GROUND MOUNTED SOUND BARRIERS -

GROUND MOUNTED SOUND BARRIERS - STEEL POSTS RECOMMENDED JAN. 31, 2019

Thoras P. Marine

RECOMMENDED JAN. 31, 2019

Millian Hadrin STRUCTURE MOUNTED SOUND BARRIER WALLS REFERENCE DRAWINGS

SHEET 1 OF 7 ACTING DIR. BUR. OF PROJECT DELIVERY BC-776M

NOTES TO FABRICATOR

- 1. PROVIDE SHOP DRAWINGS IN ACCORDANCE WITH PUBLICATION 408, SECTION 105.02(d)
- AND 1086.

 2. THE FOLLOWING INFORMATION MUST BE SHOWN ON THE SHOP DRAWINGS (IF APPLICABLE):

 GENERAL NOTES
 FABRICATION NOTES
 ITRANSPORTATION NOTES
 INSTALLATION NOTES

 ELEVATION VIEW INDICATING THE FOLLOWING MINIMUM INFORMATION:
 OVERALL WALL LENGTH
 POST SPACINGS
 POST AND PANEL CODES/DESIGNATIONS
 HORIZONTAL JOINT LOCATIONS (IF PERMITTED)
 ELEVATIONS FOR THE FOLLOWING ITEMS:
 ACOUSTIC PROFILE ELEVATIONS
 TOP OF WALL ELEVATIONS
 HORIZONTAL JOINT ELEVATIONS
 FOR POST SELEVATIONS
 TOP OF BASE PLATE ELEVATIONS
 TOP OF CAISSON ELEVATIONS
 TOP OF CAISSON ELEVATIONS
 INDIVIDUAL POST DETAILS
 INDIVIDUAL PANEL DETAILS
 CONNECTION DETAILS
 ANCHOR BOLT DETAILS
 ANCHOR BOLT DETAILS
 LIFTING INSERT DETAILS
 ANCHOR BOLT DETAILS
 ANY OTHER INFORMATION REQUIRED TO FABRICATE AND CONSTRUCT THE SOUND BARRIER WALL
- 3. THE SHOP DRAWINGS FOR THE PRECAST CONCRETE SOUND BARRIER PANELS AND THE PRECAST CONCRETE OR FABRICATED STRUCTURAL STEEL POSTS MUST BE SUBMITTED CONCURRENTLY.
- 4. PRECAST CONCRETE PANELS: THE FABRICATOR MUST ENSURE THAT THE PANELS ARE ADEQUATELY DESIGNED FOR STRESSES DUE TO STRIPPING, HANDLING, ERECTION AND TRANSPORTATION. PROVIDE AND SUBMIT DESIGN CALCULATIONS, AS REQUIRED.
- - IFTING INSERTS:

 PREPARE AND SUBMIT DESIGN CALCULATIONS FOR POST AND PANEL LIFTING INSERTS FOR ACTUAL STRENGTH OF CONCRETE AT TIME OF STRIPPING, TRANSPORTATION AND ERECTION.

 PROVIDE LIFTING INSERTS WITH A MINIMUM CAPACITY OF AT LEAST TWO TIMES THE CALCULATED LOAD ON THE INSERT.

 PROVIDE A MINIMUM OF TWO LIFTING INSERTS OR A MAXIMUM OF FOUR LIFTING INSERTS IN THE PRECAST CONCRETE PANELS.

 PROVIDE GALVANIZED INSERTS.
- 6. IF REQUIRED, PREPARE AND SUBMIT TEMPORARY BRACING CALCULATIONS AND DETAILS.
- 7. PREPARE AND SUBMIT CATALOG CUTS IN ACCORDANCE WITH PUBLICATION 408, SECTION 1086.3.
- 8. #4 GRADE 60 REINFORCEMENT BARS MAY BE SUBSTITUTED FOR WELDED WIRE FABRIC WITH AN EQUIVALENT AREA AT NO ADDITIONAL COST TO THE DEPARTMENT.
- PANELS MUST BE STORED, TRANSPORTED, HANDLED, AND ERECTED ON EDGES AT ALL TIMES. PANELS SHOULD NOT BE LAID FLAT.
- 10. FABRICATORS MUST BE PRE-APPROVED BY PENNDOT PER BULLETIN #15.

MATERIAL NOTES

- PRECAST CONCRETE SOUND BARRIER PANELS:

 PROVIDE CLASS AA CEMENT CONCRETE, MODIFIED IN THE PRECAST CONCRETE PANELS.
 f'c = 5,000 PSI
 DENSITY OF CONCRETE = UNIT WEIGHT OF CONCRETE = 150 LB./CU.FT.
 PROVIDE A MINIMUM CONCRETE STRENGTH OF 4,000 PSI BEFORE STRIPPING THE PANELS FROM THE FORMS.
- 2. REINFORCEMENT STEEL:
 - PROVIDE GRADE 60 DEFORMED REINFORCING BARS THAT MEET THE REQUIREMENTS
 OF ASTM A615, ASTM A996 OR ASTM A706. DO NOT WELD
 REINFORCING BARS UNLESS SPECIFIED. DO NOT USE RAIL STEEL A996
 REINFORCEMENT BARS WHERE BENDING OR WELDING OF REINFORCEMENT BARS IS

 - REINFORCEMENT BARS WHERE BENDING OR WELDING OF REINFORCEMENT DARS IS
 INDICATED,
 fs = 24,000 PSI

 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED REINFORCEMENT IN THE PANELS AS
 SPECIFIED ON THE CONTRACT DRAWINGS.
 PROVIDE MINIMUM LAP AND EMBEDMENT LENGTH FOR REINFORCING BARS OF 30 DIAMETERS
 OR IN ACCORDANCE WITH THE CURRENT AASHTO SPECIFICATIONS AS MODIFIED BY THE
 DESIGN MANUAL PART 4, WHICHEVER IS GREATER.
- 3. WELDED WIRE FABRIC:

 PROVIDE GRADE 65 PLAIN WELDED WIRE FABRIC THAT MEET THE REQUIREMENTS OF ASTM A185 IN THE PRECAST CONCRETE PANELS.

 fs = 24,000 PSI

 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED WELDED WIRE FABRIC IN THE PANELS AS SPECIFIED ON THE CONTRACT DRAWINGS.

 PROVIDE MINIMUM LAP FOR WELDED WIRE FABRIC IN ACCORDANCE WITH CURRENT AASHTO SPECIFICATIONS AS MODIFIED BY THE DESIGN MANUAL PART 4.

 DO NOT MIX THE USE OF WELDED WIRE FABRIC AND REINFORCEMENT STEEL IN THE PANEL, EXCEPT AS INDICATED.
- 4. PLAIN NEOPRENE BEARING PADS AND ELASTOMERIC PADS:

 PROVIDE PLAIN NEOPRENE PADS WITH A DUROMETER HARDNESS OF 50 (+/-)5

 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1113.02.
- 5. NON-SHRINK GROUT:

 PROVIDE NON-SHRINK GROUT IN ACCORDANCE WITH PUBLICATION 408, SECTION 1080.2(c).

 PLACE NON-SHRINK GROUT AFTER THE BASE PLATE IS LEVELED ON THE LEVELING NUTS.

 PACK GROUT INTO PLACE. DO NOT POUR OR INJECT GROUT.

 NON-SHRINK GROUT TO MATCH FINAL COLOR OF PANEL.

- 7. JOINT SEALING MATERIAL:

 PROVIDE JOINT SEALING MATERIAL IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.4(a).

 JOINT SEALING MATERIAL TO MATCH FINAL COLOR OF PANEL.

- 8. JOINT BACKING MATERIAL (BACKER ROD):
 PROVIDE BACKER ROD MATERIAL IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.9.
- 9. ANTIGRAFFITI COATING:

 APPLY ANTIGRAFFITI COATING IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.
- 10. PENETRATING CONCRETE STAIN:

 APPLY STAIN IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE PANELS

GENERAL NOTES - 2

RECOMMENDED JAN. 31, 2019

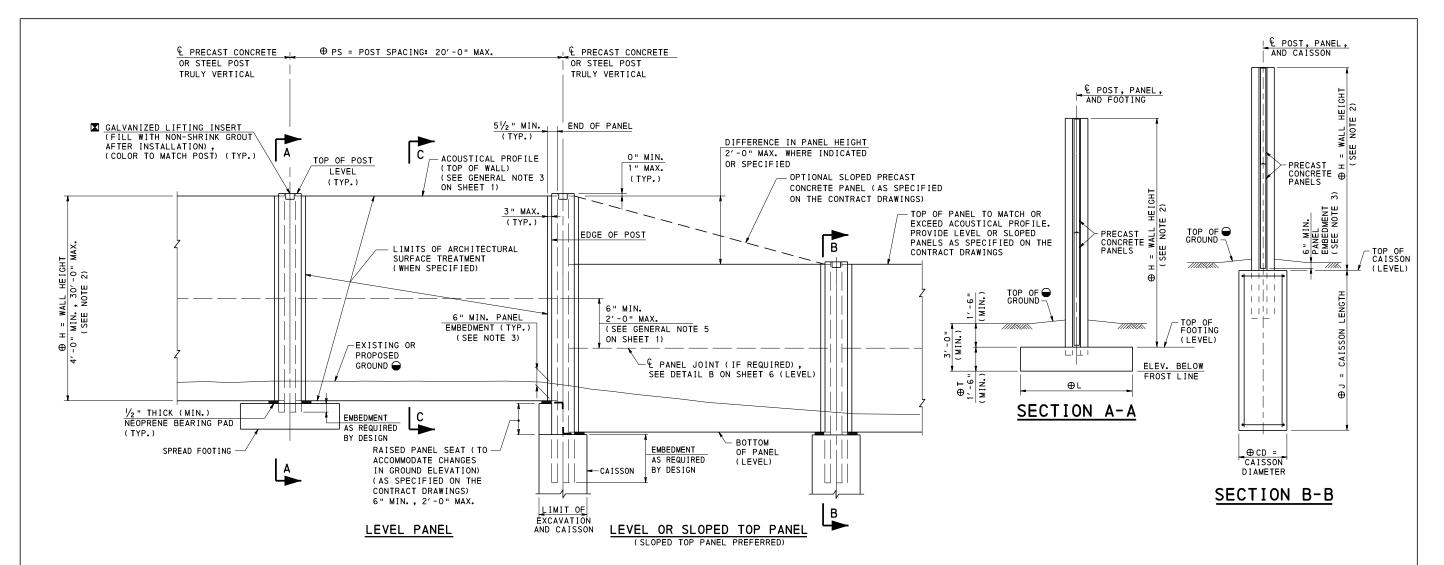
RECOMMENDED JAN. 31, 2019

RECOMMENDED JAN. 31, 2019

RECOMMENDED JAN. 31, 2019

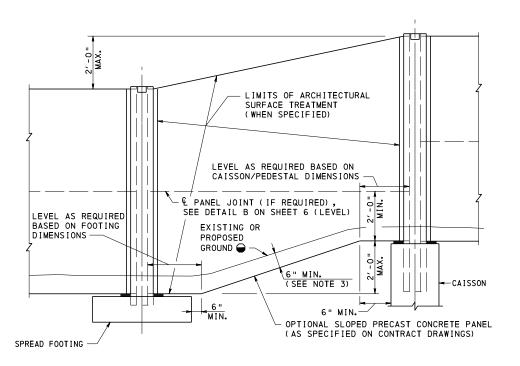
SHEET 2 OF 7

ACTING DIR. BUR. OF PROJECT DELIVERY BC-776M



GROUND MOUNTED SOUND BARRIER ELEVATION

(PRECAST CONCRETE POST SHOWN, STEEL POST SIMILAR)



OPTIONAL SLOPED BOTTOM PANEL ELEVATION

(USE IN PLACE OF RAISED PANEL SEAT)

LEGEND:

■ FABRICATOR TO VERIFY ADEQUACY

REFER TO CONTRACT DRAWINGS

GRADE GROUND TO DRAIN AWAY FROM WALL. FILL DEPTH ON EACH SIDE OF WALL TO BE WITHIN

OF LIFTING INSERTS

⊕ AS REQUIRED BY DESIGN

1'-0" DIFFERENCE.

- FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEETS 1 AND 2.
- 2. WALL HEIGHT IS DEFINED AS FOLLOWS: ● POST WITH BASE PLATE: H = HEIGHT FROM

NOTES:

- TOP OF BASE PLATE TO TOP OF WALL
 - POST WITHOUT BASE PLATE: H = HEIGHT FROM TOP OF FOOTING/CAISSON TO TOP OF WALL
- 3. PANEL EMBEDMENT MAY NEED TO BE INCREASED TO ACCOMMODATE BASE PLATES AND ANCHOR BOLT PROJECTIONS.
- 4. FOR SECTION C-C, REFER TO SHEET 6.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

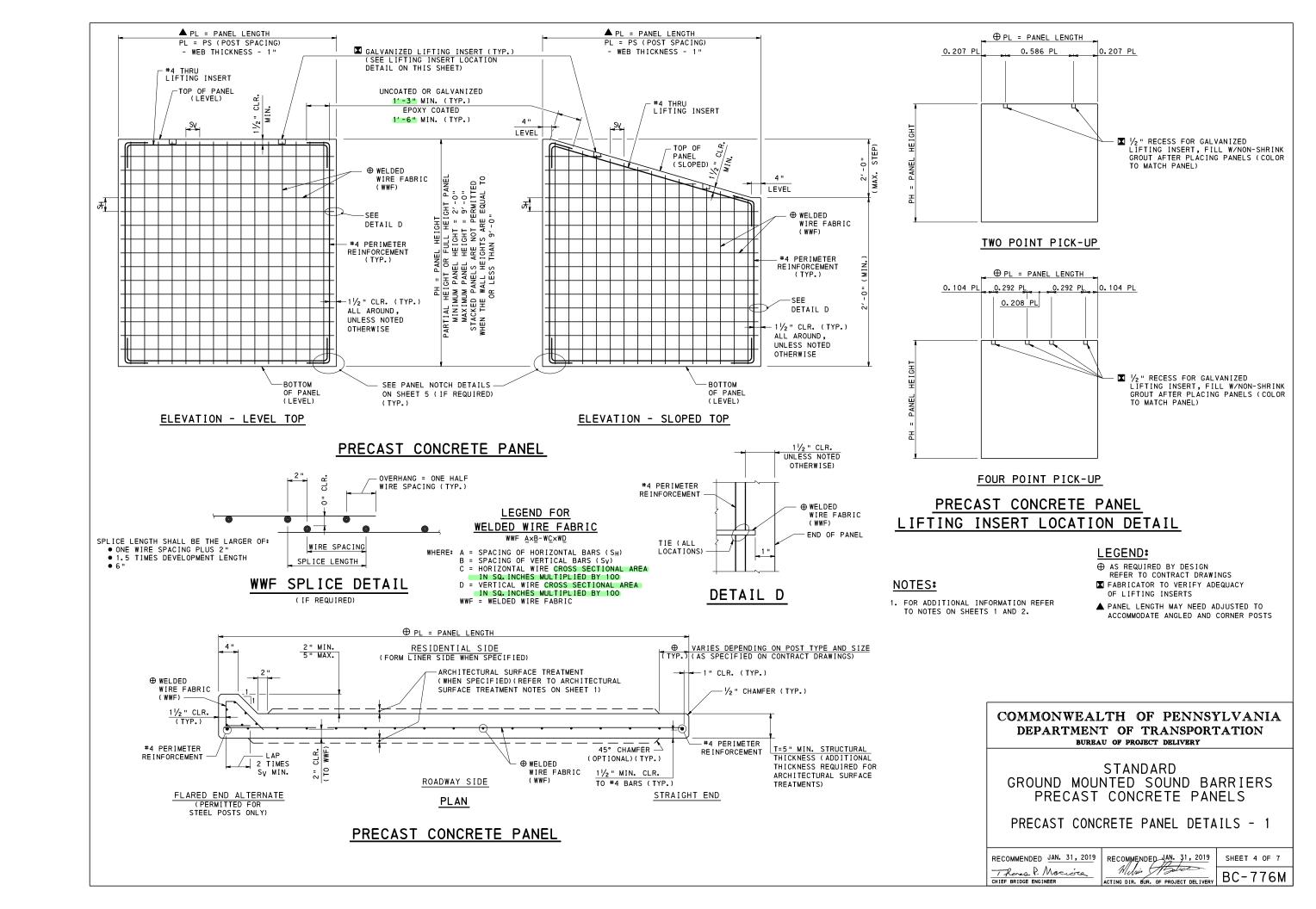
STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE PANELS

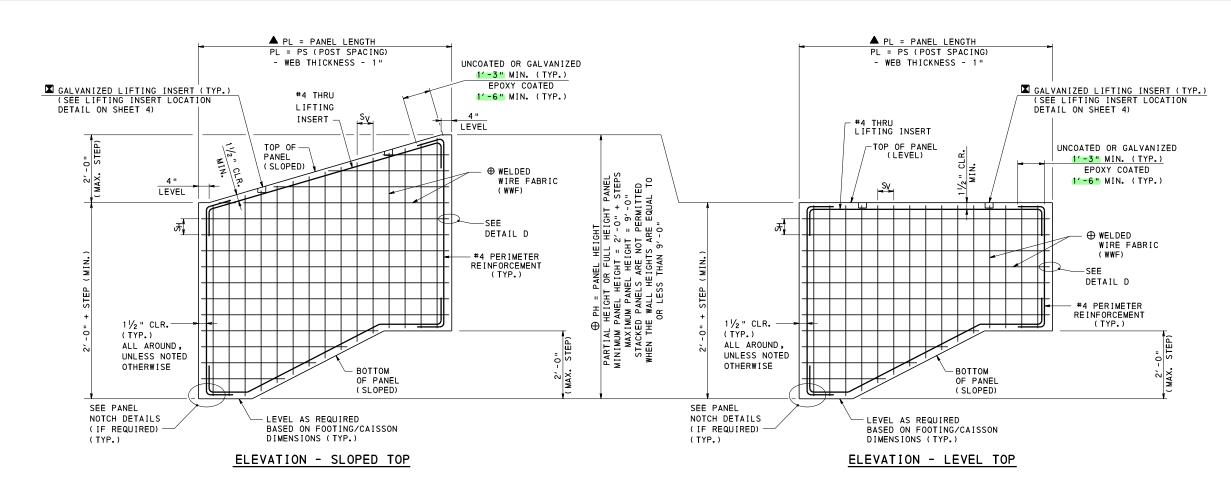
GEOMETRY AND LAYOUT

RECOMMENDED JAN. 31, 2019 Thomas P. Maciona

RECOMMENDED JAN. 31, 2019 ACTING DIR. BUR. OF PROJECT DELIVERY BC-776M

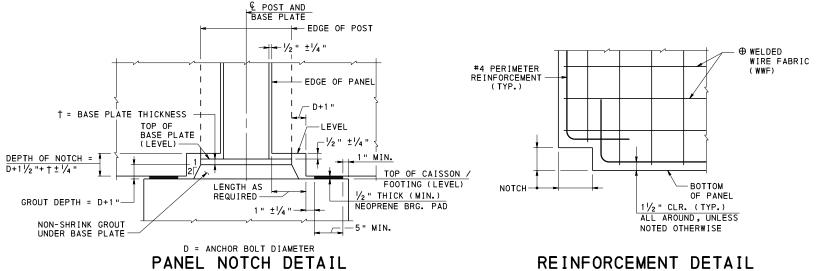
SHEET 3 OF 7





PRECAST CONCRETE PANEL WITH OPTIONAL SLOPED BOTTOM

FOR PANEL NOTCH



PANEL NOTCH DETAIL FOR BASE PLATE CONNECTIONS

(FOR DETAILS 1 AND 2) (PRECAST POST SHOWN / STEEL POST SIMILAR)

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEETS 1 AND 2.
- 2. FOR DETAIL D, REFER TO SHEET 4.

LEGEND:

- ⊕ AS REQUIRED BY DESIGN
- REFER TO CONTRACT DRAWINGS ▼ FABRICATOR TO VERIFY ADEQUACY OF LIFTING INSERTS
- ▲ PANEL LENGTH MAY NEED ADJUSTED TO ACCOMMODATE ANGLED AND CORNER POSTS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE PANELS

PRECAST CONCRETE PANEL DETAILS - 2

Thomas P. Mariora

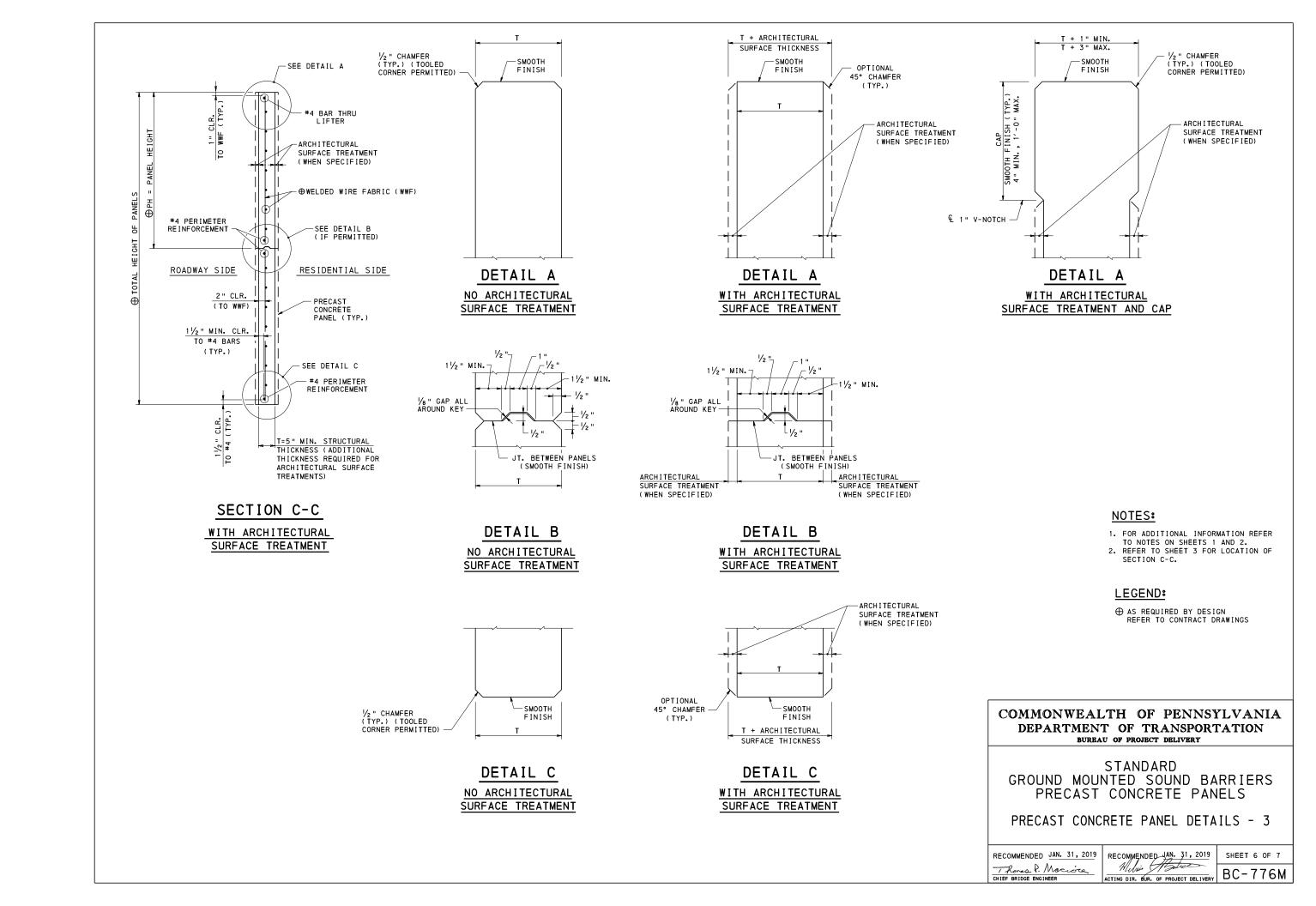
RECOMMENDED JAN. 31, 2019

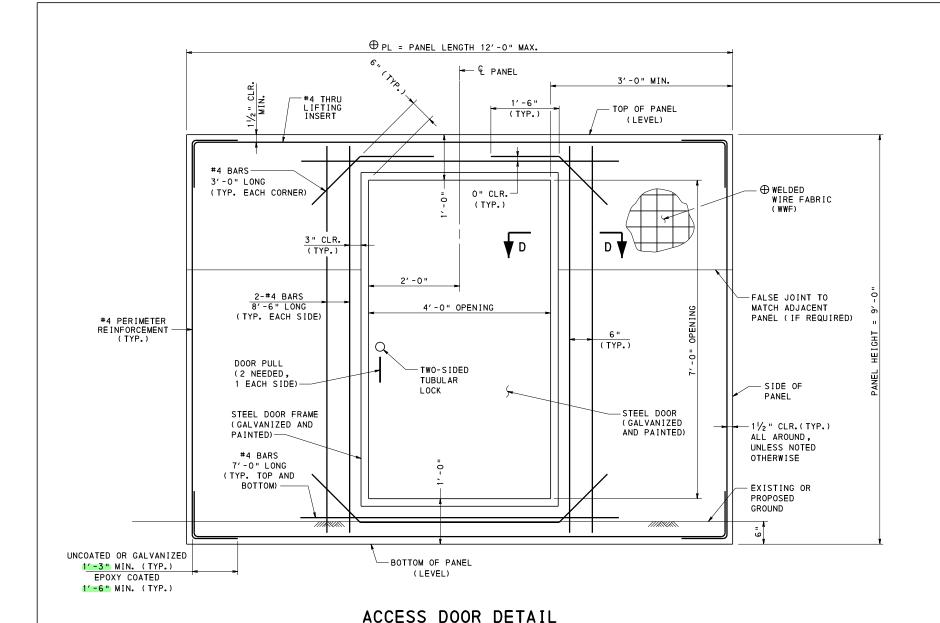
Millia House

SHEET 5 OF 7

RECOMMENDED JAN. 31, 2019

ACTING DIR. BUR. OF PROJECT DELIVERY BC-776M





STEEL DOOR FRAME

"T" MASONRY ANCHOR SPACED

AT 1'-0" MAX. (GALVANIZED)

OR AS ACCEPTED BY THE ENGINEER

ARCHITECTURAL

⊕ WELDED WIRE FABRIC (WWF)

SURFACE TREATMENT

(WHEN SPECIFIED)

(GALVANIZED AND

PAINTED)

SECTION D-D

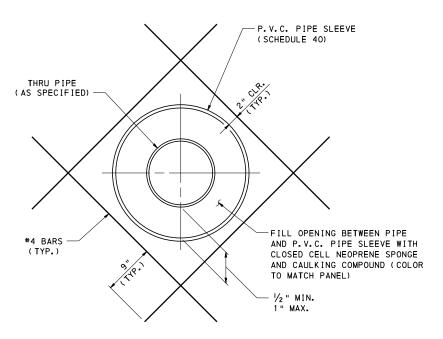
T=5" MIN. STRUCTURAL

THICKNESS (ADDITIONAL

THICKNESS REQUIRED FOR

ARCHITECTURAL SURFACE

TREATMENTS)



SLEEVE DETAIL AT OPENINGS



LEGEND:

AS REQUIRED BY DESIGN
 REFER TO CONTRACT DRAWINGS

NOTES:

1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEETS 1 AND 2.

ACCESS DOOR NOTES:

- REFER TO CONTRACT DRAWINGS FOR LOCATION OF ACCESS DOOR (IF REQUIRED) AND PROVIDE DETAILS ON THE SHOP DRAWINGS.
 STEEL DOOR AND DOOR FRAME TO BE GALVANIZED AND PAINTED TO MATCH
- COLOR OF PANEL AS SPECIFIED IN THE SPECIAL PROVISIONS.

 3. IF ACCESS DOOR IS REQUIRED IN THE SOUND BARRIER WALL, PROVIDE A 12'-0" MAXIMUM POST SPACING.
- 4. PROVIDE A 13/4" THICK EXTERIOR DOOR WITH A SMALL CELL
- HONEYCOMB OR A POLYMETHANE CORE. CORE TO BE COVERED WITH GALVANIZED STEEL WITH A 16 GAUGE THICKNESS.
- 5. MOUNT DOORS USING THREE HINGES.
- 6. PROVIDE A GALVANIZED STEEL DOOR FRAME WITH A 14 GAUGE THICKNESS.
 7. ATTACH DOOR FRAME TO PRECAST CONCRETE PANEL USING GALVANIZED STEEL
 "T" MASONRY ANCHORS OR AN ACCEPTABLE ALTERNATE APPROVED BY THE ENGINEER.
- 8. DOOR FRAME WIDTH TO BE FLUSH WITH THE STRUCTURAL THICKNESS OF THE PRECAST
- CONCRETE PANEL.

 9. PROVIDE STAINLESS STEEL DOOR PULLS (TWO NEEDED, ONE PER SIDE). MOUNT DOOR PULLS USING STAINLESS STEEL THRU-BOLTS OR AN ACCEPTABLE ALTERNATE APPROVED BY THE ENGINEER. CENTER DOOR PULLS AT 3'-0" ABOVE THE FINISHED GRADE.
- 10. PROVIDE A WEATHER-RESISTANT TWO-SIDED TUBULAR LOCKING DEVICE WITH A STAINLESS STEEL FINISH. KEY LOCKS AS SPECIFIED IN THE SPECIAL PROVISIONS OR AS DIRECTED BY THE ENGINEER.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE PANELS

PRECAST CONCRETE PANEL DETAILS - 4

RECOMMENDED JAN. 31, 2019 Thomas P. Mariora

RECOMMENDED JAN. 31, 2019

Millian Hodis ACTING DIR. BUR. OF PROJECT DELIVERY BC-776M

SHEET 7 OF 7

GENERAL NOTES

- 1. DESIGN SPECIFICATIONS:
- DESIGN SPECIFICATIONS:

 PENNDOT DESIGN MANUAL PART 4, STRUCTURES APRIL 2015 EDITION.
 1989 AASHTO "GUIDE SPECIFICATIONS FOR STRUCTURAL DESIGN OF SOUND BARRIERS", INCLUDING THE 1992 AND 2002 INTERIMS.
 2002 AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES", 17TH EDITION.
 2001 AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS", 4TH EDITION, INCLUDING THE INTERIMS THROUGH 2006.
 DESIGN IS IN ACCORDANCE WITH THE WORKING STRESS DESIGN METHOD. (NO INCREASE IN ALLOWABLE UNIT STRESSES ARE PERMITTED EXCEPT FOR GROUP III LOADINGS WHICH PERMITS A 33% OVERSTRESS.)

- 2. CONSTRUCTION SPECIFICATIONS AND WORKMANSHIP:

 PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH THE CURRENT VERSION OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, AASHTO/AWS/D1.5 BRIDGE WELDING CODE AND THE CONTRACT SPECIAL PROVISIONS. (USE AASHTO/AWS/D1.1 FOR WELDING NOT COVERED IN AASHTO/AWS/D1.5.)
- 3. WALL HEIGHTS MUST EQUAL OR EXCEED THE ACOUSTICAL PROFILE.
- 4. INSTALL ANCHOR BOLTS, POSTS, AND PANELS TRULY VERTICAL.
- PROVIDE CONCRETE COVER IN ACCORDANCE WITH THIS STANDARD AND DESIGN MANUAL PART 4.
- 6. A HIGHER STRENGTH CONCRETE, FOR CAST-IN-PLACE CONCRETE, MAY BE SUBSTITUTED FOR A LOWER CLASS CONCRETE AT NO ADDITIONAL COST TO THE DEPARTMENT.
- 7. FILL ALL LIFTING INSERTS WITH NON-SHRINK GROUT. (COLOR TO MATCH PANEL)
- 8. SEAL ALL OPEN JOINTS WITH CAULKING COMPOUND AND/OR JOINT SEALING MATERIAL. (COLOR TO MATCH PANEL).
- 9. REFER TO PUBLICATION 408. SECTION 1086.3(f) FOR FABRICATION AND ERECTIONS TOLERANCES.
- 10. REFER TO PUBLICATION 408. SECTION 1006.3(g) FOR CAISSON SHAFT TOLERANCES.
- 11. CHAMFER EXPOSED CONCRETE EDGES ON PRECAST POSTS $\frac{1}{2}$ " x $\frac{1}{2}$ ", EXCEPT AS NOTED.
- 12. CHAMFER EXPOSED CONCRETE EDGES ON CAST-IN-PLACE CONCRETE 1" x 1", EXCEPT AS NOTED.
- 13. RAKE-FINISH ALL HORIZONTAL CONSTRUCTION JOINTS. EXCEPT AS NOTED.
- 14. ALL DIMENSIONS SHOWN ARE HORIZONTAL. EXCEPT AS NOTED.
- 15. DIMENSIONS SHOWN ARE FOR A NORMAL TEMPERATURE OF 68 DEGREES F.
- 16. REINFORCEMENT IN SOME SECTIONS IS NOT SHOWN FOR CLARITY.
- 17. SPREAD FOOTINGS:

 CONSTRUCT EMBANKMENTS AND/OR CUT EXISTING GRADE TO THE TOP OF FOOTING
 - ELEVATIONS.
 EXCAVATE FOR FOOTING CONSTRUCTION.

 - EXCAVATE FOR FOOTING CONSTRUCTION.
 CONSTRUCT FOOTING.
 SPREAD FOOTINGS MAY BE ORDERED BY THE REPRESENTATIVE TO BE AT ANY ELEVATION OR ANY DIMENSIONS NECESSARY TO PROVIDE A PROPER FOUNDATION. IF SPREAD FOOTINGS ARE ADJUSTED, PANEL HEIGHTS AND POST DESIGNS WILL NEED TO BE ADJUSTED.
 USE CLASS C CEMENT CONCRETE OR NO. 2A COARSE AGGREGATE BELOW SPREAD FOOTING WHEN SPECIFIED OR DIRECTED.

CHANGE 2

- 18. CAISSONS:

 CONSTRUCT EMBANKMENTS AND/OR CUT EXISTING GRADE TO THE TOP OF CAISSON ELEVATIONS PRIOR TO CONSTRUCTION OF CAISSONS.

 THE CONTRACTOR IS RESPONSIBLE FOR MAINTAINING THE DRILLED OPENING INTACT AND FOR MAINTAINING THE STABILITY OF THE GROUND CUT SLOPE OR FILLED EMBANKMENT DURING DRILLING AND INSTALLATION OF CAISSONS.

 TEMPORARY CASING MAY BE REQUIRED DURING CAISSON CONSTRUCTION IN ORDER TO MAINTAIN AN OPEN SHAFT. IF CASING IS USED, MAINTAIN CONCRETE LEVELS ABOVE THE BOTTOM OF CASING AT ALL TIMES DURING CASING EXTRACTION TO PREVENT CAVED MATERIAL FROM CONTAMINATING THE CONCRETE.

 IF GROUNDWATER FLOW ENTERS THE CAISSON EXCAVATION DURING CONSTRUCTION, PLACE CONCRETE BY TREMIE METHODS TO ABOVE THE GROUND WATER ELEVATION IN ONE CONTINUOUS OPERATION. FILL REMAINDER OF CAISSON WITH CLASS A CONCRETE. PLACE EPOXY BONDING COMPOUND BETWEEN POURS, AS REQUIRED.
- 19. COORDINATE, LOCATE, AND CONDUCT ALL WORK RELATED TO PUBLIC AND PRIVATE UTILITIES IN ACCORDANCE WITH PUBLICATION 408, SECTION 105.06 AND 107.12, AND THE CONTRACT SPECIAL PROVISIONS.
- 20. FOR ADDITIONAL INFORMATION REFER TO BC-776M.
- 21. IF NEEDED DETAIL IS NOT FOUND IN THE SOUND BARRIER STANDARDS OR ON THE CONTRACT DRAWINGS A SPECIAL SUBMISSION REQUESTING APPROVAL FOR SPECIFIC DETAILS MUST BE MADE TO THE CHIEF BRIDGE ENGINEER.

MATERIAL NOTES

- 1. CAST-IN-PLACE CONCRETE:

 PROVIDE CLASS A CEMENT CONCRETE IN THE CAST-IN-PLACE FOOTINGS, PEDESTALS, AND CAISSONS.

 f'c = 3,000 PSI

 UNIT WEIGHT OF CONCRETE = 150 LB. / CU. FT.

- 2. PRECAST CONCRETE POSTS:

 PROVIDE CLASS AA CEMENT CONCRETE, MODIFIED IN THE PRECAST CONCRETE POSTS.

 f'c = 5,000 PSI

 UNIT WEIGHT OF CONCRETE = 150 LB. / CU. FT.

 PROVIDE A MINIMUM CONCRETE STRENGTH OF 4,000 PSI BEFORE STRIPPING
 THE POSTS FROM THE FORMS.

 PROVIDE SMOOTH FINISH ON ALL FACES OF THE PRECAST POST, UNLESS OTHERWISE SPECIFIED ON THE CONTRACT DRAWINGS.

- 3. REINFORCEMENT STEEL:

 PROVIDE GRADE 60 DEFORMED REINFORCING BARS THAT MEET THE REQUIREMENTS OF ASTM A615, ASTM A996, OR ASTM A706. DO NOT WELD REINFORCING BARS IN FOOTINGS, CAISSONS, OR WHERE BENDING OR WELDING OF REINFORCEMENT BARS IN FOOTINGS, CAISSONS, OR WHERE BENDING OR WELDING OF REINFORCEMENT BARS IS INDICATED.

 fs = 24,000 PSI

 PROVIDE UNCOATED REINFORCEMENT IN THE FOOTINGS AND CAISSONS.

 PROVIDE UNCOATED REINFORCEMENT IN THE FOOTINGS AND CAISSONS.

 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED REINFORCEMENT IN THE POSTS, RAISED PANEL SEATS, AND PEDESTALS AS SPECIFIED ON THE CONTRACT DRAWINGS.

 PROVIDE EPOXY COATED OR GALVANIZED THREADED REINFORCEMENT BARS IN THE PRECAST CONCRETE POST WITH BASE PLATES.

 PROVIDE MINIMUM LAP AND EMBEDMENT LENGTH FOR REINFORCING BARS OF 30 DIAMETERS OR IN ACCORDANCE WITH THE CURRENT AASHTO SPECIFICATIONS AS MODIFIED BY THE DESIGN MANUAL PART 4, WHICHEVER IS GREATER.

 DO NOT SPLICE VERTICAL POST REINFORCEMENT.

 MECHANICAL CONNECTIONS, WHICH MEETS THE REQUIREMENTS OF PUBLICATION 408, SECTION 1002, MAY BE USED UPON ACCEPTANCE FROM THE REPRESENTATIVE.

- 4. WELDED WIRE FABRIC:

 PROVIDE GRADE 70 DEFORMED WELDED WIRE FABRIC THAT MEET THE REQUIREMENTS

 OF ASTM A497 IN THE PRECAST CONCRETE POSTS.

 fs = 24,000 PSI

 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED WELDED WIRE FABRIC IN THE POSTS.

 PROVIDE MINIMUM LAP FOR WELDED WIRE FABRIC IN ACCORDANCE WITH CURRENT AASHTO SPECIFICATIONS AS MODIFIED BY THE DESIGN MANUAL PART 4.

 DO NOT MIX THE USE OF WELDED WIRE FABRIC AND REINFORCEMENT STEEL, EXCEPT AS INDICATED.

- 5. FABRICATED STRUCTURAL STEEL:

 PROVIDE STRUCTURAL STEEL CONFORMING TO AASHTO M270 GRADE 36

 (ASTM A709, GRADE 36) UNLESS OTHERWISE NOTED.

 GALVANIZE PLATES AND HARDWARE IN ACCORDANCE WITH PUBLICATION 408, SECTION

 - REPAIR DAMAGED GALVANIZING IN ACCORDANCE WITH PUBLICATION 408. SECTION 1105.02(s) 2.

- 6. ANCHOR BOLTS, NUTS, AND WASHERS:

 ◆ PROVIDE ANCHOR BOLTS CONFORMING TO ASTM F1554, GRADE 36 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c) 3.

 ◆ PROVIDE HEAVY HEX NUTS CONFORMING TO ASTM A563 IN ACCORDANCE

 - PROVIDE HEAVY HEX NUTS CONFORMING TO ASTM A563 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c) 3d.
 PROVIDE OVERSIZE WASHERS CONFORMING TO AASHTO M270 GRADE 36 (ASTM A709, GRADE 36).
 PROVIDE LOCK WASHERS AND FLAT WASHERS CONFORMING TO ASTM F436 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c) 3b.
 GALVANIZE ALL ANCHOR BOLTS AND HARDWARE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s).
- 7. PLAIN NEOPRENE BEARING PADS AND ELASTOMERIC PADS:

 PROVIDE PLAIN NEOPRENE PADS WITH A DUROMETER HARDNESS OF 50 (+/-) 5
 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1113.02.

8. EPOXY BONDING COMPOUND: ● PROVIDE EPOXY BONDING COMPOUND IN ACCORDANCE WITH PUBLICATION 408, SECTION 706.1.

- ON-SHRINK GROUT:

 PROVIDE NON-SHRINK GROUT IN ACCORDANCE WITH PUBLICATION 408, SECTION 1080.2(c).

 PLACE NON-SHRINK GROUT AFTER THE BASE PLATE IS LEVELED ON THE LEVELING NUTS.

 PACK GROUT INTO PLACE. DO NOT POUR OR INJECT GROUT.

 NON-SHRINK GROUT TO MATCH FINAL COLOR OF PANEL.

- 10. CAULKING COMPOUND:

 PROVIDE CAULKING COMPOUND IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.8(b).

 CAULKING COMPOUND TO MATCH FINAL COLOR OF PANEL.

- PROVIDE JOINT SEALING MATERIAL IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.4(d).
- JOINT SEALING MATERIAL TO MATCH FINAL COLOR OF PANEL.

12. JOINT BACKING MATERIAL (BACKER ROD):

• PROVIDE BACKER ROD MATERIAL IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.9.

APPLY ANTIGRAFFITI COATING IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.

14. PENETRATING CONCRETE STAIN:

● APPLY STAIN IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.

BC-734M ANCHOR SYSTEMS WALL CONSTRUCTION AND EXPANSION JOINT DETAILS BC-735M BC-736M REINFORCEMENT BAR FABRICATION DETAILS GROUND MOUNTED SOUND BARRIERS BC-776M PRECAST CONCRETE PANELS GROUND MOUNTED SOUND BARRIERS - STEEL POSTS STRUCTURE MOUNTED SOUND BARRIER WALLS RC-11M CLASSIFICATION OF EARTHWORK FOR STRUCTURES REFERENCE DRAWINGS

NOTES TO FABRICATOR

1. REFER TO BC-776M FOR NOTES TO FABRICATOR.

INDEX OF SHEETS	
SHT.NO.	SHEET TITLE
1	GENERAL NOTES
2	GEOMETRY AND LAYOUT
3	POST DETAILS
4	PANEL SEAT DETAILS
5	DETAIL 1
6	DETAIL 2
7	DETAIL 3
8	DETAIL 4
9	DETAIL 5
10	DETAIL 6
11	DETAIL 7
12	DETAIL 8

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	DESCRIPTION OF DETAILS
	DESCRIPTION OF DETAILS
DETAIL	DESCRIPTION
1	PRECAST CONCRETE POST WITH BASE PLATE CONNECTION TO CAISSON
2	PRECAST CONCRETE POST WITH BASE PLATE CONNECTION TO SPREAD FOOTING
3	PRECAST CONCRETE POST EMBEDDED IN CAISSON
4	PRECAST CONCRETE POST EMBEDDED IN SPREAD FOOTING (WITH OR WITHOUT PEDESTAL)
5	PRECAST CONCRETE ANGLED POST EMBEDDED IN CAISSON
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7	PRECAST CONCRETE ANGLED POST EMBEDDED IN SPREAD FOOTING (WITH OR WITHOUT PEDESTAL)
8	PRECAST CONCRETE CORNER POST EMBEDDED IN SPREAD FOOTING (WITH OR WITHOUT PEDESTAL)

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

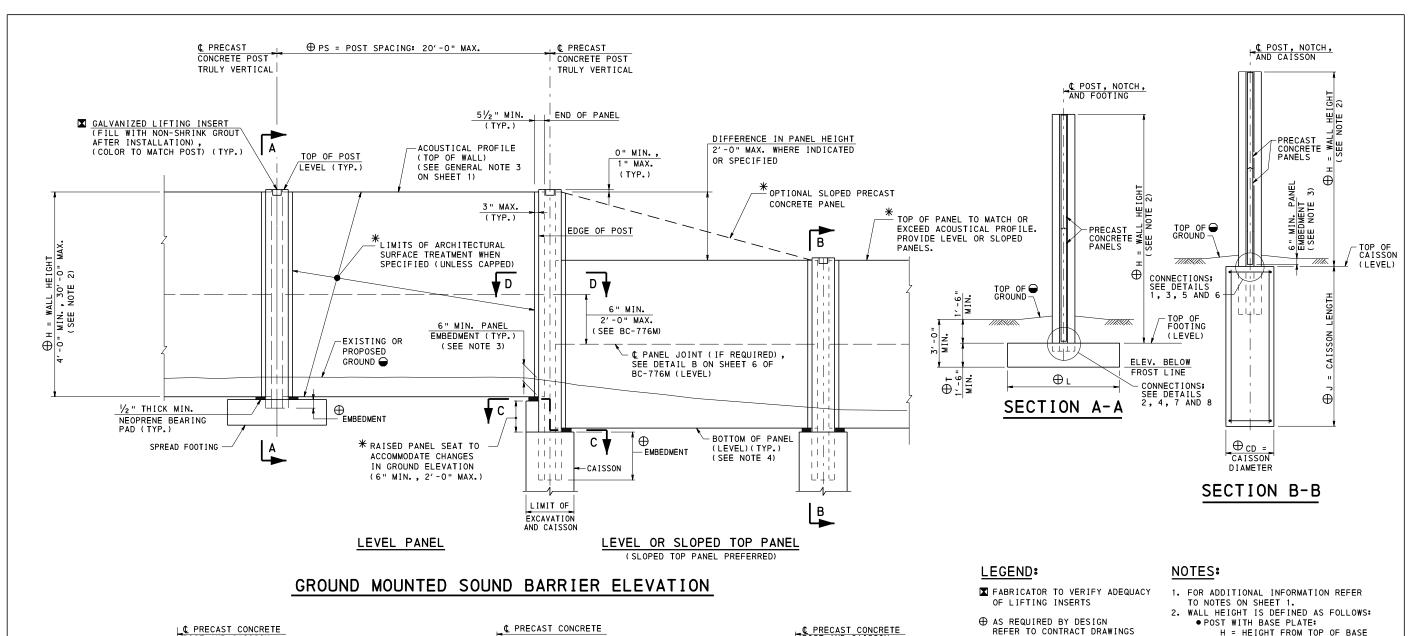
STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

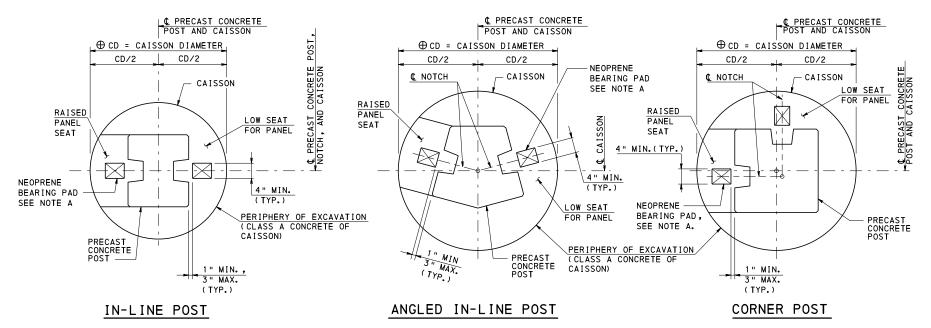
GENERAL NOTES

RECOMMENDED JAN. 31, 2019 Thomas P. Macioca

RECOMMENDED JAN. 31, 2019 Allin Holice ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

SHEET 1 OF 12





SECTION C-C

(WITHOUT BASE PLATES)

(CONCRETE CAISSON SHOWN

SPREAD FOOTING SIMILAR)

NOTE A:
NEOPRENE BEARING PAD 1/2 " MIN.

4" MIN. WIDTH, 50 DUROMETER.

ADHESIVE (TYP.)

THICKNESS BY 5" MIN. LENGTH BY

ATTACH TO CONCRETE SEAT WITH APPROVED

- GRADE GROUND TO DRAIN AWAY FROM WALL. FILL DEPTH ON EACH SIDE OF WALL TO BE WITHIN 1'-0" DIFFERENCE.
- * AS SPECIFIED ON THE
- H = HEIGHT FROM TOP OF BASE PLATE TO TOP OF WALL
- POST WITHOUT BASE PLATE: H = HEIGHT FROM TOP OF FOOTING/ CAISSON TO TOP OF WALL 3. PANEL EMBEDMENT MAY NEED TO BE
- INCREASED TO ACCOMMODATE BASE PLATES AND ANCHOR BOLT PROJECTIONS.
- 4. FOR OPTIONAL SLOPED BOTTOM PANEL REFER TO BC-776M, SHEET 3.
- 5. FOR SECTION D-D, REFER TO SHEET 3.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

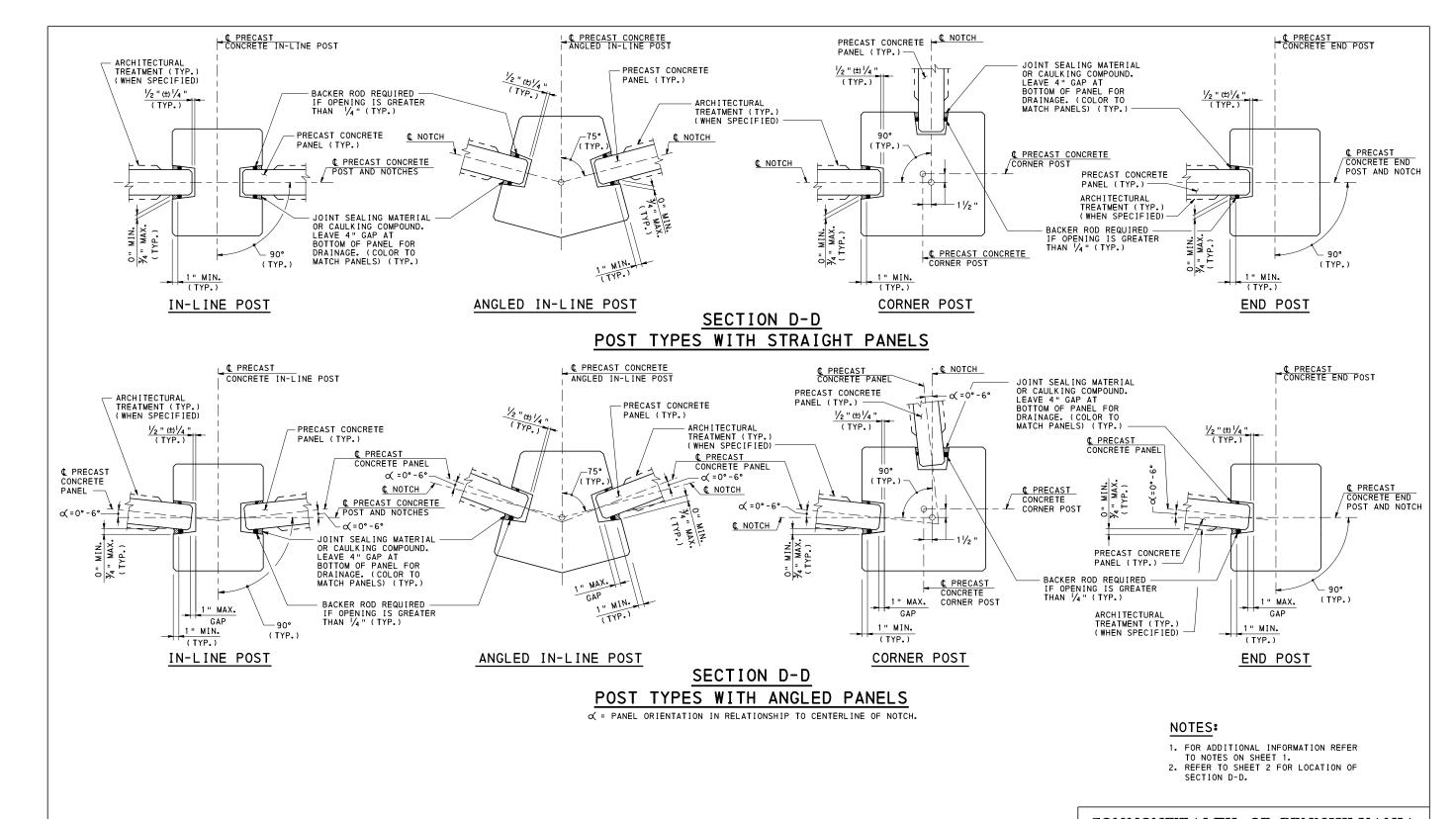
STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

GEOMETRY AND LAYOUT

RECOMMENDED JAN. 31, 2019 Thomas P. Marcioca CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019 Alwin Hodie ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

SHEET 2 OF 12



COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

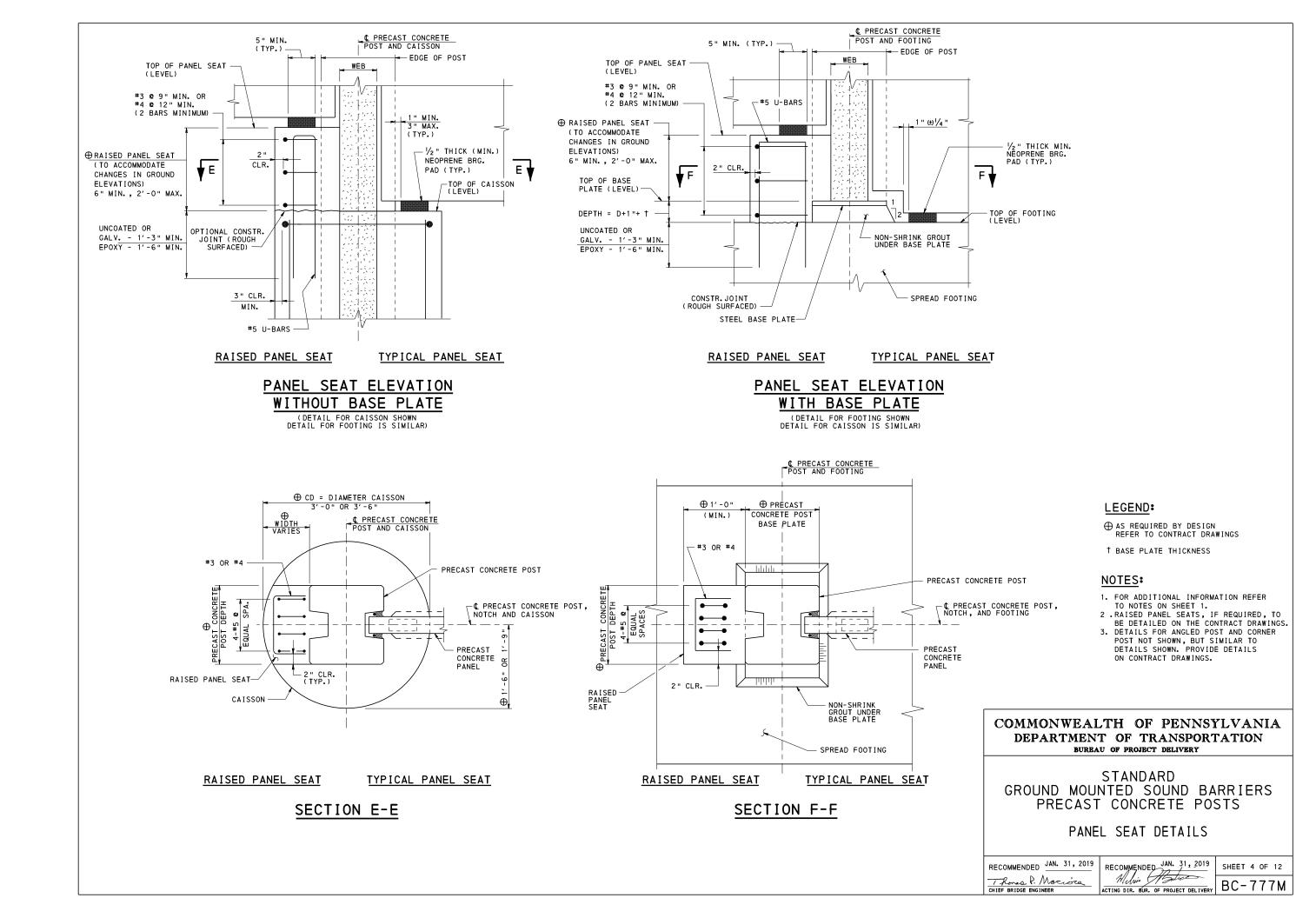
STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

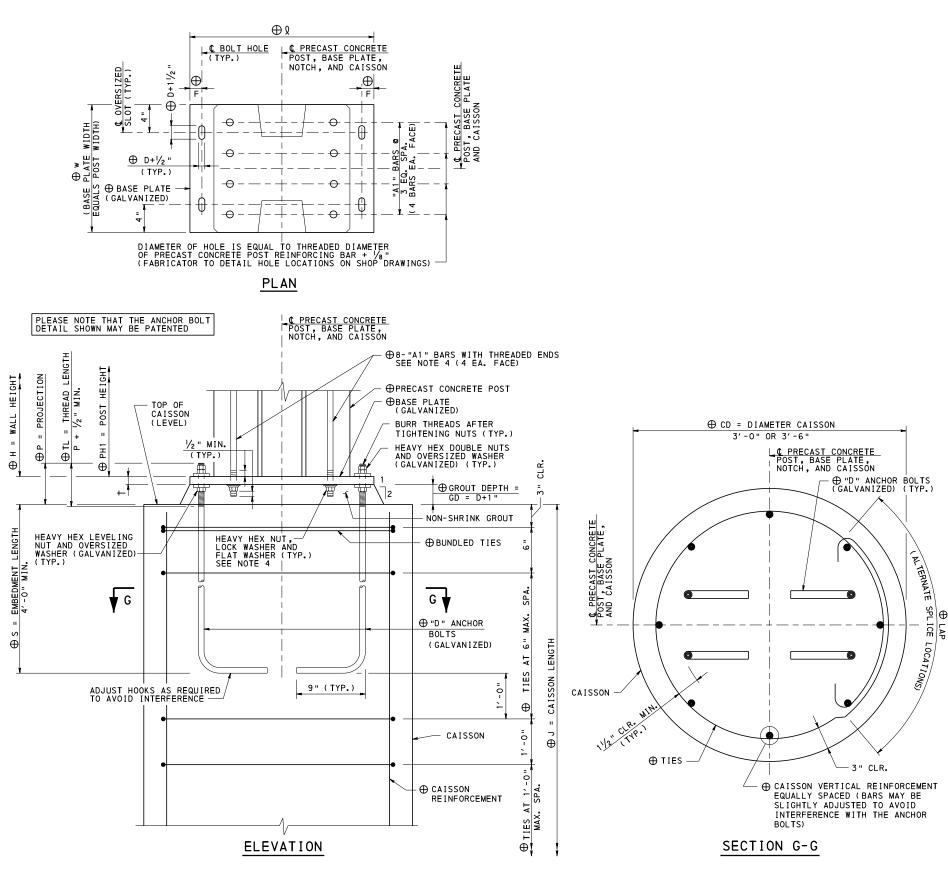
POST DETAILS

RECOMMENDED JAN. 31, 2019 Thomas P. Mariore CHIEF BRIDGE ENGINEER

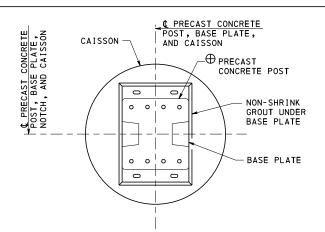
RECOMMENDED JAN. 31, 2019 Alwin Holico ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

SHEET 3 OF 12

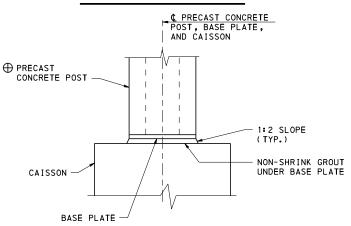




DETAIL 1 PRECAST CONCRETE POST WITH BASE PLATE CONNECTION TO CAISSON



PANEL SEAT PLAN



PANEL SEAT ELEVATION

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON
- 2. FOR PANEL SEAT DETAILS REFER TO SHEET 4.
- 3. FOR OVERSIZED WASHER DETAIL REFER TO SHEET 6.
- 4. POST "A1" BARS AND HARDWARE OPTIONS:
- A. GALVANIZED BARS IF THE THREADED BAR IS HOT DIP GALVANIZED, INCLUDING THE THREADED PORTION, USE NUTS AND WASHERS THAT ARE HOT-DIP GALVANIZED. IF THREADING IS PERFORMED AFTER GALVANIZING, COAT THE THREADED AREA WITH A COLD GALVANIZING REPAIR COMPOUND PER ASTM A780 AND USE EITHER MECHANICALLY GALVANIZED OR HOT DIP GALVANIZED WASHERS AND MECHANICALLY (ONLY) GALVANIZED NUTS. (WASHER - ASTM F436; NUT-ASTM A563).

 B. EPOXY COATED BARS - COAT THREADS WITH COLD GALVANIZING
- REPAIR COMPOUND PER ASTM A780. USE EITHER MECHANICALLY GALVANIZED OR HOT DIP GALVANIZED WASHERS AND MECHANICALLY (ONLY) GALVANIZED NUTS. (WASHER - ASTM F436; NUT - ASTM A563)

LEGEND:

AS REQUIRED BY DESIGN REFER TO CONTRACT DRAWINGS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

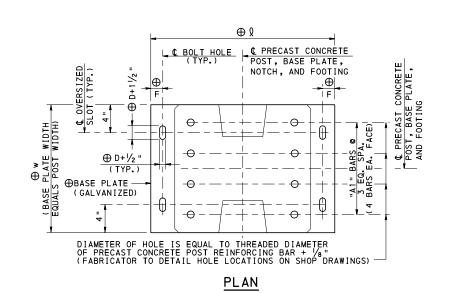
DETAIL 1

RECOMMENDED JAN. 31, 2019 Thomas P. Marcioca CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019

Alline Halice ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

SHEET 5 OF 12



PLEASE NOTE THAT THE ANCHOR BOLT DETAIL SHOWN MAY BE PATENTED

1/2 " MIN. (TYP.)

⊕ "F1" BARS

воттом оғ

FOOTING

THREAD LENGTH

() + d

#4 BARS (TYP.)

HEAVY HEX LEVELING NUT AND OVERSIZED WASHER (GALVANIZED) (TYP.)

⊕ P = PROJECTION

I Ф © PRECAST CONCRETE POST,
BASE PLATE, NOTCH, AND FOOTING

+ 8-"A1" BARS WITH THREADED ENDS SEE NOTE 4 ON SHEET 5. (4 EA. FACE)

⊕ PRECAST CONCRETE POST

BURR THREADS AFTER

TIGHTENING NUTS (TYP.)

-HEAVY HEX DOUBLE NUTS AND OVERSIZED WASHER (GALVANIZED) (TYP.)

TOP OF FOOTING (LEVEL)

GROUT DEPTH

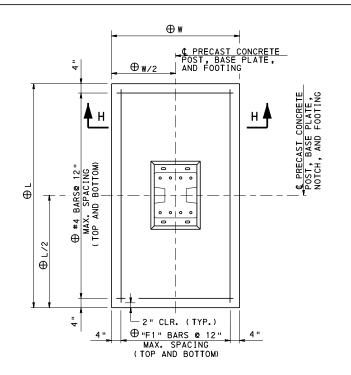
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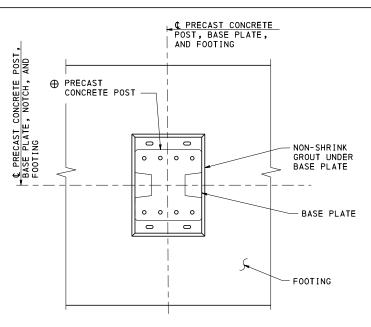
-⊕ BASE PLATE (GALVANIZED)

- NON-SHRINK GROUT

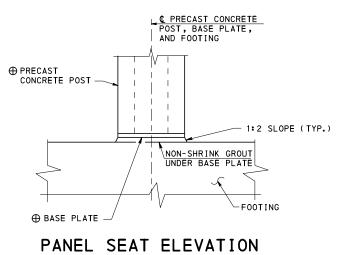
- ⊕ "D" ANCHOR BOLTS

(GALVANIZED)





PANEL SEAT PLAN



NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEET 1.
 2. FOR PANEL SEAT DETAILS REFER TO SHEET 4.

LEGEND:

⊕ AS REQUIRED BY DESIGN REFER TO CONTRACT DRAWINGS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

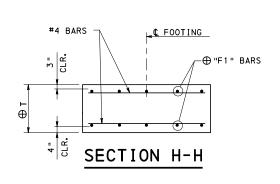
DETAIL 2

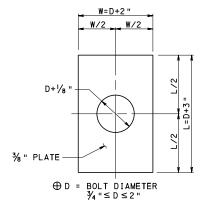
RECOMMENDED JAN. 31, 2019

Allin Haline RECOMMENDED JAN. 31, 2019 Thomas P. Macioca CHIEF BRIDGE ENGINEER

SHEET 6 OF 12 ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

SPREAD FOOTING PLAN





PRECAST CONCRETE POST WITH BASE PLATE CONNECTION TO SPREAD FOOTING

DETAIL 2

HEAVY HEX NUT, — LOCK WASHER AND FLAT WASHER (TYP.) SEE NOTE 4 ON SHEET 5

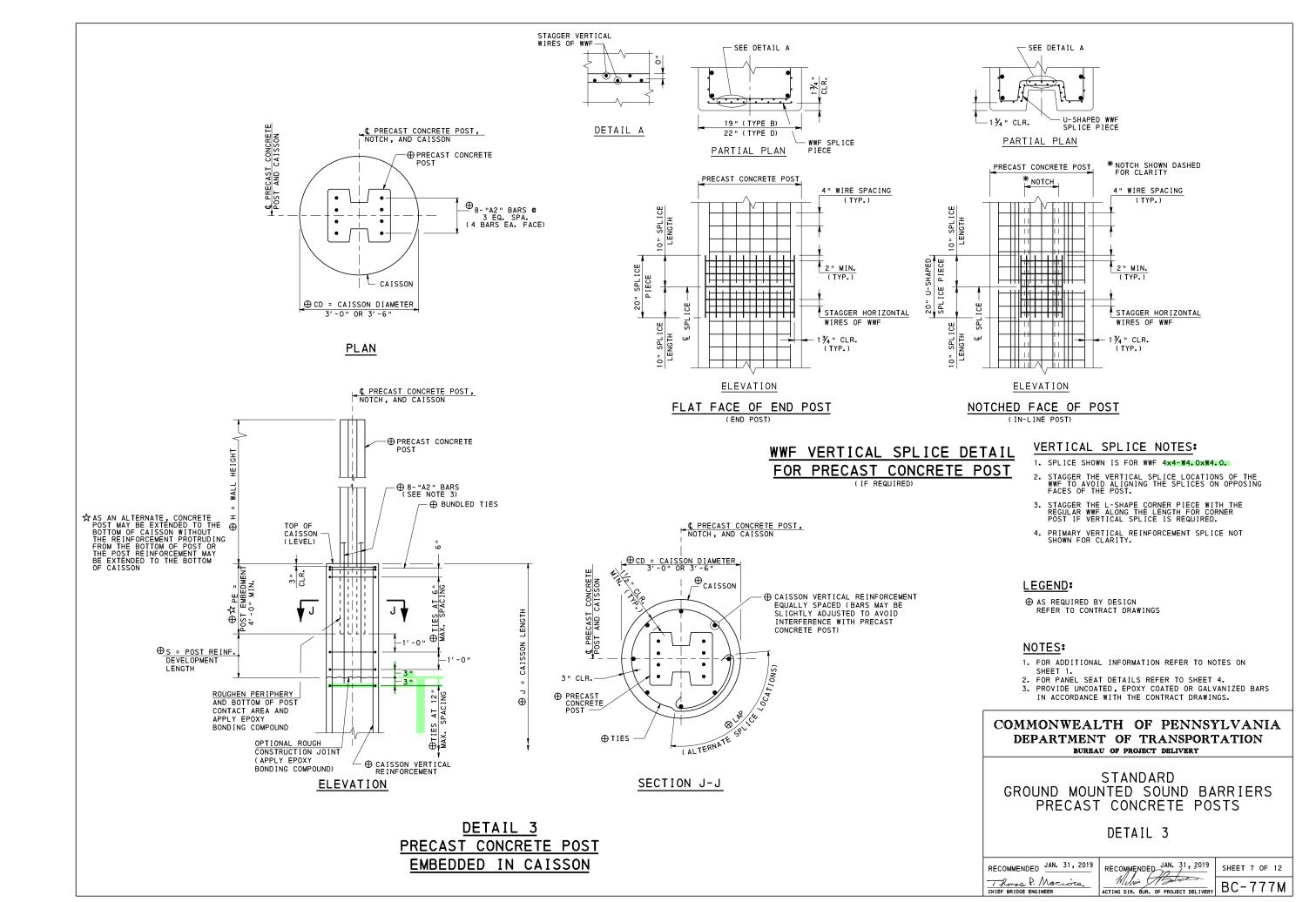
ADJUST TOP

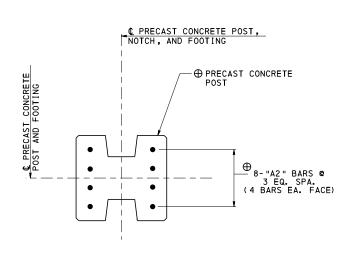
REINFORCEMENT

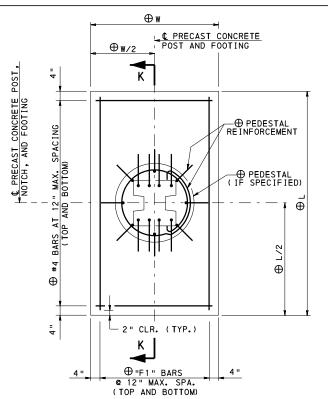
BARS TO CLEAR ANCHOR BOLTS —

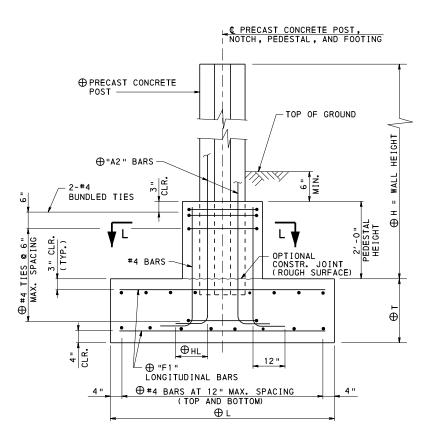
ELEVATION

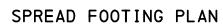
OVERSIZED WASHER DETAIL



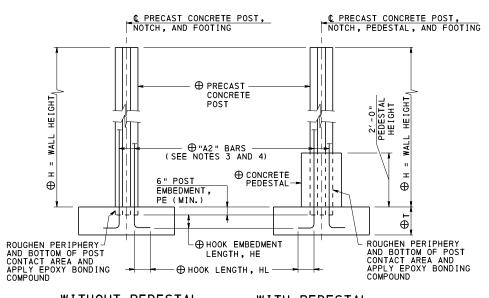




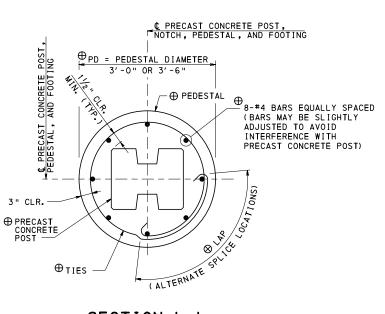




SECTION K-K (WITH PEDESTAL) ADJUST FOOTING TOP REINFORCING SPACING TO CLEAR POST.



PLAN



SECTION L-L PEDESTAL (IF SPECIFIED)

 \oplus_{TIES}

WITHOUT PEDESTAL WITH PEDESTAL

ELEVATION

DETAIL 4 PRECAST CONCRETE POST EMBEDDED IN SPREAD FOOTING (WITH OR WITHOUT PEDESTAL)

NOTES:

LEGEND:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON
- SHEET 1.
 2. FOR PANEL SEAT DETAILS REFER TO SHEET 4.

⊕ AS REQUIRED BY DESIGN
REFER TO CONTRACT DRAWINGS

- 3. PROVIDE UNCOATED OR EPOXY COATED BARS IN ACCORDANCE WITH THE CONTRACT DRAWINGS. GALVANIZED BARS NOT PERMITTED.
- 4. BARS MAY BE BENT AFTER FABRICATION OF POST. TOUCH-UP EPOXY COATED BARS WITH AN APPROVED EPOXY PAINT.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

DETAIL 4

RECOMMENDED JAN. 31, 2019 Thomas P. Macioca CHIEF BRIDGE ENGINEER

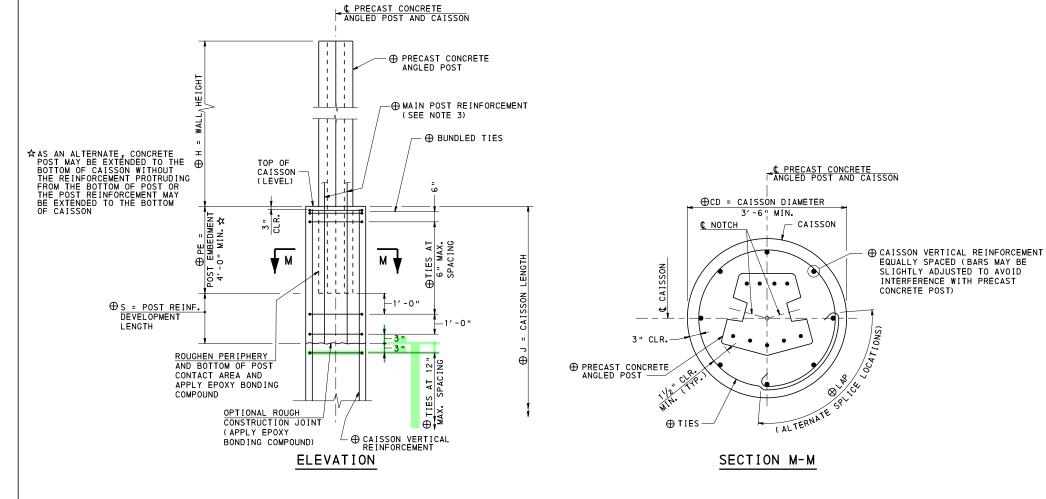
RECOMMENDED JAN. 31, 2019

Allow Assured ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

SHEET 8 OF 12

PRECAST CONCRETE ANGLED POST AND CAISSON ⊕ 4 BARS @ 3 EQ. SPA. PRECAST CONCRETE ANGLED POST ♠ NOTCH -⊕5 BARS **©** 4 EQ. SPA. ⊕ CD = CAISSON DIMENSION 3'-6" MIN.

PLAN



DETAIL 5 PRECAST CONCRETE ANGLED POST - TYPE E EMBEDDED IN CAISSON

LEGEND:

→ AS REQUIRED BY DESIGN
REFER TO CONTRACT DRAWINGS

NOTES:

- FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEET 1.
 FOR PANEL SEAT DETAILS REFER TO SHEET 4.
 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED BARS IN ACCORDANCE WITH THE CONTRACT DRAWINGS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

DETAIL 5

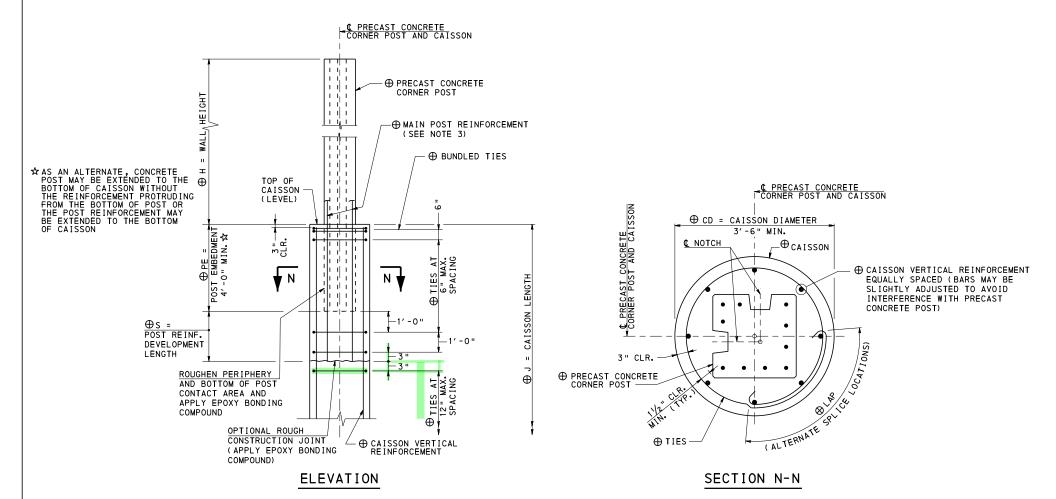
RECOMMENDED JAN. 31, 2019 Thoras P. Mariora CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019

Millian Assistance ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

SHEET 9 OF 12

CORNER POST AND CAISSON © PRECAST CONCRETE CORNER POST AND CAISSON ♠ NOTCH ⊕ PRECAST CONCRETE -⊕10 BARS (SPACED AS SHOWN) - 1 1/2 " ⊕ CD = CAISSON DIMENSION 3'-6" MIN. PLAN



DETAIL 6 PRECAST CONCRETE CORNER POST EMBEDDED IN CAISSON

LEGEND:

⊕ AS REQUIRED BY DESIGN
REFER TO CONTRACT DRAWINGS

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEET 1.
 2. FOR PANEL SEAT DETAILS REFER TO SHEET 4.
- 3. PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED BARS IN ACCORDANCE WITH THE CONTRACT DRAWINGS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

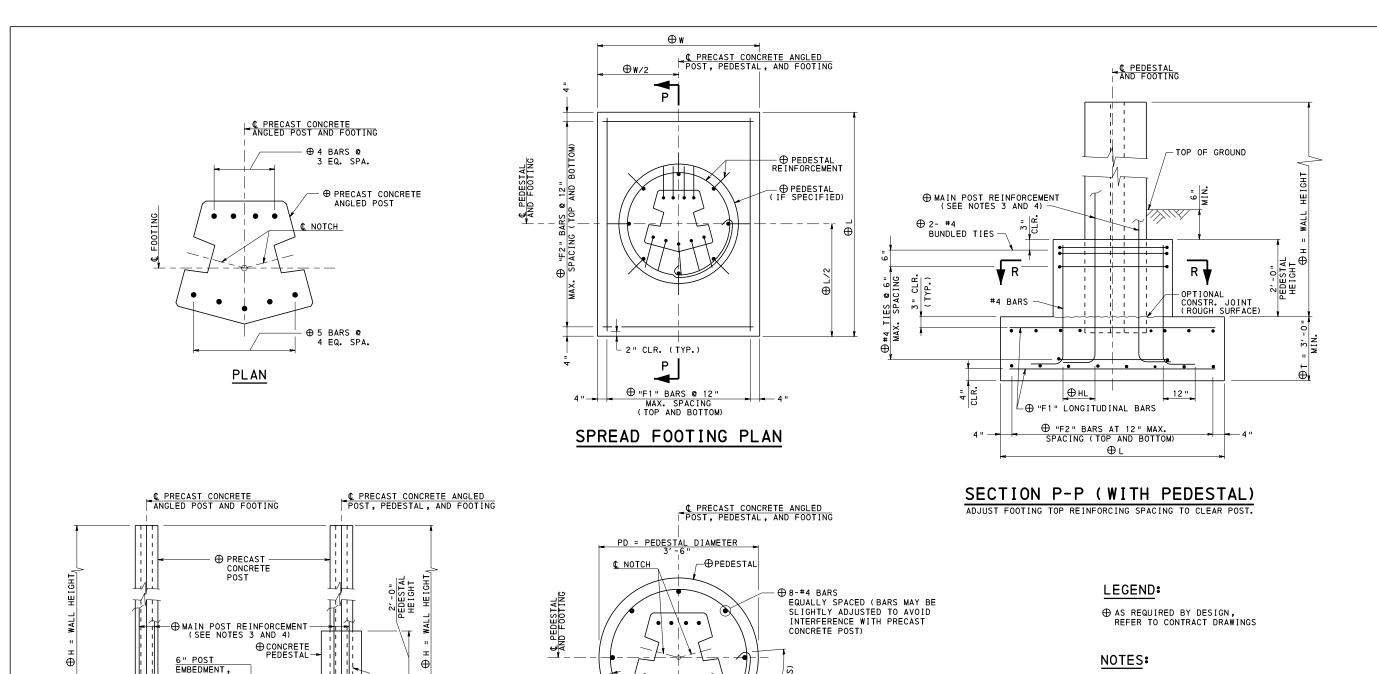
DETAIL 6

RECOMMENDED JAN. 31, 2019 Thomas P. Marcirea CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019

Millian Hadra ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

SHEET 10 OF 12



(ALTERNATE

SECTION R-R PEDESTAL (IF SPECIFIED)

3" CLR.

⊕ TIES-

⊕ T = 3′-0"

- ROUGHEN PERIPHERY
AND BOTTOM OF POST
CONTACT AREA AND
APPLY EPOXY BONDING
COMPOUND

- SHEET 1.
 2. FOR PANEL SEAT DETAILS REFER TO SHEET 4.

1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON

- 3. PROVIDE UNCOATED OR EPOXY COATED BARS IN ACCORDANCE WITH THE CONTRACT DRAWINGS. GALVANIZED BARS NOT PERMITTED.
- 4. BARS MAY BE BENT AFTER FABRICATION OF POST. TOUCH-UP EPOXY COATED BARS WITH AN APPROVED EPOXY PAINT.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

DETAIL 7

RECOMMENDED JAN. 31, 2019 Thoras P. Marcica CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019

Allwin Hadran SHEET 11 OF 12 ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

WITHOUT PEDESTAL WITH PEDESTAL

ELEVATION

-⊕ HOOK EMBEDMENT

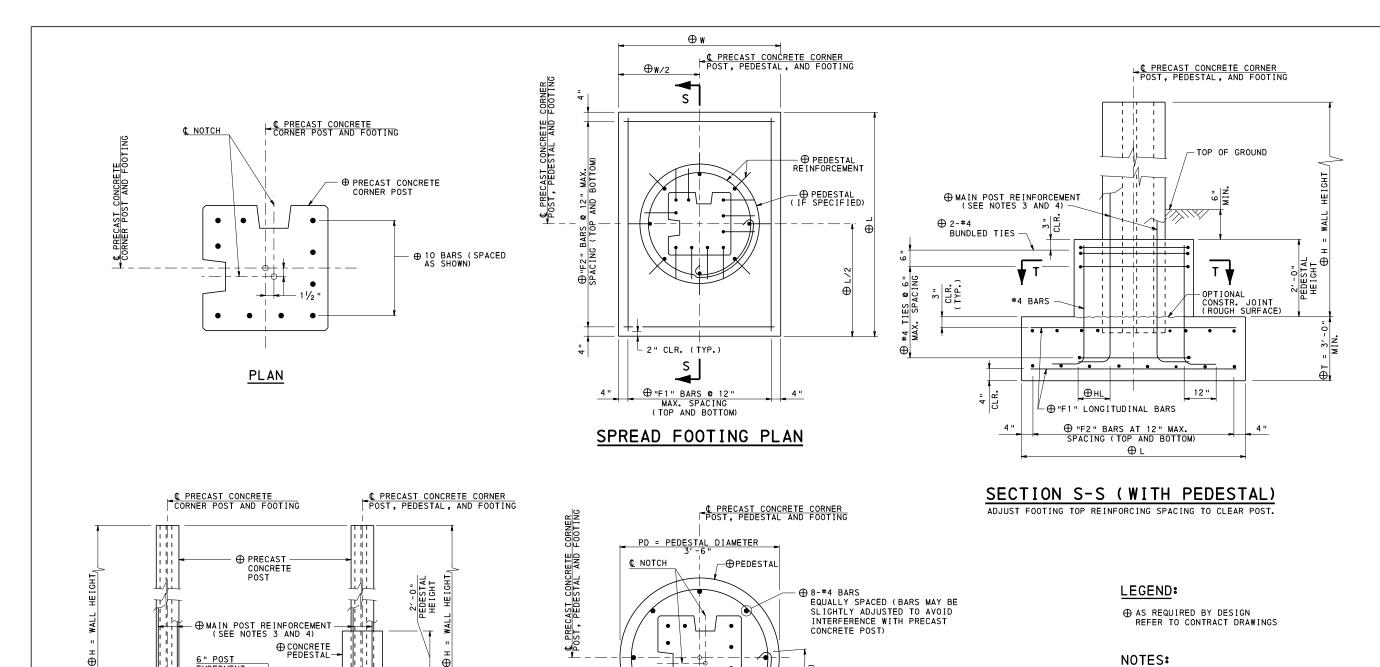
LENGTH, HE

⊕ HOOK LENGTH, HL

PE (MIN.)

ROUGHEN PERIPHERY
AND BOTTOM OF POST
CONTACT AREA AND
APPLY EPOXY BONDING
COMPOUND

DETAIL 7 PRECAST CONCRETE ANGLED POST - TYPE E EMBEDDED IN SPREAD FOOTING (WITH OR WITHOUT PEDESTAL)



3" CLR.

⊕ TIES —

SECTION T-T

PEDESTAL (IF SPECIFIED)

⊕ PRECAST CONCRETE CORNER POST

⊕T = 3′-0"

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEET 1.
 2. FOR PANEL SEAT DETAILS REFER TO SHEET 4.
- 3. PROVIDE UNCOATED OR EPOXY COATED BARS IN ACCORDANCE WITH THE CONTRACT DRAWINGS. GALVANIZED BARS NOT PERMITTED.
- 4. BARS MAY BE BENT AFTER FABRICATION OF POST. TOUCH-UP EPOXY COATED BARS WITH AN APPROVED EPOXY PAINT.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

DETAIL 8

RECOMMENDED JAN. 31, 2019 Thomas P. Macioca CHIEF BRIDGE ENGINEER

RECOMMENDED JAN. 31, 2019 SHEET 12 OF 12 ACTING DIR. BUR. OF PROJECT DELIVERY BC-777M

DETAIL 8 PRECAST CONCRETE CORNER POST - TYPE F EMBEDDED IN SPREAD FOOTING (WITH OR WITHOUT PEDESTAL)

-⊕HOOK EMBEDMENT

WITH PEDESTAL

LENGTH, HE

ELEVATION

⊕HOOK LENGTH, HL

EMBEDMENT, PE MIN.

ROUGHEN PERIPHERY
AND BOTTOM OF POST
CONTACT AREA AND
APPLY EPOXY BONDING
COMPOUND

WITHOUT PEDESTAL

GENERAL NOTES

- 1. DESIGN SPECIFICATIONS:

 PENNDOT DESIGN MANUAL PART 4, STRUCTURES APRIL 2015 EDITION.

 1989 AASHTO "GUIDE SPECIFICATIONS FOR STRUCTURAL DESIGN OF SOUND BARRIERS", INCLUDING THE 1992 AND 2002 INTERIMS.

 2002 AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES", 17TH EDITION.

 2001 AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY

 - 2001 AASHIO "SIANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY
 SIGNS, LUMINAIRES AND TRAFFIC SIGNALS", 4TH EDITION, INCLUDING THE INTERIMS THROUGH 2006.
 DESIGN IS IN ACCORDANCE WITH THE WORKING STRESS DESIGN METHOD. (NO INCREASE IN ALLOWABLE UNIT STRESSES ARE PERMITTED EXCEPT FOR GROUP III LOADINGS
 WHICH PERMITS A 33% OVERSTRESS.)
- 2. CONSTRUCTION SPECIFICATIONS AND WORKMANSHIP:

 PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH THE CURRENT VERSION OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, AASHTO/AWS/D1.5 BRIDGE WELDING CODE AND THE CONTRACT SPECIAL PROVISIONS. (USE AASHTO/AWS D1.1 FOR WELDING NOT COVERED IN AASHTO/AWS/D1.5.
- 3. WALL HEIGHTS MUST EQUAL OR EXCEED THE ACOUSTICAL PROFILE.
- 4. INSTALL ANCHOR BOLTS, POSTS, AND PANELS TRULY VERTICAL.
- PROVIDE CONCRETE COVER IN ACCORDANCE WITH THIS STANDARD AND DESIGN MANUAL, PART 4.
- 6. A HIGHER STRENGTH CONCRETE, FOR CAST-IN-PLACE CONCRETE, MAY BE SUBSTITUTED FOR A LOWER CLASS CONCRETE AT NO ADDITIONAL COST TO THE DEPARTMENT.
- SEAL ALL OPEN JOINTS WITH CAULKING COMPOUND AND/OR JOINT SEALING MATERIAL. (COLOR TO MATCH PANEL.)
- 8. REFER TO PUBLICATION 408, SECTION 1086.3(f) FOR FABRICATION AND ERECTIONS TOLERANCES.
- 9. REFER TO PUBLICATION 408, SECTION 1006.3(a) FOR CAISSON SHAFT TOLERANCES.
- 10. CHAMFER EXPOSED CONCRETE EDGES ON CAST-IN-PLACE CONCRETE 1" x 1", EXCEPT AS NOTED.
- 11. ALL FILLET WELDS SHOWN ARE MINIMUM SIZE UNLESS NOTED OTHERWISE.
- 12. RAKE-FINISH ALL HORIZONTAL CONSTRUCTION JOINTS, EXCEPT AS NOTED.
- 13. ALL DIMENSIONS SHOWN ARE HORIZONTAL, EXCEPT AS NOTED.
- 14. DIMENSIONS SHOWN ARE FOR A NORMAL TEMPERATURE OF 20 DEGREES C (68 DEGREES F).
- 15. REINFORCEMENT IN SOME SECTIONS IS NOT SHOWN FOR CLARITY.
- 16. SPREAD FOOTINGS:

 CONSTRUCT EMBANKMENTS AND/OR CUT EXISTING GRADE TO THE TOP OF FOOTING ELEVATIONS.

 EXCAVATE FOR FOOTING CONSTRUCTION.

 - EXCAVATE FOR FOUTING CONSTRUCTION.
 CONSTRUCT FOOTING.
 SPREAD FOOTINGS MAY BE ORDERED BY THE REPRESENTATIVE TO BE AT ANY ELEVATION OR ANY DIMENSIONS NECESSARY TO PROVIDE A PROPER FOUNDATION. IF SPREAD FOOTINGS ARE ADJUSTED PANEL HEIGHTS AND POST DESIGNS WILL NEED TO BE ADJUSTED.
 USE CLASS C CEMENT CONCRETE OR NO. 2A COARSE AGGREGATE BELOW SPREAD FOOTING WHEN SPECIFIED OR DIRECTED.
- 17. CAISSONS:

 CONSTRUCT EMBANKMENTS AND/OR CUT EXISTING GRADE TO THE TOP OF CAISSON ELEVATIONS PRIOR TO CONSTRUCTION OF CAISSONS.
 THE CONTRACTOR IS RESPONSIBLE FOR MAINTAINING THE DRILLED OPENING INTACT AND FOR MAINTAINING THE STABILITY OF THE GROUND CUT SLOPE OR FILLED EMBANKMENT DURING DRILLING AND INSTALLATION OF CAISSONS.
 TEMPORARY CASING MAY BE REQUIRED DURING CAISSON CONSTRUCTION IN ORDER TO MAINTAIN AN OPEN SHAFT. IF CASING IS USED, MAINTAIN CONCRETE LEVELS ABOVE THE BOTTOM OF CASING AT ALL TIMES DURING CASING EXTRACTION TO PREVENT CAVED MATERIAL FROM CONTAINTAING THE CONCRETE.
 IF GROUNDWATER FLOW ENTERS THE CAISSON EXCAVATION DURING CONSTRUCTION, PLACE CONCRETE BY TREMIE METHODS TO ABOVE THE GROUND WATER ELEVATION IN ONE CONTINUOUS OPERATION, FILL REMAINDER OF CAISSON WITH CLASS A CONCRETE. PLACE EPOXY BONDING COMPOUND BETWEEN POURS, AS REQUIRED.
- 18. COORDINATE, LOCATE, AND CONDUCT ALL WORK RELATED TO PUBLIC AND PRIVATE UTILITIES IN ACCORDANCE WITH PUBLICATION 408, SECTION 105.06 AND 107.12, AND THE CONTRACT SPECIAL PROVISIONS.
- 19. FOR ADDITIONAL INFORMATION REFER TO BC-776M
- 20. IF A NEEDED DETAIL IS NOT FOUND IN THE SOUND BARRIER STANDARDS OR ON THE CONTRACT DRAWINGS A SPECIAL SUBMISSION REQUESTING APPROVAL FOR SPECIFIC DETAILS MUST BE MADE TO THE CHIEF BRIDGE ENGINEER.

MATERIAL NOTES

- 1. CAST-IN-PLACE CONCRETE: PROVIDE CLASS A CEMENT CONCRETE IN THE CAST-IN-PLACE FOOTINGS, PEDESTALS, AND CAISSONS.
 - f'c = 3,000 PSI
 UNIT WEIGHT OF CONCRETE = 150 LB. / CU. FT.

- 2. REINFORCEMENT STEEL:

 PROVIDE GRADE 60 DEFORMED REINFORCING BARS THAT MEET THE REQUIREMENTS OF ASTM A 615, ASTM A 996, OR ASTM A 706. DO NOT WELD REINFORCING BARS UNLESS SPECIFIED. DO NOT USE RAIL STEEL ASTM A 996 REINFORCEMENT BARS IN FOOTINGS, CAISSONS, OR WHERE BENDING OR WELDING OF REINFORCEMENT BARS IS INDICATED.

 FOOTINGS, CAISSONS, OR WHERE BENDING OR WELDING OF REINFORCEMENT BARS IS INDICATED.

 PROVIDE UNCOATED REINFORCEMENT IN THE FOOTINGS AND CAISSONS.

 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED REINFORCEMENT IN THE RAISED PANEL SEATS AND PEDESTALS AS SPECIFIED ON THE CONTRACT DRAWINGS.

 PROVIDE MINIMUM LAP AND EMBEDMENT LENGTH FOR REINFORCING BARS OF 30 DIAMETERS OR IN ACCORDANCE WITH THE CURRENT AASHTO SPECIFICATIONS AS MODIFIED BY THE DESIGN MANUAL, PART 4, WHICHEVER IS GREATER.

 MECHANICAL CONNECTIONS, WHICH MEET THE REQUIREMENTS OF PUBLICATION 408, SECTION 1002, MAY BE USED UPON ACCEPTANCE FROM THE ENGINEER.
- 3. FABRICATED STRUCTURAL STEEL:

 PROVIDE STRUCTURAL STEEL CONFORMING TO AASHTO M 270, GRADE 36

 (ASTM A 709, GRADE 36) UNLESS OTHERWISE NOTED.

 PROVIDE STRUCTURAL STEEL TUBING CONFORMING TO ASTM A 53, GRADE B, TYPE E FOR THE STEEL ANGLED AND CORNER POSTS. (Fy = 35 KSI)

 WEATHERING STEEL (ASTM A 558) IS NOT PERMITTED.

 PROVIDE MINIMUM WELD SIZE OF 3/6".

 NON-DESTRUCTIVE TESTING IS REQUIRED FOR STEEL POST TO BASE PLATE WELDS. PROVIDE TESTING IN ACCORDANCE WITH AASHTO/AWS D1.5 FOR MAIN MEMBER.

 GALVANIZE AND PAINT STEEL POSTS, PLATES, AND HARDWARE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s) AND 1060.2(b).

 CLEAN AND PREPARE GALVANIZED SURFACES FOR PAINTING IN ACCORDANCE WITH PUBLICATION 408, SECTION 1060.3(b) 4.

 REPAIR DAMAGED GALVANIZING IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s) 2.
- 4. WELDED STUDS:

 PROVIDE 7/8" × 4" STUDS CONFORMING TO ASTM A 108 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(e).

- 5. ANCHOR BOLTS, NUTS, AND WASHERS:

 PROVIDE ANCHOR BOLTS CONFORMING TO ASTM F 1554, GRADE 36 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c)3.

 PROVIDE HEAVY HEX NUTS CONFORMING TO ASTM A 563A IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c)3d.

 PROVIDE OVERSIZE WASHERS CONFORMING TO AASHTO M 270, GRADE 36 (ASTM A 709, GRADE 36).

 PROVIDE LOCK WASHERS AND FLAT WASHERS CONFORMING TO ASTM F 436 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c)2b.

 GALVANIZE ALL ANCHOR BOLTS AND HARDWARE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s). SECTION 1105.02(s).
- 6. PLAIN NEOPRENE BEARING PADS:

 PROVIDE PLAIN NEOPRENE PADS WITH A DUROMETER HARDNESS OF 50 (+ / -) 5
 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1113.02.
- 7. EPOXY BONDING COMPOUND:

 PROVIDE EPOXY BONDING COMPOUND IN ACCORDANCE WITH PUBLICATION 408, SECTION 706.1.
- 8. NON-SHRINK GROUT:

 PROVIDE NON-SHRINK GROUT IN ACCORDANCE WITH PUBLICATION 408, SECTION 1080.2(c).

 PLACE NON-SHRINK GROUT AFTER THE BASE PLATE IS LEVELED ON THE LEVELING NUTS AND AFTER THE PANELS ARE INSTALLED.

 PACK GROUT INTO PLACE. DO NOT POUR OR INJECT GROUT.

 NON-SHRINK GROUT TO MATCH FINAL COLOR OF PANEL.
- 9. CAULKING COMPOUND:

 PROVIDE CAULKING COMPOUND IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.8(b).

 CAULKING COMPOUND TO MATCH FINAL COLOR OF PANEL.

- 10. JOINT SEALING MATERIAL:

 PROVIDE JOINT SEALING MATERIAL IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.4(a).

 JOINT SEALING MATERIAL TO MATCH FINAL COLOR OF PANEL.
- 11. JOINT BACKING MATERIAL (BACKER ROD):

 PROVIDE BACKER ROD MATERIAL IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.9.
- 12. ANTIGRAFFITI COATING:

 APPLY ANTIGRAFFITI COATING IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.
- 13. PENETRATING CONCRETE STAIN:

 APPLY STAIN IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.
- 14. CLOSED CELL NEOPRENE SPONGE:

 PROVIDE CLOSED CELL NEOPRENE SPONGE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1085.2(m).

BC-734M ANCHOR SYSTEMS

BC-736M

BC-776M

BC-777M

BC-779M

RC-11M

WALL CONSTRUCTION AND EXPANSION JOINT DETAILS

REINFORCEMENT BAR FABRICATION DETAILS

STRUCTURE MOUNTED SOUND BARRIER WALLS

REFERENCE DRAWINGS

CLASSIFICATION OF EARTHWORK FOR STRUCTURES

GROUND MOUNTED SOUND BARRIERS

GROUND MOUNTED SOUND BARRIERS

PRECAST CONCRETE PANELS

PRECAST CONCRETE POSTS

	INDEX OF SHEETS
SHT. NO.	SHEET TITLE
1	GENERAL NOTES
2	GEOMETRY AND LAYOUT
3	POST DETAILS
4	PANEL SEAT DETAILS
5	DETAIL 1
6	DETAIL 2
7	DETAIL 3
8	DETAIL 4
9	DETAIL 5
10	DETAIL 6

	DESCRIPTION OF DETAILS
DETAIL	DESCRIPTION
1	STEEL POST WITH BASE PLATE CONNECTION TO CAISSON
2	STEEL POST WITH BASE PLATE CONNECTION TO SPREAD FOOTING
3	STEEL POST EMBEDDED IN CAISSON
4	STEEL POST EMBEDDED IN SPREAD FOOTING WITH PEDESTAL
5	CORNER/ANGLED STEEL PIPE POST EMBEDDED IN CAISSON
6	CORNER/ANGLED STEEL PIPE POST EMBEDDED IN SPREAD FOOTING WITH PEDESTAL

NOTES TO FABRICATOR

1. REFER TO BC-776M FOR NOTES TO FABRICATOR.

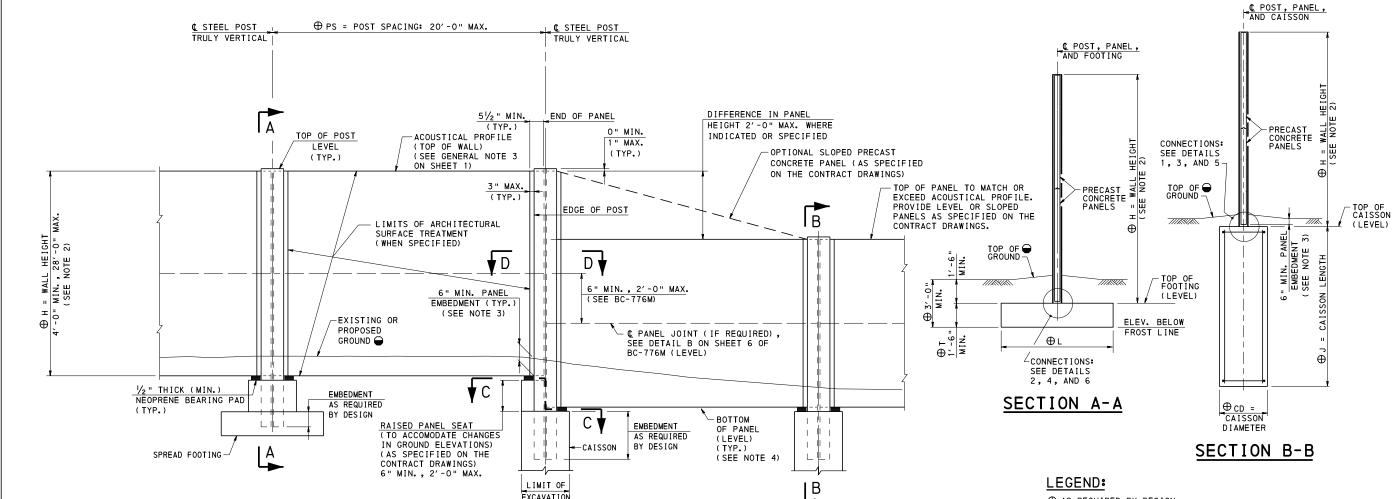
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS STEEL POSTS

GENERAL NOTES

RECOMMENDED SEPT. 30, 2016 RECOMMENDED SEPT.30, 2016 Thomas P Macioca CHIEF BRIDGE ENGINEER

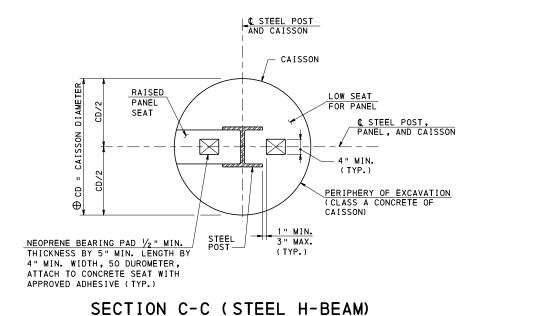
SHEET 1 OF 10 Bund SThomps DIRECTOR, BUR. OF PROJECT DELIVERY BC-778M



LEVEL OR SLOPED TOP PANEL

GROUND MOUNTED SOUND BARRIER ELEVATION

LEVEL PANEL



(CONCRETE CAISSON SHOWN (WITHOUT BASE PLATES),

PEDESTAL AND SPREAD FOOTING SIMILAR)

AND CAISSON - CAISSON OW SEAT DIAMETER RAISED PANEL - C PANEL SEAT & STEEL PIPE AND CAISSON 00 PERIPHERY OF EXCAVATION (CLASS A CONCRETE OF 4" MIN. Φ. (TYP.) CAISSON) 1 " MIN. NEOPRENE BEARING PAD 1/2 MIN. THICKNESS BY 5" MIN. LENGTH BY 3" MAX. (TYP.) STEEL PIPE POST 4" MIN. WIDTH, 50 DUROMETER, ATTACH TO CONCRETE SEAT WITH APPROVED ADHESIVE (TYP.)

SECTION C-C (STEEL PIPE POST)

(CONCRETE CAISSON SHOWN,

- → AS REQUIRED BY DESIGN
 REFER TO CONTRACT DRAWINGS
- GRADE GROUND TO DRAIN AWAY FROM WALL. FILL DEPTH ON EACH SIDE OF WALL TO BE WITHIN 1'-0" DIFFERENCE.

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEET 1.
 2. WALL HEIGHT IS DEFINED AS FOLLOWS:
- POST WITH BASE PLATE: H = HEIGHT FROM TOP OF BASE PLATE TO TOP OF WALL.
- POST WITHOUT BASE PLATE: H = HEIGHT FROM TOP OF FOOTING/CAISSON TO TOP OF WALL.
 3. PANEL EMBEDMENT MAY NEED TO BE INCREASED TO ACCOMODATE
- BASE PLATES AND ANCHOR BOLT PROJECTIONS.
- 4. FOR OPTIONAL SLOPED BOTTOM PANEL REFER TO BC-776M, SHEET 3.
- 5. FOR SECTION D-D, REFER TO SHEET 3.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS STEEL POSTS

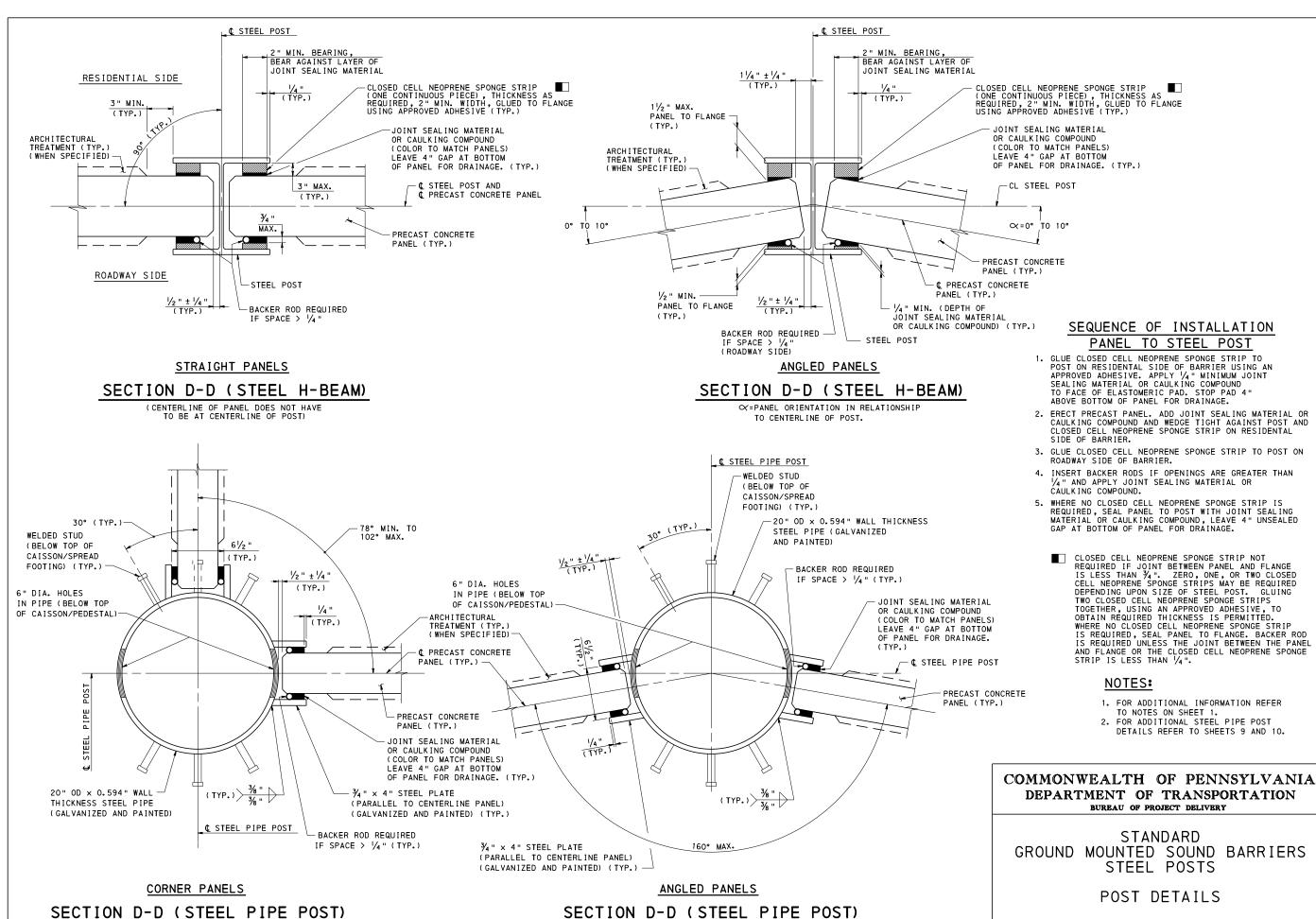
GEOMETRY AND LAYOUT

RECOMMENDED SEPT. 30, 2016

RECOMMENDED SEPT. 30, 2016 Bun & Thomas

SHEET 2 OF 10 BC-778M

Thomas P Macioca CHIEF BRIDGE ENGINEER RECTOR, BUR. OF PROJECT DELIVERY



RECOMMENDED SEPT.30, 2016

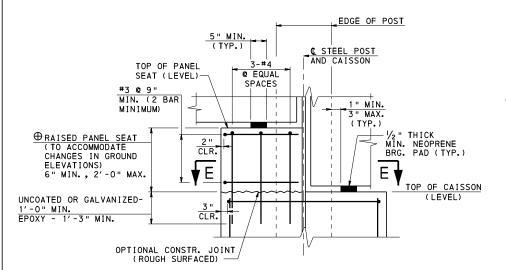
Thoma P Macioca

RECOMMENDED SEPT. 30, 2016 SHEET 3 OF 10

Bun SThomps

DIRECTOR, BUR. OF PROJECT DELIVERY BC-778M

GLUING



RAISED PANEL SEAT

⊕ 1′ -6 "

2" CLR. (TYP.)

CAISSON

RAISED PANEL SEAT

⊕STEEL POST

WEB DEPTH

3-#4 @ EQUAL SPACES

#3 TIES

TYPICAL PANEL SEAT

RESIDENTIAL SIDE

-STEEL POST

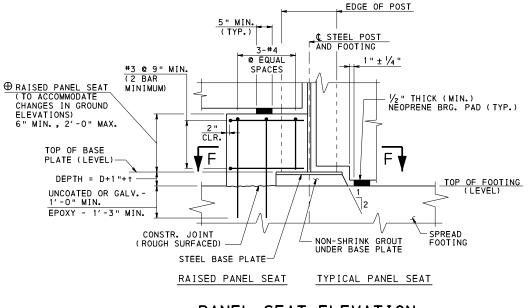
PRECAST

CONCRETE

PANEL SEAT ELEVATION WITHOUT BASE PLATE

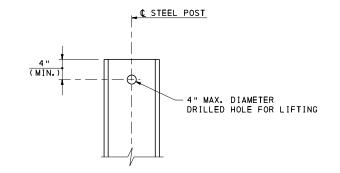
(DETAIL FOR CAISSON SHOWN DETAIL FOR FOOTING IS SIMILAR)

⊕CD = CAISSON DIAMETER 3'-0" MIN.



PANEL SEAT ELEVATION WITH BASE PLATE

(DETAIL FOR FOOTING SHOWN DETAIL FOR CAISSON IS SIMILAR)



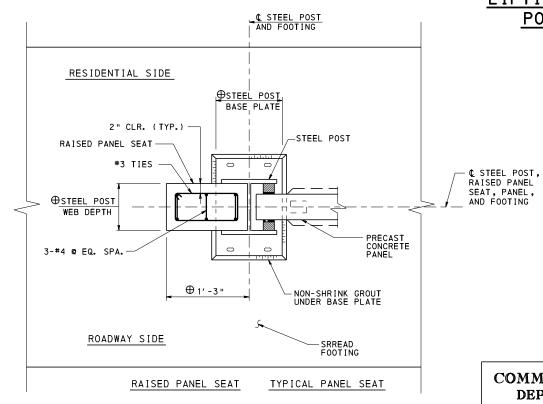
LIFTING HOLE IN STEEL POST (OPTIONAL)

C STEEL POST, RAISED PANEL SEAT, PANEL, AND CAISSON

ROADWAY SIDE

RAISED PANEL SEAT TYPICAL PANEL SEAT

SECTION E-E



LEGEND:

⊕ AS REQUIRED BY DESIGN
REFER TO CONTRACT DRAWINGS

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO
- NOTES ON SHEET 1.

 2. RAISED PANEL SEATS (IF REQUIRED) TO BE DETAILED ON THE CONTRACT DRAWINGS.
- 3. DETAILS FOR STEEL PIPE POST NOT SHOWN BUT SIMILAR TO DETAILS SHOWN. REFER TO CONTRACT DRAWINGS FOR DETAILS.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS STEEL POSTS

PANEL SEAT DETAILS

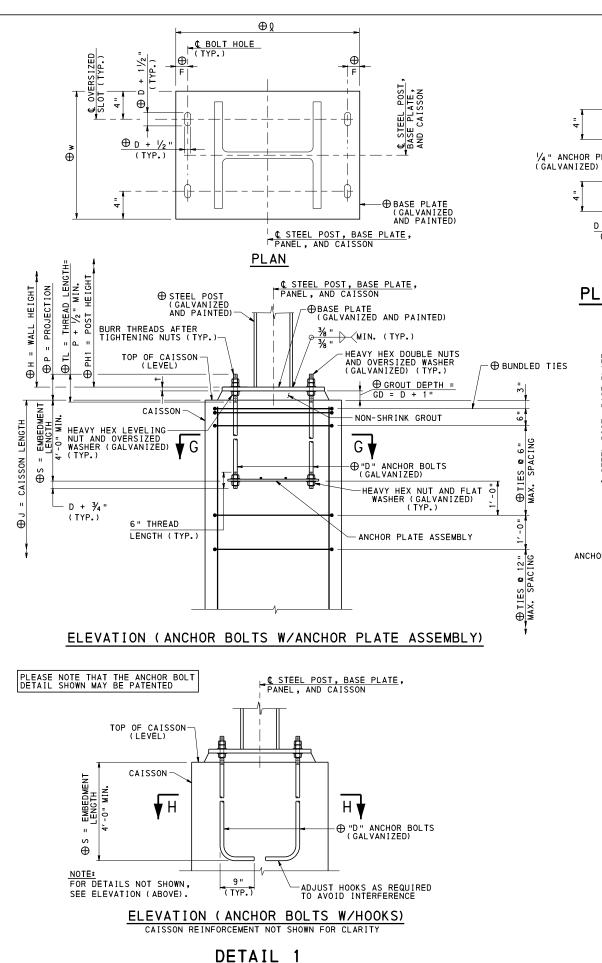
RECOMMENDED SEPT. 30, 2016 Thomas P Macioca

RECOMMENDED SEPT. 30, 2016

Bun SThomps DIRECTOR, BUR. OF PROJECT DELIVERY BC-778M

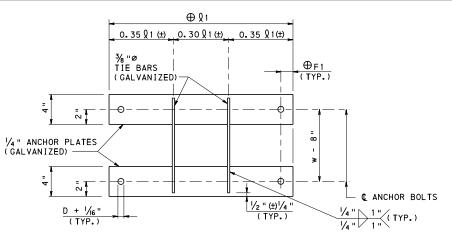
SHEET 4 OF 10

SECTION F-F

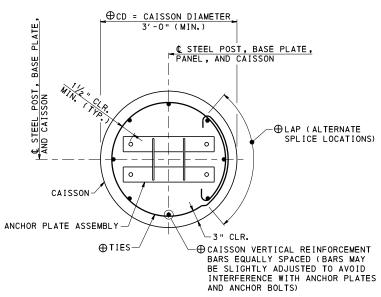


STEEL POST WITH BASE PLATE

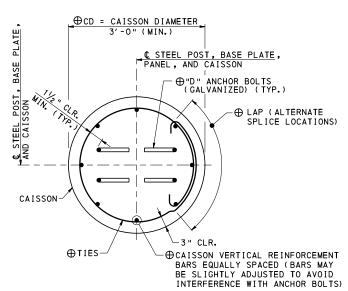
CONNECTION TO CAISSON



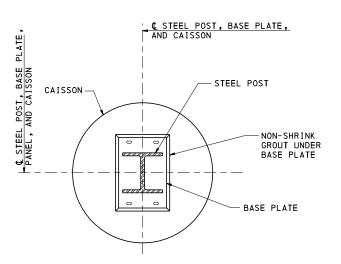
PLAN - ANCHOR PLATE ASSEMBLY



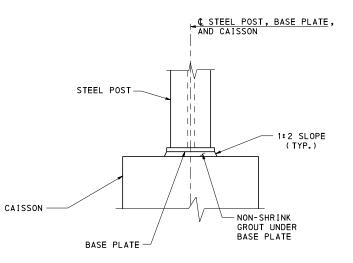
SECTION G-G



SECTION H-H



PANEL SEAT PLAN



PANEL SEAT ELEVATION

LEGEND:

⊕ AS REQUIRED BY DESIGN
REFER TO CONTRACT DRAWINGS

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER
- TO NOTES ON SHEET 1.
 2. FOR PANEL SEAT DETAILS REFER TO SHEET 4.
- 3. FOR OVERSIZED WASHER DETAIL REFER TO SHEET 6.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS STEEL POSTS

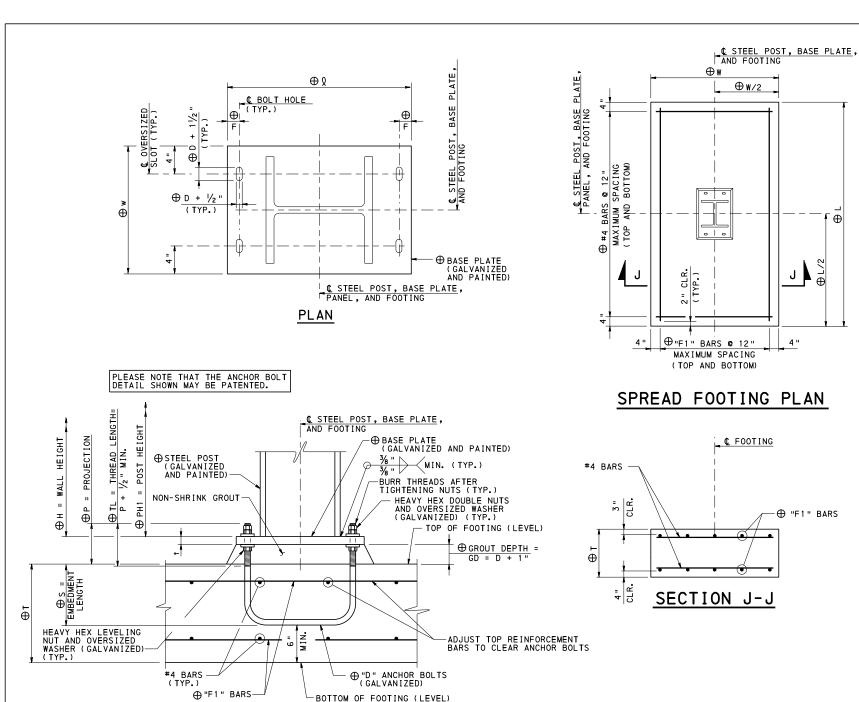
DETAIL 1

RECOMMENDED SEPT. 30, 2016 Thomas P Macioca CHIEF BRIDGE ENGINEER

RECOMMENDED SEPT. 30, 2016 Bun SThomps

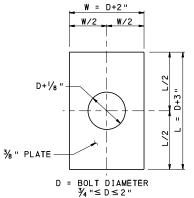
SHEET 5 OF 10

BC-778M IRECTOR, BUR. OF PROJECT DELIVERY

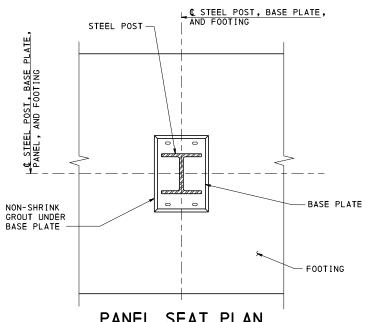


ELEVATION

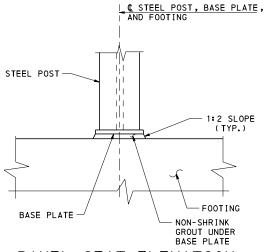
DETAIL 2 STEEL POST WITH BASE PLATE CONNECTION TO SPREAD FOOTING



OVERSIZED WASHER DETAIL



PANEL SEAT PLAN



PANEL SEAT ELEVATION

LEGEND:

⊕ AS REQUIRED BY DESIGN
REFER TO CONTRACT DRAWINGS

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER
- TO NOTES ON SHEET 1.
 2. FOR PANEL SEAT DETAILS REFER TO SHEET 4.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS
STEEL POSTS

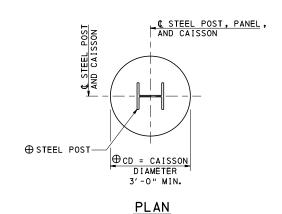
DETAIL 2

RECOMMENDED SEPT. 30, 2016 Thomas P Macioca
CHIEF BRIDGE ENGINEER

RECOMMENDED SEPT. 30, 2016 Bun & Thomps

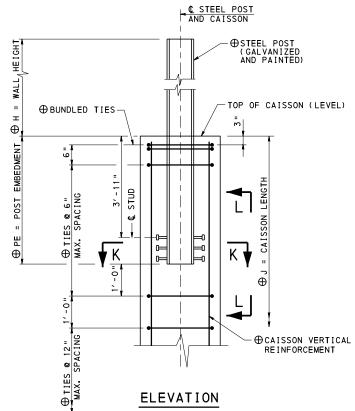
DIRECTOR, BUR. OF PROJECT DELIVERY BC-778M

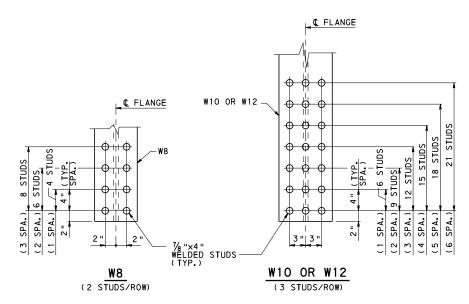
SHEET 6 OF 10



⊕CAISSON DIAMETER 3'-0" MIN. C STEEL POST, PANEL, & STEEL POST AND CAISSON ⊕ LAP (ALTERNATE
 SPLICE LOCATIONS) CAISSON -⊕ STEEL POST - + CAISSON VERTICAL REINFORCEMENT BARS EQUALLY SPACED (BARS MAY BE SLIGHTLY ADJUSTED TO AVOID INTERFERENCE WITH WELDED STUDS) ⊕ TIES

SECTION K-K ⊕STEEL POST (GALVANIZED AND PAINTED)





SECTION L-L WELDED STUDS (REQUIRED ON BOTH FLANGES)

DETAIL 3 STEEL POST EMBEDDED IN CAISSON

LEGEND:

⊕ AS REQUIRED BY DESIGN
REFER TO CONTRACT DRAWINGS

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEET 1.
- 2. FOR PANEL SEAT DETAILS REFER TO SHEET 4.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

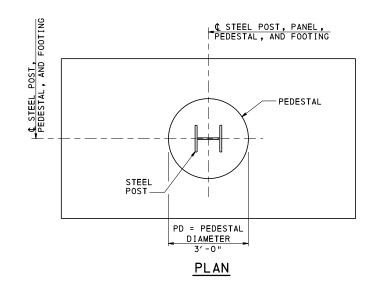
GROUND MOUNTED SOUND BARRIERS STEEL POSTS

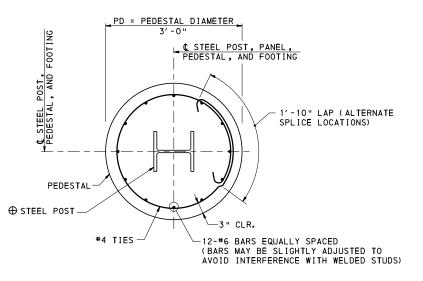
DETAIL 3

RECOMMENDED SEPT. 30, 2016 Thomas P Macioca
CHIEF BRIDGE ENGINEER

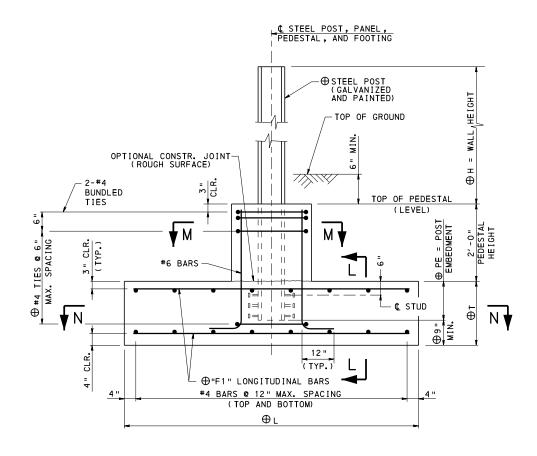
RECOMMENDED SEPT. 30, 2016

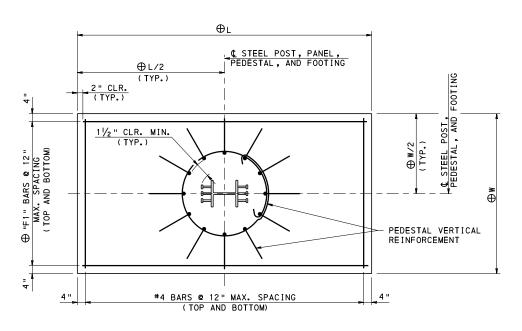
SHEET 7 OF 10 Bun & Thomps DIRECTOR, BUR. OF PROJECT DELIVERY BC-778M





SECTION M-M





SECTION N-N

LEGEND:

⊕ AS REQUIRED BY DESIGN
REFER TO CONTRACT DRAWINGS

NOTES:

- FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEET 1.
 FOR SECTION L-L REFER TO SHEET 7.
 FOR PANEL SEAT DETAILS REFER TO SHEET 4.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD GROUND MOUNTED SOUND BARRIERS
STEEL POSTS

DETAIL 4

RECOMMENDED SEPT. 30, 2016 Thomas P Macioca
CHIEF BRIDGE ENGINEER

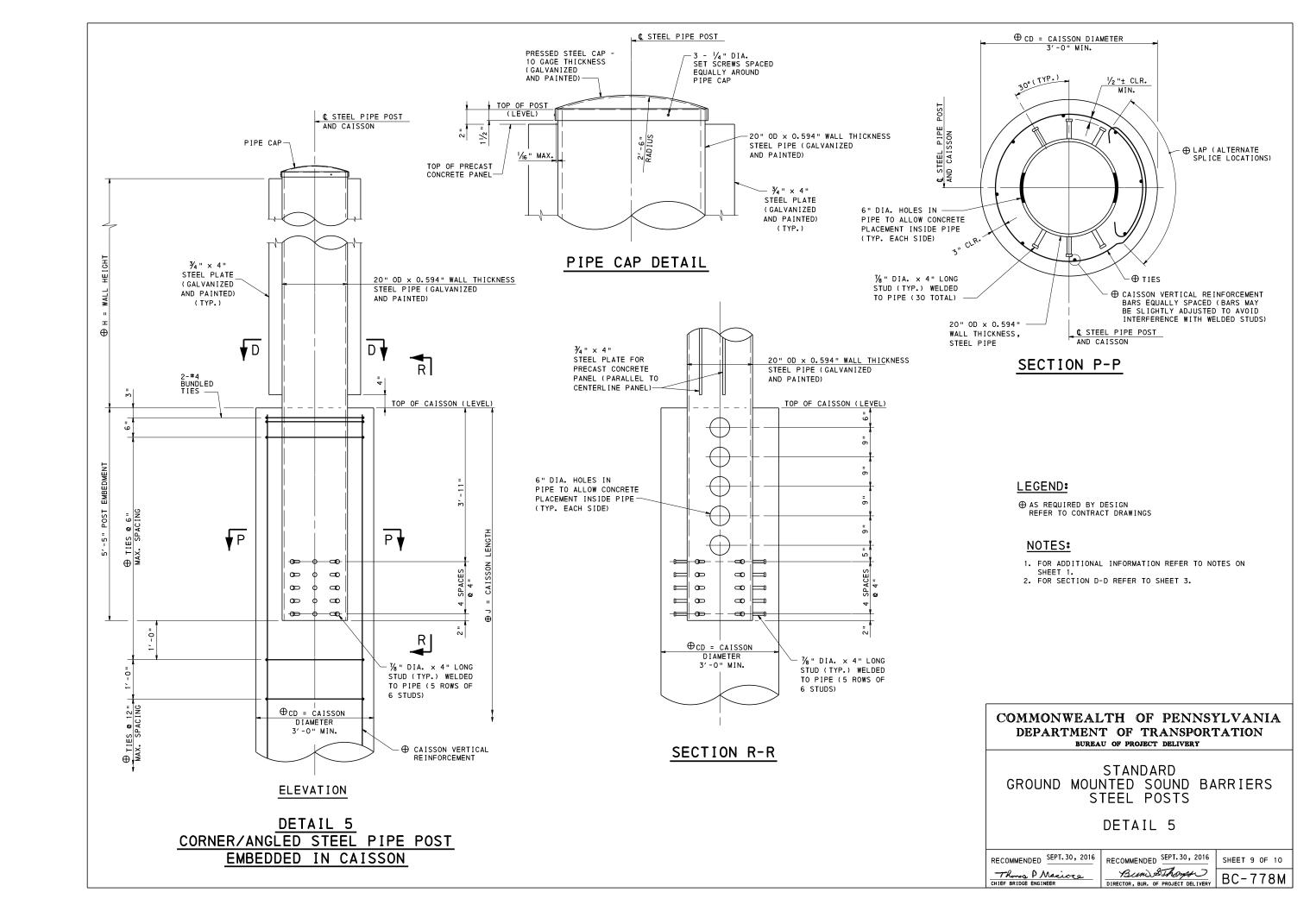
RECOMMENDED SEPT. 30, 2016 Bun & Thomps

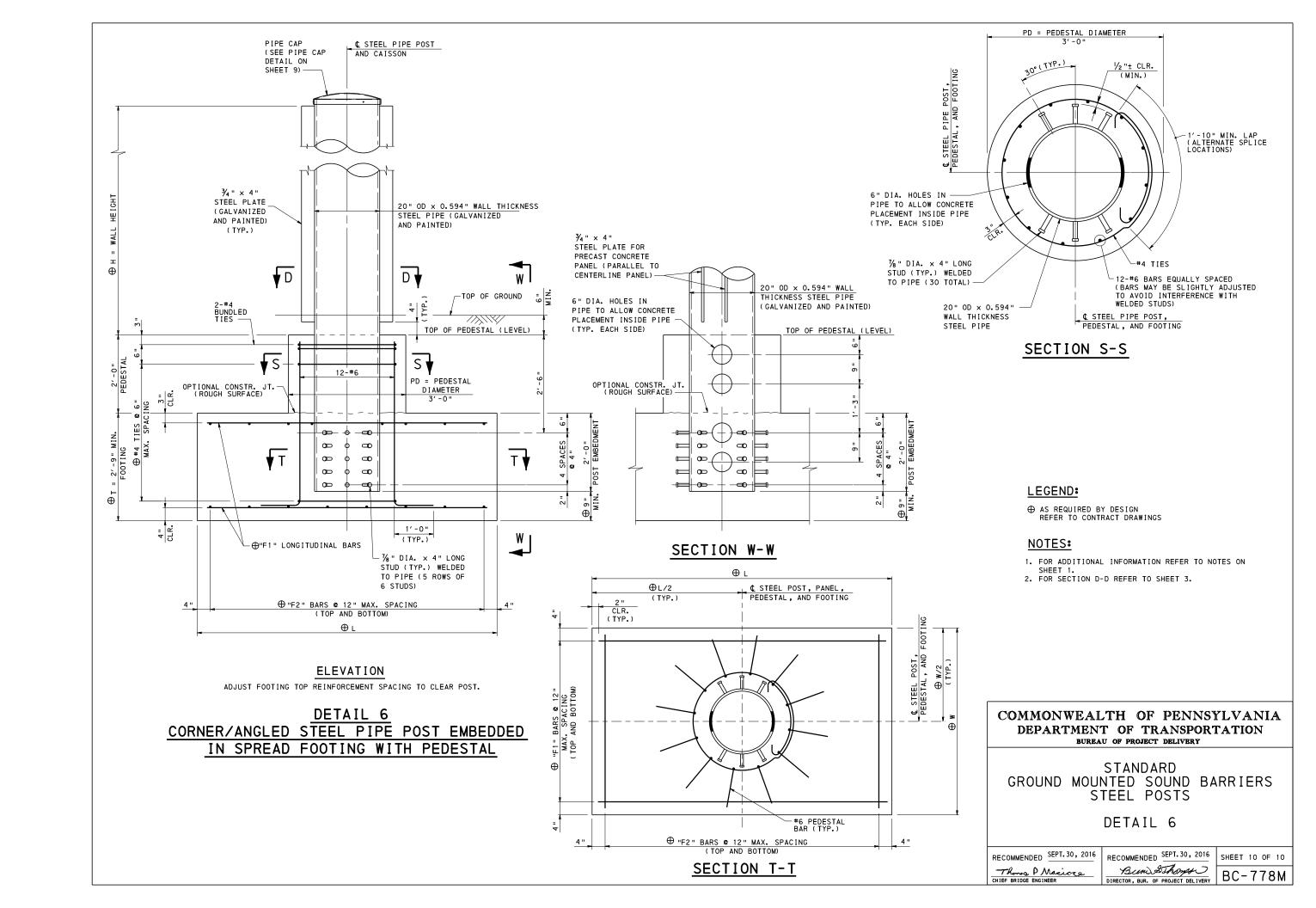
SHEET 8 OF 10 DIRECTOR, BUR. OF PROJECT DELIVERY BC-778M

ELEVATION

ADJUST FOOTING TOP REINFORCEMENT SPACING TO CLEAR POST.

DETAIL 4 STEEL POST EMBEDDED IN SPREAD FOOTING WITH PEDESTAL





GENERAL NOTES

- 1. DESIGN SPECIFICATIONS:

 - DESIGN SPECIFICATIONS:

 PENNDOT DESIGN MANUAL, PART 4, STRUCTURES, APRIL 2015 EDITION.

 1989 AASHTO "GUIDE SPECIFICATIONS FOR STRUCTURAL DESIGN OF SOUND BARRIERS", INCLUDING THE 1992 AND 2002 INTERIMS.

 2002 AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES", 17TH EDITION.

 2001 AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS", 4TH EDITION, INCLUDING INTERIMS THROUGH 2006.

 DESIGN IS IN ACCORDANCE WITH THE WORKING STRESS DESIGN METHOD. (NO INCREASE IN ALLOWABLE UNIT STRESSES ARE PERMITTED EXCEPT FOR GROUP III LOADINGS WHICH PERMITS A 33% OVERSTRESS.)
 - WHICH PERMITS A 33% OVERSTRESS.)
- 2. CONSTRUCTION SPECIFICATIONS AND WORK QUALITY:

 PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH THE CURRENT VERSION OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, AASHTO/AWS D1.5 BRIDGE WELDING CODE AND THE CONTRACT SPECIAL PROVISIONS. (USE AASHTOAWS D1.1 FOR WELDING NOT COVERED IN PROVISIONS. (USE AASHTO/AWS D1.5.)
- 3. WALL HEIGHTS MUST EQUAL OR EXCEED THE ACOUSTICAL PROFILE.
- 4. PANEL HEIGHTS:

 BARRIERS MOUNTED ON BRIDGES:
 2'-0" MINIMUM TO 10'-0" MAXIMUM
 PROVIDE A MAXIMUM POST SPACING OF 8'-0"
 WHEN PANEL HEIGHT IS GREATER THAN 9'-0"
 AND LESS THAN OR EQUAL TO 10'-0"
 STACKED PANELS NOT PERMITTED
 BARRIERS MOUNTED ON RETAINING WALLS AND MOMENT SLABS:
 2'-0" MINIMUM TO 9'-0" MAXIMUM
 PROVIDE STACKED PANELS WHEN THE WALL HEIGHT EXCEEDS 9'-0"

CHANGE 2

CHANGE 3

- 7. SLIP FORMING IS NOT PERMITTED FOR CONCRETE BARRIERS WITH STRUCTURE MOUNTED SOUND BARRIER WALLS.
- 8. INSTALL ANCHOR BOLTS, POSTS, AND PANELS TRULY VERTICAL.
- 9. PROVIDE STEEL CABLES IN THE PRECAST CONCRETE PANELS AS INDICATED ON THE CONTRACT DRAWINGS.
- 10. PROVIDE CONCRETE COVER IN ACCORDANCE WITH THIS STANDARD AND DESIGN MANUAL, PART 4.
- 12. FILL ALL LIFTING INSERTS WITH NON-SHRINK GROUT. COLOR TO MATCH PANEL.
- 13. SEAL ALL OPEN JOINTS WITH CAULKING COMPOUND AND/OR JOINT SEALING MATERIAL. (COLOR TO MATCH PANEL).
- 14. REFER TO PUBLICATION 408, SECTION 1086.3(f) FOR FABRICATION AND ERECTIONS TOLERANCES.
- 15. CHAMFER EXPOSED CONCRETE EDGES ON THE PRECAST PANELS 1/2 " x 1/2 ", EXCEPT AS NOTED.
- 16. ALL FILLET WELDS SHOWN ARE MINIMUM SIZE UNLESS NOTED OTHERWISE.
- 17. ALL DIMENSIONS SHOWN ARE HORIZONTAL, EXCEPT AS NOTED.
- 18. DIMENSIONS SHOWN ARE FOR A NORMAL TEMPERATURE OF 68 DEGREES F.
- 19. REINFORCEMENT IN SOME SECTIONS IS NOT SHOWN FOR CLARITY.
- 20. COORDINATE, LOCATE, AND CONDUCT ALL WORK RELATED TO PUBLIC AND PRIVATE UTILITIES IN ACCORDANCE WITH PUBLICATION 408, SECTION 105.06 AND 107.12, AND THE CONTRACT SPECIAL PROVISIONS.
- 21. IF A NEEDED DETAIL IS NOT FOUND IN THE SOUND BARRIER STANDARDS OR ON THE CONTRACT DRAWINGS, A SPECIAL SUBMISSION REQUESTING APPROVAL FOR SPECIFIC DETAILS MUST BE MADE TO THE CHIEF BRIDGE ENGINEER.
- 22. PROVIDE VERTICAL V-NOTCHES ON BARRIER FRONT AND REAR FACES AT ALL POST ANCHOR BOLT LOCATIONS FOR SOUND BARRIERS MOUNTED ON TOP OF BARRIERS ON BRIDGES, RETAINING WALLS AND MOMENT SLABS. SEE DETAIL ON SHEET 8.

NOTES TO FABRICATOR

- 1. PROVIDE SHOP DRAWINGS IN ACCORDANCE WITH PUBLICATION 408, SECTION 105.02(d)

- 3. THE SHOP DRAWINGS FOR THE PRECAST CONCRETE SOUND BARRIER PANELS AND THE FABRICATED STRUCTURAL STEEL POSTS MUST BE SUBMITTED CONCURRENTLY.
- 4. PRECAST CONCRETE PANELS: THE FABRICATOR MUST ENSURE THAT THE PANELS ARE ADEQUATELY DESIGNED FOR STRESSES DUE TO STRIPPING, HANDLING, ERECTION, AND TRANSPORTATION. PROVIDE AND SUBMIT DESIGN CALCULATIONS, AS REQUIRED.
- 5. LIFTING INSERTS:
- IFTING INSERTS:

 PREPARE AND SUBMIT DESIGN CALCULATIONS FOR THE PANEL LIFTING INSERTS FOR ACTUAL STRENGTH OF CONCRETE AT TIME OF STRIPPING, TRANSPORTATION AND ERECTION.

 PROVIDE LIFTING INSERTS WITH A MINIMUM CAPACITY OF AT LEAST TWO TIMES THE CALCULATED LOAD ON THE INSERT.

 PROVIDE A MINIMUM OF TWO LIFTING INSERTS OR A MAXIMUM OF FOUR LIFTING INSERTS IN THE PRECAST CONCRETE PANELS.

 - PROVIDE GALVANIZED INSERTS.
- 6. IF REQUIRED, PREPARE AND SUBMIT TEMPORARY BRACING CALCULATIONS AND DETAILS.
- 7. PREPARE AND SUBMIT CATALOG CUTS IN ACCORDANCE WITH PUBLICATION 408, SECTION 1086.3(d).

BC-735M WALL CONSTRUCTION & EXPANSION JOINT DETAILS

CONCRETE DECK SLAB DETAILS

PRECAST CONCRETE PANELS

GROUND MOUNTED SOUND BARRIERS

GROUND MOUNTED SOUND BARRIERS PRECAST CONCRETE POSTS

REINFORCEMENT BAR FABRICATION DETAILS

GROUND MOUNTED SOUND BARRIERS STEEL POSTS

REFERENCE DRAWINGS

BC-799M | MECHANICALLY STABILIZED EARTH RETAINING WALLS

- 8. #4 GRADE 60 REINFORCEMENT BARS MAY BE SUBSTITUTED FOR WELDED WIRE FABRIC WITH AN EQUIVALENT AREA AT NO ADDITIONAL COST TO THE DEPARTMENT.
- 9. PANELS MUST BE STORED, TRANSPORTED, HANDLED, AND ERECTED ON EDGES AT ALL TIMES. PANELS SHOULD NOT BE LAID FLAT.

BC-736M

BC-752M

BC-776M

10. FABRICATORS MUST BE PRE-APPROVED BY PENNDOT PER BULLETIN #15.

INDEX OF SHEETS		
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4	GEOMETRY AND LAYOUT - WALL MOUNTED	
5	PRECAST CONCRETE PANEL DETAILS - 1	
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7	DETAILS - 1	
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COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

STRUCTURE MOUNTED SOUND BARRIER WALLS

GENERAL NOTES - 1

FEB. 19, 2021 RECOMMENDED Thoma A. Macione CHIEF BRIDGE ENGINEER

RECOMMENDED FEB. 19, 2021 Bund Theyson DIRECTOR, BUR. OF PROJECT DELIVERY

SHEET 1 OF 9 BC-779M

5. PANEL JOINTS:

• PROVIDE FULL HEIGHT PANELS ON BARRIERS MOUNTED ON BRIDGES.

• MINIMIZE THE NUMBER OF PANEL JOINTS.

• PROVIDE UNIFORM STEPS.

• IF STEPS ARE REQUIRED, THE ELEVATION DIFFERENCE BETWEEN ADJACENT PANELS IS NOT PERMITTED TO BE LESS THAN 6" OR GREATER THAN 2'-0" AND MAY NOT OCCUR MORE FREQUENTLY THAN ONCE EVERY 50'-0" OF WALL LENGTH.

6. PRECAST CONCRETE POSTS ARE NOT PERMITTED FOR STRUCTURE MOUNTED SOUND BARRIERS, PROVIDE STEEL POSTS.

11. A HIGHER STRENGTH CONCRETE, FOR CAST-IN-PLACE CONCRETE, MAY BE SUBSTITUTED FOR A LOWER CLASS CONCRETE AT NO ADDITIONAL COST TO THE DEPARTMENT.

MATERIAL NOTES

- 1. CAST-IN-PLACE CONCRETE:

 PROVIDE CLASS A CEMENT IN THE CAST-IN-PLACE FOOTINGS AND CAISSONS,
 OR AS SPECIFIED ON THE CONTRACT DRAWINGS.
 f'c = 3,000 PSI

 PROVIDE CLASS AA CEMENT CONCRETE IN THE CAST-IN-PLACE BARRIERS AND MOMENT SLABS
 OR AS SPECIFIED ON THE CONTRACT DRAWINGS.
 f'c = 3,500 PSI
 UNIT WEIGHT OF CONCRETE = 150 LB./CU. FT.
- 2. PRECAST CONCRETE SOUND BARRIER PANELS:

 PROVIDE CLASS AA CEMENT CONCRETE, MODIFIED IN THE PRECAST CONCRETE PANELS.

 f'c = 5,000 PSI

 UNIT WEIGHT OF NORMAL CONCRETE = 150 LB./CU. FT.

 UNIT WEIGHT OF LIGHT WEIGHT CONCRETE = 115 LB./CU. FT.)

 PROVIDE A MINIMUM CONCRETE STRENGTH OF 4,000 PSI BEFORE STRIPPING THE PANELS FROM THE FORMS.

 PROVIDE LOW-DENSITY LIGHTWEIGHT (TYP.) CONCRETE IN THE PANELS, IF SPECIFIED ON THE CONTRACT DRAWINGS.
- 3. REINFORCEMENT STEEL:

 PROVIDE GRADE 60 DEFORMED REINFORCING BARS CONFORMING TO ASTM A615, ASTM A996, OR ASTM A706. DO NOT WELD REINFORCING BARS UNLESS SPECIFIED. DO NOT USE RAIL STEEL ASTM A996 REINFORCEMENT BARS IN BARRIERS, FOOTINGS, CAISSONS, OR WHERE BENDING OR WELDING OF REINFORCEMENT BARS IS INDICATED.

 fs = 24,000 PSI

 PROVIDE EPOXY COATED REINFORCEMENT IN THE BARRIERS AND MOMENT SLABS OR AS SPECIFIED ON THE CONTRACT DRAWINGS.

 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED REINFORCEMENT IN THE PANELS AS SPECIFIED ON THE CONTRACT DRAWINGS.

 PROVIDE MINIMUM LAP AND EMBEDDMENT LENGTH FOR REINFORCING BARS OF 30 DIAMETERS OR IN ACCORDANCE WITH THE CURRENT AASHTO SPECIFICATIONS AS MODIFIED BY THE DESIGN MANUAL, PART 4, WHICHEVER IS GREATER.
- 4. WELDED WIRE FABRIC:

 PROVIDE GRADE 65 PLAIN WELDED WIRE FABRIC CONFORMING TO ASTM A185
 IN THE PRECAST CONCRETE PANELS.

 fs = 24,000 PSI

 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED WELDED WIRE FABRIC IN THE PANELS

 AS SPECIFIED ON THE CONTRACT DRAWINGS.

 PROVIDE MINIMUM LAP FOR WELDED WIRE FABRIC IN ACCORDANCE WITH CURRENT AASHTO SPECIFICATIONS AS MODIFIED BY THE DESIGN MANUAL, PART 4.

 DO NOT MIX THE USE OF WELDED WIRE FABRIC AND REINFORCEMENT STEEL, EXCEPT AS INDICATED.
- 5. FABRICATED STRUCTURAL STEEL:

 PROVIDE STRUCTURAL STEEL CONFORMING TO AASHTO M270 GRADE 36

 (ASTM A709, GRADE 36) UNLESS OTHERWISE NOTED.

 WEATHERING STEEL (ASTM A588) IS NOT PERMITTED.

 PROVIDE MINIMUM BASE PLATE THICKNESS OF 3/4".

 PROVIDE MINIMUM WELD SIZE OF 3/8".

 NON-DESTRUCTIVE TESTING IS REQUIRED FOR STEEL POST TO BASE PLATE WELDS.
 PROVIDE TESTING IN ACCORDANCE WITH AASHTO/AWS D1.5 FOR MAIN MEMBER.

 GALVANIZE AND PAINT STEEL POSTS, PLATES, AND HARDWARE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s) AND 1060.2(b).

 CLEAN AND PREPARE GALVANIZED SURFACES FOR PAINTING IN ACCORDANCE WITH PUBLICATION 408, SECTION 1060.3(b) 4.

 REPAIR DAMAGED GALVANIZING IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s) 2.
- 6. ANCHOR BOLTS, NUTS, AND WASHERS:

 PROVIDE ANCHOR BOLTS CONFORMING TO ASTM F1554, GRADE 36 IN ACCORDANCE
 WITH PUBLICATION 408, SECTION 1105.02(c) 3,
 PROVIDE MINIMUM ANCHOR BOLT DIAMETER OF 3,
 PROVIDE HEAVY HEX LOCK NUTS AND HEAVY HEX NUTS CONFORMING TO ASTM A 563A
 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c) 3d.
 PROVIDE FLAT WASHERS CONFORMING TO ASTM F436 IN ACCORDANCE WITH PUBLICATION 106.02(c) 3b.
 GALVANIZE AND PAINT ALL ANCHOR BOLTS AND HARDWARE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s) AND 1060.2(b).
- 7. BOLTS, NUTS AND WASHERS FOR STEEL CABLE CONNECTIONS:

 PROVIDE BOLTS CONFORMING TO ASTM A307, GRADE A, IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c)1.

 PROVIDE HEAVY HEX NUTS CONFORMING TO ASTM A307, GRADE A, IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c)10.

 PROVIDE FLAT WASHERS AND LOCK WASHERS CONFORMING TO ASTM F436 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c)2b.

 GALVANIZE ALL BOLTS AND HARDWARE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s).

- 8. STEEL CABLES AND ACCESSORIES:

 PROVIDE 3/6", 7 × 19 GALVANIZED STEEL FLEXIBLE WIRE ROPE.

 MINIMUM BREAKING STRENGTH EQUALS 8 KIPS.

 STEEL ROPE MUST BE PLACED STRAIGHT AND TAUT BETWEEN CONNECTION POINTS AND WOVEN BETWEEN THE WELDED WIRE FABRIC.

 PROVIDE THIMBLES AS MANUFACTURED BY BREWER-TITCHENER #745-S OR AN APPROVED EQUAL.

 PROVIDE GALVANIZED WIRE ROPE CLIPS. TIGHTEN CLIPS IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.

 PROVIDE GALVANIZED STEEL PIPE (SCHEDULE 40) CONFORMING TO ASTM A53

 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(j) 1.

 GALVANIZE STEEL PIPE ACCORDING TO ASTM A153 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s).
- 9. PLAIN NEOPRENE BEARING PADS:

 PROVIDE PLAIN NEOPRENE PADS WITH A DUROMETER HARDNESS OF 50 (+/-) 5

 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1113.02.
- 10. CLOSED CELL NEOPRENE SPONGE:

 PROVIDE CLOSED CELL NEOPRENE SPONGE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1085.2(m).
- 11. NON-SHRINK GROUT:

 PROVIDE NON-SHRINK GROUT IN ACCORDANCE WITH PUBLICATION 408, SECTION 1080.2(c).

 PLACE NON-SHRINK GROUT AFTER THE BASE PLATE IS LEVELED ON THE LEVELING NUTS AND AFTER THE PANELS ARE INSTALLED.

 PACK GROUT INTO PLACE. DO NOT POUR OR INJECT GROUT.

 NON-SHRINK GROUT TO MATCH FINAL COLOR OF PANEL.
- 12. CAULKING COMPOUND:

 PROVIDE CAULKING COMPOUND IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.7(b).

 CAULKING COMPOUND TO MATCH FINAL COLOR OF PANEL.
- 13. JOINT SEALING MATERIAL:

 PROVIDE JOINT SEALING MATERIAL IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.4(a).

 JOINT SEALING MATERIAL TO MATCH FINAL COLOR OF PANEL.
- 14. JOINT BACKING MATERIAL (BACKER ROD):

 PROVIDE BACKER ROD MATERIAL IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.8.
- 15. ANTIGRAFFITI COATING:

 APPLY ANTIGRAFFITI COATING IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.
- 16. PENETRATING CONCRETE STAIN:

 APPLY STAIN IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.

ARCHITECTURAL SURFACE TREATMENTS

- 1. THE AVERAGE ARCHITECTURAL SURFACE TREATMENT, PER SIDE OF PANEL, IS PERMITTED TO VARY FROM O TO 1½", BUT THE TOTAL AVERAGE ARCHITECTURAL SURFACE TREATMENT, ON BOTH SIDES OF THE PANEL, MUST NOT BE GREATER THAN 1½" UNLESS OTHERWISE INDICATED ON THE CONTRACT DRAWINGS.
- 2. IF A SMOOTH ARCHITECTURAL SURFACE TREATMENT IS PROVIDED, THE TREATMENT MAY EXTEND TO THE EDGES OF PANELS AS LONG AS THE PANEL FITS BETWEEN THE FLANGES OF THE POST.
- 3. STAMPED FINISHES MAY BE PERMITTED IF ACCEPTED BY THE DISTRICT BRIDGE ENGINEER.
- REFER TO PUBLICATION 408, SECTION 1086.3 AND/OR THE CONTRACT DOCUMENTS FOR ARCHITECTURAL SURFACE TREATMENT TOLERANCES.
- 5. REFER TO CONTRACT DOCUMENTS FOR ADDITIONAL INFORMATION.

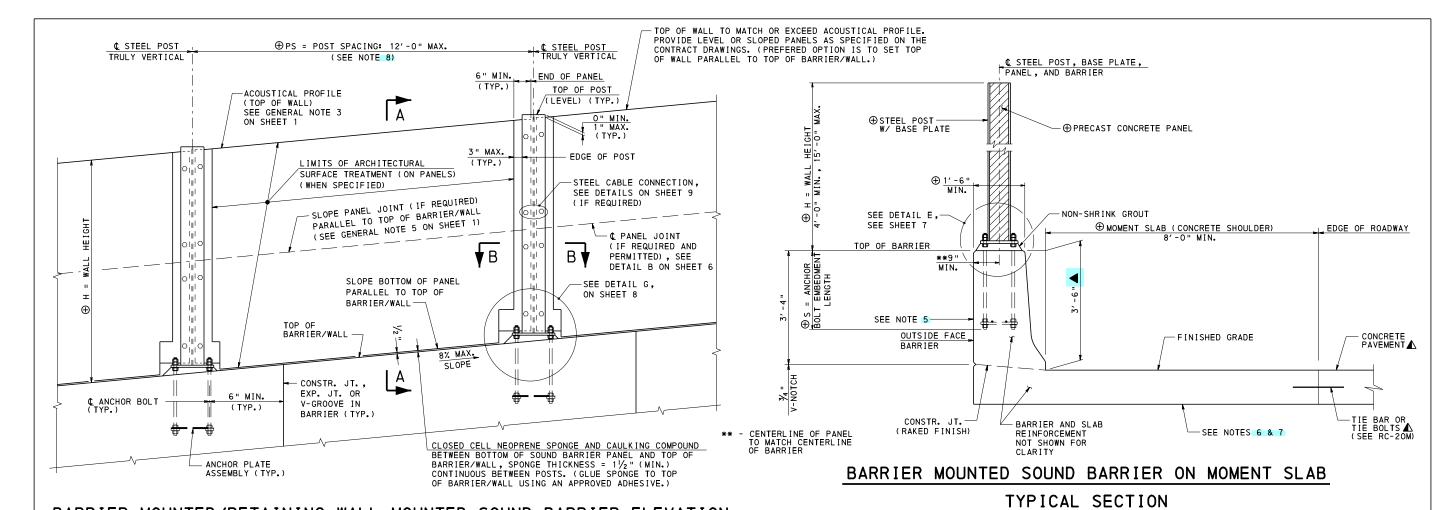
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD STRUCTURE MOUNTED SOUND BARRIER WALLS

GENERAL NOTES - 2

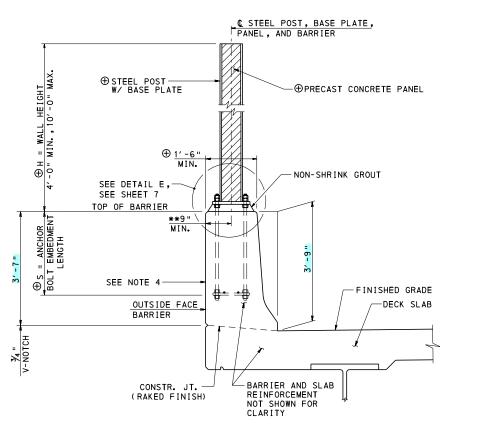
RECOMMENDED FEB. 19, 2021 FEB.19, 2021 RECOMMENDED Thomas A. Mariore
CHIEF BRIDGE ENGINEER Bund Theyson DIRECTOR, BUR. OF PROJECT DELIVERY

SHEET 2 OF 9 BC-779M

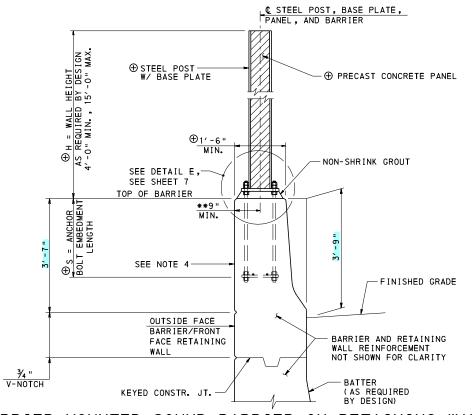


BARRIER MOUNTED/RETAINING WALL MOUNTED SOUND BARRIER ELEVATION

(TOP OF BARRIER/WALL SLOPED)



BARRIER MOUNTED SOUND BARRIER ON BRIDGE TYPICAL SECTION



BARRIER MOUNTED SOUND BARRIER ON RETAINING WALL TYPICAL SECTION

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEETS 1 AND 2.
- 2. FOR SECTION A-A, SEE SHEET 6.
- 3. FOR SECTION B-B, SEE SHEET 7.
- 4. 45" F-SHAPE CONCRETE BARRIER SHOWN, 42" F-SHAPE CONCRETE BARRIER AND 42" VERTICAL WALL CONCRETE BARRIER SIMILAR.
- 5. 42" F-SHAPE CONCRETE BARRIER SHOWN, 42" VERTICAL WALL CONCRETE BARRIER SIMILAR.
- 6. MOMENT SLAB ON MECHANICALLY STABILIZED EARTH WALLS SIMILAR, REFER TO CONTRACT DRAWINGS AND BC-799M FOR ADDITIONAL DETAILS.
- 7. CONCRETE SHOULDER SHOWN, BITUMINOUS CONCRETE SHOULDER SIMILAR, REFER TO CONTRACT DRAWINGS AND BC-799M FOR
- ADDITIONAL DETAILS.

 8. MAXIMUM POST SPACING IS 8'-0" FOR BRIDGE MOUNTED SOUND BARRIER WHEN PANEL HEIGHT IS GREATER THAN 9'-0" AND LESS THAN OR EQUAL TO 10'-0".

LEGEND:

- A ROADWAY ITEM
- ⊕ AS REQUIRED BY DESIGN, REFER TO CONTRACT DRAWINGS
- ▲ 45" F-SHAPE CONCRETE BARRIER NOT PERMITTED ON MOMENT SLAB.

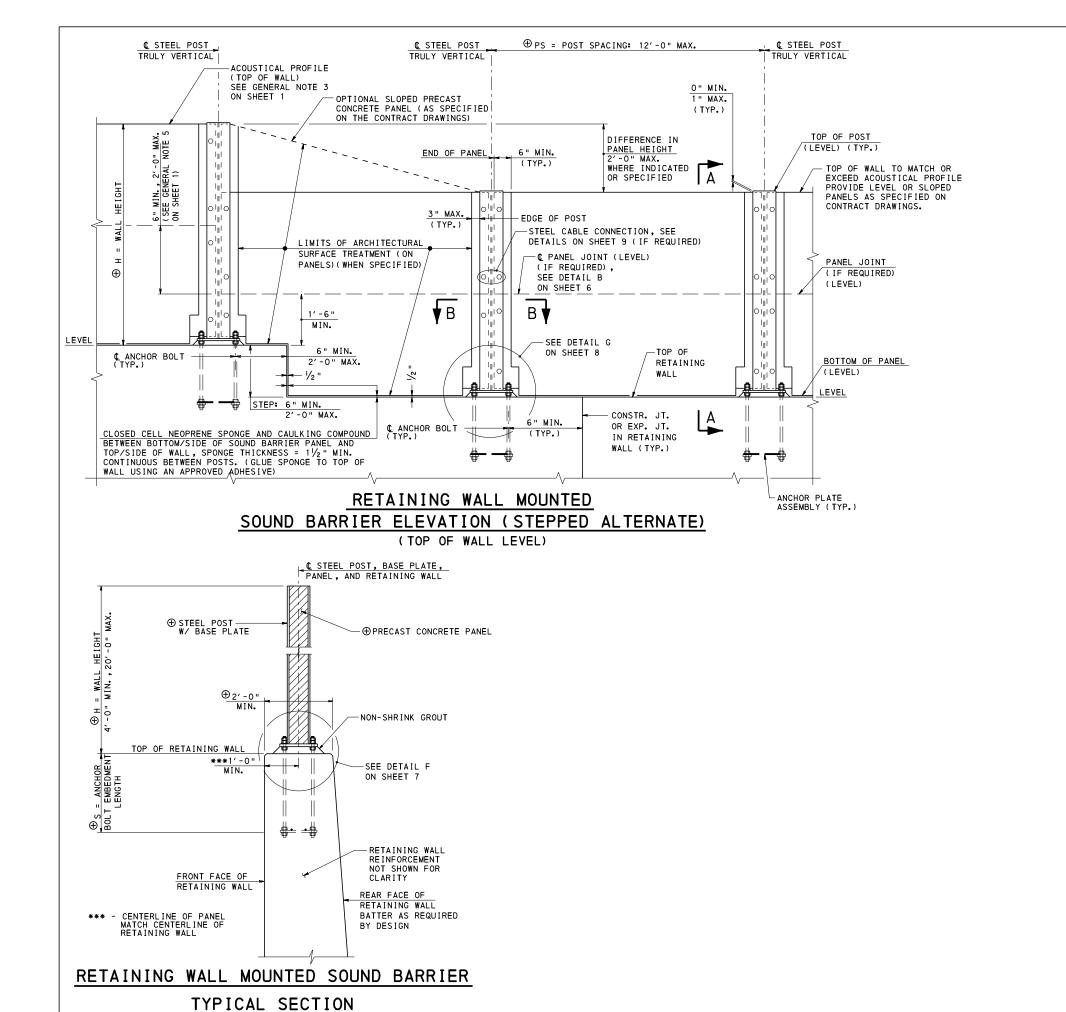
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

STRUCTURE MOUNTED SOUND BARRIER WALLS GEOMETRY AND LAYOUT - BARRIER MOUNTED

RECOMMENDED FEB. 19, 2021 Thomas A. Macione CHIEF BRIDGE ENGINEER RECOMMENDED FEB. 19, 2021 Bund Theyson IRECTOR, BUR. OF PROJECT DELIVERY

SHEET 3 OF 9 BC-779M



NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEETS 1 AND 2.
- 2. FOR SECTION A-A, SEE SHEET 6.
- 3. FOR SECTION B-B, SEE SHEET 7.

LEGEND:

⊕ AS REQUIRED BY DESIGN, REFER TO CONTRACT DRAWINGS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

BUREAU OF PROJECT DELIVERY

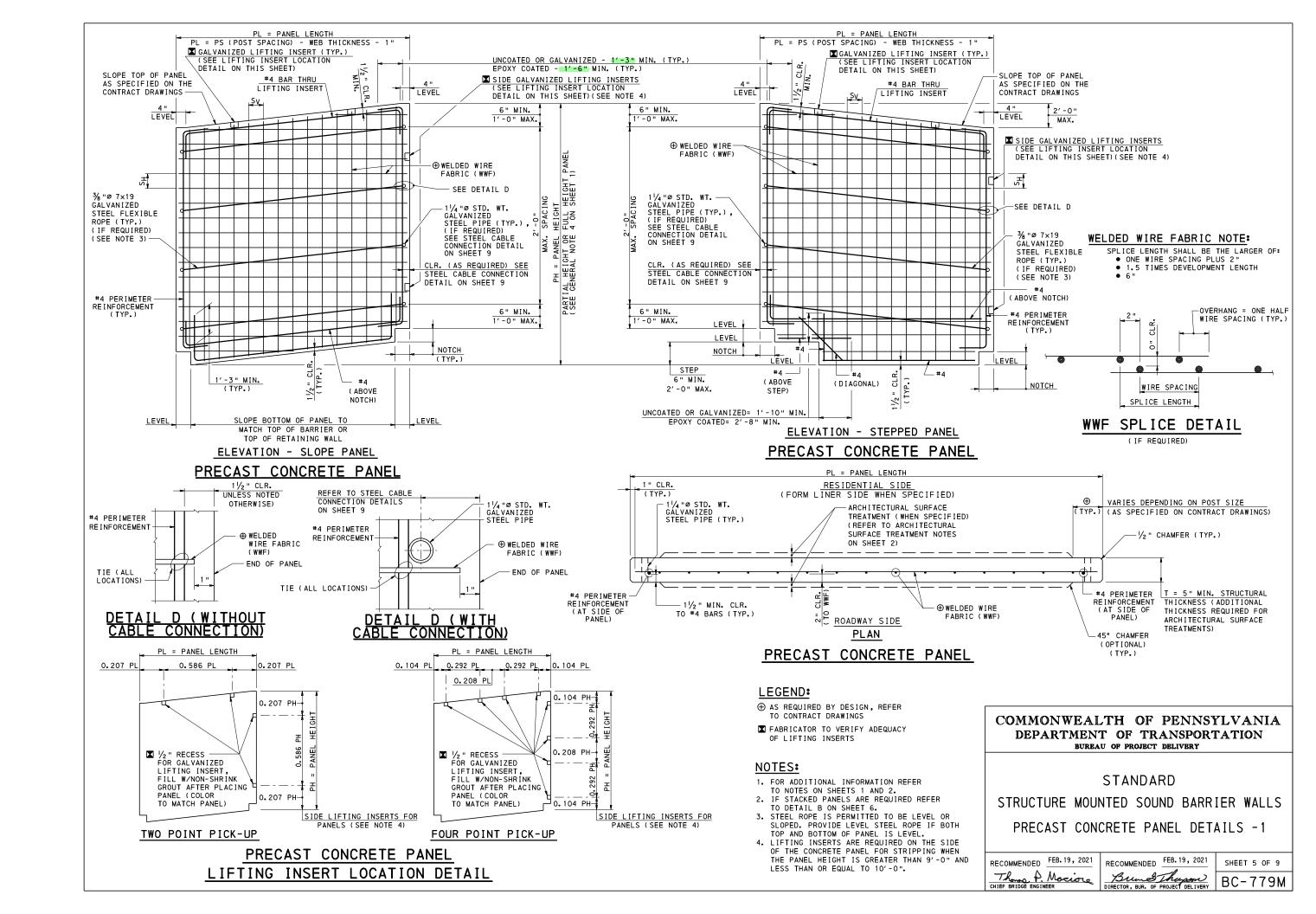
STANDARD STRUCTURE MOUNTED SOUND BARRIER WALLS

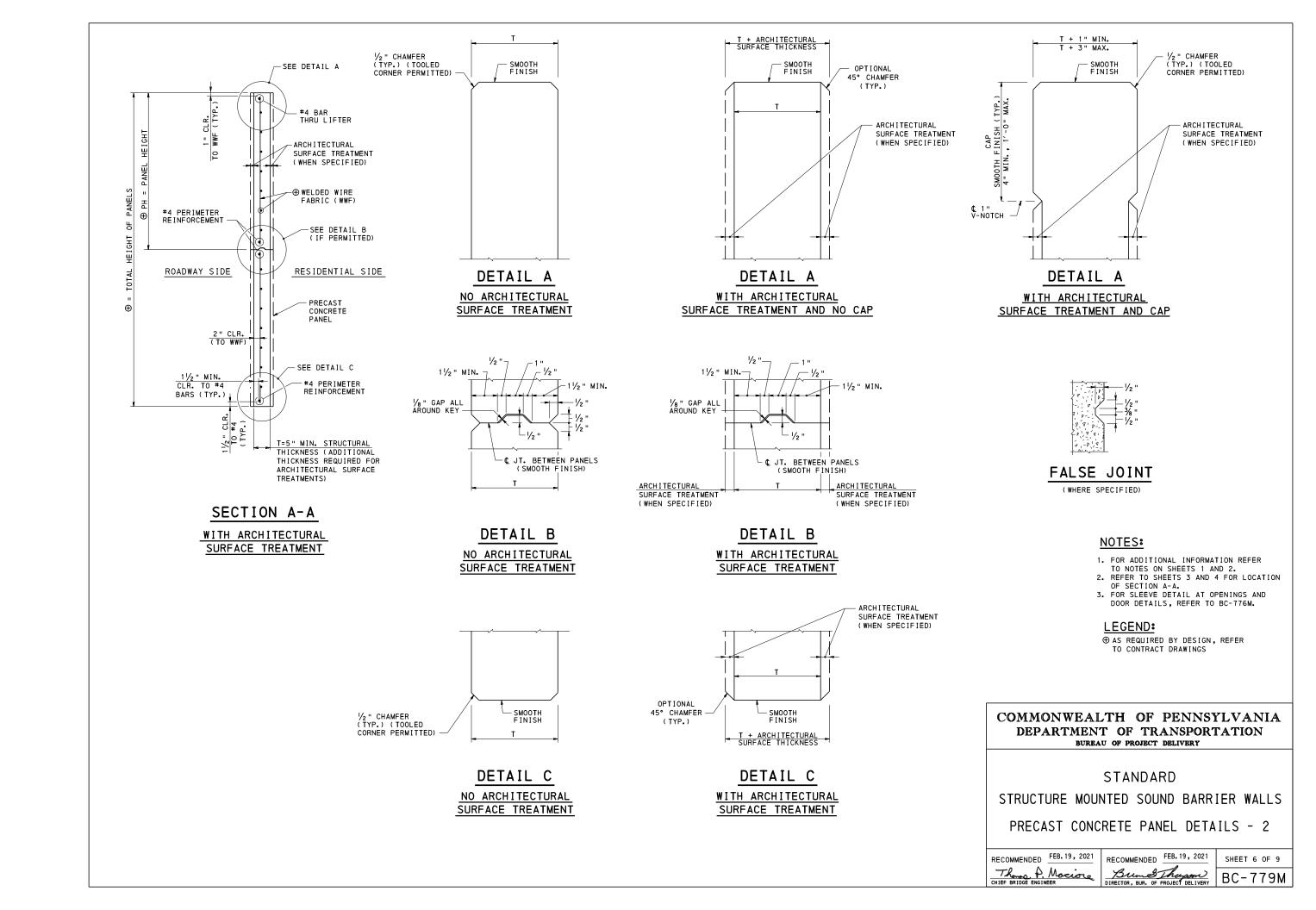
GEOMETRY AND LAYOUT - WALL MOUNTED

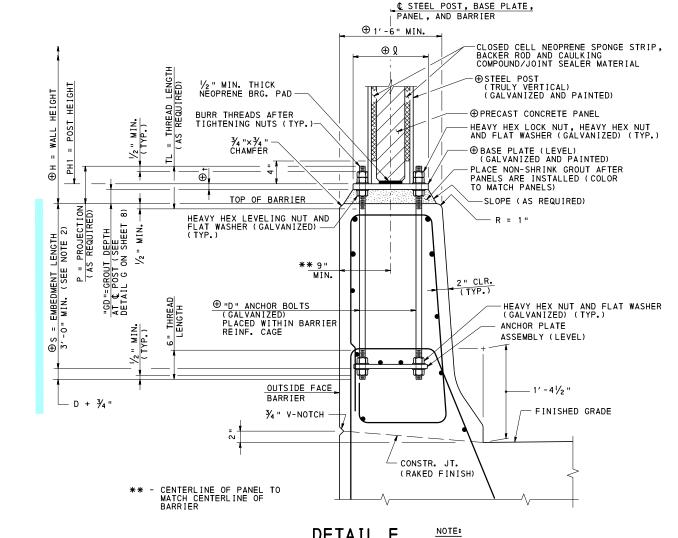
RECOMMENDED FEB. 19, 2021 Thomas P. Macione CHIEF BRIDGE ENGINEER

RECOMMENDED FEB. 19, 2021 Bund Theyson BC-779M

SHEET 4 OF 9

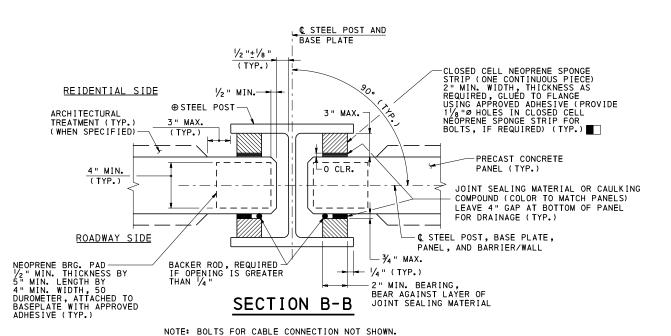




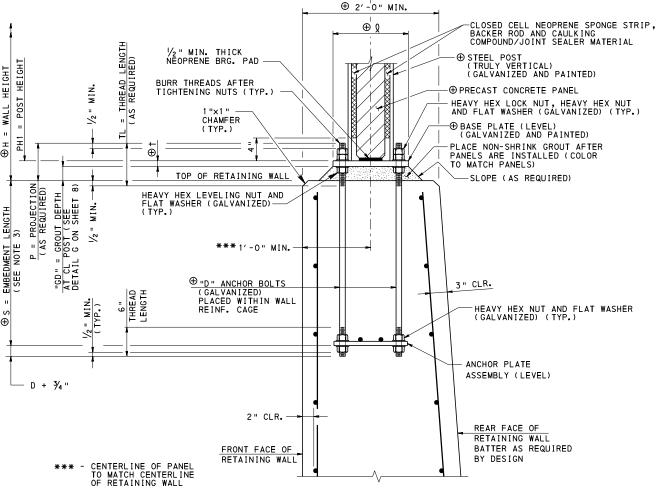


DETAIL E

45" F-SHAPE CONCRETE BARRIER SHOWN, 42" F-SHAPE CONCRETE BARRIER AND 42" VERTICAL WALL CONCRETE BARRIER SIMILAR.



REFER TO SHEET 9 FOR DETAILS.



SEQUENCE OF INSTALLATION PANEL TO STEEL POST

- 1. INSTALL POST AND BASE PLATE ASSEMBLY ON THE LEVELING NUTS
- AT CORRECT ELEVATION.
 2. GLUE NEOPRENE BEARING PADS TO BASE PLATE USING AN APPROVED
- ADHESIVE.
 GLUE CLOSED CELL NEOPRENE SPONGE STRIP TO POST ON RESIDENTIAL SIDE OF BARRIER USING AN APPROVED ADHESIVE. APPLY 4" MIN. JOINT SEALING MATERIAL TO FACE OF CLOSED CELL NEOPRENE SPONGE STRIP. STOP CLOSED CELL NEOPRENE SPONGE STRIP 4" ABOVE BASE PLATE FOR DRAINAGE.
- PLATE FOR DRAINAGE.

 FRECT PRECAST PANEL USING THE LEVELING NUTS TO ADJUST POST AND BASE PLATE ASSEMBLY TO ALIGN HOLES FOR STEEL CABLE CONNECTION. ADD JOINT SEALING MATERIAL OR CAULKING COMPOUND AND WEDGE TIGHT AGAINST POST AND PANEL ON RESIDENTIAL SIDE OF BARRIER.
- GLUE CLOSED CELL NEOPRENE SPONGE STRIP TO POST ON ROADWAY SIDE OF BARRIER.
- 6. INSERT BACKER RODS IF OPENINGS ARE GREATER THAN 1/4"
 AND APPLY JOINT SEALING MATERIAL OR CAULKING COMPOUND.
 7. WHERE NO CLOSED CELL NEOPRENE SPONGE STRIP IS REQUIRED, SEAL PANEL TO POST WITH JOINT SEALING MATERIAL OR CAULKING COMPOUND. ALLOW 4" UNSEALED GAP AT BOTTOM OF PANEL FOR DRAINAGE.
- INSTALL BOLTS (FOR STEEL CABLE CONNECTION) THRU FLANGES AND PANEL (IF REQUIRED).
- 9. PLACE NON-SHRINK GROUT UNDER BASE PLATE.

LEGEND:

⊕ AS REQUIRED BY DESIGN, REFER TO CONTRACT DRAWINGS

CLOSED CELL NEOPRENE SPONGE STRIP NOT REQUIRED IF JOINT BETWEEN PANEL AND FLANGE IS LESS THAN 3/4". ZERO, ONE, OR TWO STRIPS MAY BE REQUIRED DEPENDING UPON SIZE OF STEEL POST. GLUING TWO CLOSED CELL NEOPRENE SPONGE STRIPS TOGETHER. USING AND APPROVED ADHESIVE, TO OBTAIN REQUIRED THICKNESS IS PERMITTED. WHERE NO CLOSED CELL NEOPRENE SPONGE STRIP IS REQUIRED, SEAL PANEL TO FLANGE. BACKER ROD IS REQUIRED UNLESS THE JOINT BETWEEN THE PANEL AND FLANGE OR CLOSED CELL NEOPRENE SPONGE STRIP IS LESS THAN 1/4 ".

DETAIL F

NOTES:

FOR ADDITIONAL INFORMATION REFER
TO NOTES ON SHEETS 1 AND 2.
 ANCHOR PLATES MUST BE WITHIN CAGE FORMED

C STEEL POST, BASE PLATE, PANEL, AND RETAINING WALL

- BY LOWER BARRIER REINFORCEMENT EXTENDING OUT OF DECK SLAB, MOMENT SLAB OR RETAINING
- 3. EMBEDMENT OF ANCHOR BOLTS MUST EXTEND TO A DEPTH WHERE THE VERTICAL WALL REINFORCEMENT IS FULLY DEVELOPED.
- REFER TO SHEETS 3 AND 4 FOR LOCATION OF SECTION B-B.
- 5. REFER TO SHEET 3 FOR LOCATION OF DETAIL E.
- 6. REFER TO SHEET 4 FOR LOCATION OF DETAIL F.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

BUREAU OF PROJECT DELIVERY

STANDARD

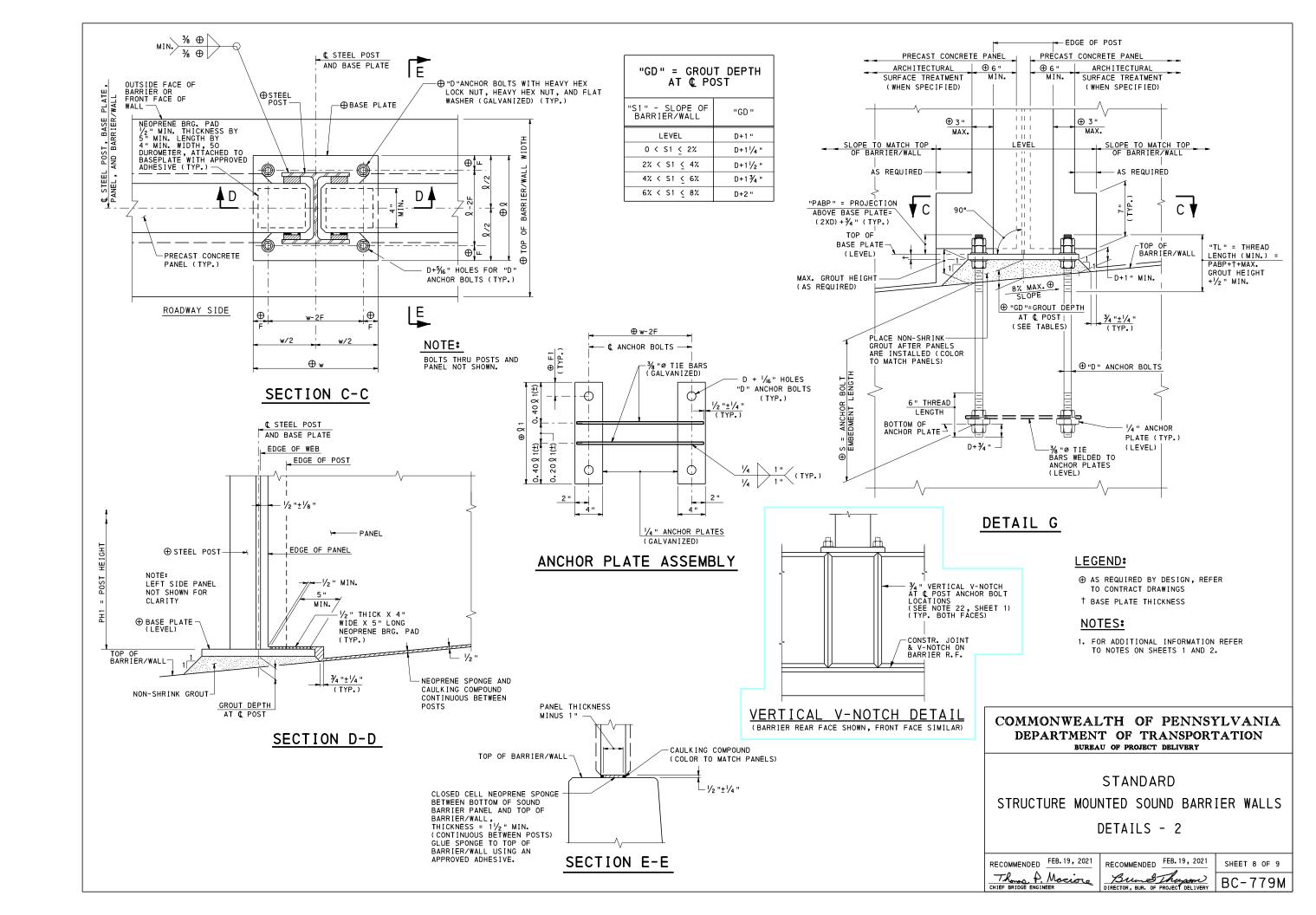
STRUCTURE MOUNTED SOUND BARRIER WALLS

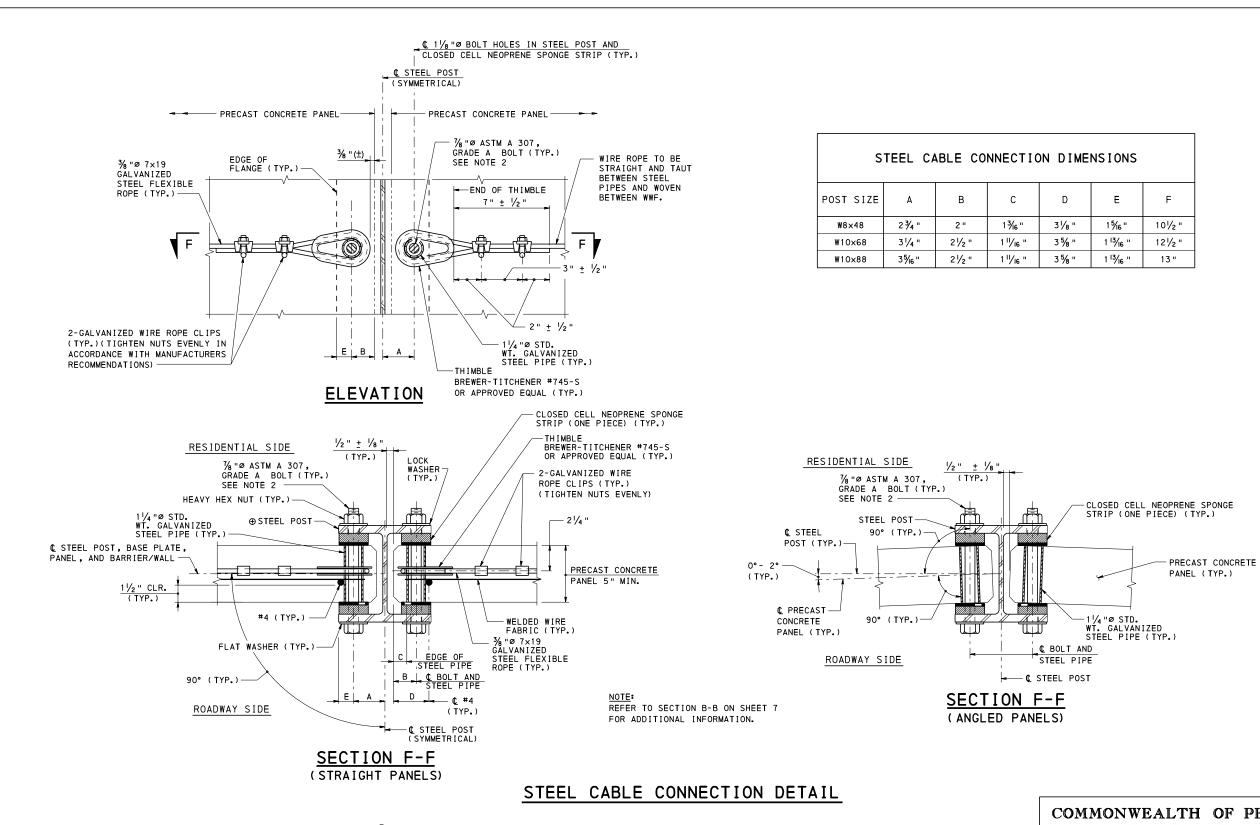
DETAILS - 1

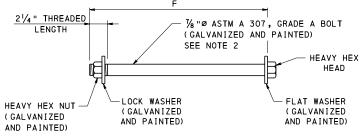
FEB. 19, 2021 RECOMMENDED Thomas A. Maciona

RECOMMENDED FEB. 19, 2021 Bum & Theyaron
IRECTOR, BUR. OF PROJECT DELIVERY

SHEET 7 OF 9 BC-779M







BOLT DETAIL

NOTES:

FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEETS 1 AND 2.
 ¾4" DIAMETER BOLTS ARE PERMITTED TO BE SUBSTITUTED
ON A LIMITED BASIS IF THE ¾6" DIAMETER BOLTS DO NOT FIT
THROUGH THE HOLES IN THE STEEL POST AND THE PIPE SLEEVE IN THE
PRECAST CONCRETE PANEL. NO MORE THAN 50% OF THE CONNECTIONS
ON ONE SIDE OF A PANEL ARE PERMITTED TO CONTAIN THE SMALLER
BOLT DIAMETER.

AS REQUIRED BY DESIGN,
 REFER TO CONTRACT DRAWINGS

LEGEND:

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

STRUCTURE MOUNTED SOUND BARRIER WALLS

STEEL CABLE CONNECTION DETAIL

RECOMMENDED FEB. 19, 2021
The Description of the Prince Chief Bridge Engineer

RECOMMENDED FEB. 19, 2021
Bund Thupon

BC-779M

GENERAL NOTES

- 1. DESIGN SPECIFICATIONS:
- ESIGN SPECIFICATIONS:

 PENNDOT DESIGN MANUAL, PART 4, STRUCTURES APRIL 2015 EDITION
 1989 AASHTO "GUIDE SPECIFICATIONS FOR STRUCTURAL DESIGN OF SOUND BARRIERS",
 INCLUDING THE 1992 AND 2002 INTERIMS.
 2002 AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES", 17TH EDITION,
 2001 AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY
 SIGNS, LUMINAIRES AND TRAFFIC SIGNALS", 4TH EDITION, INTERIMS THROUGH 2006.
 DESIGN IS IN ACCORDANCE WITH THE WORKING STRESS DESIGN METHOD. (NO INCREASE
 IN ALLOWABLE UNIT STRESSES ARE PERMITTED EXCEPT FOR GROUP III LOADINGS
 WHICH PERMITS A 33% OVERSTRESS).
- 2. CONSTRUCTION SPECIFICATIONS AND WORKMANSHIP:

 PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH THE CURRENT VERSION OF THE PENNSYLVANIA DEPARTMENT OF TRANSPORTATION PUBLICATION 408, AASHTO/AWS D1.5 BRIDGE WELDING CODE AND THE CONTRACT SPECIAL PROVISIONS. (USE AASHTO/AWS D1.1 FOR WELDING NOT COVERED IN AASHTO/AWS D1.5).
- 3. WALL HEIGHTS MUST EQUAL OR EXCEED THE ACOUSTICAL PROFILE.
- 4. STANDARD PANEL HEIGHTS:

 4'-0" MINIMUM TO 12'-0" MAXIMUM.

 PROVIDE STACKED PANELS WHEN THE WALL HEIGHT EXCEEDS 12'-0".

CHANGE 2

- END PANEL HEIGHTS:

 6'-0" MINIMUM TO 22'-0" MAXIMUM.

 PROVIDE SINGLE END PANELS, STACKED PANELS ARE NOT PERMITTED.
- 6. HORIZONTAL PANELS JOINT:

 MINIMIZE THE NUMBER OF HORIZONTAL PANEL JOINTS.

 PROVIDE UNIFORM STEPS.

 IF STEPS ARE REQUIRED, THE ELEVATION DIFFERENCE BETWEEN ADJACENT PANELS IS NOT PERMITTED TO BE LESS THAN 6" OR GREATER THAN 2'-0".
- 7. PROVIDE A MINIMUM OF TWO CABLE CONNECTIONS FOR EACH PANEL-TO-PANEL CONNECTION.
- 8. INSTALL ANCHOR BOLTS AND PANELS TRULY VERTICAL.
- 9. PROVIDE CONCRETE COVER IN ACCORDANCE WITH THIS STANDARD AND DESIGN MANUAL PART 4.
- 10. A HIGHER STRENGTH CONCRETE, FOR CAST-IN-PLACE CONCRETE, MAY BE SUBSTITUTED FOR A LOWER CLASS CONCRETE AT NO ADDITIONAL COST TO THE DEPARTMENT.
- 11. FILL ALL LIFTING INSERTS WITH NON-SHRINK GROUT. COLOR TO MATCH PANEL.
- 12. SEAL ALL OPEN JOINTS WITH CAULKING COMPOUND AND/OR JOINT SEALING MATERIAL. (COLOR TO MATCH PANEL).
- 13. REFER TO PUBLICATION 408, SECTION 1086.3 (f) FOR FABRICATION AND ERECTIONS TOLERANCES.
- 14. CHAMFER EXPOSED CONCRETE EDGES ON PRECAST PANELS 1/2" x 1/2", EXCEPT AS NOTED.
- 15. CHAMFER EXPOSED CONCRETE EDGES ON CAST-IN-PLACE CONCRETE 1" x 1", EXCEPT AS NOTED.
- 16. RAKE-FINISH ALL HORIZONTAL CONSTRUCTION JOINTS, EXCEPT AS NOTED.
- 17. ALL DIMENSIONS SHOWN ARE HORIZONTAL, EXCEPT AS NOTED.
- 18. DIMENSIONS SHOWN ARE FOR A NORMAL TEMPERATURE OF 68 DEGREES F.
- 19. REINFORCEMENT IN SOME SECTIONS IS NOT SHOWN FOR CLARITY.
- 20. SPREAD FOOTINGS MAY BE ORDERED BY THE REPRESENTATIVE TO BE AT ANY ELEVATION OR ANY DIMENSIONS NECESSARY TO PROVIDE A PROPER FOUNDATION. IF SPREAD FOOTINGS ARE ADJUSTED PANEL HEIGHTS AND WALL DESIGN WILL NEED TO BE MODIFIED.
- 21. USE CLASS C CEMENT CONCRETE OR NO. 2A COARSE AGGREGATE BELOW SPREAD FOOTING WHEN SPECIFIED OR DIRECTED.
- 22. COORDINATE, LOCATE, AND CONDUCT ALL WORK RELATED TO PUBLIC AND PRIVATE UTILITIES IN ACCORDANCE WITH PUBLICATION 408, SECTION 105.06 AND 107.12, AND THE CONTRACT SPECIAL PROVISIONS.
- 23. IF NEEDED DETAIL IS NOT FOUND IN THE SOUND BARRIER STANDARDS OR ON THE CONTRACT DRAWINGS, A SPECIAL SUBMISSION REQUESTING APPROVAL FOR SPECIFIC DETAILS MUST BE MADE TO THE CHIEF BRIDGE ENGINEER.

NOTES TO FABRICATOR

- 1. PROVIDE SHOP DRAWINGS IN ACCORDANCE WITH PUBLICATION 408, SECTION 105.02(d) AND 1086.
- - MATERIAL LISTS
 REINFORCEMENT BAR SCHEDULES
 ANY OTHER INFORMATION REQUIRED TO FABRICATE AND CONSTRUCT THE SOUND
- 3. PRECAST CONCRETE PANELS:

 THE FABRICATOR MUST ENSURE THAT THE PANELS ARE ADEQUATELY DESIGNED FOR STRESSES DUE TO STRIPPING, HANDLING, ERECTION, AND TRANSPORTATION. PROVIDE AND SUBMIT DESIGN CALCULATIONS, AS REQUIRED.
- - PREPARE AND SUBMIT DESIGN CALCULATIONS FOR THE PANEL LIFTING INSERTS
 FOR ACTUAL STRENGTH OF CONCRETE AT TIME OF STRIPPING, TRANSPORTATION
 AND ERECTION.

 - PROVIDE LIFTING INSERTS WITH A MINIMUM CAPACITY OF AT LEAST TWO TIMES THE CALCULATED LOAD ON THE INSERT.

 PROVIDE A MINIMUM OF TWO LIFTING INSERTS OR A MAXIMUM OF FOUR LIFTING INSERTS IN THE PRECAST CONCRETE STANDARD PANELS.

 PROVIDE A MINIMUM OF TWO LIFTING INSERTS OR A MAXIMUM OR EIGHT LIFTING INSERTS IN THE PRECAST CONCRETE END PANELS.
 - PROVIDE GALVANIZED INSERTS.
- 5. IF REQUIRED, PREPARE AND SUBMIT TEMPORARY BRACING CALCULATIONS AND DETAILS.
- 6. PREPARE AND SUBMIT CATALOG CUTS IN ACCORDANCE WITH PUBLICATION 408, SECTION 1086.3.
- 7. #4 GRADE 60 REINFORCEMENT BARS MAY BE SUBSTITUTED FOR WELDED WIRE FABRIC WITH AN EQUIVALENT AREA AT NO ADDITIONAL COST TO THE DEPARTMENT.
- 8. PANELS MUST BE STORED, TRANSPORTED, HANDLED, AND ERECTED ON EDGES AT ALL TIMES. PANELS SHOULD NOT BE LAID FLAT.
- 9. FABRICATORS MUST BE PRE-APPROVED BY PENNDOT PER BULLETIN #15.

INDEX OF SHEETS				
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1	GENERAL NOTES - 1			
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BC-734M ANCHOR SYSTEMS BC-735M WALL CONSTRUCTION AND EXPANSION JOINT DETAILS REINFORCEMENT BAR FABRICATION DETAILS BC-736M GROUND MOUNTED SOUND BARRIERS - PRECAST CONCRETE PANELS BC-776M RC-11M CLASSIFICATION OF EARTHWORK FOR STRUCTURES REFERENCE DRAWINGS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

> STANDARD OFFSET SOUND BARRIER WALLS GENERAL NOTES - 1

RECOMMENDED JAN. 31, 2019 Thomas P. Macioca

RECOMMENDED JAN. 31, 2019 Allin Hotics ACTING DIR. BUR. OF PROJECT DELIVERY BC-780M

SHEET 1 OF 8

MATERIAL NOTES

- 1. CAST-IN-PLACE CONCRETE:

 PROVIDE CLASS A CEMENT CONCRETE IN THE CAST-IN-PLACE FOOTINGS.

 f' c = 3,000 PSI

 UNIT WEIGHT OF CONCRETE = 150 LB./CU. FT.
- 2. PRECAST CONCRETE SOUND BARRIER PANELS:

 PROVIDE CLASS AA CEMENT CONCRETE, MODIFIED IN THE PRECAST CONCRETE PANELS.

 f'c = 5,000 PSI

 UNIT WEIGHT OF CONCRETE = 150 LB./CU. FT.

 PROVIDE A MINIMUM CONCRETE STRENGTH OF 4,000 PSI BEFORE STRIPPING

 - THE PANELS FROM THE FORMS.
- 3. REINFORCEMENT STEEL:
- REINFORCEMENT STEEL:

 PROVIDE GRADE 60 DEFORMED REINFORCING STEEL BARS THAT MEET THE REQUIREMENTS OF ASTM A615, ASTM A996, OR ASTM A706. DO NOT WELD REINFORCING STEEL BARS UNLESS SPECIFIED. DO NOT USE RAIL STEEL A996 REINFORCEMENT BARS IN FOOTINGS OR WHERE BENDING OR WELDING OF REINFORCEMENT BARS IS INDICATED.

 FRO 1DE UNCOATED REINFORCEMENT BARS IN THE FOOTING.

 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED REINFORCEMENT IN THE PANELS AS SPECIFIED ON THE CONTRACT DRAWINGS.

 PROVIDE MINDIMUM LAP AND EMBEDMENT LENGTH FOR REINFORCING BARS OF 30 DIAMETERS OR IN ACCORDANCE WITH THE CURRENT AASHTO SPECIFICATIONS AS MODIFIED BY THE DESIGN MANUAL PART 4, WHICHEVER IS GREATER.
- 4. WELDED WIRE FABRIC:

 PROVIDE GRADE 65 PLAIN WELDED WIRE FABRIC THAT MEET THE REQUIREMENTS OF ASTM A185 IN THE PRECAST CONCRETE PANELS.

 fs = 24,000 PSI

 PROVIDE UNCOATED, EPOXY COATED, OR GALVANIZED WELDED WIRE FABRIC IN THE PANELS AS SPECIFIED ON THE CONTRACT DRAWINGS.

 PROVIDE MINITUM LAP FOR WELDED WIRE FABRIC IN ACCORDANCE WITH CURRENT AASHTO SPECIFICATIONS AS MODIFIED BY THE DESIGN MANUAL PART 4.

 DO NOT MIX THE USE OF WELDED WIRE FABRIC AND REINFORCEMENT STEEL, EXCEPT AS INDICATED.

 - AS INDICATED.
- 5. FABRICATED STRUCTURAL STEEL:

 PROVIDE STRUCTURAL STEEL CONFORMING TO AASHTO M270 GRADE 36
 ASTM A709, GRADE 36 UNLESS OTHERWISE NOTED.

 GALVANIZE STEEL ANGLES AND HARDWARE IN ACCORDANCE WITH
 PUBLICATION 408, SECTION 1105. 02(s).

 REPAIR DAMAGED GALVANIZING IN ACCORDANCE WITH PUBLICATION 408,
 SECTION 1105.02(s)2.
- 6. ANCHOR BOLTS, NUTS, AND WASHERS:

 PROVIDE ANCHOR BOLTS CONFORMING TO ASTM F1554, GRADE 36 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c)3.

 PROVIDE HEAVY HEX NUTS CONFORMING TO ASTM A563A IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c)3g.

 PROVIDE WASHERS CONFORMING TO ASTM F436 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c)3b.

 GALVANIZE ALL ANCHOR BOLTS AND HARDWARE IN ACCORDANCE WITH PUBLICATION 408,
- 7. BOLTS, NUTS AND WASHERS:

 PROVIDE BOLTS CONFORMING TO ASTM A 307 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(c) 1.

 PROVIDE HEAVY HEX NUTS CONFORMING TO ASTM A 563 IN ACCORDANCE WITH

 - PUBLICATION 408, SECTION 1105.02(c) 1a.

 PROVIDE WASHERS CONFORMING TO ASTM F436 IN ACCORDANCE WITH

 - PUBLICATION 408, SECTION 1105.02(c) 2b.

 GALVANIZE ALL BOLTS AND HARDWARE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1105.02(s).
- 8. STEEL CABLES AND ACCESSORIES:

 PROVIDE 36" 7 × 19 STAINLESS STEEL (302 OR 304) FLEXIBLE WIRE ROPE (AIRCRAFT CABLE) IN ACCORDANCE WITH MIL-W-83420. MINIMUM BREAKING STRENGTH EQUALS 12 KIP.

 PROVIDE 56" OUTSIDE DIAMETER STAINLESS STEEL (302 OR 304) INTERNALLY THREADED TERMINALS SWAGED TO CABLE IN ACCORDANCE WITH MIL-T-6117.

 PROVIDE 1/2" 13 UNC STAINLESS STEEL (304) HEX HEAD CAP SCREWS. PROVIDE LENGTH AS REQUIRED.

 PROVIDE 1/6" INSIDE DIAMETER × 13/4" OUTSIDE DIAMETER STAINLESS STEEL (304) WASHERS UNDER 3/6" INSIDE DIAMETER X 1" OUTSIDE DIAMETER STAINLESS STEEL (304) WASHERS.

 PROVIDE PVC PIPE (SCHEDULE 40) CONFORMING TO ASTM D1785 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1101.09(b) 1.
- 9. PLAIN NEOPRENE BEARING PADS:

 PROVIDE PLAIN NEOPRENE PADS WITH A DUROMETER HARDNESS OF 50 (+/-) 5
 IN ACCORDANCE WITH PUBLICATION 408, SECTION 1113.02.
- 10. CLOSED CELL NEOPRENE SPONGE:

 PROVIDE CLOSED CELL NEOPRENE SPONGE IN ACCORDANCE WITH PUBLICATION 408, SECTION 1085.2(m).
- 11. NON-SHRINK GROUT:

 ◆ PROVIDE NON-SHRINK GROUT IN ACCORDANCE WITH PUBLICATION 408, SECTION 1080.2(c).

 ◆ PACK GROUT INTO PLACE. DO NOT POUR OR INJECT GROUT.

 ◆ NON-SHRINK GROUT TO MATCH FINAL COLOR OF PANEL.
- 12. CAULKING COMPOUND:

 PROVIDE CAULKING COMPOUND IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.8(b).

 CAULKING COMPOUND TO MATCH FINAL COLOR OF PANEL.

- PROVIDE JOINT SEALING MATERIAL IN ACCORDANCE WITH PUBLICATION 408, SECTION 705.4(a).

 JOINT SEALING MATERIAL TO MATCH FINAL COLOR OF PANEL.
- 14. ANTIGRAFFITI COATING:

 APPLY ANTIGRAFFITI COATING IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.
- 15. PENETRATING CONCRETE STAIN:
 - APPLY STAIN IN ACCORDANCE WITH CONTRACT DRAWINGS AND SPECIAL PROVISIONS.

CONSTRUCTION AND INSTALLATION PROCEDURES

- ONSTRUCT EMBANKMENTS AND/OR CUT EXISTING GRADE TO THE TOP OF FOOTING ELEVATIONS. EXCAVATE FOR FOOTING CONSTRUCTION. IF FILL IS REQUIRED, PLACE COMPACTED FILL MATERIAL TO THE TOP OF FOOTING ELEVATION, IN ACCORDANCE WITH CONTRACT DOCUMENTS, AND THAN EXCAVATE FOR FOOTING CONSTRUCTION. FILL MATERIAL MAY NEED TO BE IN PLACE A MINIMUM TIME DURATION PRIOR TO EXCAVATION AS REQUIRED
- (2) CONSTRUCT FOOTING. FOOTING MAY BE POURED NEXT TO EXCAVATION.
- ② CONSTRUCT FOOTING. FOOTING MAY BE POURED NEXT TO EXCAVATION.

 ③ PANEL INSTALLATION:

 A. CHECK TOP OF FOOTING FOR SMOOTHNESS. GRIND IF NECESSARY SO THAT DISCREPANCIES CAN BE ACCOMMODATED WITH NON-SHRINK GROUT.

 FLATNESS TOLERANCES: ¼ " WITHIN PANEL LENGTH

 B. IMMEDIATELY PRIOR TO PANEL INSTALLATION, PLACE A LAYER OF NON-SHRINK GROUT TO PROVIDE FULL BEARING UNDER THE PANELS. RETOOL GROUT AFTER PANEL HAS BEEN SET.

 GROUT THICKNESS: ¼ " +/- ½"

 C. PLACE FIRST PANEL ONTO THE FOOTING AND INSTALL TEMPORARY BRACING, IF REQUIRED. ADJUST PANEL UNTIL ALL FACES ARE PLUMB.

 D. SET THE SECOND PANEL, MATING THE BALL AND SOCKET EDGES TOGETHER. ADJUST PANEL UNTIL ALL FACES ARE LEVEL AND PLUMB. INSTALL CAP SCREWS, WITH WASHERS, A MINIMUM OF SIX FULL TURNS INTO THE TERMINAL TO ACHIEVE DESIGN STRENGTH. ENSURE CABLE IS NOT OVER TIGHTEND. CABLE TO BE TAUT, NOT STRESSED. INSTALL REMAINING CABLE TIES AS REQUIRED.

 IF THE CABLE TIE IS NOT TAUT OR IF SIX FULL TURNS CANNOT BE MADE, BACK OUT SCREWS AND REPLACE WITH A DIFFERENT LENGTH SCREW.

 E. CONTINUE SETTING BOTTOM PANELS.

 F. STACKED PANELS: PLACE STACKED PANELS IN A STAIR STEP PATTERN WITH THE LOWER COURSE LEADING. THE FIRST PANEL PLACED ON AN UPPER COURSE MUST BE BRACED WITH A SECOND CRANE. THE TONGUE AND GROOVE PANEL EDGES DO NOT SERVE AS A SHEAR KEY.

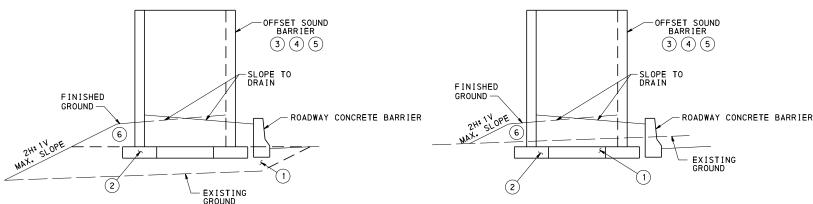
 GLUE THE NEOPRENE PADS AND CLOSED CELL NEOPRENE SPONGE TO THE TOP OF THE LOWER COURSE PANEL WITH AN APPROVED ADHESIVE.

 SET THE FIRST PANEL ON THE SECOND COURSE IN PLACE ALIGNED WITH THE PANEL BELOW AND ADJUST UNTIL ALL FACES ARE LEVEL AND PLUMB.

 SET THE SECOND PANEL ON THE SECOND COURSE, MATING THE BALL AND SOCKET EDGES TOGETHER AND ADJUST UNTIL ALL FACES ARE LEVEL AND PLUMB.

 INSTALL CABLE TIES, AS INDICATED IN NOTE D, BEFORE RELEASING PANEL

 - INSTALL CABLE TIES, AS INDICATED IN NOTE D, BEFORE RELEASING PANEL INSTALL CABLE TIES, AS INDICATED IN NOTE D, BEFORE RELEASING FROM CRANE.
 ADDITIONAL COURSES: REPEAT STEPS SHOWN ABOVE.
 G. AFTER 2 PANELS ARE SET IN A COURSE THEY ARE FREE STANDING.
 H. PROCEED SEQUENTIALLY SETTING ALL PANELS FOR THE LENGTH OF THE WALL.
 I. FILL ALL LIFTING INSERTS WITH NON-SHRINK GROUT.
- (4) SEAL ALL OPEN JOINTS, HORIZONTAL JOINTS BETWEEN PANELS, AND OPENINGS IN THE PVC PIPES WITH JOINT SEALING MATERIAL/CAULKING COMPOUND (COLOR TO MATCH PANEL).
- (5) APPLY ANTIGRAFFITI COATING AND/OR PENETRATING CONCRETE STAIN, IF SPECIFIED.
- © COMPLETE BACKFILL OPERATION ON BOTH SIDES OF THE WALL. MAXIMUM FILL DIFFERENTIAL BETWEEN SIDES OF PANELS IS LIMITED TO 2'-0" UNLESS OTHERWISE SPECIFIED.



SEQUENCE OF CONSTRUCTION (FILL)

SEQUENCE OF CONSTRUCTION (CUT)

ARCHITECTURAL SURFACE TREATMENTS

- 1. THE AVERAGE ARCHITECTURAL SURFACE TREATMENT THICKNESS, PER SIDE OF PANEL, IS PERMITTED TO VARY FROM O TO 1½ INCH, BUT THE TOTAL AVERAGE ARCHITECTURAL SURFACE TREATMENT, ON BOTH SIDES OF THE PANEL, MUST NOT BE GREATER THAN 1½ INCH UNLESS OTHERWISE INDICATED ON THE CONTRACT DRAWINGS.
- 2. STAMPED FINISHES MAY BE PERMITTED IF ACCEPTED BY THE DISTRICT BRIDGE ENGINEER.
- REFER TO PUBLICATION 408, SECTION 1086.3 AND/OR THE CONTRACT DOCUMENTS FOR ARCHITECTURAL SURFACE TOLERANCES.
- 4. REFER TO CONTRACT DOCUMENTS FOR ADDITIONAL INFORMATION.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD OFFSET SOUND BARRIER WALLS GENERAL NOTES - 2

RECOMMENDED JAN. 31, 2019 Thomas P. Mariora

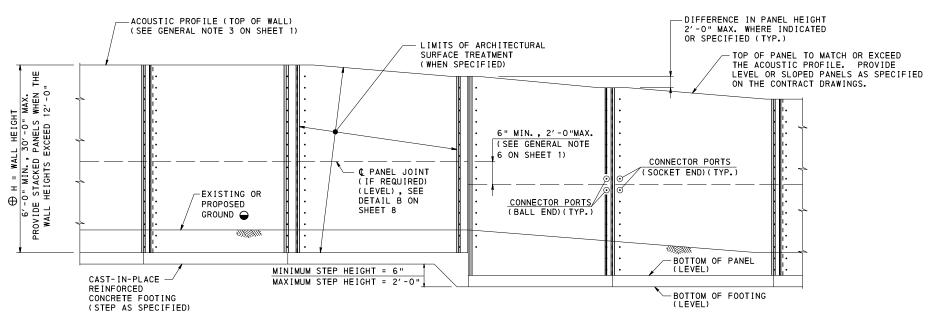
RECOMMENDED JAN. 31, 2019 Allin Houtre ACTING DIR. BUR. OF PROJECT DELIVERY BC-780M

SHEET 2 OF 8

⊕ L = LENGTH OF 4-PANEL UNIT REINFORCED CONCRETE FOOTING - LAYOUT CONTROL WORKING POINT (TYP.) STANDARD PANEL OFFSET SOUND BARRIER ⊕ D/2 CONTROL LINE 0 /2 (H) D 15'-0" BEND TO BEND (TYP.) \oplus - O ANGLE O LA C PANEL/C FOOTING

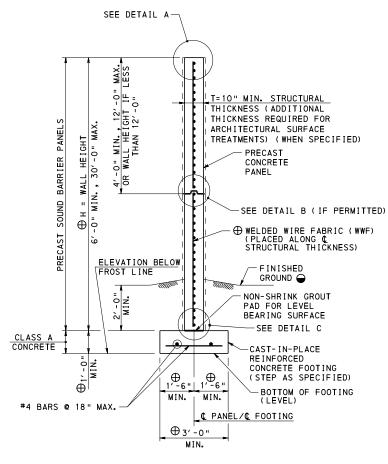
PLAN - OFFSET SOUND BARRIER

(END PANEL NOT SHOWN)



ELEVATION - OFFSET SOUND BARRIER

(END PANEL NOT SHOWN)



SECTION A-A

LEGEND:

- ⊕ AS REQUIRED BY DESIGN REFER TO CONTRACT
- ← GRADE GROUND TO DRAIN WATER AWAY FROM WALL. FILL HEIGHT ON EACH SIDE OF WALL TO BE WITHIN 2′-O" DIFFERENCE.

NOTES:

- FOR ADDITIONAL INFORMATION REFER TO NOTES ON SHEETS 1 AND 2.
 FOR DETAILS A, B, AND C REFER TO
- SHEET 8.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD OFFSET SOUND BARRIER WALLS GEOMETRY AND LAYOUT

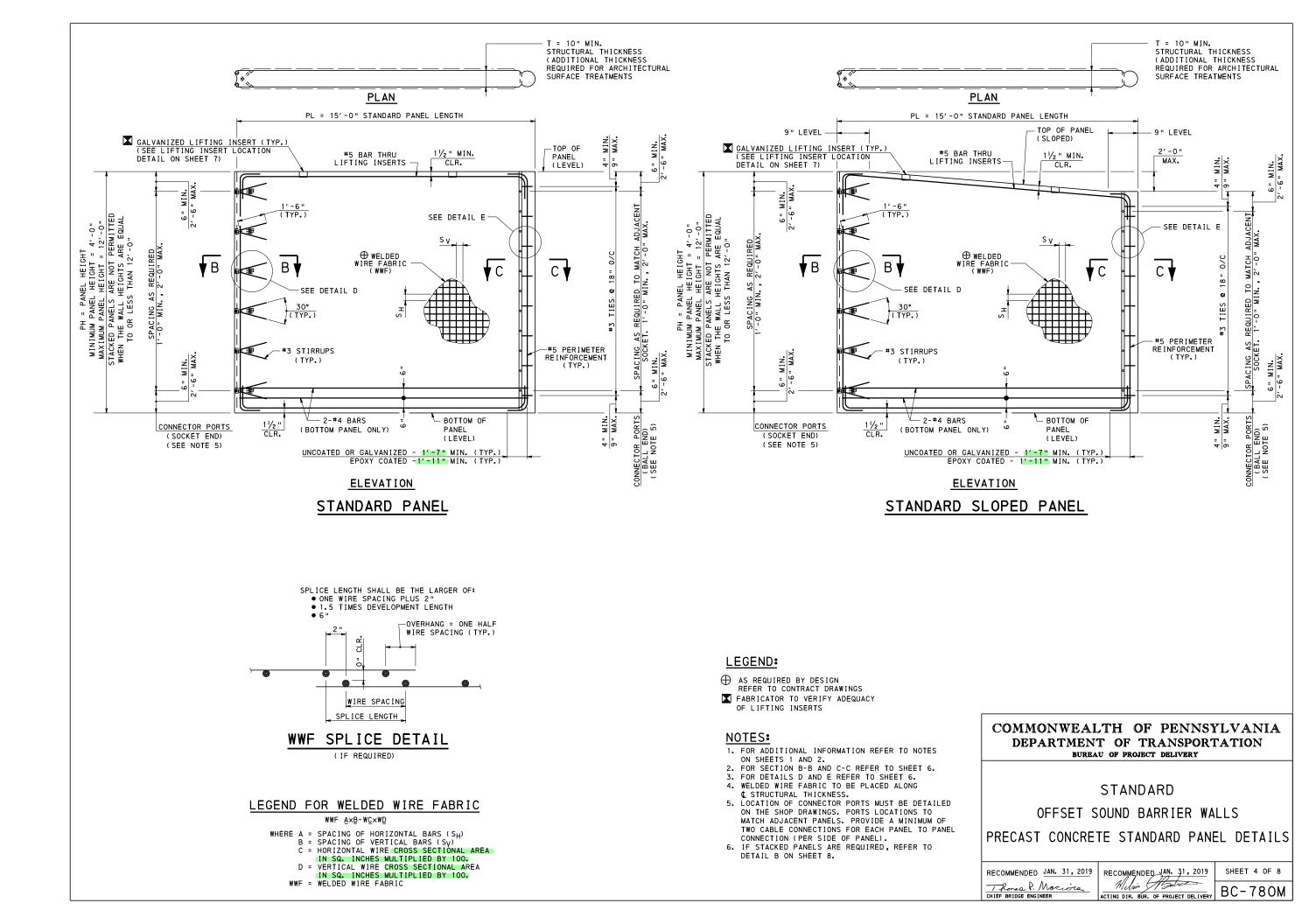
RECOMMENDED JAN. 31, 2019 Thoras P. Macroca CHIEF BRIDGE ENGINEER

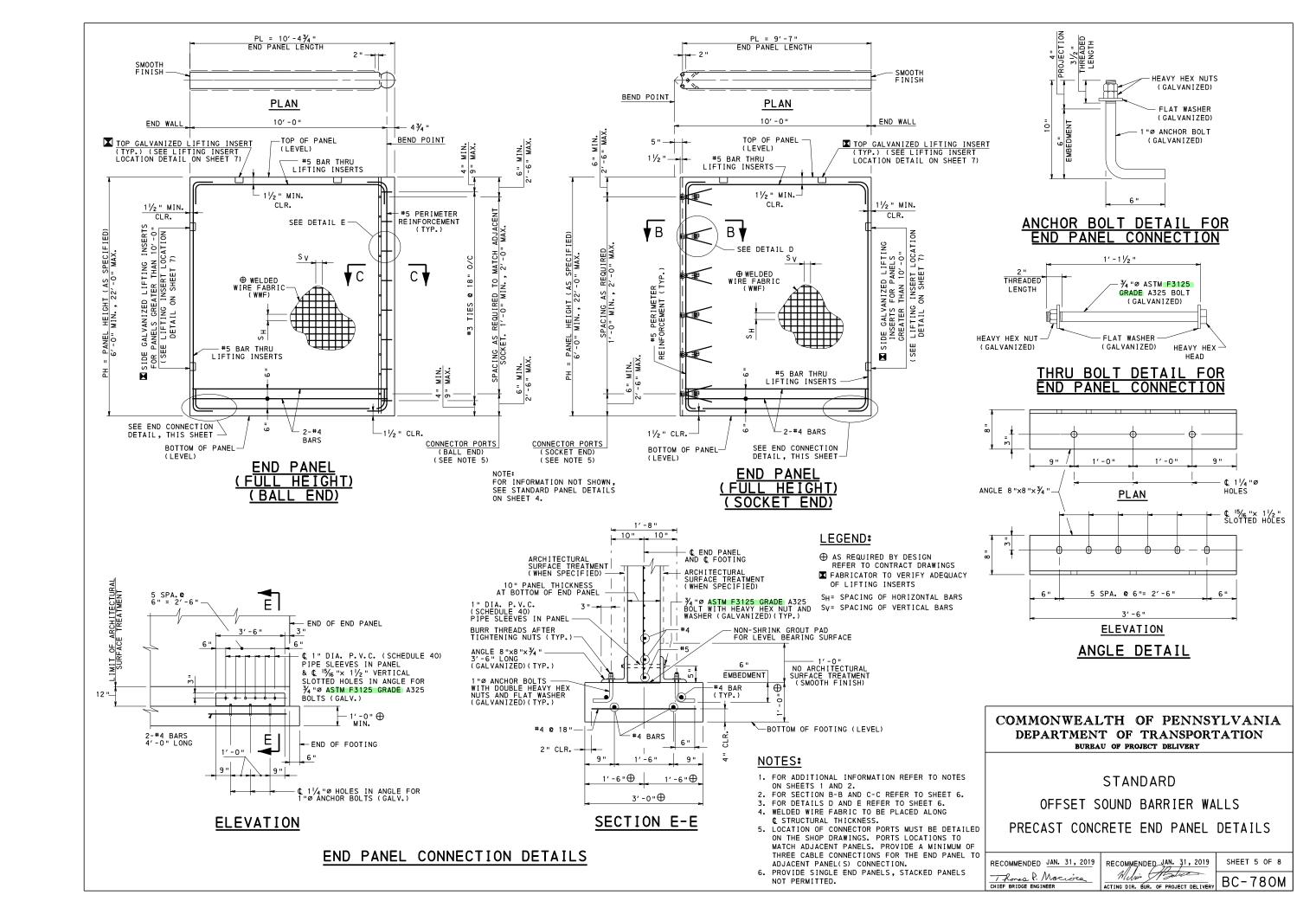
RECOMMENDED JAN. 31, 2019

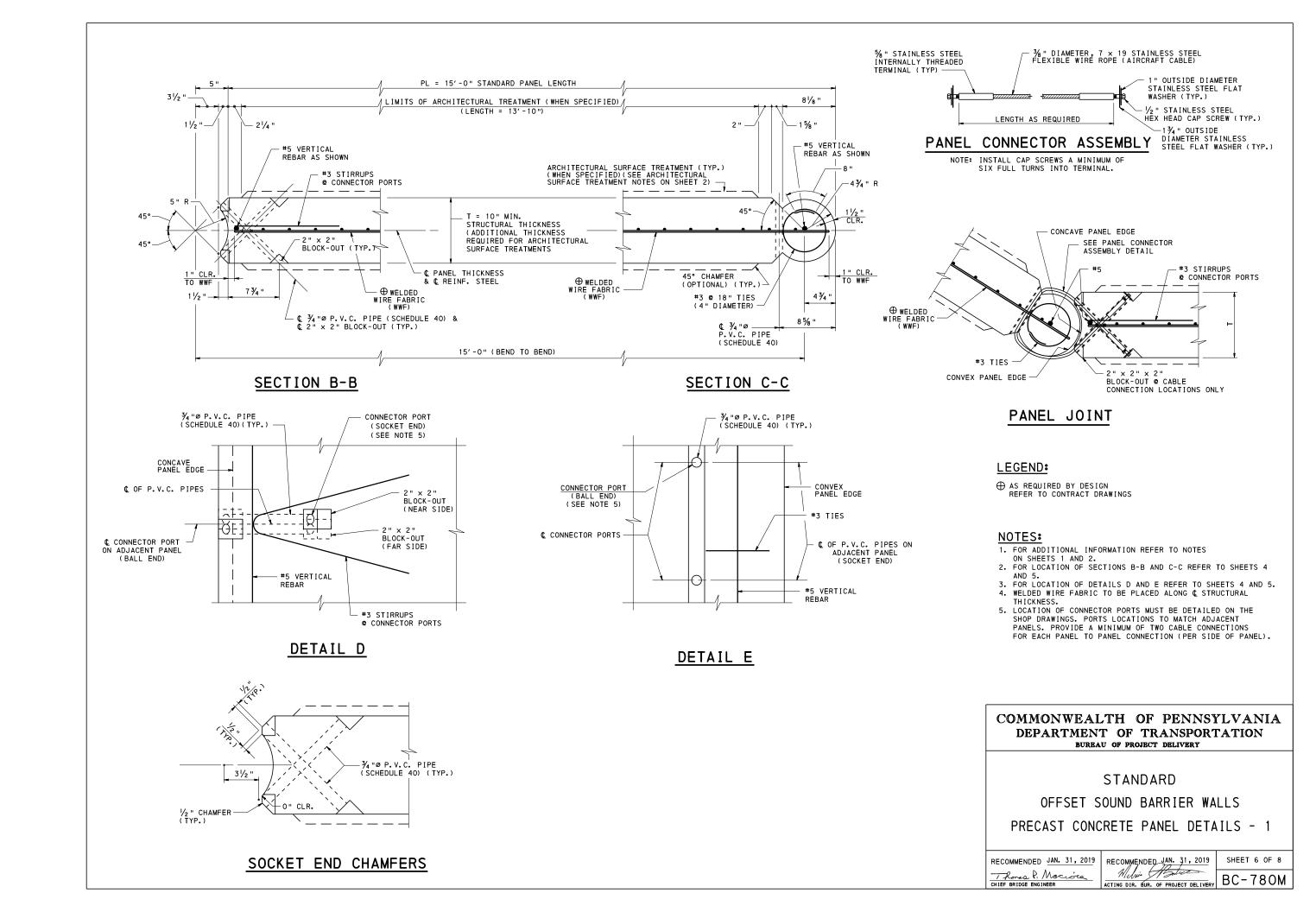
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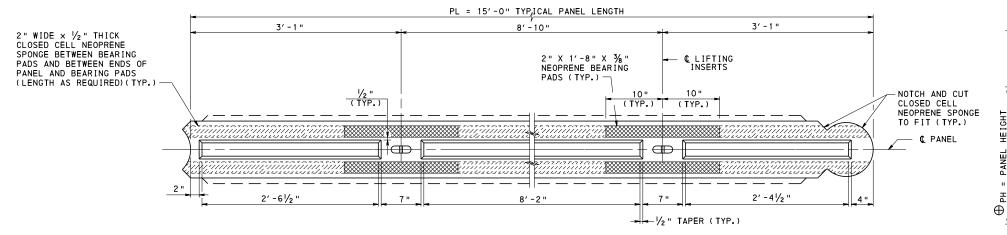
SHEET 3 OF 8

ACTING DIR. BUR. OF PROJECT DELIVERY BC-780M

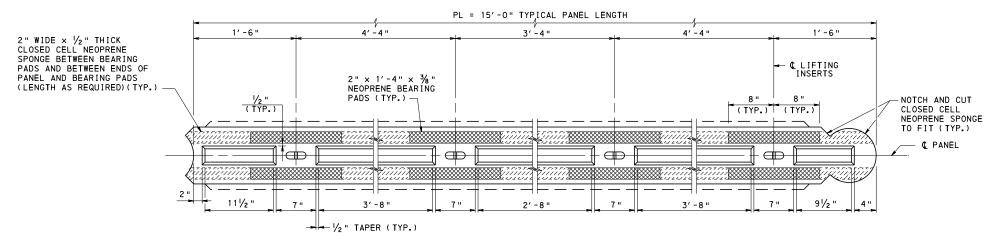






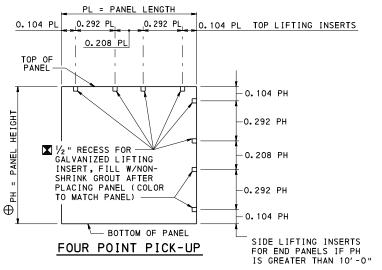


TOP OF PANEL AT JOINT PLAN VIEW TWO POINT PICK-UP



TOP OF PANEL AT JOINT PLAN VIEW FOUR POINT PICK-UP

PL = PANEL LENGTH 0.207 PL 0.586 PL 0.207 PL TOP LIFTING INSERTS TOP OF 0.207 PH ✓/2" RECESS FOR GALVANIZED LIFTING INSERT, FILL W/NONSHRINK GROUT AFTER PLACING PANEL (COLOR TO MATCH PANEL) \oplus -0.207 PH BOTTOM OF PANEL SIDE LIFTING INSERTS FOR END PANELS IF PH TWO POINT PICK-UP IS GREATER THAN 10'-0"



PRECAST CONCRETE PANEL LIFTING INSERT LOCATION DETAIL

NOTE: LIFTING INSERTS FOR SLOPED END PANELS ARE TO BE LOCATED BY THE FABRICATOR BASED ON HOW THE PANEL IS FABRICATED AND ERECTED.

LEGEND:

- FABRICATOR TO VERIFY ADEQUACY OF LIFTING INSERTS
- AS REQUIRED BY DESIGN REFER TO CONTRACT DRAWINGS

NOTES:

- 1. FOR ADDITIONAL INFORMATION REFER
- TO NOTES ON SHEETS 1 AND 2. 2. GLUE THE CLOSED CELL NEOPRENE SPONGE
- AND BEARING PADS TO PANEL WITH AN APPROVED ADHESIVE.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD OFFSET SOUND BARRIER WALLS PRECAST CONCRETE PANEL DETAILS - 2

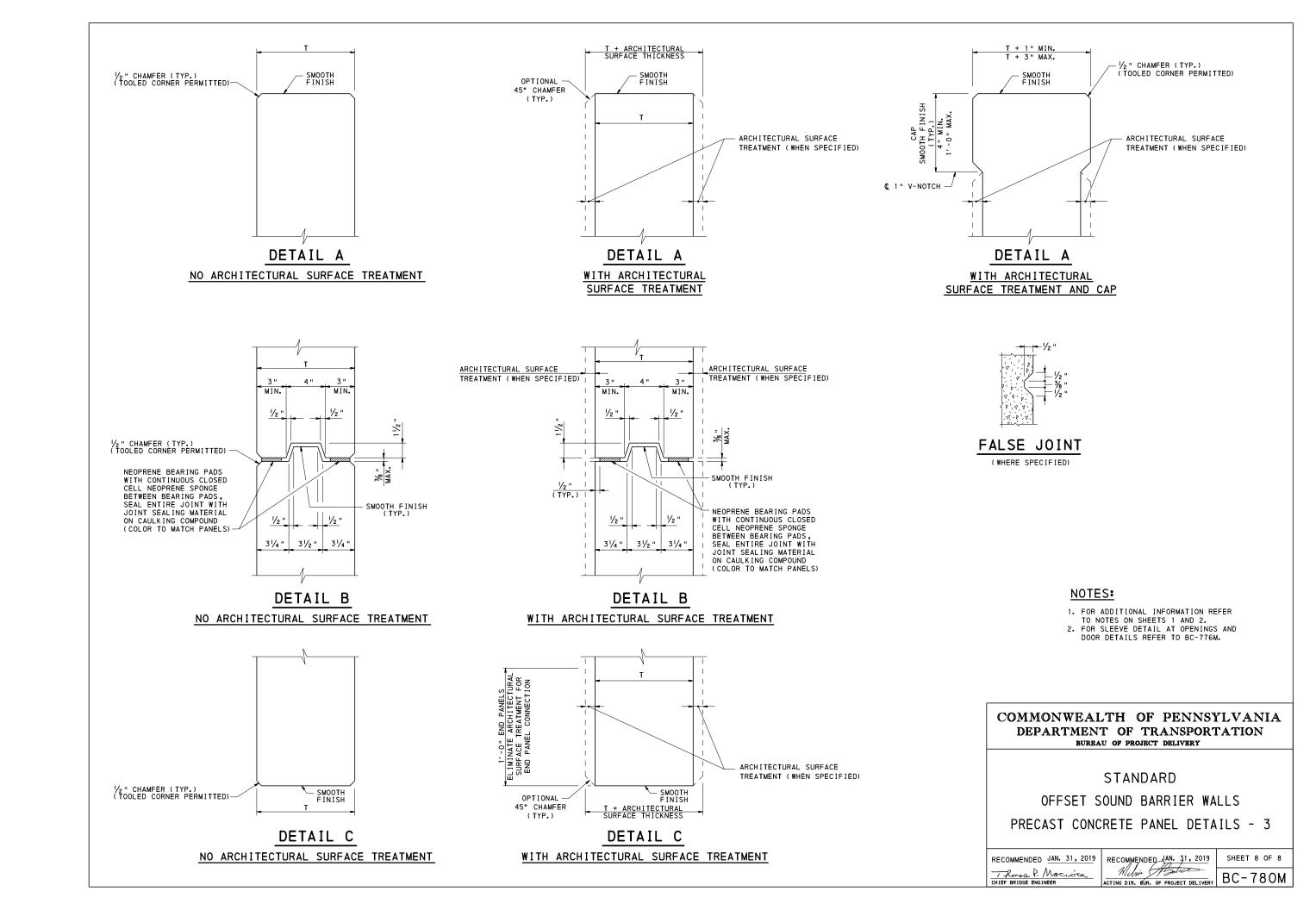
RECOMMENDED JAN. 31, 2019 Thoras P. Macroca CHIEF BRIDGE ENGINEER

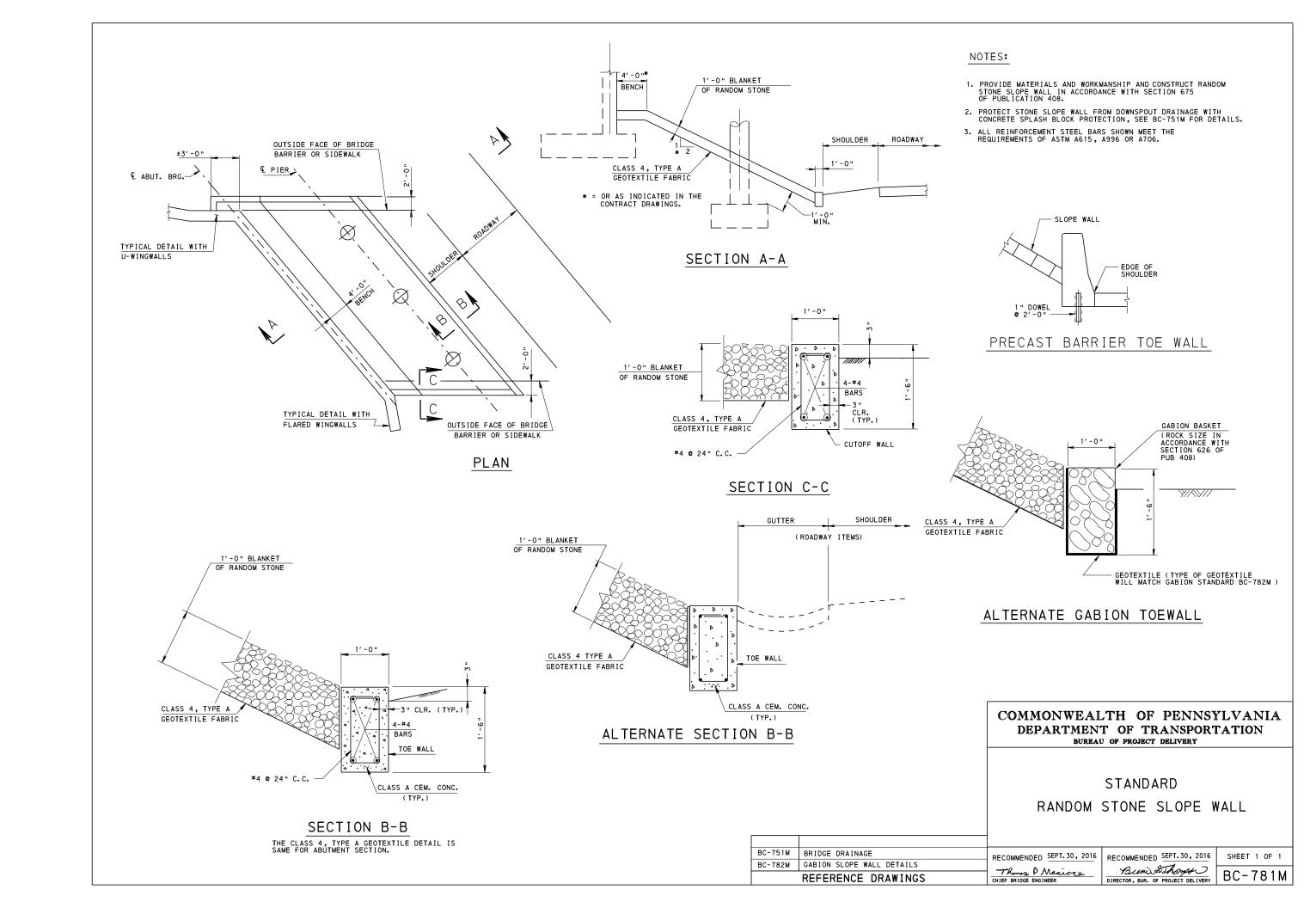
RECOMMENDED JAN. 31, 2019

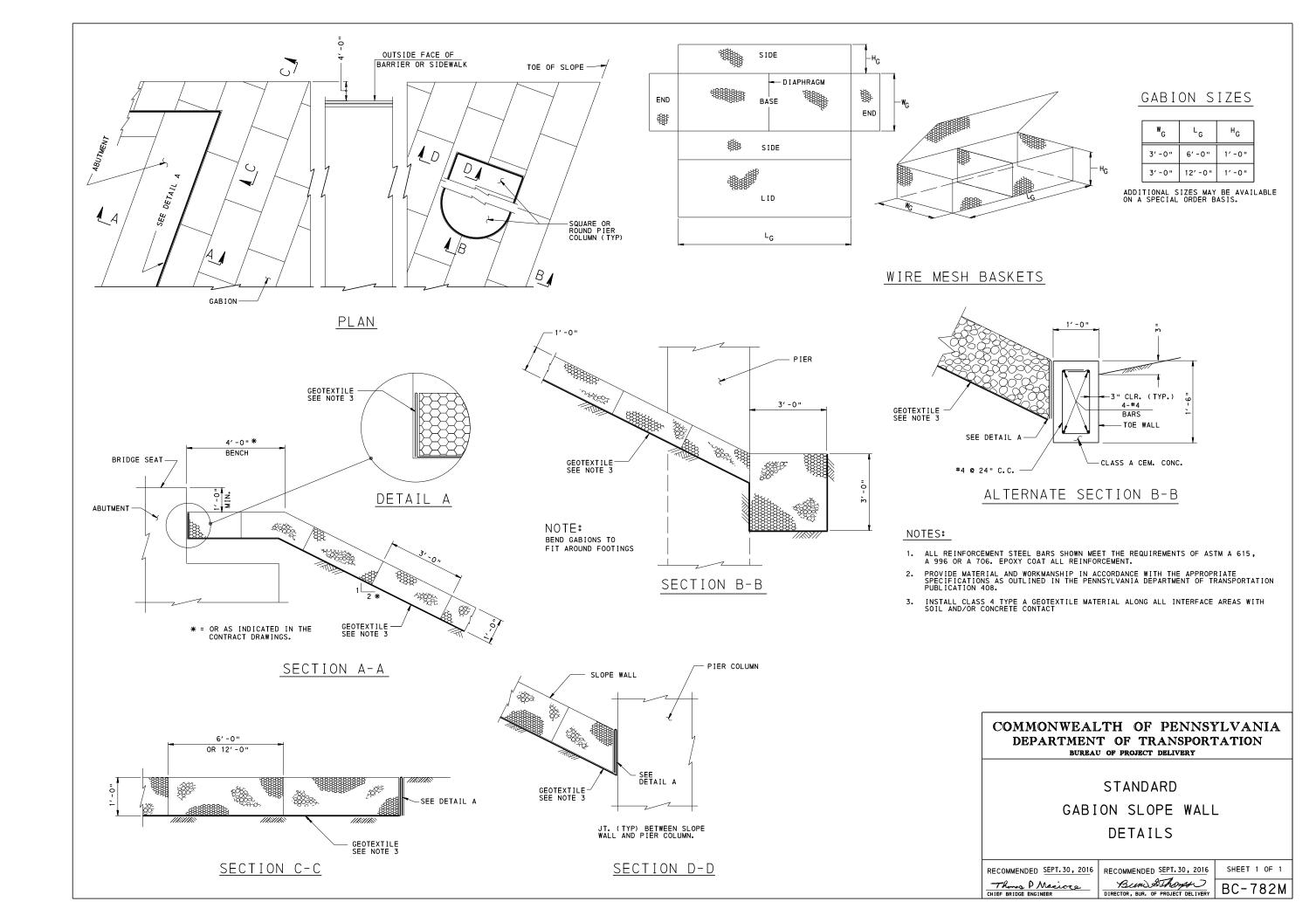
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ACTING DIR. BUR. OF PROJECT DELIVERY BC-780M

SHEET 7 OF 8







- ¾" MIN. 3/4" DEEP SAW-CUT AT 3/4" DEEP SAW-CUT AT 60° ANGLE AROUND PERIMETER OF PATCH (TYP.) 60° ANGLE AROUND PERIMETER VARIES - TOP OF REBAR BOTTOM OF REBAR _ 1/2 " CLEAR MIN. IF < ½", USE TYPE 2 REPAIR DECK MAY OR MAY NOT HAVE STAY-IN-PLACE FORMS

DECK REPAIR TYPE 2

DECK REPAIR TYPE 3

3/4" DEEP SAW-CUT AT
60° ANGLE AROUND PERIMETER

ON DECKS WITHOUT METAL

FORMS, DO NOT EXCEED 459 "BREAKBACK" OF CONCRETE.

MAX.

DECK REPAIR TYPE 2 NOTES:

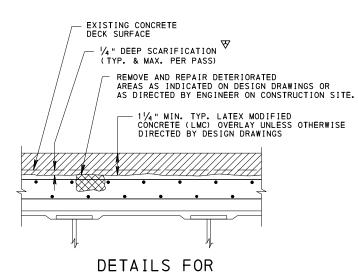
1. DECK REPAIR TYPE 3 MAY BE REQUIRED WITHIN THE AREA OF A DECK REPAIR TYPE 2.

DECK REPAIR TYPE 1 NOTES:

DECK REPAIR TYPE 1

- 1. BRIDGE DECKS WITH A SINGLE LAYER OF REINFORCEMENT ARE SIMILAR (ADJ. BOX BEAMS).
- 2. DECK REPAIR TYPE 2 OR TYPE 3 MAY BE REQUIRED WITHIN THE AREA OF A DECK REPAIR TYPE 1.

CHANGE 2



LATEX MODIFIED CONCRETE OVERLAY

(TRANSVERSE SECTION) ₩ IF DEEPER SCARIFICATION IS NEEDED, SCARIFY IN MULTIPLE PASSES.

* 24'-0" TRANSITION (PARALLEL TO CENTERLINE) TYPICAL DECK OVERLAY BRIDGE EXPANSION JOINT OR END OF 6'-0' 6'-0" 6'-0" 6'-0" BRIDGE APPROACH SLAB (SEE NOTE 2 L.M.C. OVERLAY TOP OF EXISTING DECK OR BRIDGE APPROACH SLAB SCARIFICATION

VERTICAL TRANSITION DETAIL FOR 1 1/4 " LATEX MODIFIED CONCRETE BRIDGE DECK OVERLAY

(LONGITUDINAL SECTION) (ADJUST SCARIFICATION FOR OVERLAY THICKNESS OTHER THAN 1 $\frac{1}{4}$ ")

*TRANSITION LENGTH MORE THAN 24'-0" REQUIRES DISTRICT BRIDGE ENGINEER'S APPROVAL.

- 1. PROVIDE THE TRANSITION ENTIRELY ON THE BRIDGE APPROACH SLAB, IF PRESENT.
- 2. IF A FLEXIBLE APPROACH PAVEMENT (BITUMINOUS) EXISTS, PROVIDE ADDITIONAL BITUMINOUS WEARING SURFACE FOR A SMOOTH TRANSITION TO THE BRIDGE AND MAINTAIN CONSTANT DEPTH ON THE LATEX OVERLAY.

DECK REPAIRS AND LATEX MODIFIED CONCRETE OVERLAY

FOR DECK TOP REINFORCEMENT MAT: TRANSVERSE BARS SHOWN ON TOP, SIMILAR WHEN LONGITUDINAL BARS ON TOP.

DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD REINFORCED CONCRETE REPAIR BRIDGE DECKS

BC-736M REINFORCEMENT BAR FABRICATION DETAILS REFERENCE DRAWINGS

RECOMMENDED JAN. 31, 2019 Thomas P. Macioca

THE DEPARTMENT.

RECOMMENDED JAN. 31, 2019 Allin Holis ACTING DIR. BUR. OF PROJECT DELIVERY

SHEET 1 OF 4 BC-783M

COMMONWEALTH OF PENNSYLVANIA

REMOVE DETERIORATED CONCRETE.

EXERCISE CAUTION NOT TO DAMAGE

FOR DECKS WITH PERMANENT METAL DECK FORMS, REPLACE IN KIND IF DAMAGED, OTHERWISE PROVIDE REMOVABLE FORMS.

PROVIDE MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH PUBLICATION 408.

PROVIDE REINFORCEMENT BARS CONFORMING TO THE REQUIREMENTS OF ASTM A 615, A 616 OR A 706. 3. PROVIDE LAP SPLICE LENGTHS AND EMBEDMENT LENGTHS IN ACCORDANCE WITH BC-736M.

CLEAN ALL EXISTING REINFORCEMENT BARS TO BE RETAINED WITH A WIRE BRUSH OR SAND BLAST, STRAIGHTEN AND COAT WITH AN APPROVED EPOXY PAINT FOR EPOXY COATED EXISTING REINFORCEMENT STEEL OR NEAT CEMENT FOR (NON EPOXY COATED) EXISTING REINFORCEMENT STEEL.

PROVIDE EPOXY COATED REBARS AS REQUIRED. REMOVE AND REPLACE IN KIND (EXCEPT ALWAYS USE EPOXY COATED) ALL PORTIONS OF DAMAGED OR HEAVILY CORRODED REINFORCEMENT BARS BY SATISFACTORILY SPLICING TO THE REMAINING REINFORCEMENT BARS.

CONSTRUCTION, EQUIPMENT, SURFACE PREPARATION AND PATCHING MATERIAL FOR CONCRETE BRIDGE DECK REPAIR MUST CONFORM TO SECTION 1040 OF PUB. 408.

CONSTRUCTION, EQUIPMENT, SURFACE PREPARATION, PLACING AND FINISHING FOR LATEX MODIFIED CONCRETE OVERLAY MUST CONFORM TO SECTION 1042 OF PUB. 408.

TYPE OF REPAIRS DEPICTED ON THIS STANDARD ASSUME THAT THE STRUCTURAL INTEGRITY OF THE DECK IS NOT COMPROMISED BY THE EXTENT OF THE REPAIRS.

IF BRIDGE BEAMS ARE DAMAGED DURING DECK REPAIR, BEAMS MUST BE REPAIRED OR REPLACED AT NO EXPENSE TO

LEGEND

T = THICKNESS OF CONCRETE DECK SLAB.

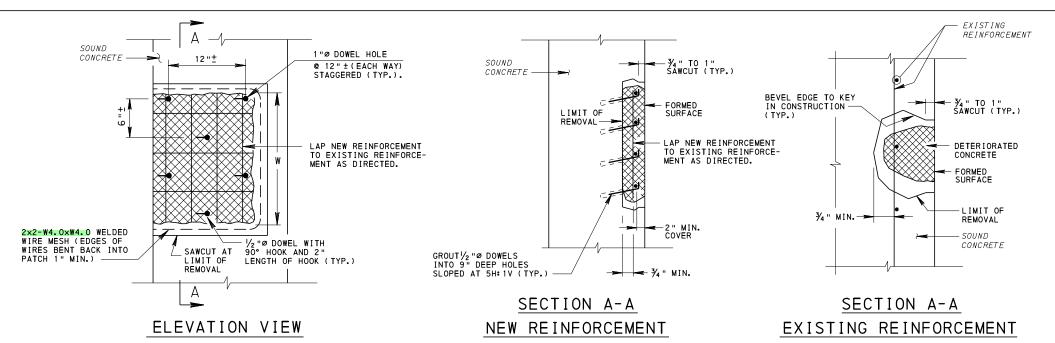
REMOVE DETERIORATED CONCRETE.

APPLY AN EPOXY BONDING COMPOUND CONFORMING TO THE REQUIREMENTS OF SECTION 1040.3(e)1 OF PUB. 408.

METAL FORMS (IF PRESENT) AND

BRIDGE BEAMS

GENERAL NOTES



FXISTING

BEVELED SAWCUT 3/4

REINFORCEMENT

CONCRETE REPAIR TYPE 2

NOTE: REPAIR TYPE 2 IS USED WHEN DEPTH OF DETERIORATED CONCRETE IS GREATER THAN ¾ " AND EXISTING REINFORCEMENT SPACED ≤ 12" ON CENTERS.

REINFORCED CONCRETE REPAIR TYPE 1 NOTES:

- SQUARE OFF DETERIORATED CONCRETE TO SOUND CONCRETE WITH A SAWCUT OF $\frac{3}{4}\,^{\rm H}$ MAXIMUM.
- REMOVE ALL LOOSE AND DELAMINATED CONCRETE TO PROVIDE A SOUND BOND BETWEEN EXISTING CONCRETE AND PATCHING MATERIAL.
- APPLY A RAPID HARDENING CONCRETE PATCHING MATERIAL FROM A MANUFACTURER LISTED IN BULLETIN 15 UNDER MISCELLANEOUS
 POLYMER MODIFIED AND SPECIAL CEMENTS, MORTARS AND CONCRETES, IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
- 4. CONCRETE REPAIRS INDICATED ARE PAYABLE UNDER 1040.3(f) 2.

LEGEND

REMOVE DETERIORATED CONCRETE.

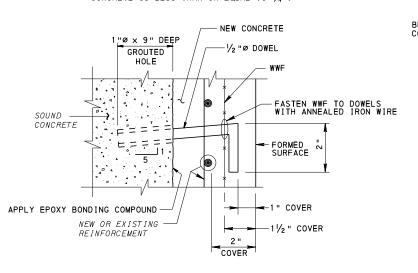
MAX. (TYP. SAWCUT AT LIMIT OF REMOVAL -DETERIORATED CONCRETE DETERIORATED LIMIT OF REMOVAL - SOUND CONCRETE **ELEVATION VIEW** SECTION B-B

В

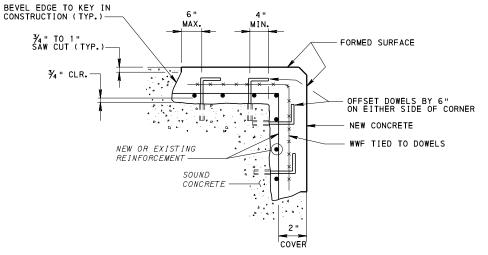
SOUND CONCRETE -

CONCRETE REPAIR TYPE

NOTE: REPAIR TYPE 1 IS USED WHEN DEPTH OF DETERIORATED CONCRETE IS LESS THAN OR EQUAL TO 3/4 ".



TYPICAL DOWEL DETAIL



TYPICAL CORNER REPAIR DETAIL

REINFORCED CONCRETE REPAIR TYPE 2 NOTES:

LIMIT OF

REMOVAL:

SOUND

- SQUARE OFF DETERIORATED CONCRETE TO SOUND CONCRETE WITH A SAWCUT OF $\frac{1}{4}$ " MINIMUM TO 1" MAXIMUM BUT NOT TO THE DEPTH OF THE REINFORCEMENT STEEL. BACK BEVEL EDGE BEYOND SAWCUT.
- USE HAND TOOLS TO REMOVE ALL LOOSE AND DELAMINATED CONCRETE THAT PROVIDES A SOUND BOND BETWEEN EXISTING CONCRETE AND NEW CONCRETE. PNEUMATIC HAMMERS WITH IMPACT RATINGS OF 30 FT/LB OR LESS MAY BE USED IF REQUIRED.
- IF DETERIORATED CONCRETE EXTENDS BEYOND THE PRIMARY REINFORCEMENT, REMOVE THE CONCRETE TO AT LEAST $\frac{7}{4}$ " BEHIND THE REINFORCEMENT.

-2" MIN. COVER

CONCRETE

FORMED SURFACE

- EXISTING

REINFORCEMENT

SAWCUT

SECTION A-A

BLISTER DETAIL NOTE: CONCRETE REPAIR TYPE 2 DETAIL FOR AREAS WITH EXISTING REINFORCEMENT HAVING LESS THAN 2" OF COVER.

— ¾ " ×¾ " CHAMFER (TYPICAL)

-1:1 SLOPE

- APPLY AN EPOXY BONDING COMPOUND BETWEEN THE EXISTING AND THE NEW CLASS AA CEMENT CONCRETE.
- 5. W REPRESENTS LEAST DIMENSION OF DETERIORATED CONCRETE.
- USE DOWELS ONLY WHEN W DIMENSION OF DETERIORATED CONCRETE IS GREATER THAN 2'-0" AND NEW OR EXISTING REINFORCEMENT CANNOT ADEQUATELY BE DEVELOPED BY LAPPING WITH EXISTING REINFORCEMENT.
- 7. USE A PACHOMETER TO LOCATE EXISTING REINFORCEMENT WHEN DRILLING DOWEL HOLES TO AVOID DRILLING THRU EXISTING BARS.
- AN APPROVED EPOXY ANCHORING SYSTEM IN 90° HOLES MAY REPLACE GROUT IN SLOPED HOLES. USE A 6" MINIMUM EMBEDMENT AND IN ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS.
- 9. A #4 DEFORMED REINFORCEMENT BENT "L" BAR MAY REPLACE THE $1/\!\!/_2$ "Ø DOWEL HOOK.
- ALTERNATE WIRE MESH MAY BE SUBSTITUTED FOR 2×2-W4.0×W4.0, PROVIDED WIRE SPACING DOES NOT EXCEED 4" AND AN EQUIVALENT STEEL AREA IS PROVIDED. NEW REINFORCEMENT BARS MAY BE OMITTED IF WIRE MESH STEEL AREA EXCEEDS EXISTING REINFORCEMENT.
- 11. CLEAN EXISTING REINFORCEMENT BY MECHANICAL MEANS.
- LAP EQUIVALENT NEW REINFORCEMENT TO THE EXISTING REINFORCEMENT AS DIRECTED.
- 13. REINFORCEMENT BARS TO BE EPOXY COATED.
- 14. CONCRETE REPAIRS INDICATED ARE PAYABLE UNDER 1040.3(f) 2.

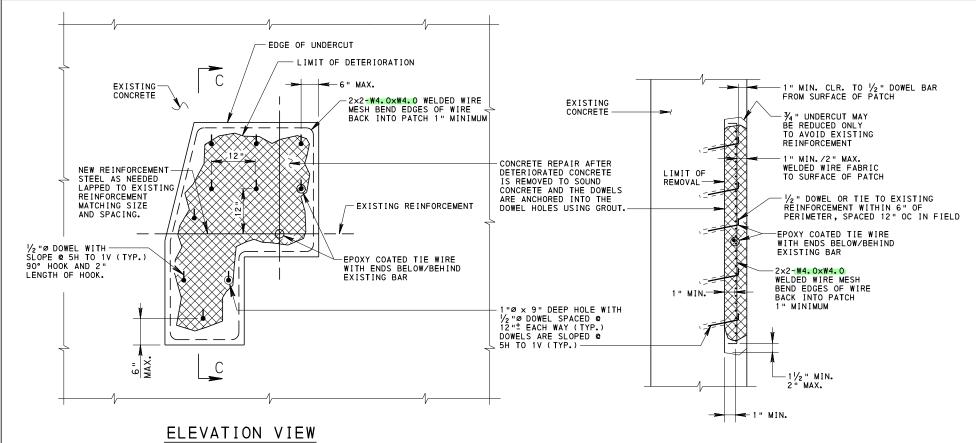
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD REINFORCED CONCRETE REPAIR

RECOMMENDED JAN. 31, 2019 Thomas P. Marioca

RECOMMENDED JAN. 31, 2019 ACTING DIR. BUR. OF PROJECT DELIVERY BC-783M

SHEET 2 OF 4



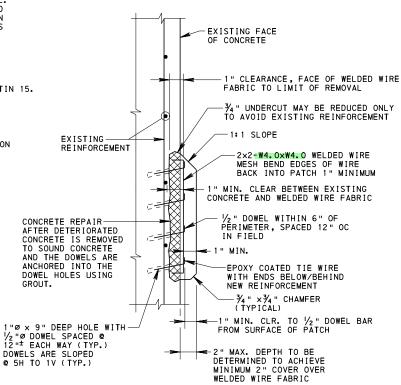
NOTE: PROVIDE EPOXY COATED WIRE TIE TO CONNECT EXISTING PROVIDE EPOXY COATED WIRE TIE TO CONNECT EXISTING REINFORCEMENT AND 2×2-W4.0×W4.0 WELDED WIRE MESH ALONG THE PERIMETER OF THE REMOVAL AREA AT A MAXIMUM SPACING OF 6" FROM THE EDGE OF THE REMOVAL. PROVIDE TIES AT 12" SPACING IN BOTH HORIZONTAL AND VERTICAL DIRECTIONS ALONG THE PERIMETER AND WITHIN THE AREA OF REMOVAL. IF EXISTING REINFORCEMENT IS SPACED AT GREATER THAN 12" SPACING OR NOT LOCATED TO PROVIDE TIE OCCATIONS AS LISTED ABOVE PROVIDE TO PROVIDE TIE LOCATIONS AS LISTED ABOVE, PROVIDE $\frac{1}{2}$ " GROUTED DOWELS AS SHOWN ON THE DRAWING TO PROVIDE TIE LOCATIONS AT THE SAME SPACINGS.

USE ONLY AN APPROVED POLYMER MODIFIED AND SPECIAL CEMENTS, MORTARS AND CONCRETES AS LISTED IN BULLETIN 15.

CONCRETE REPAIR TYPE 2A

NOTE: REPAIR TYPE 2A IS USED WHEN DEPTH OF DETERIORATION IS GREATER THAN 3/4" AND EXISTING REINFORCEMENT IS SPACED GREATER THAN 12" ON CENTERS.

SECTION C-C



SECTION C-C BLISTER DETAIL

NOTE: SHALLOW REMOVAL CONDITION IF PATCH CANNOT ENGAGE EXISTING REINFORCEMENT.

REINFORCED CONCRETE REPAIR TYPE 2A NOTES:

- SQUARE OFF DETERIORATED CONCRETE TO SOUND CONCRETE WITH A SAWCUT OF $34\,^{\circ}$ MINIMUM BUT NOT TO THE DEPTH OF THE REINFORCEMENT STEEL.
- REMOVE ALL LOOSE AND DELAMINATED CONCRETE TO PROVIDE A SOUND BOND BETWEEN EXISTING CONCRETE AND NEW CONCRETE.
- IF DETERIORATED CONCRETE EXTENDS BEYOND THE PRIMARY REINFORCEMENT, REMOVE THE CONCRETE TO AT LEAST 1" BEHIND THE REINFORCEMENT.
- APPLY AN EPOXY BONDING COMPOUND BETWEEN THE EXISTING AND THE NEW CONCRETE.
- 5. WIRE MESH MAY BE SUBSTITUTED FOR NEW REINFORCEMENT IF
- 6. CLEAN EXISTING REINFORCEMENT BY MECHANICAL MEANS.
- NEW REINFORCEMENT TO BE EPOXY COATED.
- 8. CONCRETE REPAIR TYPE 2A ARE PAYABLE AS CONCRETE REPAIRS TYPE 2.

LEGEND

- REMOVE DETERIORATED CONCRETE.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

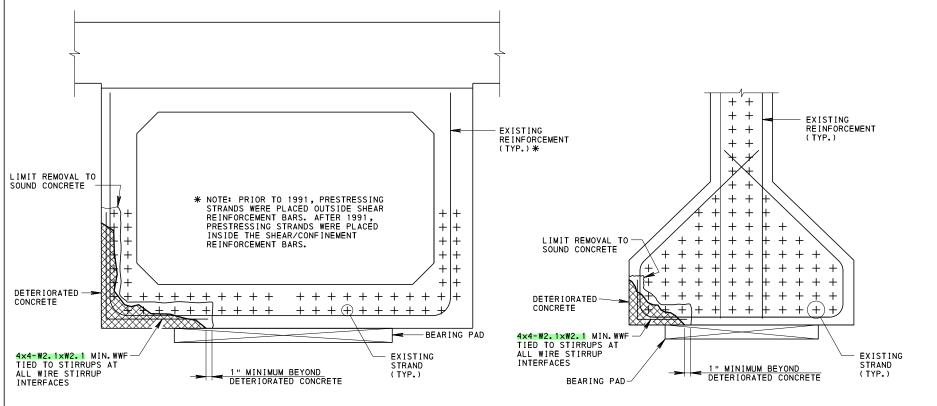
STANDARD REINFORCED CONCRETE REPAIR

RECOMMENDED JAN. 31, 2019

RECOMMENDED JAN. 31, 2019 Mww Hours

SHEET 3 OF 4

Thoras P. Marcirea CHIEF BRIDGE ENGINEER ACTING DIR. BUR. OF PROJECT DELIVERY BC-783M



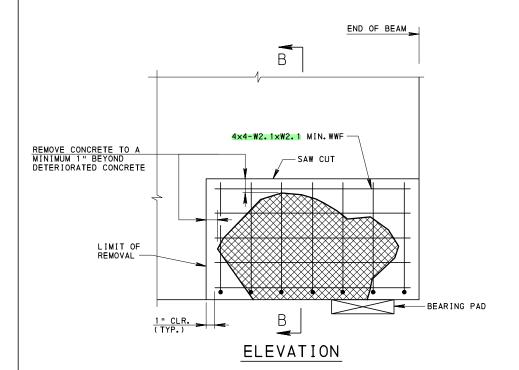
CONCRETE REPAIR - PRESTRESSED

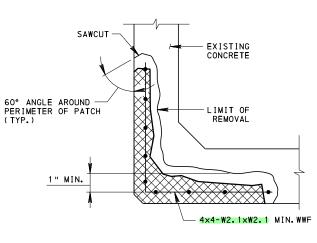
CONCRETE SPREAD BOX BEAM

(ADJACENT BOX BEAM SIMILAR)

CONCRETE REPAIR - PRESTRESSED

CONCRETE I-BEAM





SECTION B-B

CONCRETE REPAIR - PRESTRESSED

CONCRETE BOX BEAM

(PRESTRESSED CONCRETE I BEAM SIMILAR)

REINFORCED CONCRETE REPAIR PRESTRESSED CONCRETE BEAM NOTES:

- 1. REMOVE ALL LOOSE AND DELAMINATED CONCRETE TO PROVIDE A SOUND BOND BETWEEN EXISTING CONCRETE AND REPAIR MATERIAL. LIMIT REMOVAL TO A MINIMUM OF 1" BEYOND THE VISIBLE DETERIORATED AREA TO EXPOSE SOUND CONCRETE.
- 2. REMOVE DETERIORATED CONCRETE ADJACENT TO AND AROUND THE PRESTRESSING STRANDS AS REQUIRED TO EXPOSE SOUND CONCRETE. DO NOT DAMAGE PRESTRESSING STRANDS DURING CONCRETE REMOVAL. USE SURFACE PREPARATION EQUIPMENT IN ACCORDANCE WITH SECTION 1040.3(c) OF PUBLICATION 408, HOWEVER, THE WEIGHT OF PNEUMATIC HAMMERS MUST NOT EXCEED A NOMINAL 15-POUND CLASS.
- 3. SQUARE OFF DETERIORATED CONCRETE TO SOUND CONCRETE WITH A SAW CUT OR GRINDER. DEPTH OF CUT TO BE A MINIMUM OF 1/4", BUT NOT TO EXCEED 5/8" OR THE DEPTH OF THE REINFORCEMENT, WHICHEVER IS SMALLER.
- 4. CLEAN ALL EXISTING REINFORCEMENT BARS TO BE RETAINED AND PRESTRESSING STRANDS BY MECHANICAL MEANS TO NEAR WHITE APPEARANCE. COAT EXISTING REINFORCEMENT BARS AND STRANDS WITH APPROVED BONDING COMPOUND IF NO CORROSION WAS PRESENT PRIOR TO CLEANING. COAT EXISTING REINFORCEMENT BARS AND STRANDS WITH APPROVED GALVANIZED SPRAY CONTAINING A MINIMUM OF 92% ZINC WHEN CORROSION WAS PRESENT PRIOR TO CLEANING.
- 5. PROVIDE A SOUND CONCRETE SURFACE WITH EXPOSED AGGREGATE WITH A MINIMUM SURFACE PROFILE OF 1/8" OR AS REQUIRED BY REPAIR MATERIAL MANUFACTURER'S RECOMMENDATIONS.
- 6. DRILL AND INSERT 3/8" DIAMETER GALVANIZED STEEL EXPANSION ANCHOR PINS ON 4" CENTERS FOR REPAIR AREAS WITH DEPTHS GREATER THAN 3 INCHES WHEN REINFORCEMENT BARS ARE NOT PREVALENT (SPACING GREATER THAN 8"). LOCATE EXPANSION ANCHOR PINS AT MIDPOINT OF CLEAR SPACING BETWEEN PRESTRESSING STRANDS.
- 7. APPLY MECHANICAL ANCHORAGE USING GALVANIZED 4x4-W2.1xW2.1 MIN. WELDED WIRE FABRIC TIED TO EXISTING REINFORCEMENT WHEN DETERIORATED CONCRETE IS GREATER THAN 1'-0" IN ANY DIRECTION. PROVIDE 1" CLEAR DISTANCE TO LIMIT OF REMOVAL.
- 8. AREA TO BE REPAIRED MUST BE CLEAN, SOUND AND FREE OF CONTAMINANTS PRIOR TO APPLICATION OF BONDING AGENT AND REPAIR MATERIAL.
- 9. REPAIR CRACKS IN EXISTING CONCRETE AFTER REMOVING DETERIORATED CONCRETE AND PRIOR TO CONSTRUCTING CONCRETE REPAIR. USE EPOXY INJECTION CRACK REPAIR IN ACCORDANCE WITH PUBLICATION 408, SECTION 1091.
- 10. APPLY AN APPROVED BONDING AGENT, AS LISTED IN BULLETIN 15 THAT IS COMPATIBLE WITH THE APPROVED REPAIR MATERIAL, UNLESS THE MANUFACTURER'S INSTRUCTIONS EXPRESSLY STATE THAT A BONDING AGENT IS NOT REQUIRED.
- 11. APPLY A RAPID HARDENING CONCRETE PATCHING MATERIAL FROM A MANUFACTURER LISTI IN BULLETIN 15 UNDER MISCELLANEOUS POLYMER MODIFIED AND SPECIAL CEMENTS, MORTARS AND CONCRETES, IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
- 12. APPLY REPAIR MATERIAL THAT HAS A COMPRESSIVE STRENGTH EQUAL TO OR GREATER THAN THAT OF THE ORIGINAL CONCRETE (IF KNOWN), BUT NOT LESS THAN 4,500 PSI AND 5,500 PSI AT 7 AND 28 DAYS, RESPECTIVELY.
- 13. CURE REPAIR MATERIAL IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS FOR A MINIMUM OF 24 HOURS. IMPLEMENT ADDITIONAL CURING PROTECTIONS IN ACCORDANCE WITH PUBLICATION 408, SECTION 1001.3(P) 4 AND SECTION 1001.3(P) 5, AS REQUIRED.
- 14. PROVIDE REPAIR MATERIAL WITH MINIMUM 200 PSI BOND STRENGTH TO THE EXISTING CONCRETE AS TESTED IN ACCORDANCE WITH ASTM D4541 PULL-OFF TEST.
- 15. A CONCRETE BLISTER MAY BE USED FOR AREAS WITH EXISTING REINFORCEMENT HAVING INADEQUATE COVER OR FOR ACCESS FOR CONCRETE PLACEMENT IN FORMS. REFER TO BLISTER DETAIL, SHEET 2. DO NOT REDUCE VERTICAL UNDERCLEARANCE WITHOUT DISTRICT BRIDGE ENGINEER APPROVAL.
- 16. FOR ADJACENT BOX BEAMS, INSERT 1/2" JOINT MATERIAL BETWEEN BEAMS AND PUMP CONCRETE INTO FORM THROUGH PORT AT BOTTOM FLANGE FORM. PROVIDE 1" VENTS AT TOP OF REPAIR AREA.
- 17. APPLY AN APPROVED PENETRATING SEALER AFTER REPAIR MATERIAL HAS CURED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- 18. APPLY CONCRETE REPAIR TYPE 2 TO CONCRETE DIAPHRAGMS AS NEEDED, SEE SHEET 2 FOR REPAIR.
- 19. FOR GENERAL NOTES, SEE SHEET 1.

LEGEND

- REMOVE DETERIORATED CONCRETE.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD

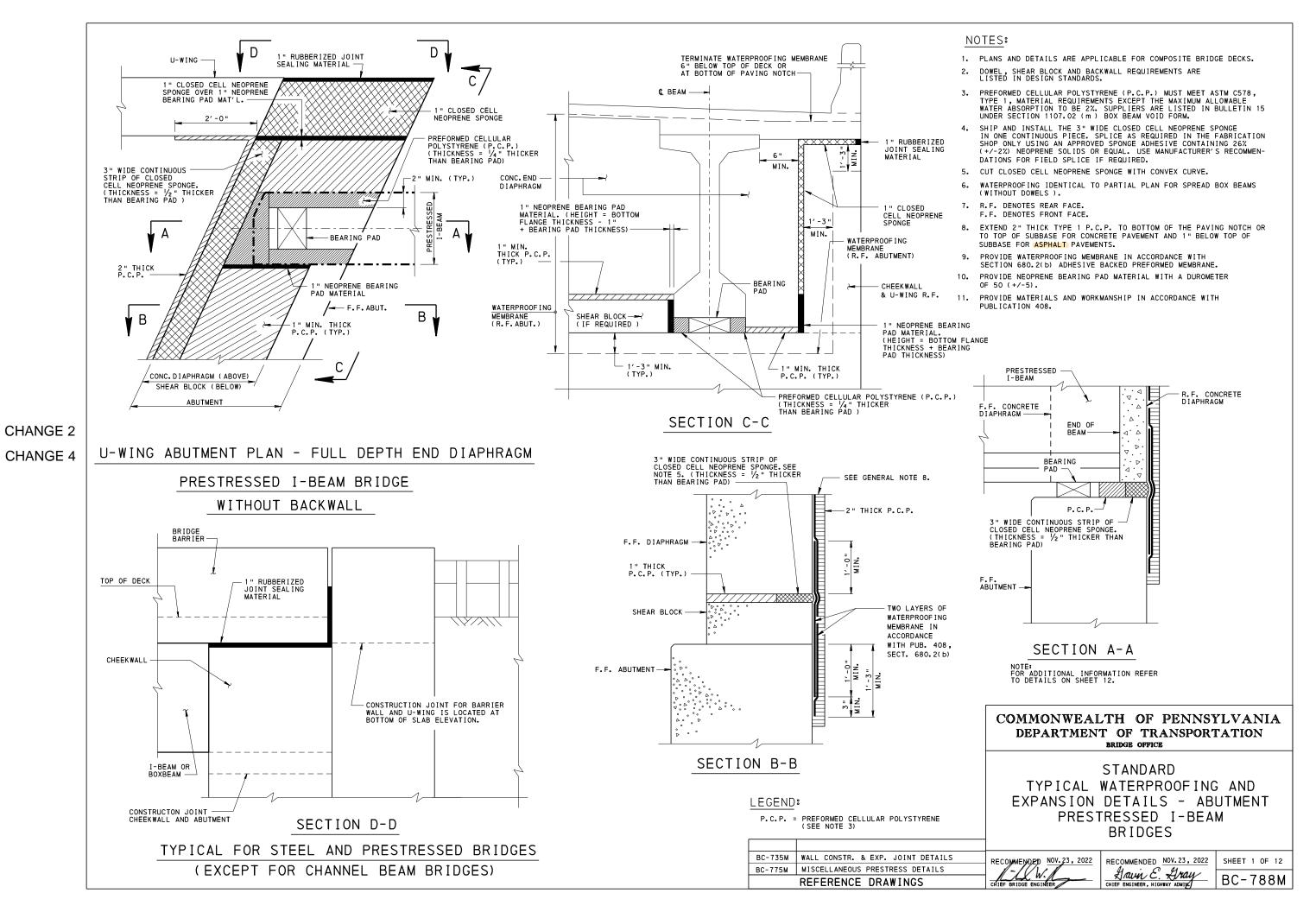
REINFORCED CONCRETE REPAIR
PRESTRESSED CONCRETE BEAM

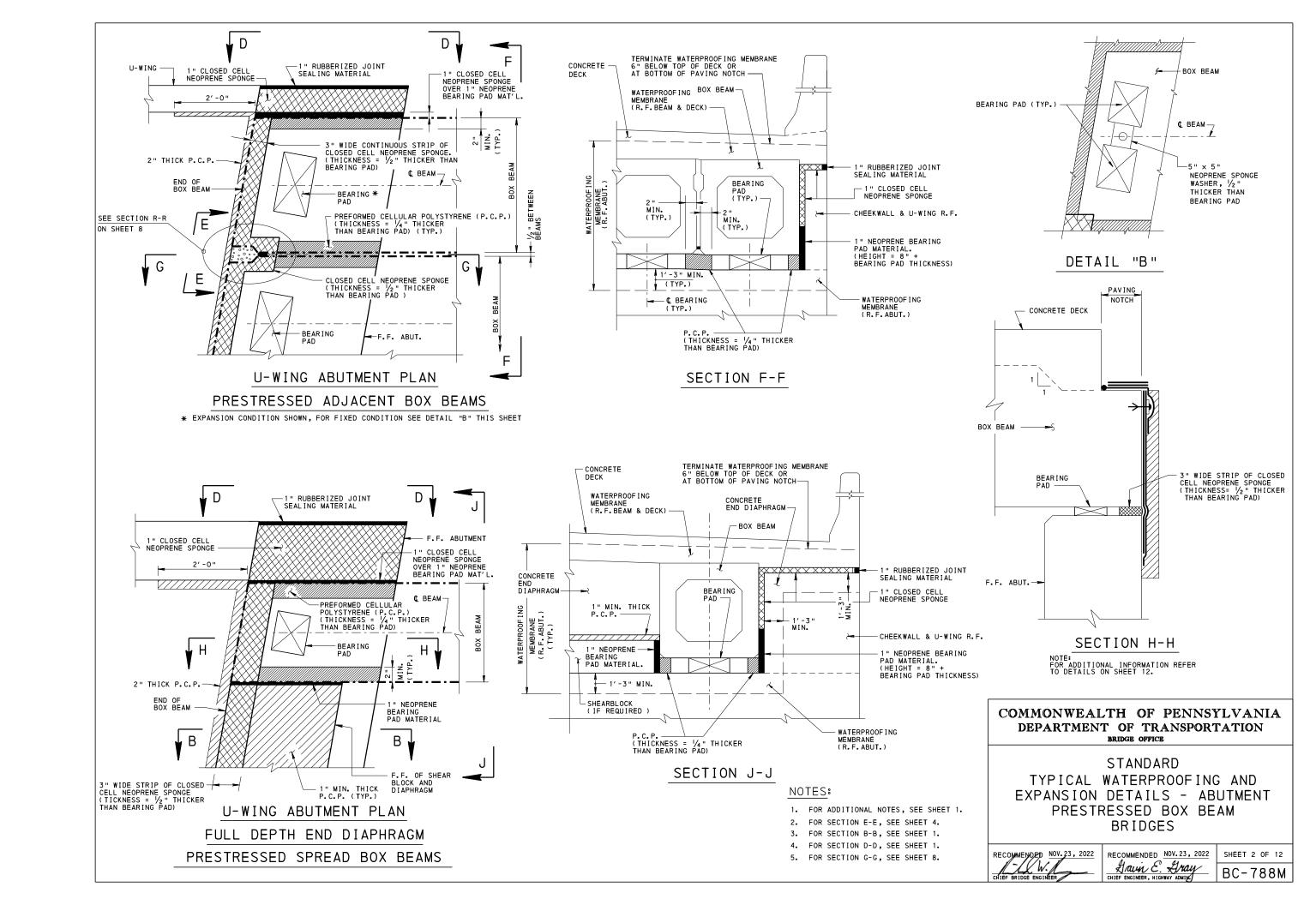
RECOMMENDED JAN. 31, 2019

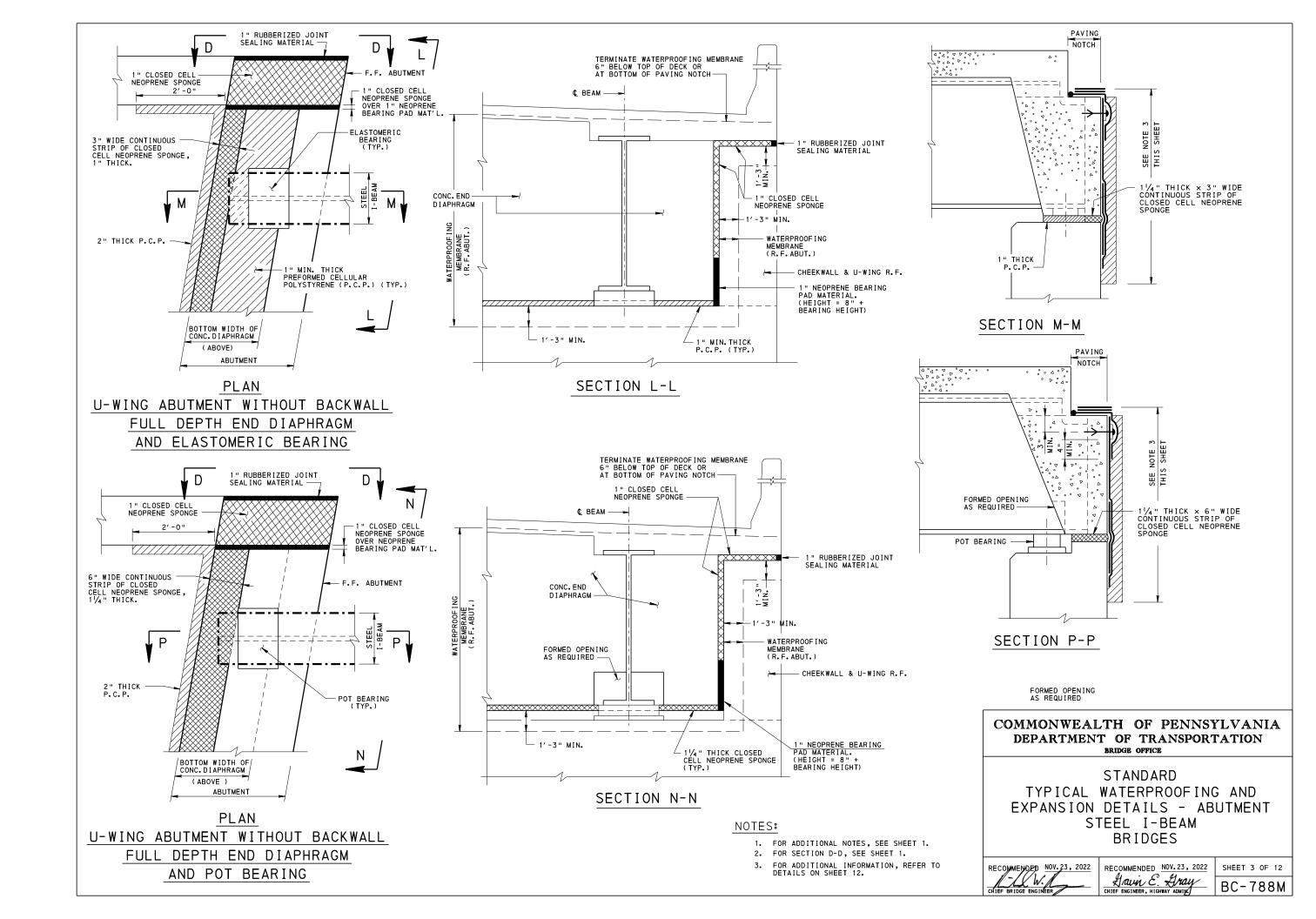
Thoras P. Marcine
CHIEF BRIDGE ENGINEER

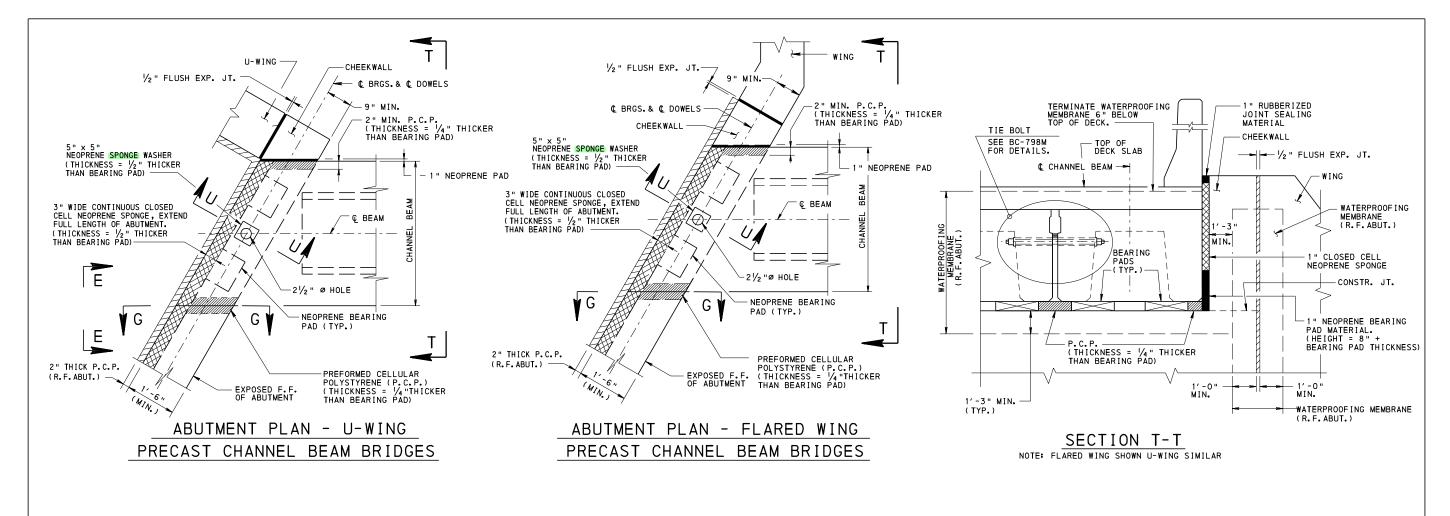
RECOMMENDED JAN. 31, 2019
Millian Houter

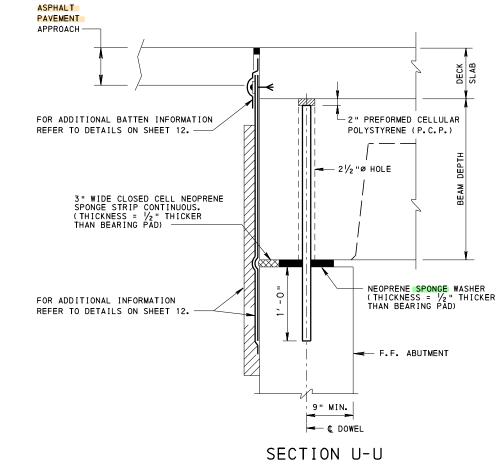
PROJECT DELIVERY BC-783M

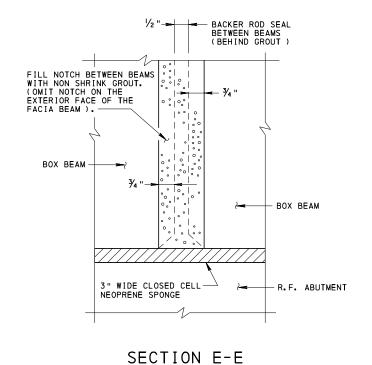












NOTES:

- 1. FOR ADDITIONAL NOTES, SEE SHEET 1.
- 2. FOR SHEAR KEY DETAIL SEE STANDARD DRAWING BC-775M
- 3. FOR SECTION G-G, SEE SHEET 8.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

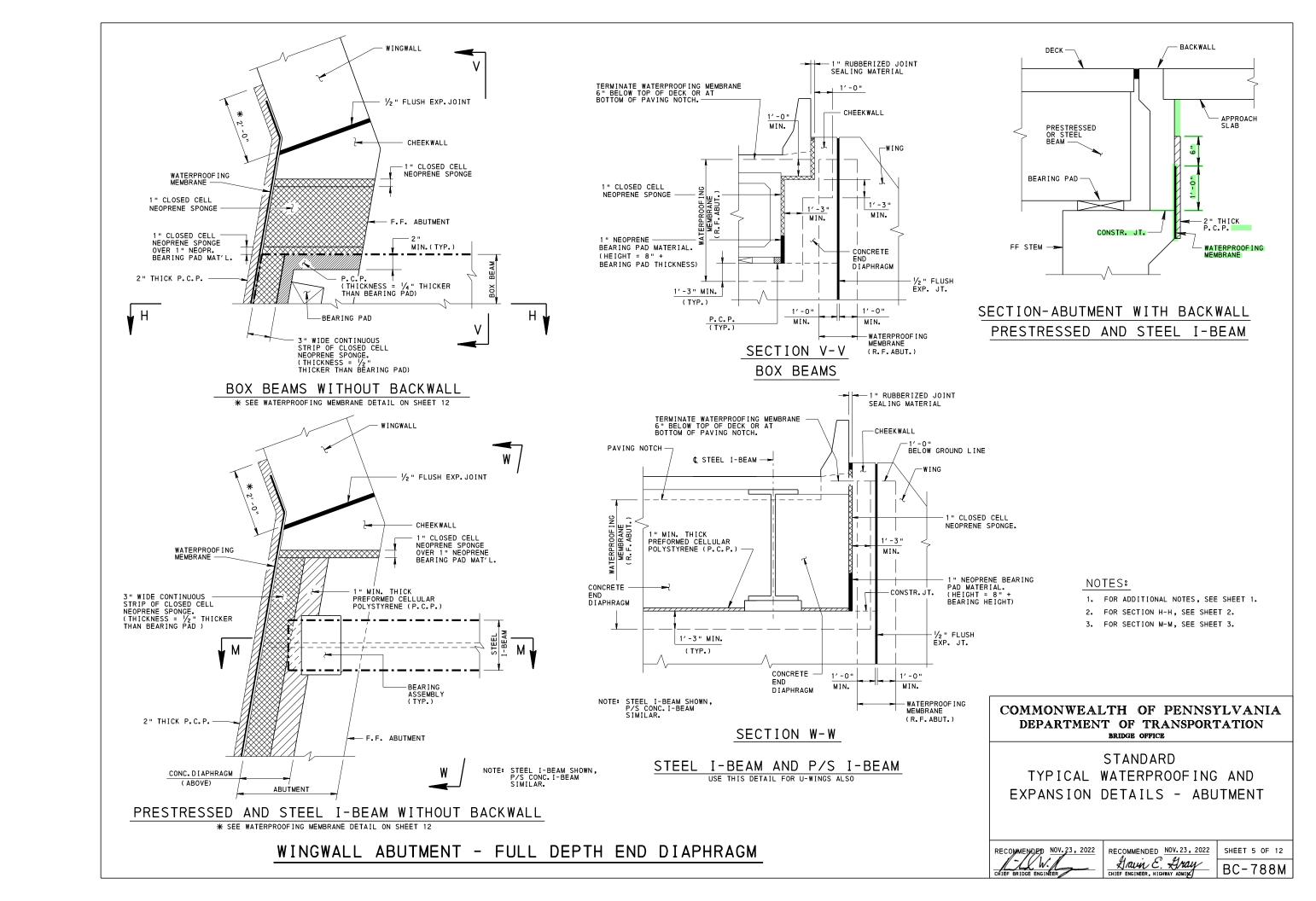
STANDARD
TYPICAL WATERPROOFING AND
EXPANSION DETAILS - ABUTMENT
PRECAST CHANNEL BEAM
BRIDGES

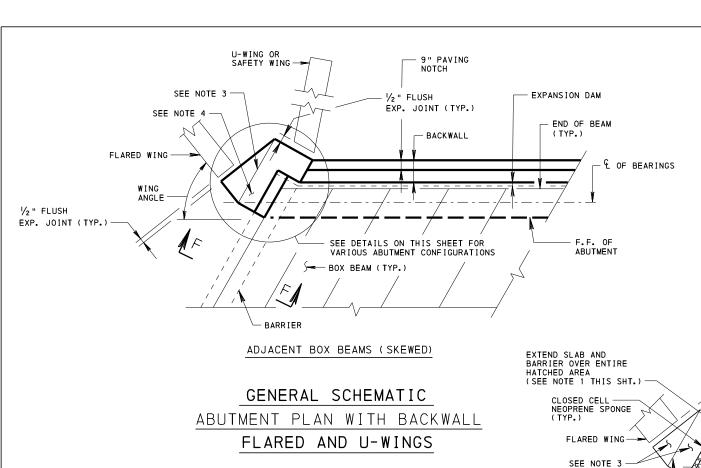
RECOMMENDED NOV. 23, 2022

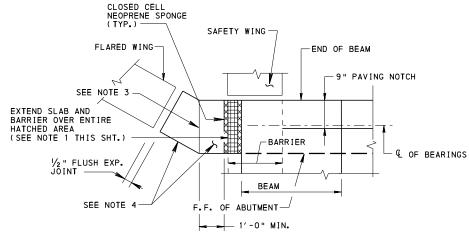
RECOMMENDED NOV. 23, 2022 SHEET 4 OF 12

Havin & Hornay
CHIEF ENGINEER, HIGHWAY ADMIN,

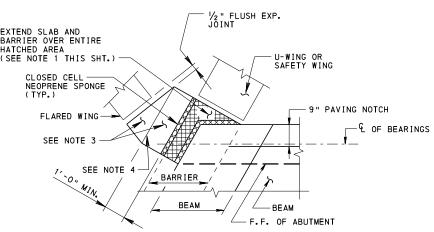
BC-788M



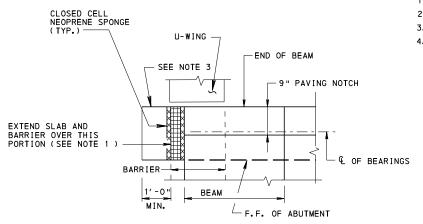




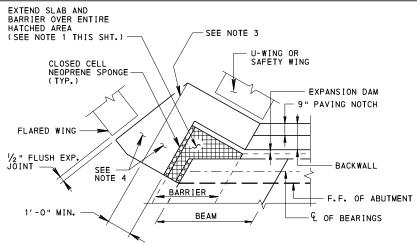
DETAIL FOR 90° ABUTMENT
WITHOUT BACKWALL
FLARED WING



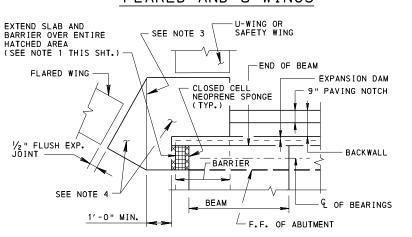
DETAIL FOR SKEWED ABUTMENT WITHOUT BACKWALL FLARED AND U-WINGS



DETAIL FOR 90° ABUTMENT
WITHOUT BACKWALL
U-WINGS



DETAIL FOR SKEWED ABUTMENT WITH BACKWALL FLARED AND U-WINGS



DETAIL FOR 90° ABUTMENT
WITH BACKWALL
FLARED AND U-WINGS

DETAIL NOTES:

- 1. PLACE 1" CLOSED CELL NEOPRENE SPONGE UNDER SLAB.
- 2. SEE SHEET 2 FOR SECTION F-F.
- 3. LIMIT OF CURTAIN WALL FOR U-WINGS.
- 4. CURTAIN WALL FOR FLARED WINGS AND INCLUDES THE CURTAIN WALL FOR U-WINGS PLUS ANY ADDITIONAL CONCRETE NEEDED TO FRAME INTO THE FLARED WINGWALL.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD
TYPICAL WATERPROOFING AND
EXPANSION DETAILS - ABUTMENT

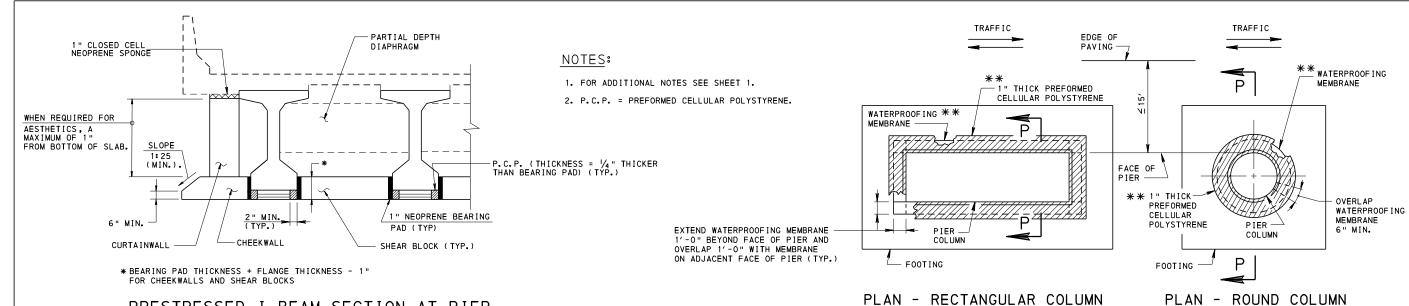
RECOMMENDED NOV. 23, 202

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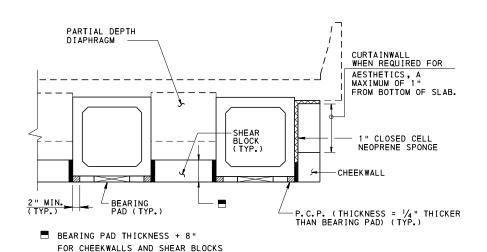
Havin E. Gray

CHIEF ENGINEER, HIGHWAY ADMIN.

SHEET 6 OF 12
BC-788M

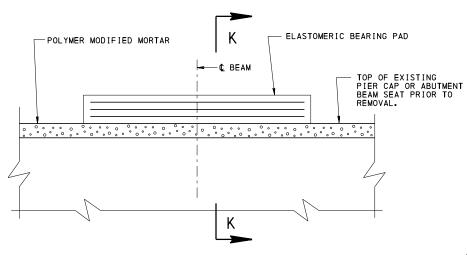


PRESTRESSED I-BEAM SECTION AT PIER



BOX BEAM SECTION AT PIER

CHEEKWALL CONDITION FOR BOX BEAMS SIMILAR



TYPICAL PIER AND ABUTMENT EXPANSION BEARING

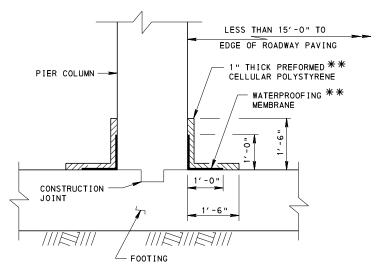
STEEL OR PRESTRESSED REHABILITATION
SLOPED TO DRAIN

REMOVE TOP OF 1/2" OF EXISTING CONCRETE OF PIER CAP OR ABUTMENT BEAM SEAT, ROUGHEN REMAINING CONCRETE AND APPLY 1/2" THICK POLYMER MODIFIED MORTAR FOR REHABILITATION PROJECTS.

ELASTOMERIC BEARING PAD

C BEARINGS

SECTION K-K
SLOPED TO DRAIN



PIER WATERPROOFING DETAILS

SECTION P-P

** WATERPROOFING MEMBRANE AND 1" THICK PREFORMED CELLULAR
POLYSTYRENE TO BE USED AROUND THE PIER COLUMNS WHEN THE
DISTANCE FROM EDGE OF PAVING TO THE PIER COLUMN FACE IS ≤ 15 FEET.
ALSO PROVIDE WATERPROOFING WHENEVER PIER FOOTING IS SUBJECT
TO HIGH GROUND WATER LEVEL SUCH AS IN OR NEAR A WATERWAY.

PIER WATERPROOFING INSTALLATION NOTE:
INSTALL 2'-0" WIDE WATERPROOFING MEMBRANE TO FIT PIER COLUMN AND TOP OF FOOTING AS SHOWN,
RUN MEMBRANE CONTINUOUS ALONG APPLICABLE FACE OF PIER COLUMN. USE AN ADHESIVE BACKED,
PREFORMED WATERPROOFING MEMBRANE PER PUB. 408, SECTION 680.2 (b). PROVIDE 1'-6" MINIMUM
PREFORMED CELLULAR POLYSTYRENE IN EACH DIRECTION OVER WATERPROOFING MEMBRANE
AS PROTECTION.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD
TYPICAL WATERPROOFING AND
EXPANSION DETAILS - PIER
PRESTRESSED CONCRETE
I-BEAM AND BOX BEAM BRIDGES

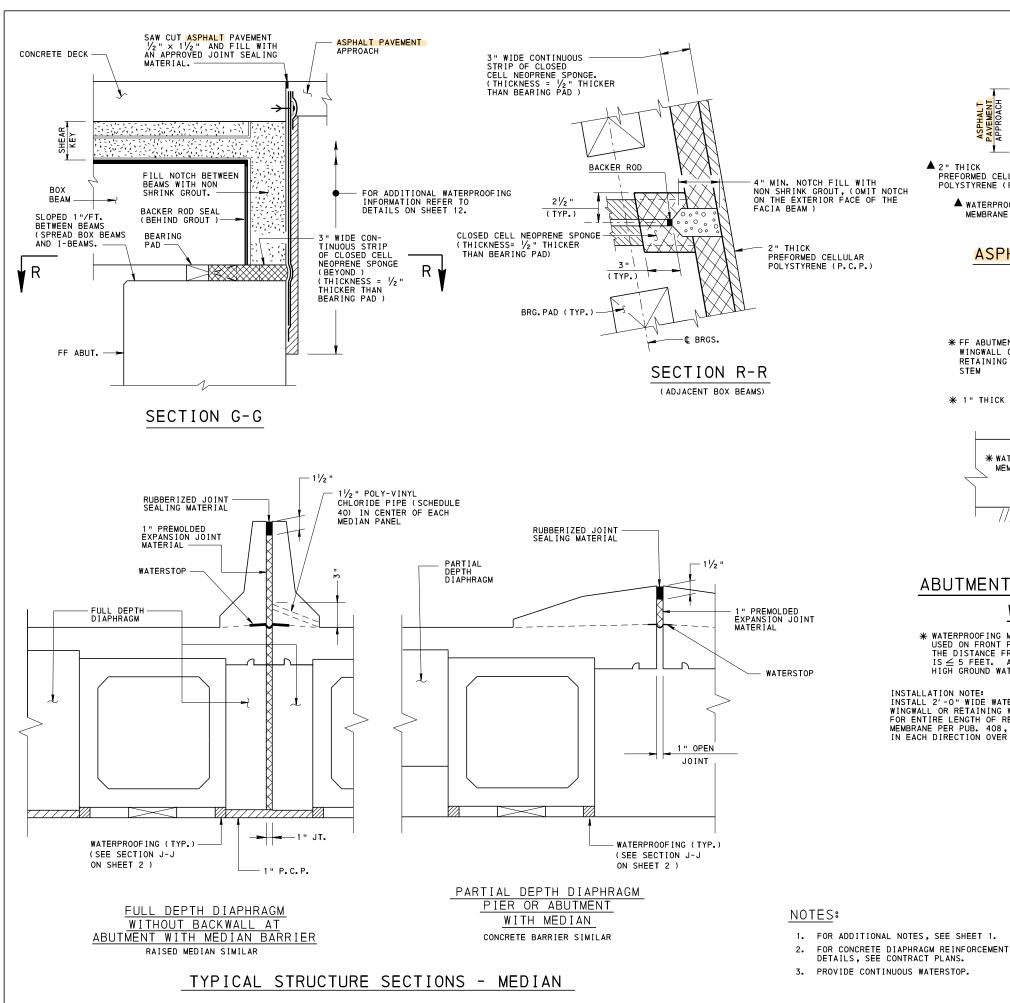
RECOMMENDED NOV. 23, 2022

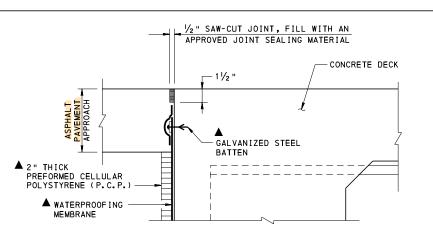
RECOMMENDED NOV. 23, 2022

Haw E. Hay

CHIEF ENGINEER, HIGHWAY ADMIN.

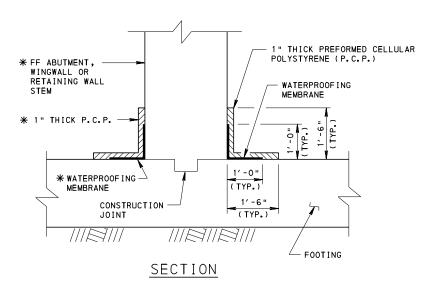
SHEET 7 OF 12
BC-788M





ASPHALT PAVEMENT APPROACH AT STRUCTURE

▲ FOR ADDITIONAL INFORMATION REFER TO DETAILS ON SHEET 12.



ABUTMENT, WINGWALL OR RETAINING WALL

WATERPROOFING DETAIL

* WATERPROOFING MEMBRANE AND 1" THICK PREFORMED CELLULAR POLYSTYRENE TO BE USED ON FRONT FACE OF ABUTMENT, WINGWALL OR RETAINING WALL SIEM ONLY WHERE THE DISTANCE FROM EDGE OF PAVING TO THE FRONT FACE OF THE RESPECTIVE STEM IS \$\inqual 5\$ FEET. ALSO PROVIDE WATERPROOFING WHENEVER FOOTING IS SUBJECT TO HIGH GROUND WATER LEVEL SUCH AS IN OR NEAR A WATERWAY.

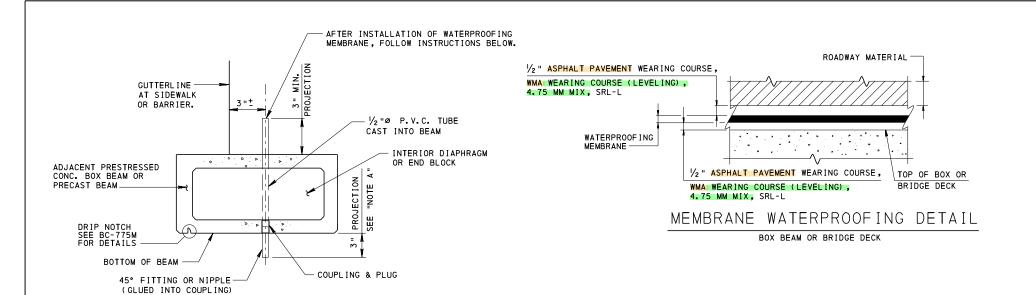
INSTALLATION NOTE:
INSTALL 2'-O" WIDE WATERPROOFING MEMBRANE TO FIT APPLICABLE FACE(S) OF THE ABUTMENT,
WINGWALL OR RETAINING WALL STEM AND TOP OF FOOTING AS SHOWN. RUN MEMBRANE CONTINUOUS
FOR ENTIRE LENGTH OF RESPECTIVE WALL OR STEM. USE AN ADHESIVE BACKED, PREFORMED WATERPROOFING
MEMBRANE PER PUB. 408, SECTION 680.2 (b). PROVIDE 1'-6" MINIMUM PREFORMED CELLULAR POLYSTYRENE IN EACH DIRECTION OVER WATERPROOFING MEMBRANE AS PROTECTION.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD TYPICAL WATERPROOFING AND EXPANSION DETAILS - MISCELLANOUS P/S CONCRETE I-BEAM AND BOX BEAM BRIDGES

RECOMMENDED NOV. 23, 2022 Havin E. Gray
HIEF ENGINEER, HIGHWAY ADMIN

SHEET 8 OF 12 BC-788M

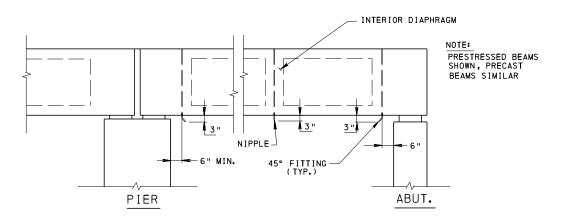


NOTES:

- 1. DETAILS SHOWN TO BE USED FOR PRESERVATION PROJECTS ONLY.
- 2. SPACE 1/2 " Ø P.V.C. TUBES, WHERE PRACTICABLE AS FOLLOWS:
- A) AT 20 FT. CENTERS (MAX.), OR
- B) IN THE INTERIOR DIAPHRAGMS OR END
- C) IF THE BRIDGE IS SUPERELEVATED ONE TUBE IS TO BE PLACED AT THE LOW END AND LOW SIDE OF THE STRUCTURE, AHEAD OF ABUTMENT AND PIER(S) IF MORE THAN SINGLE SPAN.
- D) USE 45° FITTING TURNED AWAY FROM SUBSTRUCTURE UNIT WHEN THE CLEARANCE IS LESS THAN 2'-0".
- 3. PROVIDE MASTIC IN ACCORDANCE WITH SECTION 680.2(a) OF PUB. 408.

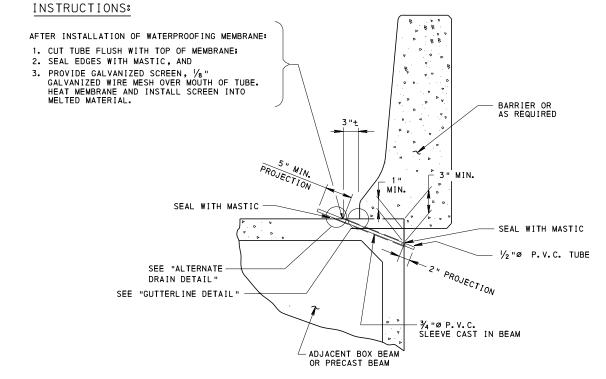
NOTE A:

COUPLING AND PLUG; REMOVE PLUG AFTER FABRICATION OF BEAM AND INSERT 3" NIPPLE (INCIDENTAL TO PRECAST OR P/S CONCRETE BRIDGE BEAM(S), TO BE DETAILED ON FABRICATOR'S SHOP DRAWINGS.



TYPICAL LONGITUDINAL SECTION

ALTERNATE DRAINS

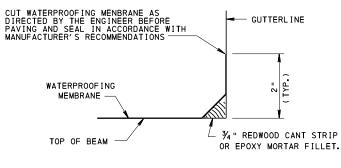


TYPICAL DRAIN DETAIL

PRESTRESSED CONCRETE BEAM SHOWN,

PRECAST BEAM SIMILAR

MEMBRANE DRAIN DETAIL AT CURB



GUTTERLINE DETAIL

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

TYPICAL WATERPROOFING AND EXPANSION DETAILS-MISCELLANEOUS PRESTRESSED OR PRECAST BRIDGE

RECOMMENDED NOV. 23, 2022

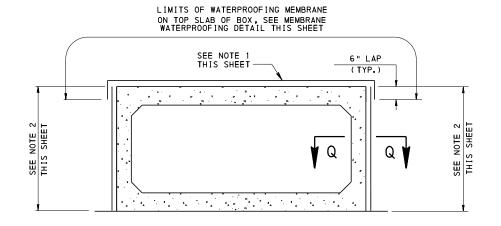
RECOMMENDED NOV. 23, 2022

Havin E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN

BC-788M

SHEET 9 OF 12



TYP. PRECAST BOX SECTION NO SCALE

ROADWAY MATERIAL 1/2 " ASPHALT PAVEMENT WEARING COURSE, WMA WEARING COURSE (LEVELING),

BOX TOP SLAB

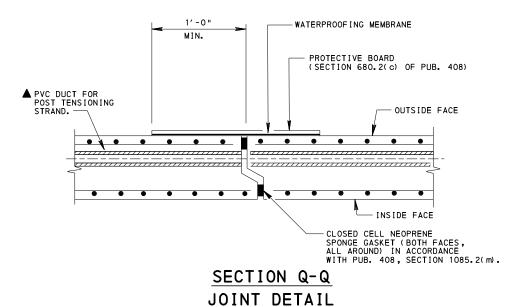
WATERPROOF ING MEMBRANE

MEMBRANE WATERPROOFING DETAIL PRECAST BOX CULVERT

NOTES:

4.75 MM MIX, SRL-L

- 1. PROVIDE APPROVED WATERPROOFING MEMBRANE FOR THE ENTIRE TOP WIDTH AND LENGTH OF THE BOX AND 2'-O" WIDTH ± ALONG THE SIDE JOINTS.
- 2. LIMITS OF WATERPROOFING AT SIDE OF PRECAST BOX SECTION JOINTS 2'-0" WIDTH ± . PLACE THIS BEFORE THE TOP SLAB WATERPROOFING.



SEAL AROUND EACH DUCT JOINT WITH A NEOPRENE SPONGE DONUT.

■ POST TENSIONING DUCTS MAY BE PLACED WITHIN THE WALL OR SLAB ANYWHERE BETWEEN LAYERS OF REINFORCEMENT TO AVOID THE SLOPED PORTION OF THE JOINT SO AS TO PROMOTE SEALING OF THE DUCT.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

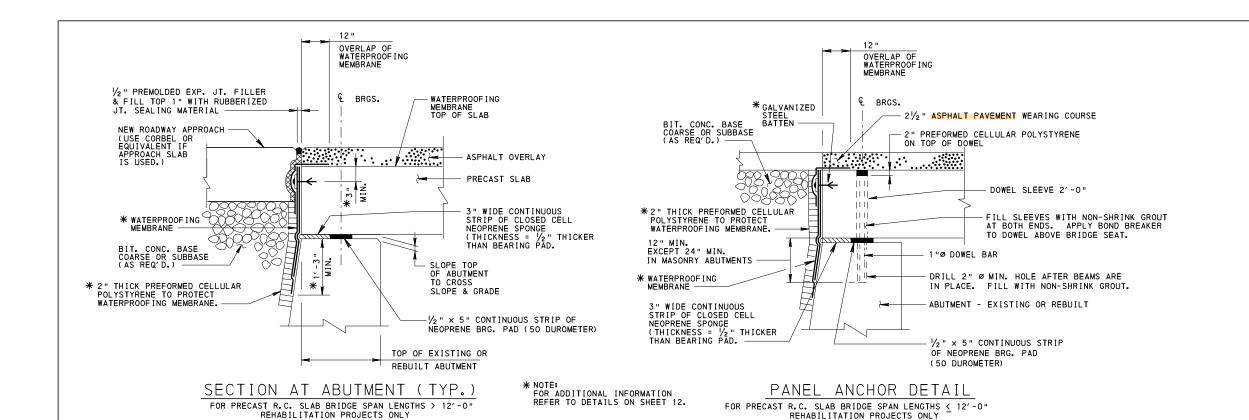
STANDARD TYPICAL WATERPROOFING AND EXPANSION DETAILS PRECAST R.C. BOX CULVERTS

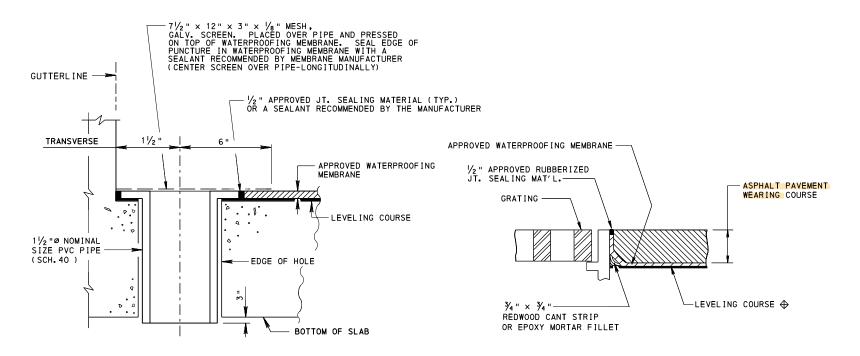
RECOMMENDED NOV. 23, 2022

RECOMMENDED NOV. 23, 2022 SHEET 10 OF 12 Havin E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN

BC-788M





DRAIN PIPE THROUGH DECK SLAB

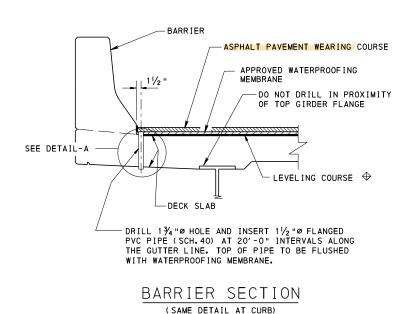
◆ (SAME DETAIL © CURB & MEDIAN BARRIER, IF APPLICABLE)

DETAIL-A

ASPHALT PAVEMENT OVERLAY AND WATERPROOFING MEMBRANE DETAILS AT DECK DRAINS

WATERPROOFING MEMBRANE AT SCUPPER

REHABILITATION PROJECTS ONLY



COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD
TYPICAL WATERPROOFING AND
EXPANSION DETAILS
R.C. BRIDGE DECK

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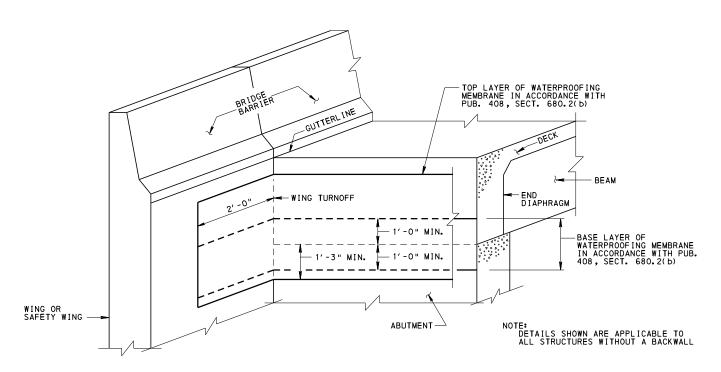
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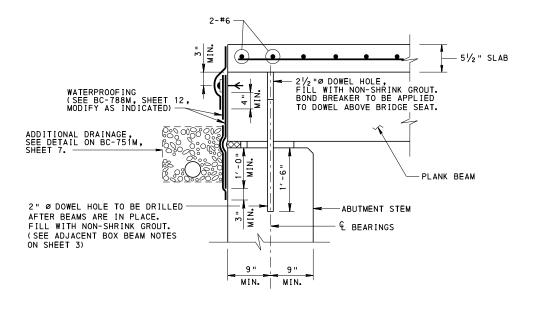
Law E. Lray

CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 11 OF 12

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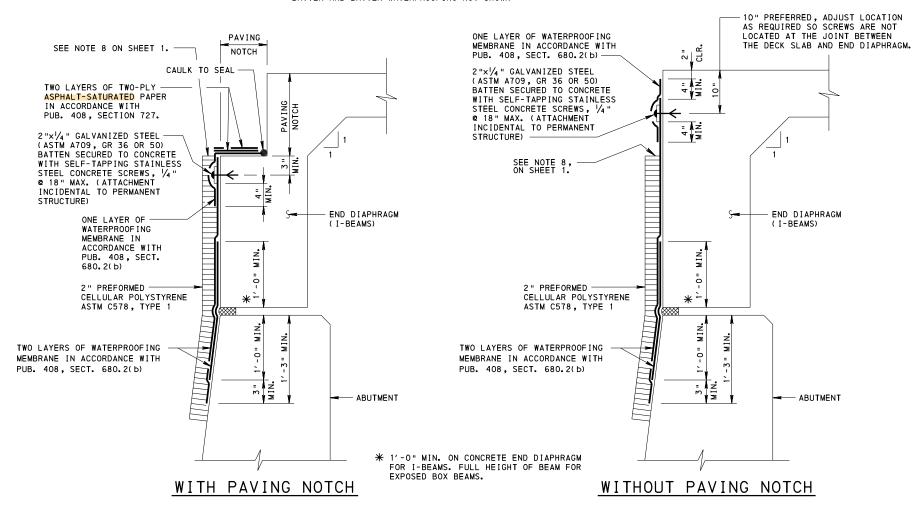




MEMBRANE WATERPROOFING DETAIL

BATTEN AND BATTEN WATERPROOFING NOT SHOWN

TYPICAL LONGITUDINAL SECTION FOR PLANK BEAMS



WATERPROOFING DETAIL AT ABUTMENT WITHOUT BACKWALL

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD
MISCELLANEOUS
WATERPROOFING DETAILS

RECOMMENDED NOV. 23, 2022

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Law E. Hray

CHIEF ENGINEER, HIGHWAY ADMIN.

SHEET 12 OF 12 BC-788M

GENERAL NOTES

DESCRIPTION - THIS WORK IS GROUTING OF STEEL POST-TENSIONING TENDONS IN CONCRETE STRUCTURES. THIS SPECIFICATION APPLIES TO ALL GROUTED POST-TENSIONING OPERATIONS IN BRIDGE SUPERSTRUCTURE AND SUBSTRUCTURE, INDICLUDING BOX-GIRDERS, I-GIRDERS, SEGMENTAL GIRDERS, SPLICED GIRDERS, PIER CAPS, AND PIERS.

MATERIAL

USE MATERIALS MEETING THE REQUIREMENTS CLASS C (PRE-PACKAGED). THIXOTROPIC GROUT AS SPECIFIED IN THE CURRENT EDITION OF POST TENSIONING INSTITUTE'S GUIDE SPECIFICATION FOR GROUTED POST-TENSIONING (PTI/ASBI M 50) AND POST-TENSIONING INSTITUTE'S SPECIFICATION FOR GROUTING OF POST-TENSIONED

USE PRE-PACKAGED GROUT MATERIALS WITHIN 6 MONTHS OF THE MANUFACTURE DATE OR PER MANUFACTURER'S RECOMMENDATIONS FOR SHELF-LIFE. WHICHEVER IS LESS.

USE GROUT FROM ONE PLANT FOR A GIVEN PROJECT LOCATION.

FILLERS ARE NOT PERMITTED IN GROUT.

EQUIPMENT

PROVIDE THE NECESSARY STRESSING AND GROUTING EQUIPMENT TO PERFORM THE WORK PER PTI M50/M55. USE OF HIGH-SPEED SHEAR COLLOIDAL MIXER IS REQUIRED. VERIFY ALL EQUIPMENT CALIBRATIONS PRIOR TO USE.

CONSTRUCTION

A. GROUTING PERSONNEL QUALIFICATIONS:

PROVIDE PERSONNEL MEETING THE FOLLOWING MINIMUM REQUIREMENTS TO PERFORM GROUTING OPERATIONS:

- 1. A FOREMAN CERTIFIED AS AN AMERICAN SEGMENTAL BRIDGE INSTITUTE (ASBI) CERTIFIED TECHNICIAN AND PTI LEVEL 2 BONDED PT-FIELD SPECIALIST.
 2. MINIMUM OF 25% OF THE PERSONEL PERFORMING THE GROUTING OPERATIONS MUST BE
- CERTIFIED AS PTI LEVEL 1 BONDED PT-FIELD INSTALLATION.

B. TENDON INSTALLATION:

USE POST-TENSION TENDON DUCT MATERIAL PER PTI M 50 AND M 55.
POST-TENSION TENDON INSTALLATION SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION
OF PTI M 50 AND M 55. PRIOR TO GROUTING OPERATIONS, PERFORM PRESSURE TESTING OF
INSTALLED TENDONS PER PTI M 50 AND M 55.

PROVIDE THE MINIMUM INNER DIAMETER OF INLETS AND OUTLETS WITH AT LEAST 3/4 IN FOR STRAND TENDONS AND 3/8 IN FOR SINGLE BAR TENDONS. PLACE INLETS AND OUTLETS AT LOCATIONS RECOMMENDED BY THE DESIGN OR CONSTRUCTION ENGINEER AND AT THE FOLLOWING

- AT THE ANCHORAGE AREA OF THE TENDON. AT THE HIGHEST POINTS OF THE DUCT, WHEN THE VERTICAL DISTANCE BETWEEN THE HIGHEST AND LOWEST POINT IS MORE THAN 20 IN. WHERE OUTLETS ARE PLACED AT THE HIGH POINTS, AT A DISTANCE NOT TO EXCEED 36
- IN BOTH DIRECTIONS FROM THE HIGH-POINT OUTLETS.
 AN ADDITIONAL OUTLET AT A SHORT DISTANCE DOWNSTREAM (AT APPROXIMATELY THE
- LOCATION WHERE THERE HAS BEEN A VERTICAL DROP OF A DISTANCE EQUAL TO ONE HALF DUCT DIAMETER) OF A HIGH POINT OUTLET.
- AT MAJOR CHANGES IN THE CROSS-SECTION OF THE DUCT, SUCH AS COUPLERS AND ANCHORAGES.
- AT ALL LOW POINTS (FREE-DRAINING).
- PRIMARY INLET: AT OR NEAR THE LOWEST POINT OF THE TENDON.

PROVIDE INLETS AND OUTLETS WITH POSITIVE SHUT-OFFS AND CHECK TO BE SURE THEY CAN BE PROPERLY OPENED AND CLOSED.

C. GROUTING PLAN:

CHANGE 2

SUBMIT A GROUTING PLAN TO THE ENGINEER FOR REVIEW AND ACCEPTANCE AT LEAST 4 WEEKS PRIOR TO GROUTING. PERFORM A MOCK-UP TEST WITH THE EQUIPMENT AND MATERIALS THAT WILL BE USED ON THE JOB AND SUBMIT RESULTS WITH THE GROUTING PLAN:

- NAMES. EXPERIENCE AND QUALIFICATIONS OF GROUTING CREW AND SUPERVISORS
- NAMES, EXPERIENCE AND QUALIFICATIONS OF GROUTING CREW AND SUPERVISOR:
 ASBI CERTIFICATION DOCUMENTATION FOR THE TECHNICIAN AND BACKUP.
 TYPE OF EQUIPMENT (TYPE, SPEED, CAPACITY, INCLUDING BACK-UP EQUIPMENT).
 TARGETED FLOW RATE DURING PUMPING AND METHOD OF CONTROLLING RATE.
 TYPE OF GROUT AND WATER CONTENT THAT WILL BE USED FOR INITIAL ON-SITE MIX.
 ESTIMATED QUANTITY OF GROUT FOR EACH TENDON GROUP.
 MATERIAL SUPPLIER NAME AND AVAILABLE DATA SHEETS.
 LIST OF ON-SITE TESTING AND FREQUENCY.

- INLET AND VENTION DETAILS (TYPE, LOCATION, SIZE).
 PLANS FOR PROTECTION OF TENDONS PRIOR TO GROUTING (PRIOR TO
- INSTALLATION AND AFTER INSTALLATION.
 PROVIDE A PLAN (ADDITIONAL PROTECTION) IF TENDONS WILL REMAIN
- UNGROUTED MORE THAN 7 DAYS.
 11. AIR TESTING PROCEDURE.
- GROUT MIXING AND PUMPING PROCEDURES.
- ORDER OF TENDON GROUTING.
- DIRECTION OF GROUTING AND SEQUENCE OF VENT SHUTOFF.
- PRECOURES FOR BLOCKAGES (INCLUDING MEANS OF FLUSHING IF NECESSARY)
 PROCEDURES FOR INSPECTION OF TENDONS AFTER GROUTING.
- 17. PROCEDURES FOR FILLING ANY VOIDS.
- SAMPLE GROUTING RECORD FORMS. 19. RESULTS OF TENDON MOCK-UP.

- D. PRE-GROUTING OPERATIONS:
- PLACE A PERMANENT PLASTIC END CAP WITH A VENT OVER EACH ANCHOR HEAD IMMEDIATELY AFTER STRAND TAILS ARE CUT.
 BEFORE GROUTING BEGINS, CHECK AND PREPARE DUCTS BY BLOWING THROUGH OIL-FREE AND
- WATER-FREE COMPRESSED AIR. OPEN AND CLOSE EACH VENT IN TURN TO BLOW OUT MOISTURE. INSPECTION TO ENSURE THAT ALL MATERIALS ARE OF THE SPECIFIED TYPE AND QUANTITY. INSPECTION TO ENSURE THAT ALL EQUIPMENT IS IN SATISFACTORY CONDITION. INSPECTION OF DUCTS TO ENSURE THAT THEY ARE FREE OF WATER, DEBRIS,
- AND OTHER OBSTRUCTIONS
- INSPECTION OF DUCTS TO ENSURE THAT THERE WILL BE NO GROUT LEAKAGE BETWEEN

- INSPECTION OF DUCIS TO ENSURE THAT THERE WILL BE NO GROUT LEARAGE BEING ADJACENT DUCTS IN THE JOINT AREAS OR AT COUPLERS FOR EXTERNAL TENDONS. TEMPERATURE MEASUREMENT OF AIR, WATER, AND PREPACKAGED GROUT TO ENSURE THAT THEY ARE WITHIN THE ACCEPTABLE LIMITS.
 INSPECTION OF ALL PREPACKAGED GROUT OR CEMENT PACKAGES FOR EVIDENCE OF AGE AND DAMPNESS, SUCH AS LUMPS AND HARDENED PIECES.
 COMMENCE GROUTING OPERATIONS ONLY AFTER THE ENGINEER'S ACCEPTANCE TO PROCEED IS PECTIVED.
- GROUTING OPERATION:
 - PERFORM GROUTING OPERATIONS IN ACCORDANCE WITH PTI M 50 AND M55. AT A MINIMUM, MONITOR AND CONDUCTION QUALITY TESTING TO VERIFY MATERIAL PROPERTIES PER PRODUCTION TESTS AS SPECIFIED IN PTI M 55. FOLLOW PTI M 50 AND M 55 FOR REQUIRED PRE-GROUTING OPERATIONS AND POST-GROUTING
- OPERATION, MIXING CONDITIONS AND TEMPERATURES, GROUTING OPERATIONS AND PROCEDURES, AND POST GROUTING OPERATIONS AND VERIFICATION TESTING.

GROUT ONLY WHEN BOTH THE AMBIENT AIR TEMPERATURE AND SUBSTRATE TEMPERATURE ARE AT LEAST 41 DEGREES F DURING THE GROUTING OPERATION AND FOR 3 HOURS THEREAFTER. ADDITIONALLY, GROUT TEMPERATURE MUST BE MAINTAINED ABOVE 35 DEGREES F FOR 3 CONSECUTIVE DAYS AFTER

F. MINIMUM QUALITY CONTROL TESTING:

NOTIFY THE DEPARTMENT'S REPRESENTATIVE PRIOR TO PERFORMING QUALITY CONTROL TESTING. QUALITY CONTROL TESTING IS TO BE PERFORMED BY THE ASSI/PTI GROUT TECHNICIAN. PERFORM QUALITY CONTROL TESTING TO CONFIRM THE PRODUCTION GROUT, AT MINIMUM,

THE TESTING OF PRODUCTION GROUT SHALL BE CARRIED OUT AS DESCRIBED IN THE SECTION (TESTING, QUALITY CONTROL AND QUALITY ASSURANCE) OF PTI M 55 WITH THE FOLLOWING MINIMUM NUMBER OF TESTS:

- A MINIMUM OF ONE PRESSURE BLEED TEST (SECTION 4.4.6) SHALL BE PERFORMED PER PROJECT DURING FIELD TRIAL TESTING. ADDITIONAL TESTS ARE REQUIRED FOR EACH TRUCK LOAD OF PREPACKAGED GROUT DELIVERED TO THE PROJECT, AND EVERY 20 TONS (40,000 lb) OF UNMIXED MATERIALS BEFORE THE ADDITION OF WATER. THE SAMPLE OF GROUT FOR THE TEST IS TO BE TAKEN AT THE MIXER.
 A WET DENSITY TEST SHALL BE PERFORMED AT THE MIXER INITIALLY AND EVERY TWO HOURS. AND
- A WEI DENSITY LEST SHALL BE PERFORMED AT THE MIXER INTITIALLY AND EVERT TWO HOURS, AND AT THE LAST OUTLET OF EACH TENDON PER MINIMUM VALUE IN 4.4.8.

 MINIMUM OF TWO FLUIDITY TESTS (FLOW CONE), ONE AT THE MIXER AND ONE AT THE DUCT OUTLET AS PER SECTION 4.4.5, REPEAT TESTING EVERY 2 HOURS OF GROUTING OPERATIONS. THE EFFLUX TIME SHALL BE WITHIN 5 SECONDS OF THE VALUES ESTABLISHED DURING LABORATORY TESTING.

 IF AN EXPANSIVE ADMIXTURE IS USED IN THE GROUT FOR EXTERNAL TENDONS USING PLASTIC DUCTS, THEN ONE VOLUME CHANGE TEST (SECTION 2.4.4) PER DAY SHALL BE CARRIED OUT.
- A MINIMUM OF ONE CHLORIDE ION CONCENTRATION TEST SHALL BE PERFORMED PER PROJECT DURING FIELD TRIAL TESTING. ADDITIONAL TESTS ARE REQUIRED FOR EVERY 20 TONS (40,000 Lb) UNMIXED MATERIALS DELIVERED TO THE PROJECT BEFORE THE ADDITION OF WATER.

TABLE F.1: QUALITY CONTROL TESTING REQUIREMENTS				
TEST	FREQUENCY	PERFORMANCE CRITERIA	TEST METHOD	
GROUT STRENGTH TEST (AVERAGE OF 3 CUBES)	1 PER DAY	= 3,000 PSI AT 7 DAYS = 5,000 PSI AT 28 DAYS	ASTM C942	
FLUIDITY TEST	1 EVERY* 2 HOURS	WITHIN * 5 SEC OF VALUES ESTABLISHED DURING LABORATORY TESTING	ASTM C939**	
SCHUPACK PRESSURE BLEED TEST	1 PER DAY*	ASTM 1741		
API MUD BALANCE TEST	2 PER DAY***	VALUE SHALL BE BETWEEN THE VALUES OBTAINED DURING PRE-QUALIFICATION TESTING AND MINIMUM AND MAXIMUM WATER DOSAGE.		
CHANGE IN VOLUME TESTS (IF EXPANSIVE AGENT IS USED)	1 PER DAY	0%- +0.1% IN 24 HRS. MAX +0.2% AT 28 DAYS	ASTM 1090	

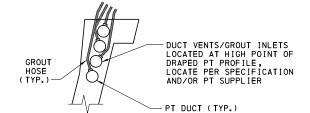
- TAKEN AT THE INLET.
- USE EITHER THE STANDARD ASTM C939 FLOW CONE TEST OR THE MODIFIED TEST DEPENDING ON WHICH WAS APPROVED AND USED INITIALLY IN THE LABORATORY
- OR WHEN THERE IS A VISUAL OR APPARENT CHANGE IN THE CHARACTERISTICS OF THE GROUT. IN ADDITION TO THE TWO PER DAY, TAKE AN ADDITIONAL SAMPLE FROM THE DUCT OUTLET TO ENSURE THAT EXTRA WATER IS NOT PRESENT IN THE

G. MIXING CONDITIONS:

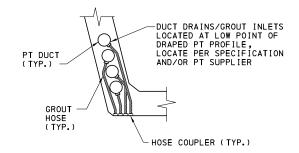
HOT WEATHER, EMPLOY HOT-WEATHER MIXING AND PROTECTION METHODS WHEN THE AMBIENT AIR TEMPERATURE EXCEEDS 90°F. COOL-MIX WATER TO MAINTAIN GROUT TEMPERATURE AT 80°F OR BELOW AT THE TIME OF GROUTING. ICE MAY BE

RECESSARY AND IF USED MUST BE ADDED TO THE MIX WATER TO LOWER THE WATER TEMPERATURE PRIOR TO MIXING. DO NOT ALLOW ICE IN THE GROUT MIXTURE. COLD WEATHER. MAINTAIN A DAILY RECORD OF MINIMUM AND MAXIMUM AMBIENT AIR TEMPERATURES DURING COLD WEATHER. RECORD THE TEMPERATURE OF THE GROUT DRY INGREDIENTS AND THE STRUCTURE SURROUNDING THE DUCTS TO BE GROUTED.

WHEN AMBIENT AIR TEMPERATURE HAS BEEN BELOW (32°F) AFTER DUCT PLACEMENT AND PRIOR TO GROUTING, BLOW DRY AIR THROUGH THE DUCTS TO EXPEL ANY TRAPPED WATER, FROST, OR ICE. GROUT ONLY WHEN BOTH THE AMBIENT AIR TEMPERATURE AND SUBSTRATE TEMPERATURE ARE AT LEAST 41 DEGREES F DURING THE GROUTING OPERATION AND FOR 3 HOURS THEREAFTER. ADDITIONALLY, GROUT TEMPERATURE MUST BE MAINTAINED ABOVE 35 DEGREES F FOR 3 CONSECUTIVE DAYS AFTER CROUTING



TOP DUCT VENT DETAIL



BOTTOM DUCT DRAIN DETAIL NOTE: DUCT DRAIN FOR U-GIRDER SHOWN, I-GIRDER SIMILAR.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

> STANDARD POST-TENSIONING OF CONCRETE GIRDERS GROUTING SPECIFICATIONS

BUREAU OF PROJECT DELIVERY

RECOMMENDED JAN. 31, 2019

Thomas P. Marcira

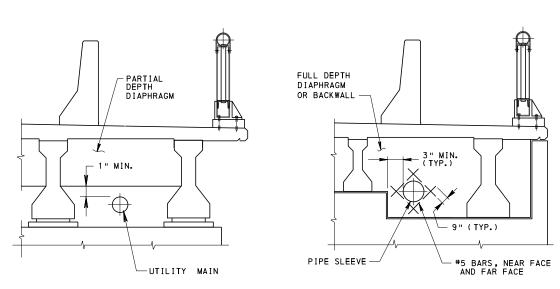
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SHEET 1 OF 1

REFERENCE DRAWINGS

ACTING DIR. BUR. OF PROJECT DELIVERY

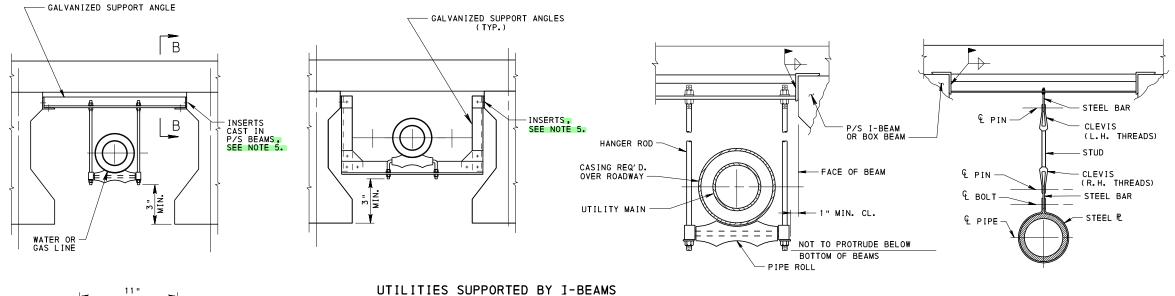


LOCATION OF SLEEVES OR CASINGS

¾"Ø HEX HEAD FLANGE BOLTS (GALVANIZED)

NUT & LOCKWASHER (GALV.)

-SUPPORT ANGLE WITH SLOTTED HOLES.



CHANGE 2

21/2"

GALVANIZED — STEEL ANGLE BRACKET W/ SLOTTED HOLES

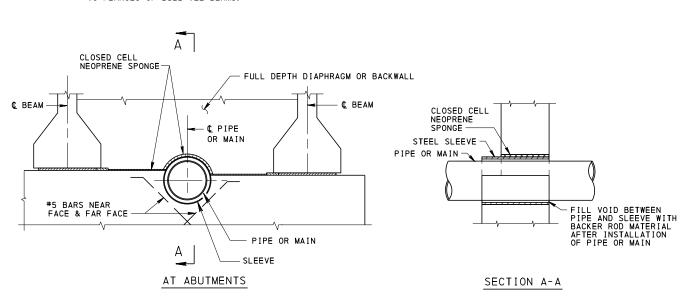
SECTION B-B

•

SECTION C-C

(TYP.)

- SPREAD BOX BEAMS ARE SIMILAR.
- INSERTS ARE NOT PERMITTED FOR CONNECTION TO FLANGES OF BULB-TEE BEAMS.



CASINGS AND SLEEVES

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF PROJECT DELIVERY

STANDARD UTILITY ATTACHMENT & SUPPORT DETAILS PRESTRESSED BRIDGES

RECOMMENDED JAN. 31, 2019 Thomas P. Marcira CHIEF BRIDGE ENGINEER

GENERAL NOTES:

1. NO UTILITIES MAY PROTRUDE BELOW THE BOTTOM OF THE BEAMS (EXCEPTIONS AT END SPANS NOT OVER TRAFFIC, SUBJECT TO APPROVAL).

NO UTILITIES MAY BE HUNG UNDER THE OVERHANG OR FROM THE DECK, EXCEPT WHERE THERE IS NO OTHER FEASIBLE SOLUTION. THIS IS SUBJECT TO THE BRIDGE ENGINEER'S APPROVAL.

DRILLING IN P/S BEAMS OR FIELD WELDING OF STEEL BEAMS MUST BE EVALUATED ON A CASE BY CASE BASIS AND APPROVED BY THE BRIDGE ENGINEER.

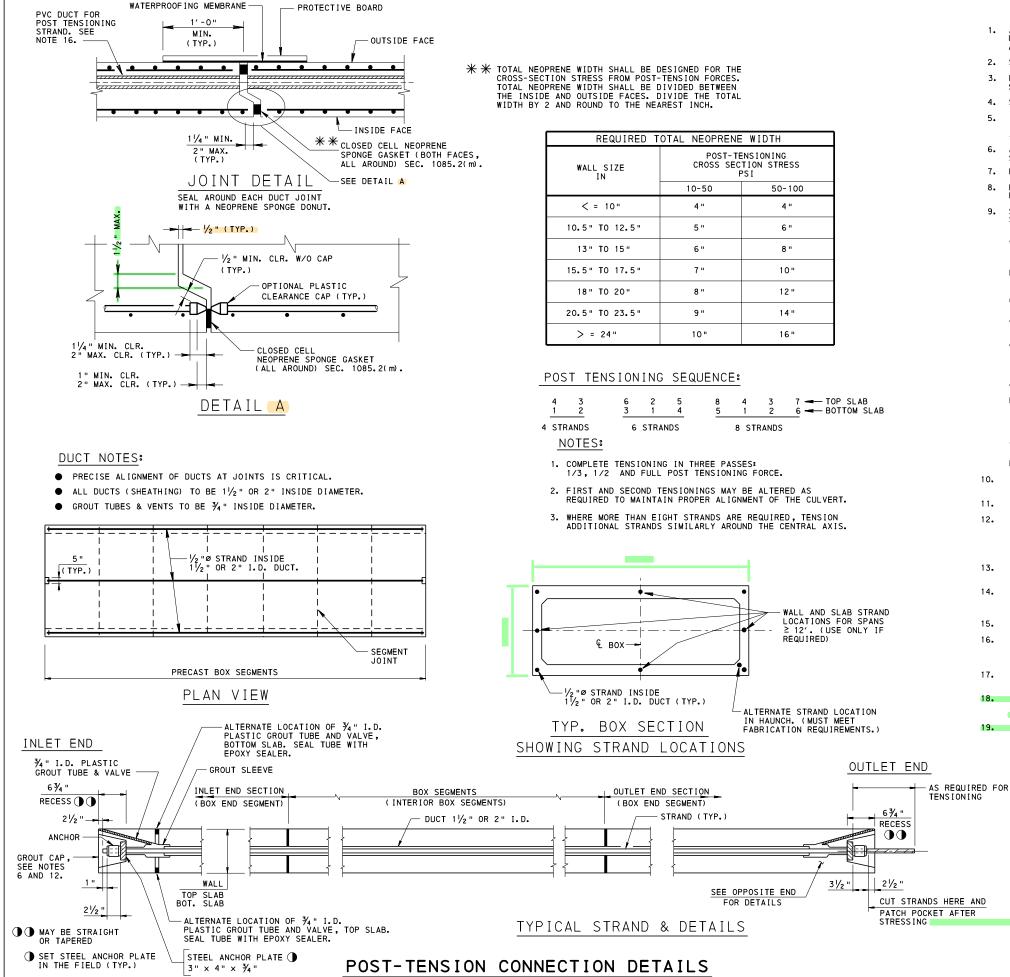
4. ALL HANGERS, SUPPORTS AND THEIR ASSOCIATED HARDWARE TO BE EITHER GALVANIZED OR ZINC RICH PRIMER AND APPLY FINISH COAT TO MATCH STEEL BEAM COLOR.

5. INSERTS TO BE GALVANIZED OR COMPLETLY ZINC-ELECTROPLATED.

RECOMMENDED JAN. 31, 2019

Millia Hadre ACTING DIR. BUR. OF PROJECT DELIVERY BC-794M

SHEET 1 OF 1



CHANGE 2

CHANGE 4

CHANGE 5

INSTRUCTIONS FOR POST-TENSIONING

- ALWAYS USE POST-TENSIONING WHEN END WALLS ARE NOT USED. USE THIS DETAIL WITH PRECAST OR CAST-IN-PLACE END WALLS AS PER DESIGN DRAWINGS.
- 2. SHOW ALL DETAILS ON SHOP DRAWINGS.
- 3. PROVIDE $\ensuremath{V_2}\xspace$ " DIAMETER POLY STRANDS OR APPROVED EQUAL HAVING A YIELD STRENGTH OF 270 KSI.
- 4. SNUG FIT ALL JOINTS BEFORE POST-TENSIONING.
- 5. INSTALL STRANDS IN PRECAST SECTIONS. STRESS EACH STRAND TO AN EFFECTIVE FORCE OF 10 PSI OVER THE CROSS SECTION OF ANY SECTION. CHECK RAM AREA AND CALIBRATION CURVES OF EQUIPMENT FURNISHED FOR GAGE PRESSURES.
- 5. AFTER STRESSING, GROUT ALL STRAND DUCTS. REFER TO PUB.408 SEC.1085 FOR TIME LIMITATIONS ASSOCIATED WITH GROUTING.
- 7. PLACE GROUT MIX INTO TUBING USING PRESSURE GROUT.
- PROVIDE POST TENSIONING OPERATIONS AND MATERIALS IN ACCORDANCE WITH PUBLICATION 408, SECTION 1108. SHOP DRAWINGS ARE REQUIRED.
- SUBMIT POST TENSIONING COMPUTATIONS WITH SHOP DRAWINGS SHOWING THE STRAND PATTERN AND REQUIRED POST-TENSIONING FORCE. BASE DESIGN UPON THE FOLLOWING CRITERIA:

b. MAXIMUM TOTAL POST TENSION FORCE SHOULD NOT CREATE A PRESSURE

- d. THE TOTAL POST TENSION FORCE IS THE SUM OF THE FORCE REQUIRED TO OVERCOME SOIL FRICTION PLUS THE FORCE REQUIRED TO CREATE A PRESSURE OF 10 PSI OVER THE CROSS SECTION OF THE CULVERT.
- GREATER THAN 100 PSI OVER THE CROSS SECTION OF ANY SEGMENT.
- d. MAXIMUM LOAD ON A $\frac{1}{2}$ " DIAMETER STRAND IS 29 KIP. USE 0.6" DIAMETER STRAND WITH HIGHER LOAD WHEN PERMITTED.
- e. USE A COFFICIENT OF SOIL FRICTION OF 0.6.

C. MINIMUM TOTAL POST TENSION FORCE IS 100 KIPS.

- f. PLACE STRANDS SYMMETRICALLY ABOUT BOTH AXES OF THE CULVERT CROSS SECTION.
- g. USE A MINIMUM OF 4 STRANDS.
- h. MAXIMUM STRAND SPACING IS 8'-0", EXCEPT FOR CULVERTS LESS THAN 12'-0" SPAN.
- i. MINIMUM STRAND SPACING IS 2'-0".
- j. PLACE CORNER STRANDS AT THE LOCATION OF CENTERLINES BETWEEN WALL AND SLAB OR AT A MAXIMUM DISTANCE OF 2'-0" FROM THIS LOCATION.
- K. LOCATE STRANDS SO AS TO NOT INTERFERE WITH REINFORCEMENT DETAILS.
- O. PROVIDE SEALS OR GASKETS AROUND THE DUCTS AT THE JOINTS TO MAKE THE JOINTS GROUT TIGHT.
- 11. ALL POST-TENSIONING MUST BE WITNESSED BY THE ENGINEER.
- 12. AFTER POST-TENSIONING IS APPROVED, CUT STRANDS TO PROVIDE A MINIMUM OF 2½" CLEAR FROM OUTSIDE FACE OF CONCRETE AND COAT RECESS WITH EPOXY BONDING COMPOUND. FILL ALL RECESSES WITH AN APPROVED PRODUCT LISTED IN BULLETIN 15 UNDER MISCELLANEOUS POLYMER MODIFIED AND SPECIAL CEMENTS, MORTARS AND CONCRETES TO FORM A SEAL AND CAP.
- 3. POST-TENSION AND GROUT BEFORE BACKFILLING AND PLACING TRAFFIC
- 4. ALL POST TENSIONING CHUCKS MUST BE OF THE REUSABLE TYPE. OPERATORS MUST EXERCISE PROPER PRECAUTIONS WHEN RE-ALIGNING WEDGES AFTER RELEASE OF TENDONS AND PRIOR TO RETENSIONING AND RE-SEATING.
- 15. KEEP JOINT CLEAN AT POST-TENSIONING STAGE.
- 6. POST TENSIONING DUCTS MAY BE PLACED WITHIN THE WALLS OR SLAB ANYWHERE BETWEEN THE LAYERS OF REINFORCEMENT TO AVOID THE SLOPED PORTION OF THE JOINT SO AS TO PROMOTE SEALING OF THE DUCT.
- 17. REMOVE A MINIMAL AMOUNT OF POLYSTRAND TO ACCOMODATE INTERMEDIATE SPLICES AT BOX ENDS.
- 18. ALTERNATE POST-TENSIONING SEQUENCE VARYING FROM DETAILS SHOWN ON THIS STANDARD MUST BE DETAILED ON SHOP DRAWINGS AND ACCEPTED BY DISTRICT BRIDGE ENGINEER.
- 9. POST TENSIONING DUCTS MUST BE ADEQUATELY SECURED TO PREVENT DEFLECTION DURING CONCRETE PLACEMENT. DUCTS THAT ARE NOT STRAIGHT MUST BE ACCEPTED BY CHIEF STRUCTURAL MATERIALS ENGINEER.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BURBAU OF BRIDGE

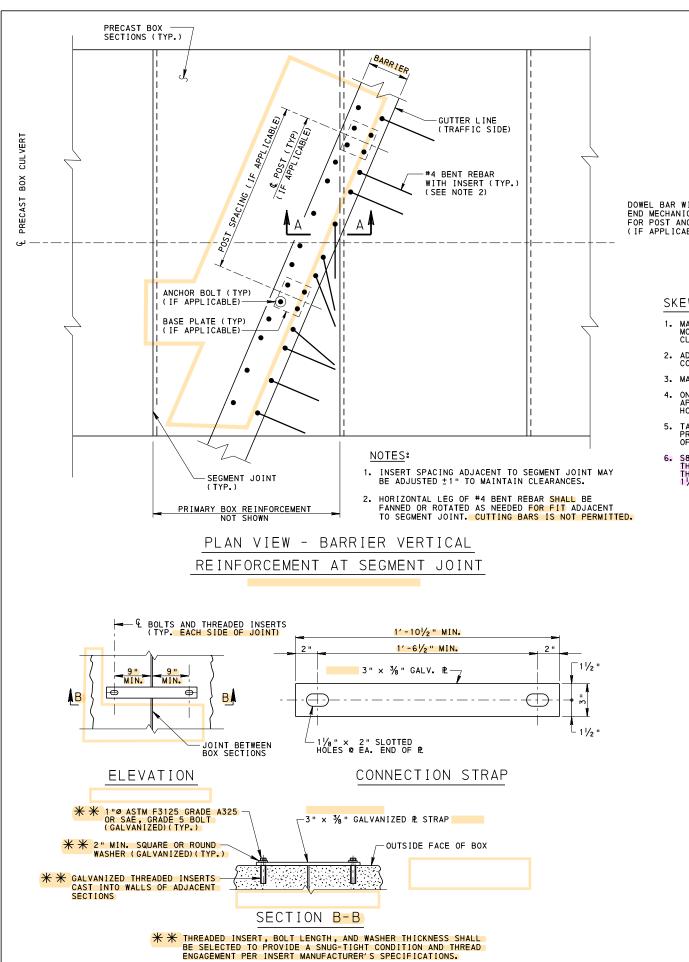
STANDARD
MECHANICAL CONNECTION DETAILS
PRECAST BOX CULVERT

RECOMMENDED FEB. 14, 2023

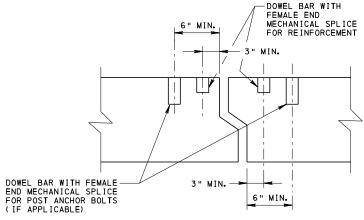
RECOMMENDED FEB. 14, 2023

LAWN E. LAWN
CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 1 OF 3
BC-798M



GALVANIZED STRAP CONNECTION DETAIL



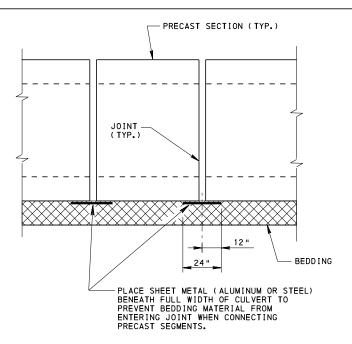
SKEWED BARRIER LAYOUT GUIDELINES:

1. MAINTAIN THE REQUIRED POST SPACING (IF APPLICABLE) AND MOVE THE POSTS AS A GROUP TO AVOID CONFLICTS WITH THE CULVERT JOINTS.

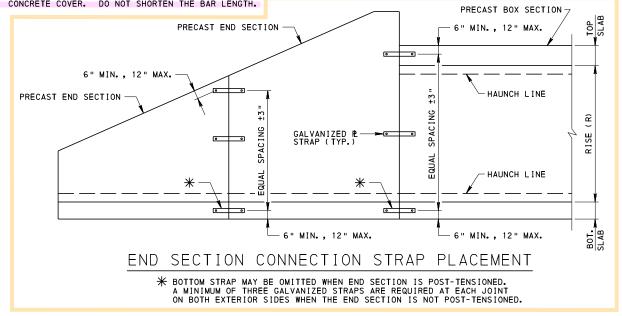
SECTION A-A

(SEGMENT JOINT)

- 2. ADJUST CULVERT SEGMENT LENGTHS WHEN POSSIBLE TO AVOID CONFLICTS WITH POSTS (IF APPLICABLE).
- 3. MAINTAIN THE MINIMUM EDGE DISTANCES SHOWN ON SECTION A-A.
- 4. ONE (1) OF THE FOUR (4) ANCHOR BOLTS FOR A POST (IF APPLICABLE) MAY USE A LOOP FERRULE INSERT SUBSTITUTE OR HOOK BOLT EMBEDDED ONLY IN THE CURB CONCRETE.
- 5. TAILS OF THE DOWELS MAY BE ROTATED IN ANY DIRECTION TO PROVIDE 1½" MIN. CONCRETE COVER. DO NOT SHORTEN TAILS OF THE DOWEL BARS.
- 6. S8 BARS FOR THE PA 3-RAIL BRIDGE BARRIER ARE NOT SHOWN IN THE PLAN VIEW BUT NEED TO BE INCLUDED. IF REQUIRED BEND THE BARS NEAR THE GUTTER LINE AND ROTATE BARS TO PROVIDE 11/2" MIN. CONCRETE COVER. DO NOT SHORTEN THE BAR LENGTH.



ELEVATION VIEW AT PRECAST SEGMENT JOINTS



COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGE

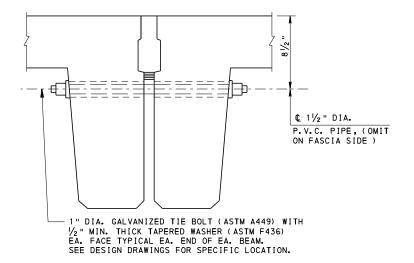
STANDARD
MECHANICAL CONNECTION DETAILS
PRECAST R.C. BOX CULVERT

RECOMMENDED FEB. 14, 2023

RECOMMENDED FEB. 14, 2023

LAWN E. LAWN
CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 2 OF 3
BC-798M



TIE BOLT DETAIL - PRECAST CHANNEL BEAM

COMMONWEALTH OF PENNSYLVANIA
DEPARTMENT OF TRANSPORTATION
BUREAU OF BRIDGE

STANDARD
MECHANICAL CONNECTION DETAILS
PRECAST SLAB AND
PRECAST CHANNEL BEAM

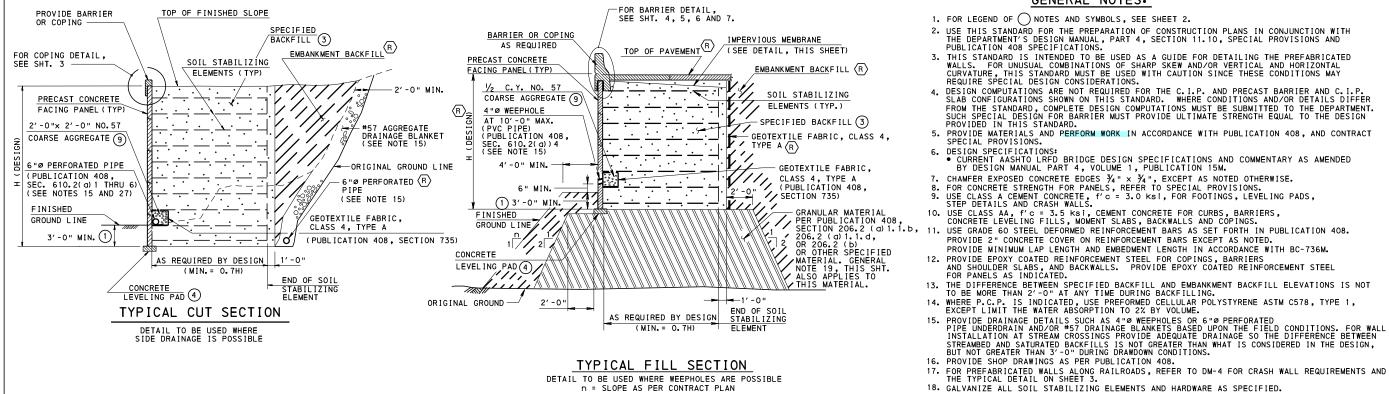
RECOMMENDED FEB. 14, 2023

RECOMMENDED FEB. 14, 2023

Haw E. Hray

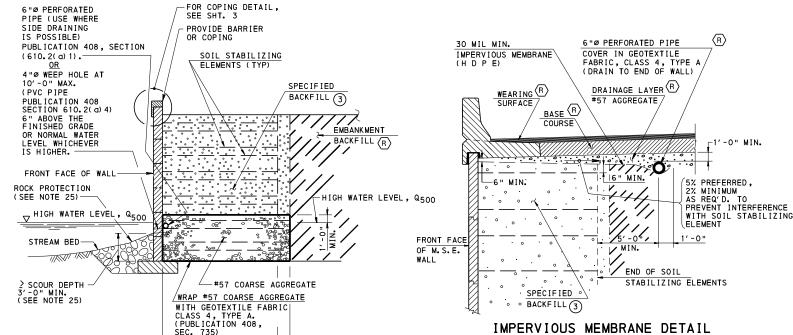
CHIEF ENGINEER, HIGHWAY ADMIN

SHEET 3 OF 3
BC-798M



TYPICAL FILL SECTION

DETAIL TO BE USED WHERE WEEPHOLES ARE POSSIBLE n = SLOPE AS PER CONTRACT PLAN



$(MIN_{\bullet} = 0.7H)$ TYPICAL SECTION AT STREAM

AS REQUIRED BY DESIGN

DETAIL TO BE USED WHERE HIGH WATER LEVEL IN FRONT OF THE WALL IS ANTICIPATED, AND THE STREAM VELOCITY, V, IS LESS THAN 2.0 fps

SHEET NO. DETAILS AND GENERAL NOTES CRASH WALL AND MISCELLANEOUS WALL DETAILS C. I.P. TRAFFIC BARRIER PRECAST TRAFFIC BARRIER MOMENT SLAB AND BARRIER JOINT SIDEWALK AND ALTERNATE BARRIER AND GUIDE RAIL TRANSITION DRAINAGE INSTALLATIONS
SHOULDER RELIEF JOINT AND INLET INSTALLATION REINFORCED EARTH WALL PANELS REINFORCED EARTH WALL PANELS RETAINED EARTH WALL PANELS RETAINED EARTH WALL PANEL AND WIRE MESH TOLERANCES

CONCRETE AND STEEL I-BEAM BRIDGES CEMENT CONCRETE PAVEMENT JOINTS RC-27M | PLAIN CONCRETE PAVEMENT RC-50M GUIDE RAIL TO BRIDGE BARRIER TRANSITIONS

REFERENCE DRAWINGS

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION

STANDARD

MECHANICALLY STABILIZED EARTH RETAINING WALLS DETAILS AND GENERAL NOTES

NOV.23, 2022

GENERAL NOTES:

2. USE THIS STANDARD FOR THE PREPARATION OF CONSTRUCTION PLANS IN CONJUNCTION WITH THE DEPARTMENT'S DESIGN MANUAL, PART 4, SECTION 11.10, SPECIAL PROVISIONS AND PUBLICATION 408 SPECIFICATIONS.

3. THIS STANDARD IS INTENDED TO BE USED AS A GUIDE FOR DETAILING THE PREFABRICATED WALLS. FOR UNUSUAL COMBINATIONS OF SHARP SKEW AND/OR VERTICAL AND HORIZONTAL CURVATURE, THIS STANDARD MUST BE USED WITH CAUTION SINCE THESE CONDITIONS MAY REQUIRE SPECIAL DESIGN CONSIDERATIONS.

PROVIDE 2" CONCRETE COVER ON REINFORCEMENT BARS EXCEPT AS NOTED.

DESIGN COMPUTATIONS ARE NOT REQUIRED FOR THE C.I.P. AND PRECAST BARRIER AND C.I.P. SLAB CONFIGURATIONS SHOWN ON THIS STANDARD. WHERE CONDITIONS AND/OR DETAILS DIFFER FROM THE STANDARD, COMPLETE DESIGN COMPUTATIONS MUST BE SUBMITTED TO THE DEPARTMENT. SUCH SPECIAL DESIGN FOR BARRIER MUST PROVIDE ULTIMATE STRENGTH EQUAL TO THE DESIGN PROVIDED IN THIS STANDARD.

PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH PUBLICATION 408, AND CONTRACT SPECIAL PROVISIONS.

19. REMOVE UNSUITABLE OR UNSTABLE FOUNDATION MATERIAL BELOW THE TOP OF LEVELING PAD AND REPLACE WITH SPECIFIED GRANULAR MATERIAL. PRIOR TO WALL CONSTRUCTION, COMPACT THE FOUNDATION AREA WITH A SMOOTH VIBRATORY ROLLER ACCORDING TO PUB. 408.

20. BACKFILL MATERIALS (DESIGN PROPERTIES):

WEIGHT OF SPECIFIED BACKFILL = 90-120 LBS. PER CUBIC FOOT
PER THE DESIGN AND AS SPECIFIED ON DESIGN DRAWINGS. #57 COARSE AGGREGATE
IS PERMITTED AS SPECIFIED BACKFILL IF MATERIAL MEETS REQUIREMENTS OF

21. THE M.S.E. WALL DESIGNER/SUPPLIER MUST CERTIFY ALL ASSUMPTIONS MADE IN THE DESIGN. PLACE THE FOLLOWING NOTE NEAR THE P.E. SEAL ON THE FIRST SHEET OF THE DRAWINGS. "I CERTIFY THAT ALL ASSUMPTIONS MADE IN DESIGNING THIS WALL HAVE BEEN VALIDATED THROUGH CONSTRUCTION DETAILS, SPECIAL NOTES AND/OR INSTRUCTIONS TO THE FABRICATOR, ERECTOR AND CONTRACTOR."

22. SOME OF THE TECHNICAL DETAILS WERE PROVIDED BY REINFORCED EARTH COMPANY AND RETAINED EARTH (FOSTER GEOTECHNICAL) COMPANY. FOR PROPRIETARY RIGHTS CONTACT APPROPRIATE

DO NOT CUT REINFORCEMENT STRIPS OR MESH. BEND OR SKEW ONLY AS SHOWN IN DETAIL OR NOTES ON SHEETS 2, 8 & 11.

26. PUNCTURING IMPERVIOUS MEMBRANE IS TO BE AVOIDED, FORMWORK SUPPORTS UTILIZING STAKES DRIVEN THROUGH IMPERVIOUS MEMBRANE ARE PROHIBITED. IMPERVIOUS MEMBRANE'S INTEGRITY MUST NOT BE COMPROMISED.

27. METAL PIPES AS SPECIFIED IN PUBLICATION 408, SECTIONS 610.2(a) 7 AND 8, ARE NOT PERMITTED.

24. DURING THE SPECIFIED BACKFILL PLACEMENT, KEEP THE BACKFILL AT OR JUST ABOVE THE REINFORCEMENT CONNECTION TO PANEL, PRIOR TO MAKING THE CONNECTION. REMOVE AND REPLACE ANY FACE PANEL THAT DOES NOT MEET CONSTRUCTION TOLERANCE, SPECIFIED IN THE SPECIAL PROVISIONS. AVOID PLACING HEAVY EQUIPMENT OVER BACKFILL COVERING WALL STRAPS WHICH MIGHT CAUSE MOVEMENT OF WALL PANELS.

28. REFER TO SHEET 8 FOR HORIZONTAL DRAINAGE PIPES WHICH ARE INSTALLED WITHIN SPECIFIED BACKFILL AREA.

29. THE 45" F-SHAPE CONCRETE BARRIER IS NOT PERMITTED ON MOMENT SLABS.

25. PROVIDE ROCK PROTECTION AS REQUIRED BY SCOUR CALCULATIONS.

1. FOR LEGEND OF () NOTES AND SYMBOLS, SEE SHEET 2.

EMBANKMENT BACKFILL \$ = 30°

SPECIAL PROVISIONS.

SPECIFIED BACKFILL ϕ = 34° #57 COARSE AGGREGATE USE ϕ = 34°

RECOMMENDED NOV. 23, 2022 Havin E. Gray

BC-799M

SHEET 1 OF 13

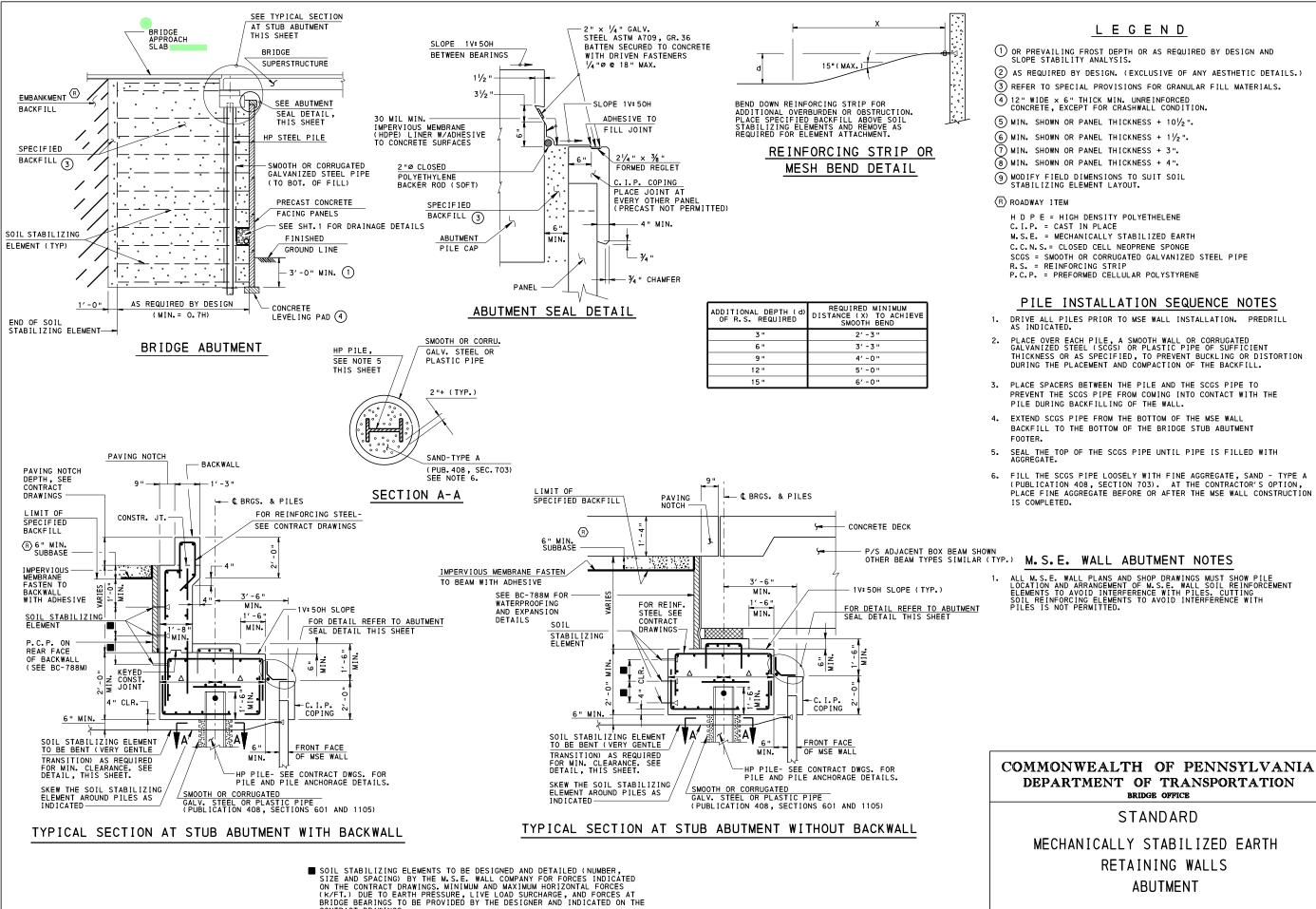
30. THE 42" F-SHAPE CONCRETE BARRIER ATTACHED TO A MOMENT SLAB IS DESIGNATED AS MASH TL-4. THE 32" F-SHAPE CONCRETE BARRIER ATTACHED TO MOMENT SLAB IS DESIGNATED AS MASH TL-3. THE ALTERNATE SIDEWALK WITH 42" VERTICAL WALL CONCRETE BARRIER ATTACHED TO A MOMENT SLAB IS DESIGNATED AS MASH TL-2 NOTE: PROVIDE IMPERVIOUS MEMBRANE WHEN SPECIFIED. BC-735M WALL CONSTR. & EXP. JOINTS DETAILS INDEX OF SHEETS BC-736M REINFORCEMENT BAR FABRICATION DETAILS BRIDGE OFFICE BC-752M CONCRETE DECK SLAB DETAILS BC-767M NEOPRENE STRIP SEAL DAM FOR PRESTRESSED BC-788M TYP. WATERPROOFING AND EXPANSION DETAILS RC-11M | CLASSIFICATION OF EARTHWORK FOR STRUCTURES RC-12M BACKFILL AT STRUCTURES

NOTE: Strike-off Letter revisions on Shts. 4, 5 & 7

CHANGE 2

CHANGE 3

CHANGE 4

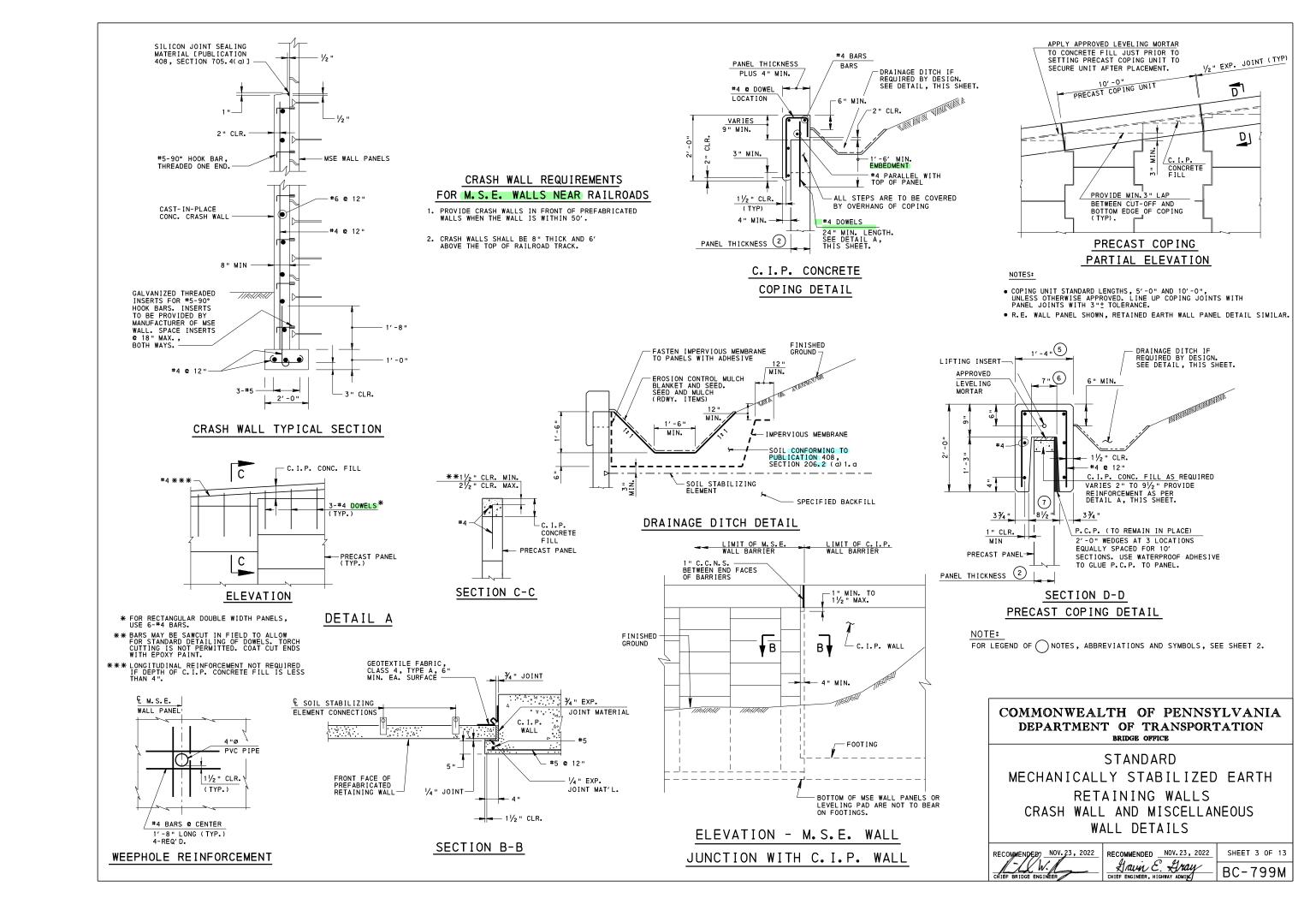


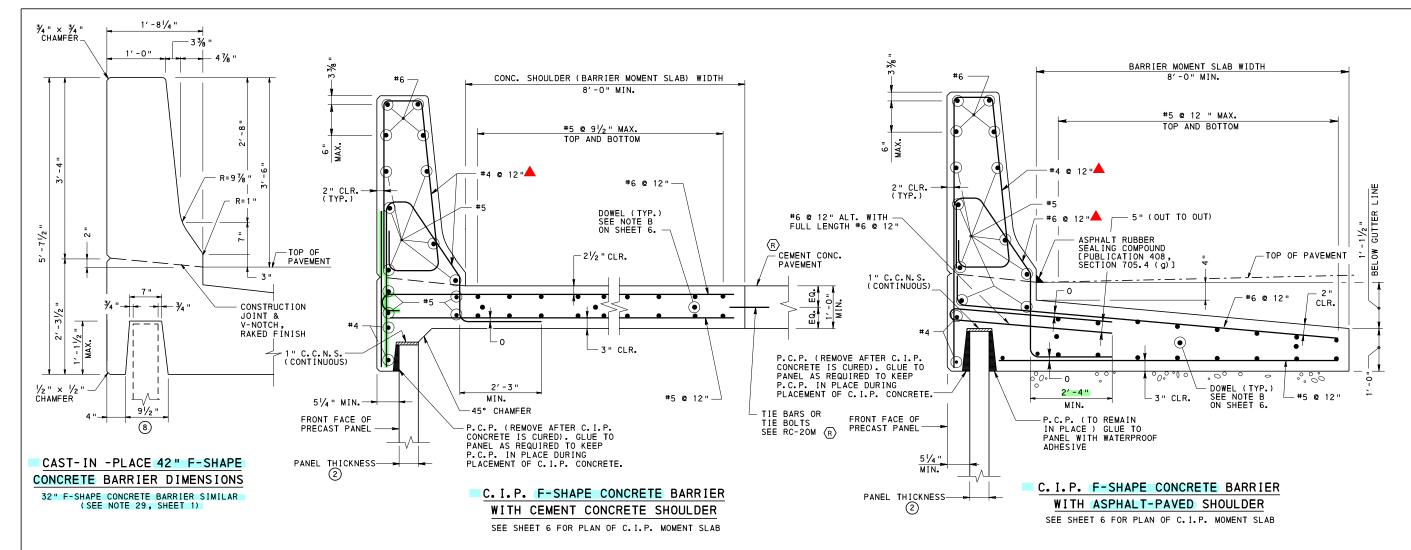
CONTRACT DRAWINGS.

Δ 1/2 OF PILE CAP DESIGN WIDTH

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Havin E. Gray





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▲ WITHIN 10 FT. OF AN EXPANSION JOINT AND AT THE END OF THE BARRIER, REDUCE SPACING OF VERTICAL REINFORCMENT BARS IN THE BARRIER TO HALF THE SHOWN SPACING.

TRAFFIC BARRIER AND MOMENT SLAB NOTES:

- PLACE EXPANSION JOINT IN BARRIER WITH PAVEMENT JOINT, EXCEPT NOT TO FALL WITHIN 6'-0" OF CENTERLINE OF LIGHT POLE OR 2'-0" OF CENTERLINE OF JUNCTION BOX. SEE SHEET 9 FOR INLET INSTALLATION DETAILS.
- 2. FOR BRIDGE BARRIER TO GUIDE RAIL TRANSITION, SEE SHEET. 7.

NOTE:

1. FOR LEGEND OF NOTES, ABBREVIATIONS AND SYMBOLS, SEE SHEET 2.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

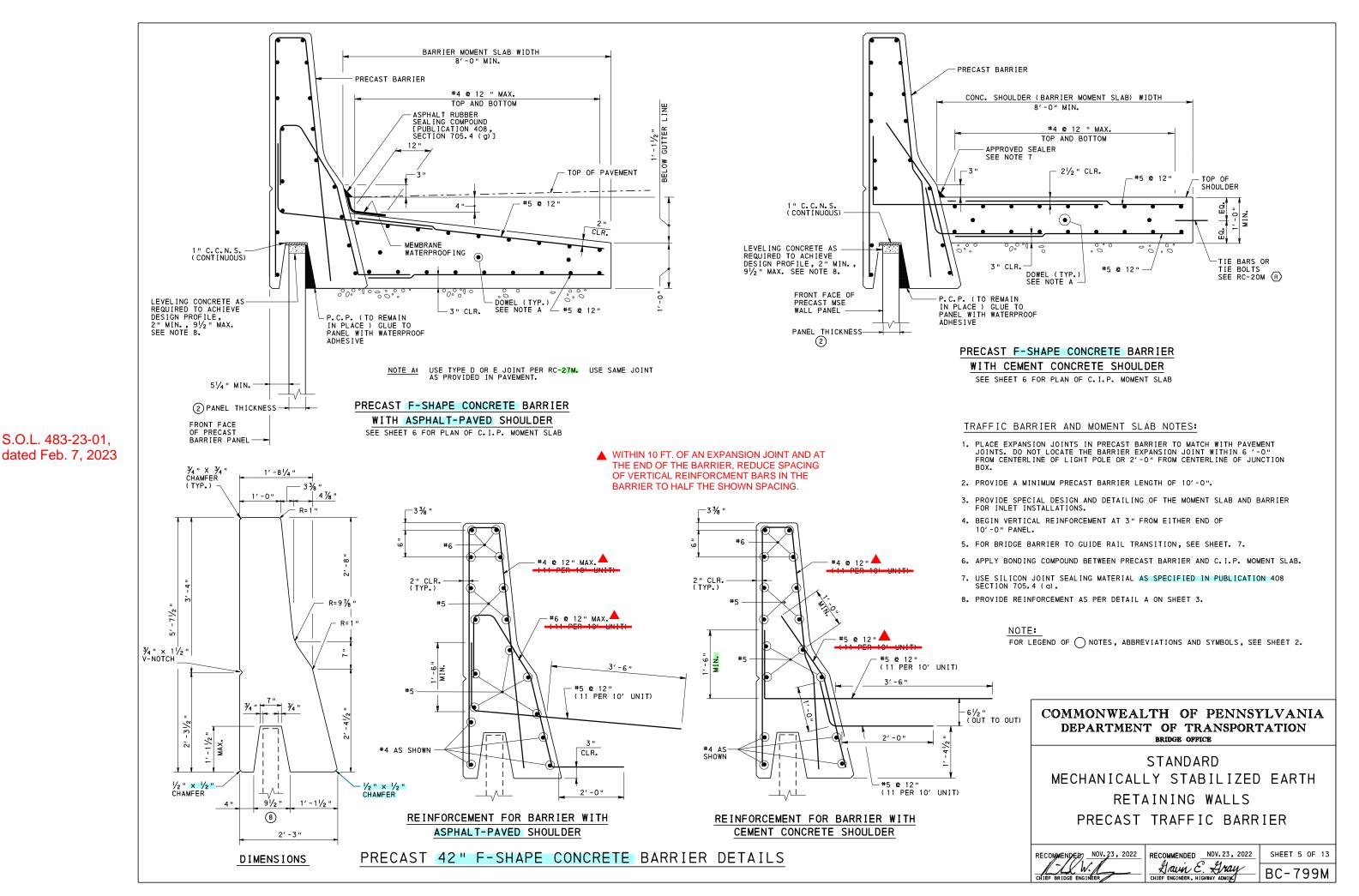
MECHANICALLY STABILIZED EARTH
RETAINING WALLS

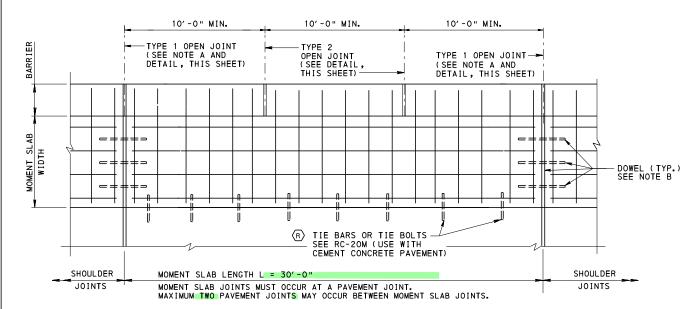
C. I.P. TRAFFIC BARRIER

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Lawn E. Gray
CHIEF ENGINEER, HIGHBAY ADMIN BC-799M



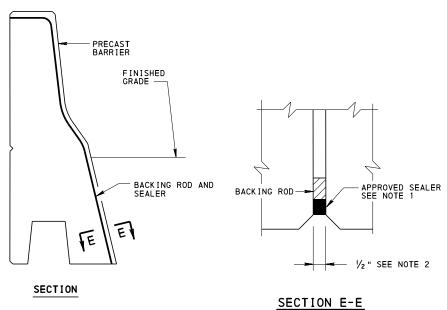


NOTE A: PROVIDE TYPE 1 OPEN JOINTS AT SAME LOCATIONS AS THOSE PROVIDED FOR THE MOMENT SLAB.

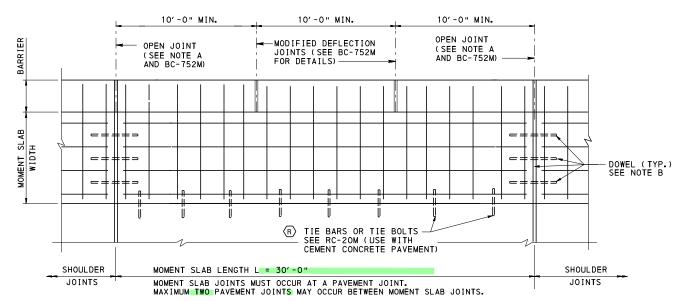
NOTE B: USE TYPE D OR E JOINT PER RC-27M. USE SAME JOINT AS PROVIDED IN PAVEMENT.

PLAN - BARRIER MOMENT SLAB

(PRECAST BARRIER)



TYPE 1 AND TYPE 2 OPEN JOINT IN PRECAST F-SHAPE CONCRETE BARRIER



NOTE A: PROVIDE OPEN JOINTS IN BARRIER AT SAME LOCATIONS AS THOSE PROVIDED FOR THE MOMENT SLAB.

NOTE B: USE TYPE D OR E JOINT PER RC-27M. USE SAME JOINT AS PROVIDED IN PAVEMENT.

PLAN - BARRIER MOMENT SLAB

(C.I.P. BARRIER)

BARRIER MOMENT SLAB NOTES:

- 1. ALL OPEN JOINTS IN THE BARRIER MUST BE FILLED WITH BACKING ROD AND SEALED WITH SILICONE
 JOINT SEALING MATERIAL AS SPECIFIED IN PUBLICATION 408, SECTION 705.4 (a).
- 2. EXPOSED JOINTS & BARRIER MAY VARY FROM 1/2" TO 1" WIDTH FOR TYPE 1 OPEN JOINT AND 1/4" TO 3/4" WIDTH FOR TYPE 2 OPEN JOINT, TO ALLOW FOR HORIZONTAL AND/OR VERTICAL CURVATURE

NOTE:

FOR LEGEND OF SYMBOLS, SEE SHEET 2.

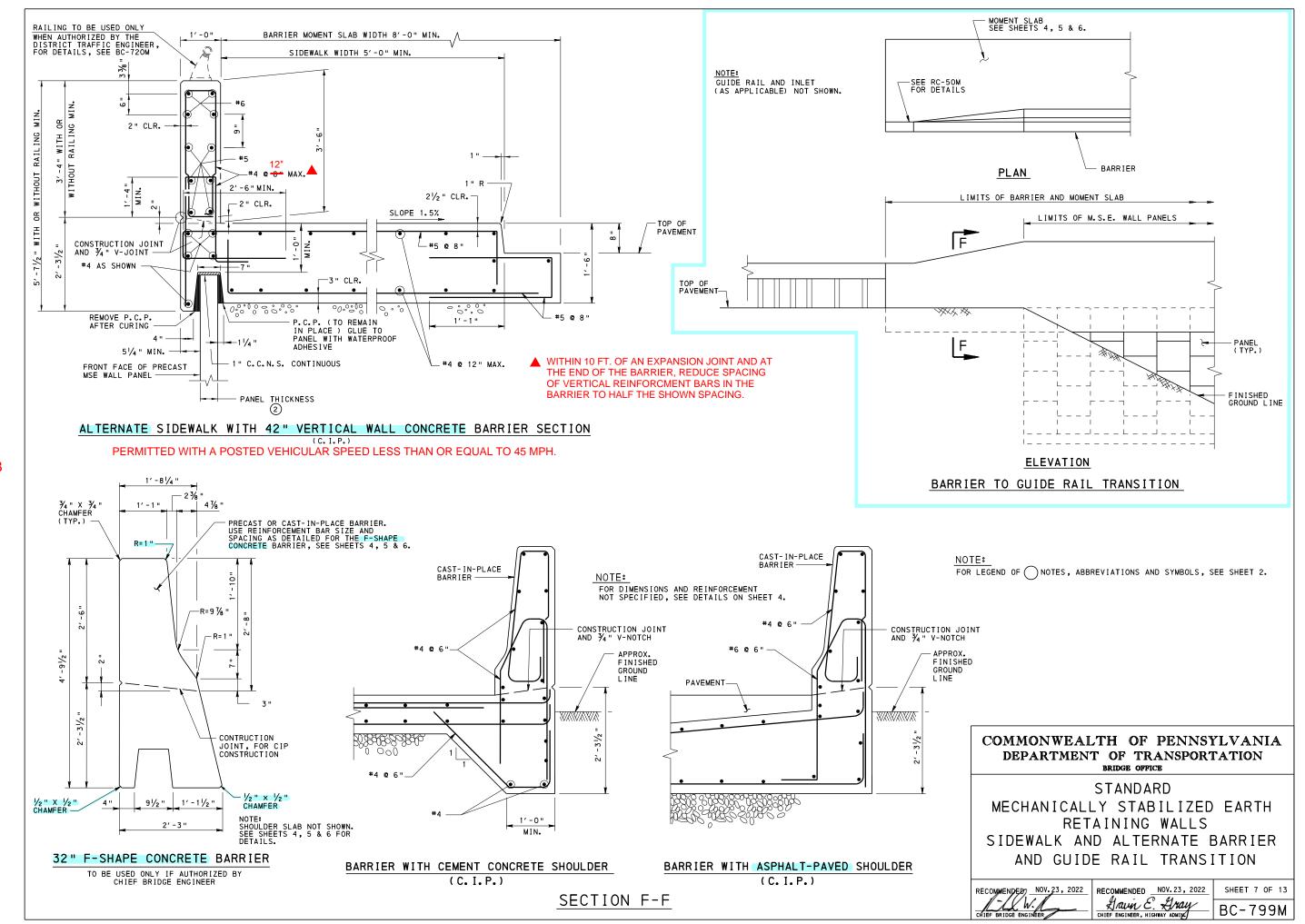
COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD MECHANICALLY STABILIZED EARTH RETAINING WALLS MOMENT SLAB AND BARRIER JOINT

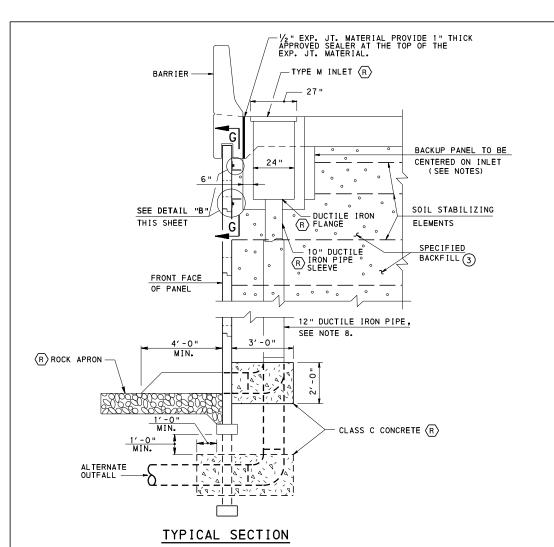
RECOMMENDED NOV. 23, 2022

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SHEET 6 OF 13 Havin E. Hray
CHIEF ENGINEER, HIGHWAY ADMIN BC-799M

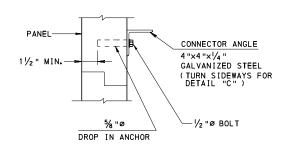


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CONNECTOR ANGLE SEE DETAIL B, AS PER DESIGN THIS SHEET (TYP) % "×¾ " SLOTTED HOLE IN CONN. ANGLE FOR 1/2 "Ø GALV. BOLT ANCHOR IN PANEL (TYP.) HOLE SPACING 3" (TYP.) AS SPECIFIED BY THE PANEL MANUFACTURER PANEL AS PER DESIGN MIN. MIN.

SECTION G-G

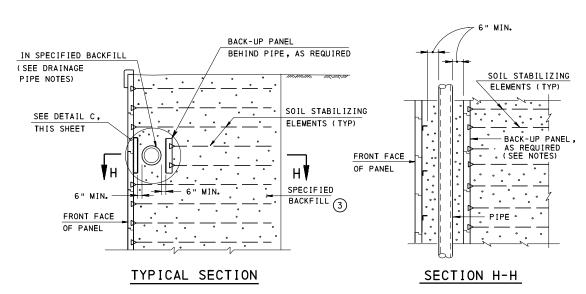


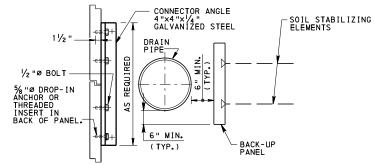
DETAIL B

DRAINAGE PIPE NOTES :

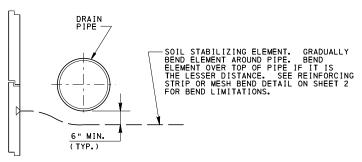
- 1. FOR HORIZONTAL DRAIN PIPES, PROVIDE NONFERROUS PIPE WITH A 100-YEAR DESIGN LIFE AND WATERTIGHT JOINTS. PROVIDE POLYETHYLENE PIPES CONFORMING TO PUBLICATION 408, SECTION 601.2(a)6.g FOR THERMOPLASTIC PIPES, PROVIDE WATERTIGHT JOINTS IN ACCORDANCE WITH ASTM D3212. FOR CONCRETE PIPES, PROVIDE WATERTIGHT JOINTS IN ACCORDANCE WITH ASTM C443. TAKE SPECIAL CARE IN DETAILING TO MAINTAIN PIPE JOINTS INTACT.
- 2. TAKE SPECIAL CARE TO PROPERLY COMPACT GRANULAR BACKFILL AROUND PIPE SO AS NOT TO DAMAGE IT. USE LIGHT MECHANICAL TAMPER.
- 3. THE NUMBER OF BACKUP PANELS ARE TO BE DETERMINED BY THE CONTRACTOR AND APPROVED BY THE ENGINEER.
- 4. THE COST FOR CONNECTOR DEVICES IS INCIDENTAL TO THE BID PRICE FOR CONTRACT ITEMS.
- 5. GALVANIZE ALL CONNECTOR ANGLES, BOLTS AND ANCHORS.
- 6. IF NECESSARY, MAKE MODIFICATIONS TO THESE DETAILS ON THE CONSTRUCTION PLANS.
- 7. DRAINAGE PIPES WITHIN MECHANICALLY STABILIZED EARTH WALLS IS NOT A PREFERRED CONDITION. DRAINAGE SHOULD BE EXITED OUTSIDE THE WALL IN ACCORDANCE WITH THE INLET BEHIND WALL DETAIL, THIS SHEET, WHENEVER POSSIBLE.
- 8. THE NON-FERROUS PIPE REQUIREMENTS IN NOTE 1, DO APPLY TO VERTICAL PIPES.

INLET BEHIND WALL





DETAIL C - WITH BACKUP PANEL



DETAIL C - WITHOUT BACKUP PANEL

DRAINAGE PIPE BEHIND WALL

USE FOR PIPES LARGER THAN 12" AND LESS THAN 30" O.D. DRAINAGE INSTALLATIONS WITH PIPES LARGER THAN 30" O.D. REQUIRES APPROVAL FROM THE CHIEF BRIDGE ENGINEER. SEE NOTE 7 FOR ADDITIONAL INFORMATION.

NOTE

FOR LEGEND OF () NOTES, ABBREVIATIONS AND SYMBOLS, SEE SHEET 2.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

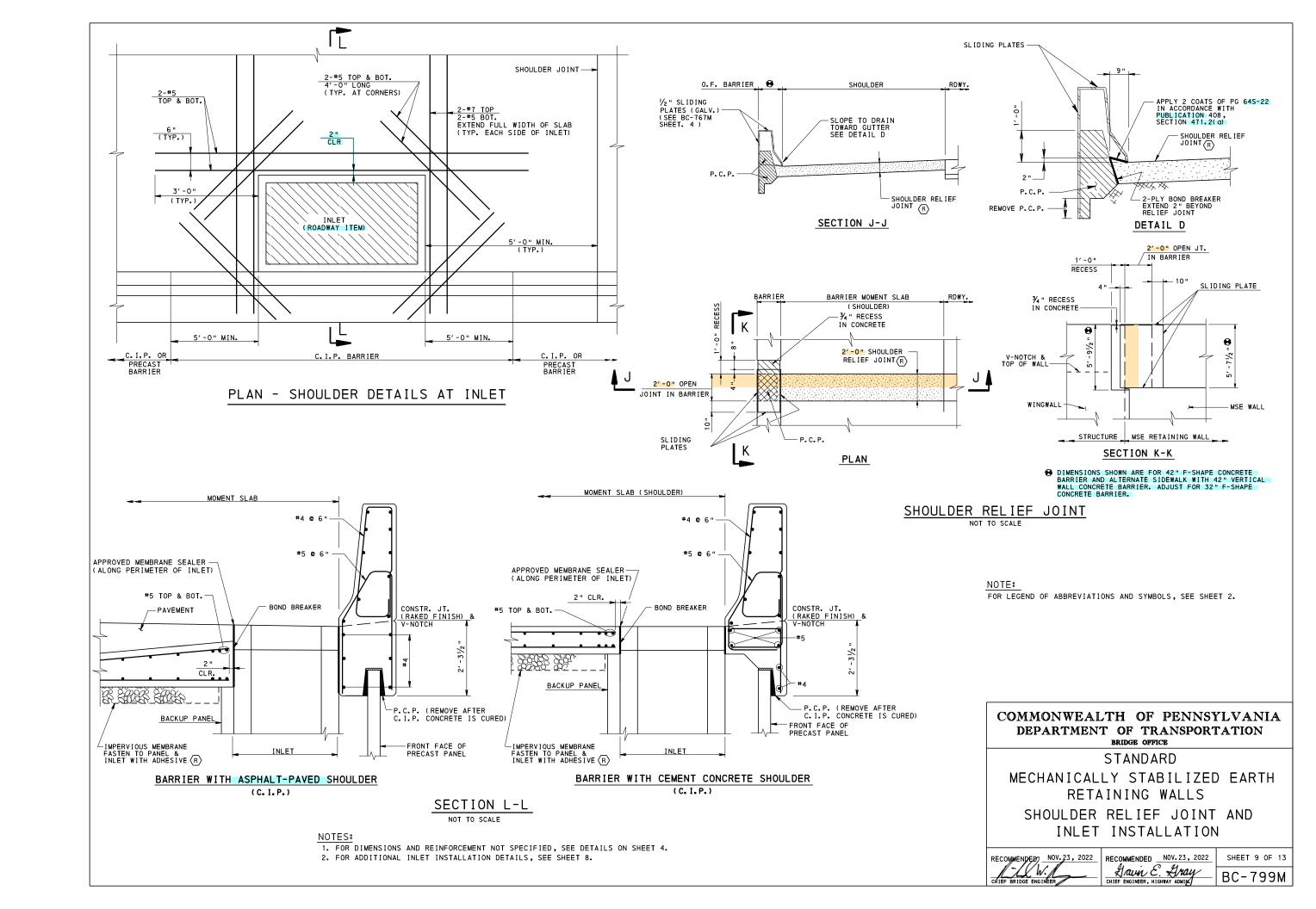
STANDARD

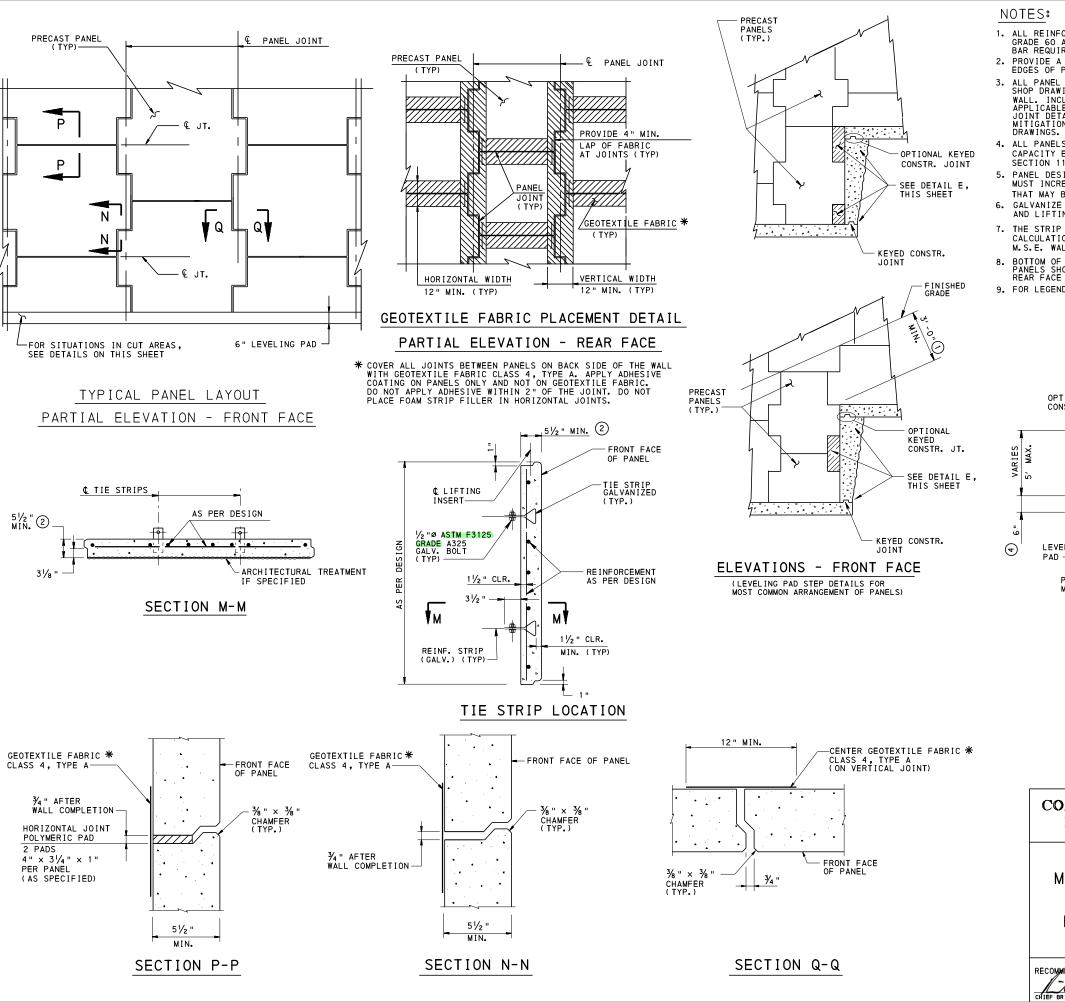
MECHANICALLY STABILIZED EARTH RETAINING WALLS DRAINAGE INSTALLATIONS

RECOMMENDED NOV. 23, 2022

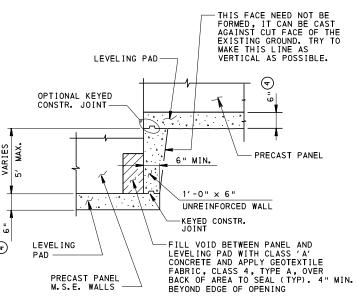
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Law E. Law B. BC-799M





- 1. ALL REINFORCEMENT BARS ARE EPOXY COATED AND A 615
 GRADE 60 AS INDICATED. SEE BC-736M FOR REINFORCEMENT
- 2. PROVIDE A 3/6" × 3/6" CHAMFER ON ALL EXPOSED EDGES OF PANELS (FRONT FACE ONLY).
- 3. ALL PANEL TYPES AND OTHER RELATED ELEMENTS WILL BE DETAILED ON SHOP DRAWINGS. INCLUDE LAYOUT (PLAN AND ELEVATION) OF COMPLETE WALL. INCLUDE WEEP HOLES DETAILS, LOCATION OF ABUTMENT PILES IF APPLICABLE, ALL OBSTRUCTIONS, BARRIER LAYOUT, SHOULDER SLAB AND JOINT DETAILS, INLET LOCATIONS, LIGHTPOLES, ETC. SHOW OBSTRUCTION MITIGATION DETAILS AND DESIGN ON THE CONSTRUCTION DRAWINGS AND SHOP DRAWINGS.
- 4. ALL PANELS SHALL HAVE TWO LIFTING INSERTS OF 2 TON CAPACITY EACH. GALVANIZE IN ACCORDANCE WITH PUBLICATION 408 SECTION 1105.02 (s).
- 5. PANEL DESIGN THICKNESS IS 51/2". THICKNESS OF CONCRETE MUST INCREASE TO ACCOMMODATE ANY ARCHITECTURAL SURFACE FINISH THAT MAY BE SPECIFIED.
- GALVANIZE ALL REINFORCING STRIPS, CONNECTION APPURTENANCES AND LIFTING HARDWARE.
- 7. THE STRIP SKEW MAY BE INCREASED TO 25° MAXIMUM PROVIDED THAT CALCULATIONS SHOWING THE STRUCTURAL ADEQUACY OF ALL AFFECTED M.S.E. WALL COMPONENTS ARE SUBMITTED AND ACCEPTED.
- 8. BOTTOM OF BOTTOM PANEL, TOP OF TOP PANEL, AND EXPOSED EDGES OF PANELS SHOULD BE FABRICATED WITH A FLAT SURFACE SQUARE TO THE REAR FACE OF PANEL.
- 9. FOR LEGEND OF NOTES AND SYMBOLS, SEE SHEET 2.



DETAIL E

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD

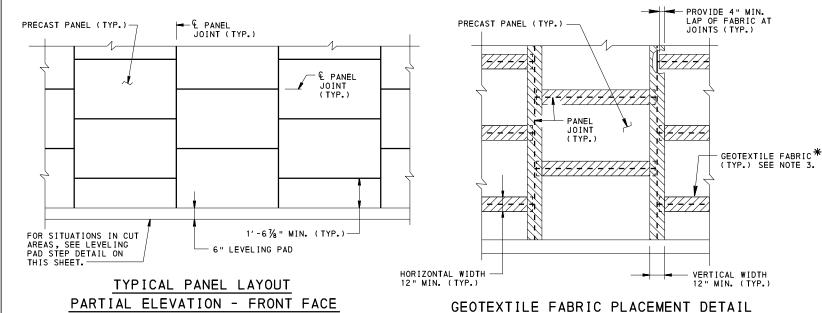
MECHANICALLY STABILIZED EARTH RETAINING WALLS REINFORCED EARTH WALL PANELS

RECOMMENDED NOV.23, 2022 RECOMMENDED Lawin &

RECOMMENDED NOV. 23, 2022 SHEET 10 OF 13

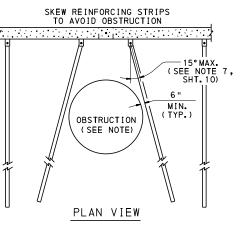
Law E. Law BC-799M

CHIEF ENGINEER, HIGHBAY ADMIN.



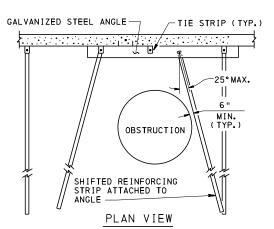
* COVER ALL JOINTS BETWEEN PANELS ON BACK SIDE OF THE WALL WITH GEOTEXTILE FABRIC CLASS 4, TYPE A. APPLY ADHESIVE COATING ON PANELS ONLY AND NOT ON GEOTEXTILE FABRIC. DO NOT APPLY ADHESIVE WITHIN 2" OF THE JOINT. DO NOT PLACE FOAM STRIP FILLER IN HORIZONTAL JOINTS.

PARTIAL ELEVATION - REAR FACE



REINFORCING STRIP SKEW DETAIL

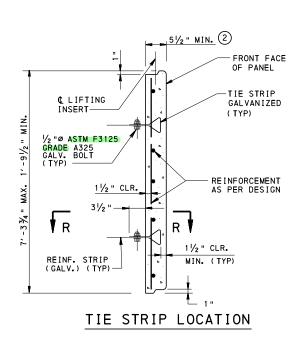
NOTE: SIZE OF OBSTRUCTION LIMITED BY THE MIN. CLEARANCE AND STRIP INCLINATION LIMITS SHOWN.

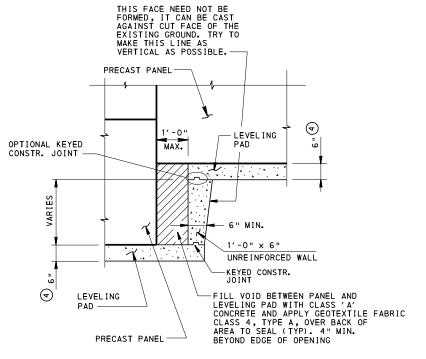


ALTERNATE REINFORCING STRIP SKEW DETAIL

FRONT FACE OF PANEL REINFORCEMENT AS PER DESIGN — 3½ " 5 ½ h MIN.

SECTION R-R





NOTES:

- 1. FOR ADDITIONAL NOTES, SEE SHEET 10.
- 2. ALL PANELS SHALL HAVE TWO LIFTING INSERTS OF 2 TON CAPACITY EACH.
- 3. STAGGERED HORIZONTAL PANEL JOINTS, MINIMUM DISTANCE 2'-51/2".
- 4. FOR LEGEND OF () NOTES AND SYMBOLS, SEE SHEET 2.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

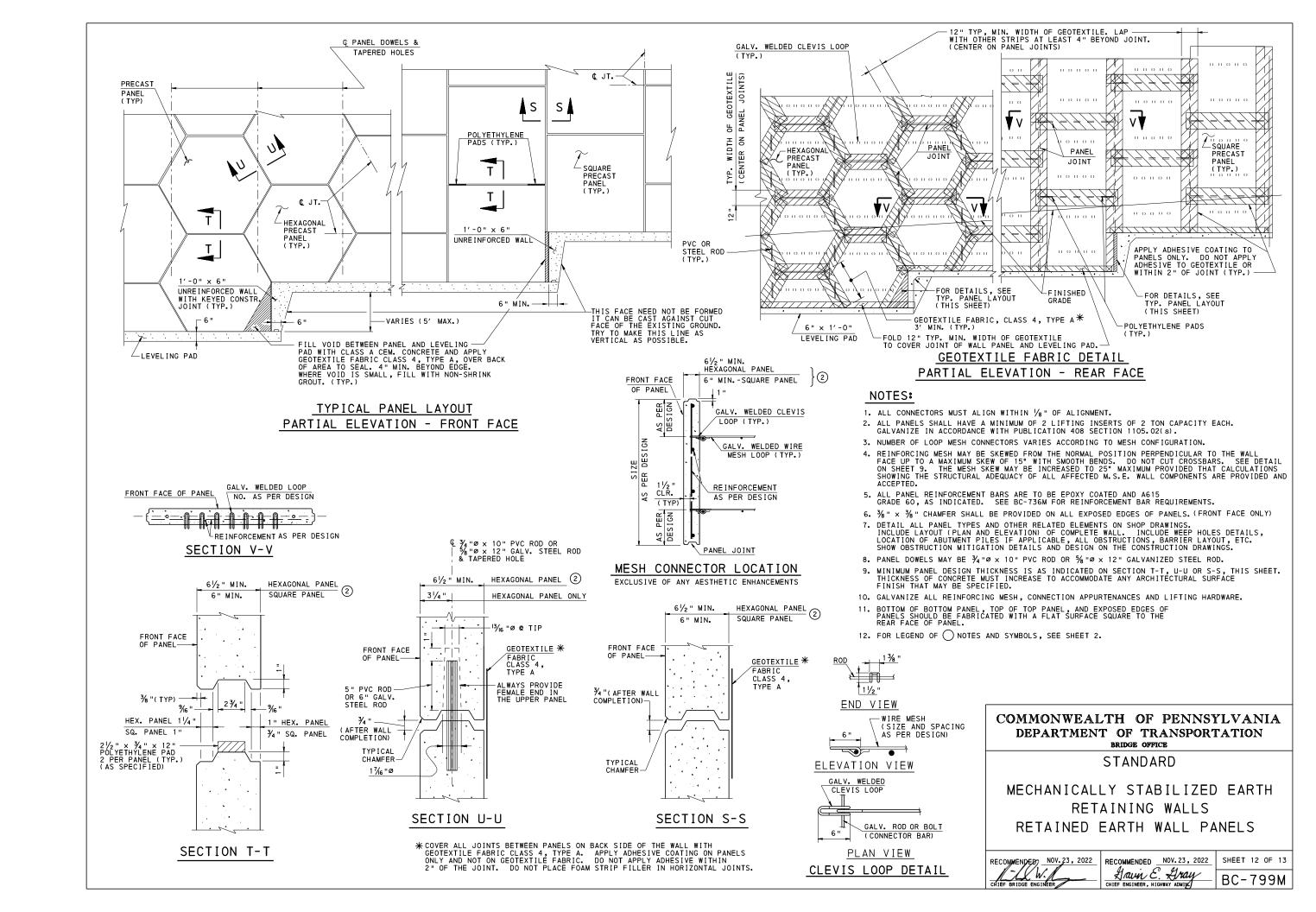
STANDARD

MECHANICALLY STABILIZED EARTH RETAINING WALLS REINFORCED EARTH WALL PANELS

RECOMMENDED NOV. 23, 2022

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CHIEF ENGINEER, HIGHWAY ADMIN BC-799M

LEVELING PAD STEP DETAIL



PANEL LOOP AND WIRE MESH LOOP TOLERANCES

- 1. FABRICATE PANELS AND WIRE MESH TO PREFERABLY ACHIEVE FULL CONTACT OF THE WIRE MESH TO THE PANEL CONNECTION. THE MAXIMUM PERMISSIBLE GAP BETWEEN THE CONNECTING BAR(S) AND PANEL/WIRE LOOPS AFTER ASSEMBLY WILL BE 1/8" AS SHOWN IN DETAIL A.
- 2. SUBMIT A QUALITY CONTROL PLAN DESCRIBING METHODS AND PROCEDURES USED TO ACHIEVE A MAXIMUM 1/8" GAP AS PER DETAIL A. DIVIDE THE QUALITY CONTROL PLAN INTO TWO PARTS: PART 1: FABRICATION, AND PART II : ERECTION. AS A MINIMUM, INCLUDE THE FOLLOWING IN THE QUALITY

- PART I: FABRICATION

 A. METHOD OF POSITIONING/MAINTAINING THE CLEVIS LOOPS IN THE PANEL DURING CONCRETE PLACEMENT, VIBRATION AND FINISHING.

 B. PROPOSED FINAL FABRICATION TOLERANCES OF THE CLEVIS WITH RESPECT TO EMBEDMENT AND ALIGNMENT.
- C. MEASUREMENT METHOD (INCLUDING TOOLS) USED TO VERIFY FABRICATION
- D. METHOD OF HANDLING, STORING AND SHIPPING THE PANELS TO AVOID CONTACT WITH AND/OR CHANGE IN POSITION OF THE CLEVIS LOOPS.

PART II: ERECTION
A. PROPOSED TOLERANCES FOR ALIGNMENT OF THE WIRE MESH LOOPS.
B. MEASUREMENT METHOD (INCLUDING TOOLS) USED TO VERIFY WIRE MESH LOOP ALIGNMENT AND FINAL CONNECTION TOLERANCES.

SUBMIT THE QUALITY CONTROL PLAN TO THE CHIEF STRUCTURAL MATERIALS ENGINEER AND THE DISTRICT STRUCTURAL CONTROL ENGINEER FOR REVIEW AND APPROVAL. APPROVAL FROM BOTH THE CHIEF STRUCTURAL MATERIALS ENGINEER AND THE DISTRICT STRUCTURAL CONTROL ENGINEER ARE REQUIRED PRIOR TO ACCEPTANCE OF THE SHOP DRAWINGS.

- 3. AS AN ALTERNATIVE TO THE PREPARATION OF A QUALITY CONTROL PLAN, OR WHERE THE QUALITY CONTROL PLAN IS REJECTED BY THE CHIEF STRUCTURAL MATERIALS ENGINEER AND/OR THE DISTRICT STRUCTURAL CONTROL ENGINEER, COMPLY WITH THE FOLLOWING:
 - OMPLY WITH THE FOLLOWING:

 A. PANELS: FABRICATE PANELS WITH LOOPS THAT ARE POSITIONED WITHIN 1/6" OF THE DEFINED POSITION. ACCEPTANCE WILL BE ESTABLISHED BY PLACEMENT OF A STRAIGHT BAR THROUGH ALL LOOPS IN A ROW OF A PANEL. REFER TO DETAIL C.

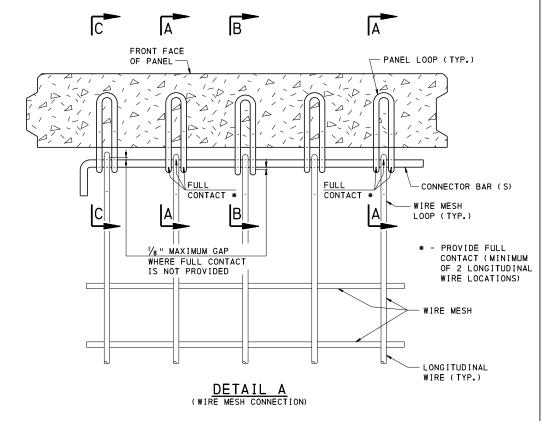
 B. WIRE MESH: FABRICATE LOOPS OF WIRE MESH TO WITHIN 1/16"
 OF A POSITION DEFINED BY A STRAIGHTEDGE PLACED IN CONTACT WITH

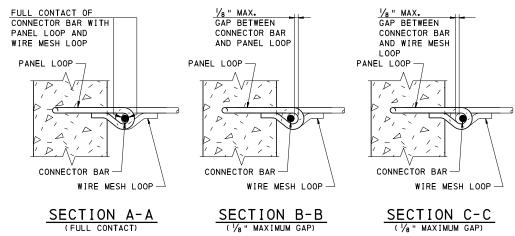
 - AT LEAST TWO LOOPS. REFER TO DETAIL B.

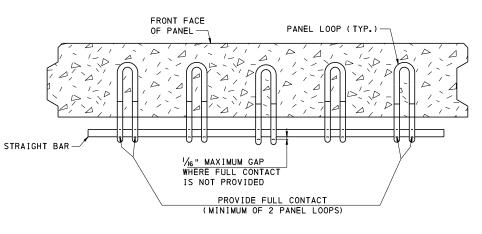
 C. MAINTAIN PANEL AND WIRE MESH TOLERANCES DURING TRANSPORTATION AND ASSEMBLY TO ACHIEVE A MAXIMUM 1/8 " GAP BETWEEN CONNECTOR BAR(S) AND PANEL LOOPS AND WIRE MESH LOOPS. REFER TO
- 4. BENDING OR REPOSITIONING PANEL LOOPS AFTER PANEL FABRICATION WILL NOT BE ACCEPTED AS MEANS OF ACHIEVING PROPER TOLERANCES.
- 5. REFER TO THE MECHANICALLY STABILIZED EARTH RETAINING WALL SYSTEMS SPECIAL PROVISION FOR ADDITIONAL WIRE MESH TOLERANCES.

METHOD FOR ESTABLISHING ACCEPTANCE OF WIRE MESH CONNECTION DURING CONSTRUCTION

- 1. CONNECT WIRE MESH TO PANEL WITH CONNECTING BAR(S).
- 2. PULL WIRE MESH AWAY FROM THE PANEL WITH SUFFICIENT EFFORT SO THAT THE CONNECTOR BAR(S) MAKE FULL CONTACT WITH THE MESH WIRE LOOPS AND THE PANEL LOOPS AT A MINIMUM OF TWO WIRE MESH LOOP LOCATIONS.
- 3. MEASURE THE GAPS, IF ANY, BETWEEN THE CONNECTOR BAR(S) AND THE PANEL LOOPS, AND BETWEEN THE CONNECTOR BAR(S) AND THE WIRE MESH LOOPS.
- 4. THE MAXIMUM ACCEPTABLE GAP BETWEEN THE CONNECTOR BAR(S) AND THE PANEL AND WIRE MESH LOOPS IS 1/2".
- 5. THE USE OF WOODEN WEDGES DRIVEN BETWEEN THE PANEL AND CONNECTOR BAR(S) TO ASSIST IN ENGAGING THE CONNECTOR ARRS) WITH THE LOOPS TO ACHIEVE THE 1/8 " MAXIMUM GAP WILL NOT BE PERMITTED. THE USE OF WOODEN WEDGES TO STABILIZE THE MESH FROM MOVEMENT DURING BACKFILL OPERATIONS WILL BE PERMITTED, BUT THE WEDGES MUST BE DRIVEN AT LOCATIONS OF FULL CONTACT ONLY OR AT ANY LOCATION AFTER GAPS ARE MEASURED AND FOUND TO BE WITHIN TOLERANCE.







DETAIL C **

(PANEL LOOPS)

** TOLERANCES IN DETAILS "B" AND "C" ARE APPLICABLE UNLESS SUPERCEDED IN A QUALITY CONTROL PLAN.

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION BRIDGE OFFICE

STANDARD MECHANICALLY STABILIZED EARTH RETAINING WALLS RETAINED EARTH WALL PANEL AND WIRE MESH TOLERANCES

RECOMMENDED NOV. 23, 2022

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CHIEF ENGINEER, HIGHWAY ADMIN BC-799M

